A. INTRODUCTION

As described in Chapter 1, "Project Description," the Proposed Actions would allow Columbia University to develop a new Manhattanville campus, and would facilitate redevelopment of other nearby properties within the approximate 35-acre Manhattanville Project Area. This chapter examines the potential impacts of these Proposed Actions in the 2015 and 2030 analysis years on the traffic network and on parking conditions in the study area. The potential impacts on transit and pedestrian facilities are described in Chapter 18.

To maintain flexibility for programmatic changes that may occur in the course of the long-range development of the Manhattanville campus, minimum and maximum build-outs for each of the development components were established. Based on these limits and the trip generation characteristics associated with each of these components, a reasonable worst-case development scenario for evaluating transportation-related impacts, determined by maximizing the space allocation for the highest trip-generating components, was developed. This analysis approach is further discussed below. In addition, since the publication of the DEIS, the traffic studies presented in this chapter have been revised to provide detailed midday peak hour analyses and to reflect recently developed traffic information from the 125th Street Corridor Rezoning and Related Actions and East 125th Street Development background projects.

PRINCIPAL CONCLUSIONS

The Proposed Actions would add a substantial number of vehicle trips in the Project Area. Largely due to traffic improvements that are proposed as part of this project, no significant adverse traffic impacts would occur at intersections bordering and within the Project Area. At study area locations that are farther away from the Project Area, particularly at locations where congested conditions would already exist for conditions in the future without the Proposed Actions, significant adverse traffic impacts may occur. The analysis results show that the Proposed Actions would result in significant adverse traffic impacts at four, four and five intersections during the AM, midday, and PM peak hours, respectively, in 2015, and at three, three, and seven intersections during the AM, midday, and PM peak hours, respectively, in 2030, as listed below.

2015 AM Peak Hour

- Amsterdam Avenue and West 125th Street
- Frederick Douglass Boulevard and West 125th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

2015 Midday Peak Hour

• Amsterdam Avenue and West 125th Street

- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street
- Broadway and West 145th Street

2015 PM Peak Hour

- Riverside Drive and West 135th Street
- Amsterdam Avenue and West 125th Street
- Frederick Douglass Boulevard and West 125th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

2030 AM Peak Hour

- Amsterdam Avenue and West 125th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

2030 Midday Peak Hour

- Amsterdam Avenue and West 125th Street
- Second Avenue and East 125th Street
- Broadway and West 145th Street

2030 PM Peak Hour

- Riverside Drive and West 135th Street
- Amsterdam Avenue and West 135th Street
- Amsterdam Avenue and West 125th Street
- Broadway and West 145th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street
- First Avenue and East 125th Street

While adequate off-street parking would be provided for the projected demand from the Columbia University development and adequate on-street supply would be available for the demand due to development within the rezoning area outside of Subdistrict A, there would be significant adverse parking impacts from the displacement of existing off-street facilities within the Project Area, resulting in a total shortfall of up to approximately 260 parking spaces in 2015 and just over 120 parking spaces in 2030.

Potential measures to mitigate these projected significant adverse impacts are described in Chapter 23, "Mitigation."

B. METHODOLOGY

OVERVIEW

The planning for a transportation impact analysis begins with understanding the travel characteristics associated with the Proposed Actions and the roadway network and regional transportation systems surrounding the Project Area. Depending on the size and anticipated trip generation of the Proposed Actions, various transportation elements may need to be evaluated

quantitatively. The determination of analysis needs for projects in New York City is based on guidance outlined in the *City Environmental Quality Review (CEQR) Technical Manual*. Impacts on vehicular flow, parking supply and demand, and vehicle-pedestrian safety are evaluated as part of the "Traffic and Parking" chapter of an EIS, while those on transit services and pedestrian flow are depicted in the "Transit and Pedestrians" section.

Once the analysis needs have been determined, a study area is developed for each of the specific transportation elements. At the same time, the appropriate analysis time periods are determined. Typically, the weekday AM, midday and PM peak hours are selected as representative peak periods for analysis. However, based on the types of use, these periods could vary to also include late evening or weekend hours. To provide the basic parameters for analysis, baseline traffic, parking, transit, and pedestrian data, along with physical and operational characteristics, are collected for developing the baseline conditions, often referred to as the "existing conditions." Once the basic analysis parameters have been established, operating levels for each of the transportation analysis areas are determined.

The future without the Proposed Actions, or the No Build condition, builds on the existing conditions analysis by incorporating background growth, other nearby projects expected to be completed, and anticipated changes in the transportation network. Again, operating levels are computed. The analysis results become the future baseline onto which projected increments associated with the Proposed Actions are layered to formulate the "Build condition." If the future Build analysis concludes that the Proposed Actions would result in significant transportation-related impacts, as defined by the *CEQR Technical Manual*, mitigation measures to alleviate these impacts are examined.

The following sections summarize the various aspects of the "Traffic and Parking" assessment, such as defining study areas, detailing analysis methodologies, and summarizing future trip projections, and set the framework upon which existing and future analyses would be based.

STUDY AREA

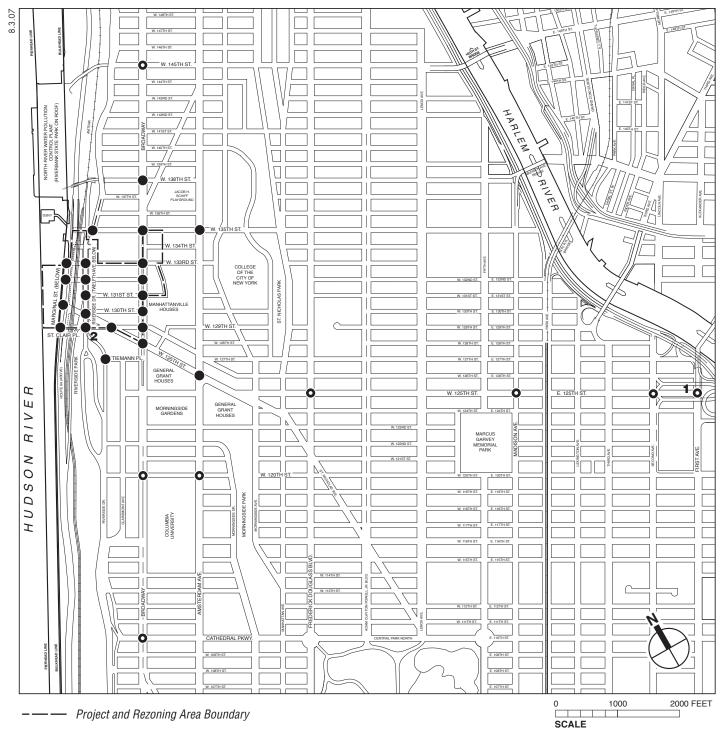
To assess the potential traffic impacts associated with the Proposed Actions, primary and secondary study areas were defined, considering the proposed Columbia University and nearby developments, principal access routes to and from the Project Area, traffic conditions in the surrounding area, and key intersections likely to be affected by project-generated trips. In total, 23 signalized and nine unsignalized intersections, determined through this EIS's scoping process, were selected for analysis. The geographic locations of these intersections are depicted in Figure 17-1.

PRIMARY STUDY AREA

The primary study area is bounded to the north by West 138th Street, to the east by Amsterdam Avenue, to the south by Tiemann Place and West 125th Street, and to the west by the Hudson River. Within this area, the following intersections, at which project-related traffic is likely to be most concentrated, were selected for analysis.

Signalized Intersections

- Marginal Street at West 125th Street
- Riverside Drive at West 135th Street
- Twelfth Avenue at West 133rd Street



- Primary Study Area Intersection
- Secondary Study Area Intersection
- 1 Intersection Analyzed Partly as Signalized and Partly as Unsignalized due to its Configuration
- 2 Intersection of Twelfth Avenue and St. Claire Place is Located Immediately Below the Intersection of Riverside Drive and St. Claire Place

Proposed Manhattanville in West Harlem Rezoning and Academic Mixed-Use Development FEIS

- Twelfth Avenue at West 132nd Street
- Twelfth Avenue at West 125th Street
- Broadway at West 138th Street
- Broadway at West 135th Street
- Broadway at West 133rd Street
- Broadway at West 132nd Street
- Broadway at West 131st Street
- Broadway at West 130th Street
- Broadway at West 129th Street
- Broadway at West 125th Street
- Amsterdam Avenue at West 135th Street
- Amsterdam Avenue at West 125th Street

Unsignalized Intersections

- Twelfth Avenue at West 131st Street
- Twelfth Avenue at St. Clair Place
- Riverside Drive at St. Clair Place
- Marginal Street at West 133rd Street
- Marginal Street at West 132nd Street
- Marginal Street at St. Clair Place
- Riverside Drive at Tiemann Place
- West 125th Street at West 129th Street

SECONDARY STUDY AREA

A secondary study area was defined to include additional key locations where project-generated traffic is expected to traverse. This area encompasses upper Manhattan to West 145th Street and Harlem east of the Project Area to the East River. The southern boundary of the secondary study area extends to West 110th Street (Cathedral Parkway) in Morningside Heights. Nine intersections within this area were selected for analysis, as listed below.

Signalized Intersections

- Broadway at West 145th Street
- Broadway at West 120th Street
- Broadway at Cathedral Parkway
- Amsterdam Avenue at West 120th Street
- First Avenue at East 125th Street
- Second Avenue at East 125th Street
- Madison Avenue at East 125th Street
- Frederick Douglass Boulevard at West 125th Street

Unsignalized Intersections

• First Avenue at East 125th Street Southbound Right Turn

STUDY AREA INTERSECTION AND ROADWAY CHARACTERISTICS

The Henry Hudson Parkway (Route 9A) is the primary highway connection to the study area. Located west of the site along the Hudson River waterfront, it connects with the Cross Bronx Expressway and the New Jersey Turnpike (via the George Washington Bridge) to the north, as well as the Gowanus and Brooklyn-Queens Expressway (via the Brooklyn Battery Tunnel) to the south. The northbound exit and southbound entrance ramps are located at the southwest corner of the project site at St. Clair Place. The northbound entrance and southbound exit ramps are located at the northwest corner of the project site at West 133rd Street.

Local through routes connecting to the Project Area include West 125th Street, Broadway, Amsterdam Avenue, and Riverside Drive. West 125th Street is a major crosstown roadway operating east-west with two moving lanes in each direction. On-street parking is permitted on both sides of the street. Broadway, a designated New York City Department of Transportation (NYCDOT) truck route, is a major north-south thoroughfare adjacent to the project site. Two moving lanes are available in each direction, and on-street parking is permitted on both sides of the street. Within the primary study area, the No. 1 subway operates on an elevated viaduct above Broadway between West 122nd and West 135th Streets. Amsterdam Avenue, also a designated NYCDOT truck route, is a two-way, four-lane arterial serving north-south traffic, with on-street parking permitted on both sides of the street. Riverside Drive is a major north-south, four-lane arterial elevated above Twelfth Avenue and connects to the local street network at St. Clair Place and West 135th Street. Within the study area, parking is permitted on both sides of Riverside Drive. In addition to these through routes, Twelfth Avenue and Marginal Street facilitate access to and from the Henry Hudson Parkway and circulation for local traffic. Twelfth Avenue is a two-lane, north-south roadway beneath the Riverside Drive viaduct from St. Clair Place to West 135th Street, with parking permitted on both sides. North of West 135th Street, it is one-way southbound. Marginal Street, operating southbound with two to three travel lanes between West 133rd Street and St. Clair Place, borders the waterfront and serves as the connecting roadway to the Henry Hudson Parkway on- and off-ramps.

OPERATIONAL ANALYSIS METHODOLOGY

SIGNALIZED INTERSECTION CAPACITY ANALYSIS

The operation of signalized intersections within the study area was analyzed in accordance with CEQR guidelines by applying the methodologies presented in the 2000 *Highway Capacity Manual (HCM)*, using Highway Capacity Software (HCS) 2000 Version 4.1e. This procedure evaluates signalized intersections for average delay per vehicle and level of service (LOS).

LOS for signalized intersections are based on the average stopped delay per vehicle for the various lane group movements within the intersection. This delay is the basis for an LOS determination for individual lane groups (grouping of movements in one or more travel lanes), the approaches, and the overall intersection. The LOS criteria for signalized intersections are defined in Table 17-1:

Table 17-1 Level of Service Criteria for Signalized Intersections

	0
LOS	Average Delay
А	≤ 10.0 seconds
В	> 10.0 and ≤ 20.0 seconds
С	> 20.0 and ≤ 35.0 seconds
D	> 35.0 and ≤ 55.0 seconds
Е	> 55.0 and ≤ 80.0 seconds
F	> 80.0 seconds
Sources: Transportation Research Bo	ard. Highway Capacity Manual, 2000.

Although the HCM methodology calculates a volume-to-capacity (v/c) ratio, there is no strict relationship between v/c ratios and LOS as defined in the HCM. A high v/c ratio indicates substantial traffic passing through an intersection, but a high v/c ratio combined with low average delay actually represents the most efficient condition in terms of traffic engineering standards, where an approach or the whole intersection processes traffic close to its theoretical maximum with minimal delay. However, very high v/c ratios—especially those approaching or greater than 1.0 are often correlated with a deteriorated LOS. Other important variables affecting delay include cycle length, progression, and green time. LOS A and B indicate good operating conditions with minimal delay. At LOS C, the number of vehicles stopping is higher, but congestion is still fairly light. LOS D describes a condition where congestion levels are more noticeable and individual cycle failures (a condition where motorists may have to wait for more than one green phase to clear the intersection) can occur. The midpoint of this service level (45 seconds of delay) is considered the threshold of acceptable operating conditions. Conditions at LOS E and F reflect poor service levels, and cycle failures are frequent. The HCM methodology provides for a summary of the total intersection operating conditions, by identifying the two critical movements (the worst-case from each roadway) and calculating a summary of critical v/c ratio, delay, and LOS.

Significant Impact Criteria

According to the criteria presented in the *CEQR Technical Manual*, impacts are considered significant and require examination of mitigation if they result in an increase in the Build condition of 5 or more seconds of delay in a lane group over No Build levels beyond mid-LOS D. For No Build LOS E, a 4-second increase in delay is considered significant. For No Build LOS F, a 3-second increase in delay is considered significant. Also, if the No Build LOS F condition already corresponds with a delay in excess of 120 seconds, an increase of 1.0 or more seconds of delay is considered significant. In addition, impacts are considered significant if levels of service deteriorate from acceptable A, B, or C in the No Build condition to marginally unacceptable LOS D (a delay in excess of 45 seconds, the midpoint of LOS D), or unacceptable LOS E or F in the future Build condition. The above sliding scale is applicable only if the Proposed Actions are projected to generate five or more vehicle trips through the <u>affected lane group</u> in the peak hour.

UNSIGNALIZED INTERSECTION CAPACITY ANALYSIS

For unsignalized intersections, the total delay is defined as the total elapsed time from which a vehicle stops at the end of the queue until the vehicle departs from the stop line. This includes the time required for the vehicle to travel from the last-in-queue to the first-in-queue position.

The average total delay for any particular minor movement is a function of the service rate or capacity of the approach and the degree of saturation. The LOS criteria for unsignalized intersections are summarized in Table 17-2.

Table 17-2 Level of Service Criteria for Unsignalized Intersections

LOS	Average Delay
Α	≤ 10.0 seconds
В	> 10.0 and ≤ 15.0 seconds
С	> 15.0 and ≤ 25.0 seconds
D	> 25.0 and ≤ 35.0 seconds
Е	> 35.0 and ≤ 50.0 seconds
F	> 50.0 seconds
Source: Transportation Research Board	d. Highway Capacity Manual, 2000.

The LOS thresholds for unsignalized intersections are different from those for signalized intersections. The primary reason is that drivers expect different levels of performance from different types of transportation facilities. The expectation is that a signalized intersection is designed to carry higher traffic volumes than an unsignalized intersection. In addition, certain driver behavioral considerations combine to make delays at signalized intersections less onerous than at unsignalized intersections. For example, drivers at signalized intersections are able to relax during the red interval, whereas drivers on minor approaches to unsignalized intersections must remain attentive to identifying acceptable gaps and vehicle conflicts. Also, there is often much more variability in the amount of delay experienced by individual drivers at unsignalized intersections. For these reasons, the total overall scale of delay thresholds for unsignalized intersections is lower than that of signalized intersections.

Significant Impact Criteria

The same sliding scale of significant delays described for signalized intersections applies for unsignalized intersections. For the minor street to trigger significant impacts, at least 90 passenger car equivalents (PCE) must be identified in the future Build condition in any peak hour.

PARKING CONDITIONS ASSESSMENT

The parking analysis identifies the extent to which on-street and off-street parking is available and utilized under existing and future conditions. Typically, this analysis encompasses a study area within ¼ mile of the project site. If the analysis produces a shortfall in parking in the ¼ mile study area, the study area could be extended to ½ mile to identify additional parking supply. The analysis, which takes into consideration anticipated changes in area parking supply, provides a comparison of parking needs versus availability to determine if a parking shortfall is likely to result from additional demand generated by the Proposed Actions.

Outside of the Manhattan central business district (CBD), a parking shortfall that exceeds the number of off-street parking spaces and more than half the available on-street spaces within ½ mile of the project site may be considered significant. However, smaller shortfalls may be deemed insignificant. In some cases, if the shortfalls could be accommodated by parking spaces within ½ mile, the parking impacts may not necessarily be considered significant.

For the Proposed Actions, on-street and off-street parking shortfalls would occur within ¼ mile from the Project Area boundaries. Therefore, the parking study area was extended to a ½-mile radius for both on-street and off-street parking analyses. However, due to the small number of off-street parking facilities within this ½-mile radius, some of which would be displaced by the Proposed Actions, and the sheer size of the development Project Area, the off-street parking analysis was further extended to 1 mile from the Project Area boundaries to identify potential off-street parking supply. Furthermore, since there would be adequate on-site parking to accommodate all off-street demand resulting from the Proposed Actions, this extension of the off-street parking study area was determined to be appropriate in identifying potential off-street parking impacts.

TRAFFIC SAFETY EVALUATION

An evaluation of traffic safety is necessary for locations within the traffic study area that have been identified as high accident locations, where five or more pedestrian accidents in any year in the most recent three-year period were recorded. For these locations, accident trends would be identified to determine whether projected vehicular and pedestrian traffic would further impact safety at these locations or whether existing unsafe conditions could adversely impact the flow of the projected new trips. The determination of potential significant safety impacts is often subjective and depends largely on the type of area where the Proposed Actions are anticipated to occur. For example, in busy Manhattan midtown and downtown areas, where vehicular and pedestrian activities are high and motorists and pedestrians are acclimated to travel under congested conditions, it is unlikely that significant safety impacts would be determined even if the Proposed Actions are anticipated to generate a substantial amount of vehicular and pedestrian traffic. On the other hand, in quiet residential neighborhoods, if substandard roadway geometry had historically resulted in unsafe crossing, even nominal increases in activities anticipated from the Proposed Actions may adversely affect safety and constitute significant safety impacts.

TRAVEL DEMAND PROJECTIONS

The projection of future trips incorporated background growth, No Build projects that are expected to be completed absent the Proposed Actions by the respective 2015 and 2030 future analysis years, and the reasonable worst-case transportation scenario (as defined in Table 17-3) developed for the Proposed Actions. Details related to the development of travel demand estimates for the No Build projects and uses programmed as part of the Proposed Actions are discussed below.

Table 17-3 Comparison of Transportation Analysis Scenarios

Use	Illustrative Plan	Maximum Build-Out	Minimum Build-Out		Worst-Case
	(gsf)	(gsf)	(gsf)	2015	2030
Academic Mixed-Use Area (Subdistrict A)					
Community Facility Uses					
Academic Research	2,596,957	2,700,000	960,000	337,990	2,367,931
General or Other Academic	1,266,928	2,000,000	1,000,000	705,000	2,000,000
Columbia University Housing	582,115	1,300,000	350,000	0	350,000
Recreation	239,313	350,000	0	0	0
Commercial Uses					
Active Ground-Floor Uses (Retail)	162,618	600,000	130,000	60,449	130,000
Primary Space Subtotal	4,847,931	-	-	1,103,439	4,847,931
Below-Grade Support Uses					
Research Support	296,201			58,563	296,201
Academic Program	69,830			69,830	69,830
Swimming and Diving Center	145,431			0	145,431
Parking and Loading/Service/Storage	1,403,996			125,932	1,403,996
Centralized Steam/Chilled Water Plant	70,199			50,870	70,199
Support Space Subtotal	1,985,657			305,195	1,985,657
Grand Total	6,833,588	ı	ı	1,408,634	6,833,588
Non-University Rezoning Areas	Residentia	<u>Offic</u>	<u>e</u>	Retail I	Health Center
Subdistrict B	0	55,000	gsf 125	5,000 gsf	0
Other Area	99 DU	0	18	,000 gsf	62,000 gsf

NO BUILD PROJECTS

Typically, No Build projects within ½ mile of a project site are incorporated into the future No Build condition. For the Proposed Actions, since the traffic study areas defined earlier encompass intersections within a large area in upper Manhattan, No Build projects beyond the typical ½ mile were included. Figure 17-2 provides an illustration of these future developments. All of the No Build sites are anticipated to be completed on or before the initial analysis year 2015¹. Trips generated by each of the No Build projects were developed based on information provided in approved studies, including the Retail and Industrial Zoning Text Amendments FGEIS (1996), Harlem Park Development EAS (2004), Harlem Center Project EAS (1999), East River Plaza FEIS (1999), East Harlem Rezoning EAS (2003), and Frederick Douglass Rezoning EAS (2004), standard references, such as the CEOR Technical Manual, Pushkarev and Zupan's Urban Space for Pedestrians, and the U.S. census database. For the six Columbia University buildings included in the No Build analysis, trip projections also accounted for information obtained from the Columbia University Department of Facilities Planning and Space Management and from the April/May 2004 on-line travel survey (to be discussed under the Build trip estimates). Brief discussions of these No Build projects and their corresponding trip generation characteristics are provided below. Some projects listed were anticipated for

¹ The 125th Street Corridor Rezoning and Related Actions project is expected to be completed in 2017. For a conservative analysis, its projected trips were incorporated into the 2015 analysis; however, the mitigation measures proposed to alleviate impacts related to these trips in the DEIS for the rezoning were incorporated only into the 2030 analysis.



Figure 1*7*-2

Traffic Study Area:
Developments Proposed or
Under Construction for 2015 and 2030

completion during the preparation of this study. <u>Because data collection occurred before the completion of these projects</u>, projected traffic volumes <u>for these projects</u> were included in the No Build analysis.

- 1. <u>560 Riverside Drive</u>: Located between St. Clair Place and Tiemann Place, this Columbia University housing complex would be renovated, resulting in the addition of a new entrance on West 125th Street. No alteration to the existing use and no incremental trips would be generated with this project.
- 2. <u>Harlem Piers</u>: Located along the Hudson River between West 125th and West 137th Streets, this project would result in noticeable improvements to the existing waterfront destinations by enhancing the piers and recreational open space with a gateway plaza and landscaped areas. The new Harlem Piers, containing approximately 120,400 gross square feet (gsf) of waterfront open space, 9,000 gsf of site building, and one ferry slip, would function as passive parkland and primarily serve the surrounding communities. Scheduled for completion in 2008, this project is expected to generate 154 AM, 156 midday, and 171 PM peak hour person trips, and 9 AM, 16 midday, and 16 PM peak hour vehicle trips.
- 3. <u>Striver's Garden</u>: Located on Frederick Douglass Boulevard at West 135th Street, this project would contain 46,000 gsf of ground-floor retail and 170 residential condominium or cooperative units. Completed in 2005, it is expected to generate 193 AM, 478 midday, and 357 PM peak hour person trips, and 31 AM, 36 midday, and 39 PM peak hour vehicle trips.
- 4. <u>Citarella</u>: Located on West 126th Street between Morningside and Amsterdam Avenues in the former Taystee factory, this project is scheduled for completion sometime before 2015. The trip generation estimates assumed the 80,000-gsf building to be comprised of 50,000-gsf of office space and 30,000-gsf of retail space. Under this assumption, 170, 411 and 243 person trips, and 32, 35 and 31 vehicle trips would be generated during the AM, midday and PM peak hours, respectively.
- 5. Mink Building: Located on Amsterdam Avenue between West 126th and West 128th Streets, this project would entail the reuse of an existing 120,000-gsf building for new office space. Scheduled for completion in 2008, it is expected to generate 302, 337, and 255 person trips, and 70, 47, and 56 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 6. West 127th Street Cornerstone: Occupying the entire block bounded by St. Nicholas Avenue, Frederick Douglass Boulevard, West 127th Street, and West 128th Street, this project would contain 40,000 gsf of commercial space and 200 residential units. Expected to be completed in 2010, it would generate estimated 205, 436, and 355 person trips, and 30, 32, and 35 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 7. Mart 125: Located on West 125th Street between Frederick Douglass and Adam Clayton Powell Jr. Boulevards, this project is scheduled for completion before 2015. While the precise mixture of uses is not yet determined, it is assumed, for trip generation purposes, to include 40,000 gsf for office use and 10,000 gsf of retail space. Overall, 115, 202, and 131 person trips, and 27, 21, and 22 vehicle trips during the AM, midday, and PM peak hours, respectively, are anticipated for this project.
- 8. <u>Harlem Dowling</u>: Located along Adam Clayton Powell Jr. Boulevard at West 127th Street, this project is scheduled for completion prior to 2015. The 35,000-gsf commercial development would be 10 stories high and is expected to include ground-level retail uses. For trip generation purposes, it is assumed to contain 31,000 gsf for office use and 4,000 gsf

- of retail space. Due to its proximity to No Build site No. 9, trip projections for the two sites were combined, as described below.
- 9. <u>United City Methodist Society</u>: Located along Adam Clayton Powell Jr. Boulevard between West 129th and West 130th Streets, this six-story, mixed-use building, which is expected to be completed before 2015, would feature ground-floor commercial uses and community facility space totaling 34,200 gsf, and 40 residential dwelling units. Combined with No Build site No. 8, the total development program would include 65,200 gsf of office space (community facility space assumed as office space for trip generation purposes), 4,000 gsf of retail space, and 40 residential units. It is estimated that the two projects would generate 199, 235, and 190 person trips, and 51, 29, and 40 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 10. <u>Harlem Park</u>: Located on the southwest corner of Park Avenue and East 125th Street, the original project would contain 230 hotel rooms, 146,000 gsf of office space, 69,000 gsf of neighborhood retail space, and 100 residential dwelling units. Although the project was scheduled for completion in 2005, it has not yet begun construction, and the programming of on-site uses has recently been changed to include only office space, totaling approximately 600,000 gsf. Since no further environmental review was required for this change in use, previous trip projections and analysis conclusions are likely to remain similar with the new plan. For trip generation purposes, the original program, as described above and analyzed in the *Harlem Park Development EAS*, was assumed. In total, this project is expected to generate 915, 3,232, and 1,916 person trips, and 97, 157, and 125 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 11. <u>Vincent Cyrus Plaza</u>: Located along East 128th Street between Madison and Park Avenues, this project, which has been completed, contains 104 units of low-income housing. Trip estimates show that 76, 40, and 90 person trips, and 10, 4, and 12 vehicle trips during the AM, midday, and PM peak hours, respectively, would be generated.
- 12. East 125th Street Development Project: Located between Second and Third Avenues and bounded by East 124th and East 127th Streets, this potential development, scheduled for completion in 2012, encompasses several parcels that could total approximately 300,000 gsf of media/office space, a 30,000-gsf cultural facility (500-seat auditorium), 470,000 gsf of destination retail/entertainment space, a 100,000-gsf hotel with approximately 130 rooms, and up to 1,000 residential units. Based on the transportation planning assumptions that accompanied this project's draft scoping document, the proposed development components are estimated to generate 2,345, 4,790, and 5,371 person trips and 321, 625, and 767 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 13. <u>Harlem Auto Mall</u>: Located between Second and Third Avenues and bounded by East 127th and East 128th Streets, this Potamkin Development Company-sponsored project would entail 180,000 gsf of showroom, auto service, and repair facilities. Completed in March 2006, the <u>projected increments of</u> 54, 69, and 96 vehicle trips during the AM, midday, and PM peak hours, respectively, <u>as summarized</u> in the approved *Harlem Auto Mall EAS* <u>were incorporated into the No Build traffic analysis networks.</u>
- 14. East River Plaza: Located adjacent to the FDR Drive between East 116th and East 119th Streets, this big box retail project would include approximately 500,000 gsf of retail space to house, among others, Home Depot and Costco stores. In addition, the development program would include 24,500 gsf of office space and a 1,250-car garage. Scheduled for completion before 2015, East River Plaza is estimated to generate 4,687 person trips and 1,688 vehicle

- trips during the midday peak hour, and 4,547 person trips and 1,621 vehicle trips during the PM peak hour.
- 15. <u>Malcolm Shabazz Merchants Market</u>: Located along Fifth Avenue between 115th and 116th Streets, this project, scheduled for completion before 2015, would result in up to 150 residential dwelling units. Due to its proximity to No Build sites No. 16 and 17, trip projections for the three sites were combined, as described below.
- 16. <u>Shabazz Gardens</u>: Located along Malcolm X Boulevard (Lenox Avenue) between West 117th and West 118th Streets, this mixed-use project, which is currently under construction, would contain 170 residential units and 12,000 gsf of retail space. Due to its proximity to No Build sites No. 15 and 17, trip projections for the three sites were combined, as described below.
- 17. The Kalahari: Located along Malcolm X Boulevard (Lenox Avenue) at West 116th Street, this project, scheduled to open in December 2007, would contain 249 condominium units, an independent film center, education center, and a gym (analyzed as 265 dwelling units and approximately 50,000 gsf of retail space). Combined with No Build sites No. 15 and 16, the total development program would include 585 residential units and 62,000 gsf of retail space. It is estimated that the three projects would generate 521, 780, and 788 person trips, and 62, 58, and 72 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 18. New Columbia Academic Building: Located at the southwest corner of Broadway and West 125th Street, this new 250,840-gsf academic building would replace the existing uses and is expected to be completed in 2010. It would result in 421 person trips during both the AM and PM peak hours, and 422 person trips during the midday peak hour. Additionally, 57, 28, and 56 vehicle trips would be made during the AM, midday, and PM peak hours, respectively. In the Build condition, the academic use within this building would be reduced to 154,240 gsf, with the remaining space accommodating No Build site No. 22.
- 19. New Columbia Academic/Research Building: Located at the southeast corner of Broadway and West 120th Street, this new 170,000-gsf academic/research building is expected to be completed in 2010. The new space would accommodate primarily existing population within the Columbia University Morningside Campus. Based on information provided by Columbia University, only a minimal number of new users would be generated by the operation of this new building. The additional trips associated with these new users were assumed to be part of the background growth.
- 20. <u>City College Dormitory</u>: Located on St. Nicholas Terrace at West 130th Street, this project comprises a 600-bed dormitory for City College students and housing for up to five faculty members. Scheduled for completion in 2006, it is expected to generate 259 AM, 134 midday, and 304 PM peak hour person trips, and 6 AM, 4 midday, and 6 PM peak hour vehicle trips.
- 21. New Columbia Administrative Building (the Studebaker Building): Located on West 131st Street, between Broadway and Twelfth Avenue, the existing building would be converted into 220,500 gsf of office and accessory space for Columbia University administrative use. Scheduled for completion in 2008, this building is expected to generate 559, 364, and 559 person trips, and 111, 32, and 103 vehicle trips during the AM, midday, and PM peak hours, respectively.
- 22. <u>Science, Math and Engineering Secondary School</u>: Located on the east side of Broadway on the existing U-Haul site, between West 132nd and West 133rd Streets, a 90,000-gsf public school would be constructed for grades 6-12, accommodating 650 to 700 students and 35 to 45

- staff/faculty members. Scheduled for completion sometime before 2015, it is expected to generate 637 AM and mid-afternoon peak hour person trips, and 112 AM and mid-afternoon peak hour vehicle trips. In the Build condition, this school would relocate to No Build site No. 18.
- 23. New Columbia Office Building: Located on Broadway, between West 131st and West 132nd Streets, the existing former Warren Nash Service Station building would be converted into approximately 208,000 gsf of office space for use by Columbia University. Scheduled for completion sometime before 2015, it is expected to generate 526 AM, 342 midday, and 527 PM peak hour person trips, and 105 AM, 32 midday, and 97 PM peak hour vehicle trips. In the Build condition, this use would be replaced by a component of the Proposed Actions.
- 24. New Columbia Office Building: Located on the east side of Broadway on the existing U-Haul site, between West 132nd and West 133rd Streets, a new 127,296-gsf Columbia University office building would be constructed, along with No Build site No. 22. Scheduled for completion sometime before 2015, it is expected to generate 323 AM, 210 midday, and 323 PM peak hour person trips, and 64 AM, 20 midday, and 59 PM peak hour vehicle trips. In the Build condition, this use would be replaced by a component of the Proposed Actions.
- 25. <u>CUNY Research Buildings & Building Conversions</u>: Within City College's South Campus, approximately 350,000 gsf of research facilities for use by City College would be constructed between 2010 and 2013. These uses are expected to generate 785 AM, 682 midday, and 784 PM peak hour person trips, and 125 AM, 48 midday, and 124 PM peak hour vehicle trips.
- 26. <u>Project Area Rezoning A</u>: Located at 655 West 125th Street, a rezoning application has been filed to convert the existing use to yield approximately 53,000 gsf of added office space or 54 new residential units. This potential use, if approved, could be completed sometime before 2015. However, since this No Build project would be replaced by a component of the Proposed Actions and its exact use is speculative, it was not included in the analysis.
- 27. <u>Project Area Rezoning B</u>: Located at 614 West 131st Street, a rezoning application has been filed to convert the existing use to yield approximately 10,000 gsf of added office space or 25 new residential units. This potential use, if approved, could be completed sometime before 2015. However, since this No Build project would be replaced by a component of the Proposed Actions and its exact use is speculative, it was not included in the analysis.
- 28. <u>Project Area Rezoning C</u>: Located at 3300-3320 Broadway, a rezoning application has been filed to convert the existing use to yield approximately 120,000 gsf of added office space or 116 new residential units and 36,000 gsf of retail space. This potential use, if approved, could be completed sometime before 2015. However, since this No Build project would be replaced by a component of the Proposed Actions and its exact use is speculative, it was not included in the analysis.
- 29. <u>Project Area Rezoning D</u>: Located at 3261 Broadway, a rezoning application has been filed to convert the existing use to yield approximately 40,200 gsf of added office space or 66 new residential units and 20,000 gsf of retail space. This potential use, if approved, could be completed sometime before 2015. However, since this No Build project would be replaced by a component of the Proposed Actions and its exact use is speculative, <u>it</u> was not included in the analysis.
- 30. <u>Barnard Student Center</u>: Located along Broadway south of West 120th Street, an 80,000-gsf Nexus building would be constructed to replace the existing McIntosh Center as Barnard's

campus resource center. The building would include space for a small number of classrooms, academic departments, dining facilities, two event-hall theaters, and a 100-seat black box theater. Anticipated for completion in 2009, this facility would largely result in a reallocation of existing trips within the Barnard campus and not result in added trips to the study area.

- A. <u>East Harlem Rezoning</u>: Recently approved and expected to be completed prior to 2015, this rezoning project would involve residential (totaling 2,519 dwelling units) and neighborhood retail developments on 56 blocks in East Harlem, generally bounded by East 122nd Street, Pleasant Avenue, 100 feet east of Lexington Avenue, and East 99th Street. In total, 1,851, 956, and 2,177 person trips, and 245, 130, and 271 vehicle trips were estimated for the AM, midday and PM peak hours, respectively.
- B. Frederick Douglass Boulevard Rezoning: Recently approved and expected to be completed prior to 2015, this rezoning project would allow for increases in residential density and extension of existing commercial overlay on West 116th Street. The rezoning area is bounded by Adam Clayton Powell Jr. Boulevard to the east and Morningside Avenue to the west, between West 110th and West 124th Streets. In total, 1,830 residential dwelling units and nearly 135,000 gsf of retail space would be developed, resulting in 2,930, 4,552, and 4,899 non-vehicle trips, and 126, 181, and 184 vehicle trips during the AM, midday and PM peak hours, respectively.
- C. 125th Street Corridor Rezoning and Related Actions: The DEIS for this project was issued in October 2007. Once approved, a two-block-wide corridor centered on 125th Street, from Broadway to Second Avenue, would be rezoned and construction is anticipated to be completed by 2017. The DEIS identifies 26 projected development sites in the rezoning area. For analysis purposes, the development was assumed for the 2015 analysis year.

PROPOSED ACTIONS

Future trips associated with the Proposed Actions were derived through a vigorous process of determining the reasonable worst-case transportation scenario, developing trip rates for various Columbia University uses and population components, and interpreting travel characteristics through an on-line travel survey and review of other approved documents.

Reasonable Worst-Case Transportation Scenario

As described in Chapter 1, the Academic Mixed-Use Area, or Subdistrict A, would facilitate, with the Proposed Actions' Illustrative Plan, five different above-grade uses, including academic research, academic, housing for graduate students, faculty, and other employees, recreation, and retail. Below grade, additional space would be created for academic and academic research support, as well as to provide parking, loading/service/storage, central energy plants, and a swimming and diving center. In total, approximately 6.8 million gsf of development space would be constructed.

Based on trip generation rates developed for each of the possible development components (discussed in further detail in the next section), iterations of estimating future trips for different combinations of space allocation were conducted to determine a reasonable worst-case transportation scenario for impact assessment of transportation-related elements in the 2015 Phase 1 build-out and the 2030 final analysis year. In addition, the remaining parcels within the overall 35-acre rezoning area were assumed to be completed in a similar timeframe as the 2015 Phase 1 build-out. These remaining developable parcels, divided into Subdistricts B and Other Area, respectively, include 55,000 gsf of office and 125,000 gsf of retail developments west of the Academic Mixed-Use Area, across Twelfth Avenue, and 99 residential units, 18,000 gsf of retail development, and a 62,000-gsf health center, all to the northeast across Broadway.

Overall, the transportation analysis accounted for nearly 2.7 million gsf of academic research facilities, just over 2.0 million gsf of academic building space, 350,000 gsf of housing for graduate students, faculty, and other employees, 130,000 gsf of ground-floor retail, and 440,000 gsf of surrounding mixed-use developments. A summary of the above breakdown and comparisons with the Illustrative Plan is shown in Table 17-3.

Trip Generation Estimates

The projection of future trips associated with the Proposed Actions accounted for general travel characteristics related to typical uses as well as those specific to Columbia University facilities and users. In addition to information presented in standard references and approved studies, new data on travel rates and characteristics were attained through in-depth consultations with Columbia University officials and through an on-line survey of Columbia University faculty, administrators, other employees, and students. The consultations provided key information on the users of different University facilities, such as population distribution, absentee rates, class/work schedules, and average visitations, while the on-line survey generated valuable data on the travel characteristics for specific user groups, including modal split, housing information, geographical distribution, and parking patterns. Trip rates for employees (faculty, post-doctorate researchers, researchers, and administrators) were based on these surveys and on existing density ratios in Columbia facilities. Trip rates for graduate students were based on these surveys and on projections of future graduate student populations at Manhattanville. Collectively, these efforts provided the basis for developing the trip estimates for different components of the University development assumed in the reasonable worst-case transportation scenario.

In 2015, with five of the 18 buildings scheduled to be completed, 2,780 students and 1,716 employees (faculty, post-doctorate researchers, researchers, administrators, and non-Columbia employees) were projected for the Illustrative Plan. Upon the completion of the remaining 13 buildings in 2030, the total campus population would be 4,322 students and 6,411 employees. For the analysis of the reasonable worst-case transportation scenario, the number of students would remain the same as projected, regardless of the mix of potential uses assumed for each of the proposed buildings. However, employee population would vary from the Illustrative Plan, as discrete density ratios (i.e., population per 1,000 gsf) among different personnel groups at different building types were used to project the total population under this reasonably conservative transportation analysis scenario.

To determine travel characteristics of different users within the Columbia University community, an on-line travel survey was administered over a three-week period in the spring of 2004. The survey was designed for all Columbia University personnel and students based at the Morningside and Medical Center campuses. Overall, responses were received from just under 25 percent of the target population. The results show that approximately 25 percent of all employees and 5 percent of all students travel via auto. Transit shares, which include subway and bus trips, make up nearly 50 and 35 percent of the total employee and student trips, respectively. With over half of all students residing in University housing or dormitory, travel on foot to and from University facilities is their primary mode of travel (56 percent), while the walk only mode comprises only 18 percent of the total employee trips. These statistics were further dissected to reflect specific travel patterns for different University affiliations. For example, the faculty has an auto rate of 35 percent, while administrators and other employees have rates of 23 and 13 percent, respectively. Graduate and professional students have a combined auto rate of over 6 percent, while undergraduate students have a rate of around 3 percent. A memorandum summarizing the planning and administration of the survey and the survey results is included in Appendix H.

Based on the information provided by Columbia University's Department of Facilities Planning and Space Management, the above travel patterns, and reasonable assumptions, transportation characteristics were developed for each of the University personnel groups. For example, the absentee rates for graduate students, post-doctorate and general researchers, and administrators were projected at 5 percent, with the faculty reaching a 20 percent absentee rate. Each member of these personnel groups was assumed to make 3.5 trips a day. These trips encompass commuting to and from the campus and 75 percent of the total population making other discretionary two-way trips (i.e., lunch) during the course of day. In terms of trip distribution, graduate students and faculty members are expected to have a more evenly distributed arrival or departure pattern while researchers and administrators would tend to adhere to a more regular schedule and arrive or depart during a smaller window of time. These characteristics, along with those associated with daily visitations, truck deliveries, university housing, and retail uses are summarized in Tables 17-4a and 17-4b. Total projected trips for the 2015 and 2030 analysis years are presented in Tables 17-5 and 17-6, respectively. As part of the transportation analyses, 12 University-operated shuttle bus trips were assumed for serving the projected shuttle trips during each of the analysis hours.

Table 17-4a
Travel Demand Assumptions for Columbia University Personnel

Daily Trip Rates		duate nts (1,6)	Facult	y (1,6)		rchers ,6)	Admins (1	strators ,6)	Visito Res	ors (1) Acad					
Person Trips	3	.5	3.	.5	3	.5	3	.5	0.3	0.4		All rates	are per	person,	except for
Absentee Rate	5	%	20	1%	5	%	5	%	-						or academic
Mandal On th	Grad	duate			Resea	rchers	Admins	strators							
Modal Split	Stude	nt (2,4)	Facult	y (2,4)	(2	,4)	(2	,4)	Visito	rs (4,6)					
	AM/PM	MD	AM/PM	MD	AM/PM	MD	AM/PM	MD	AM/PM	MD					
Auto	6.0%	5.0%	35.0%	5.0%	15.9%	5.0%	20.4%	5.0%	25.0%	5.0%					
Taxi	2.5%	2.0%	3.5%	2.0%	0.5%	2.0%	1.5%	2.0%	2.0%	2.0%					
Subway	40.0%	10.0%	31.0%	10.0%	49.0%	10.0%	49.5%	10.0%	50.0%	10.0%					
Bus	3.5%	5.0%	4.5%	5.0%	6.0%	5.0%	10.5%	5.0%	11.0%	5.0%					
Shuttle	1.0%	3.0%	2.0%	3.0%	6.0%	3.0%	2.0%	3.0%	2.0%	3.0%					
Commuter Rail	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%					
Walk Only	47.0%	75.0%	24.0%	75.0%	22.6%	75.0%	16.1%	75.0%	10.0%	75.0%					
Total	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%					
Vehicle Occupancy		duate		(0)		rchers		strators		(0)					
	Stude	nts (2)	Facul		(2	2)	(2	2)		ors (2)				_	
Auto	Stude 1.	nts (2) 20	1.3	20 ` ′	(2	2) 20	(2	2) 20	1.	20 `	4			pancy fig	ures are
	Stude 1.	nts (2)		20 ` ′	(2	2)	(2	2)	1.		•		cle occup per veh		ures are
Auto Taxi/Black Car	Stude 1. 1.	nts (2) 20 30 uate Stu	1.: 1.:	20 ` 30	(; 1. 1.	2) 20 30	(; 1. 1.	2) 20 30 esearche	1. 1.	20 30	•	persons	per veh	icle	
Auto Taxi/Black Car Temporal	Stude 1. 1. Grad	ents (2) 20 30 uate Stu (3,5)	1.: 1.: dents	20`´ 30 F a	(2 1. 1. aculty (3	2) 20 30 , 5)	(; 1. 1. Re	2) 20 30 esearche (3,5)	1. 1. ers	20 30 Admi r	nistrator	persons	per veh	icle 'isitors (5)
Auto Taxi/Black Car Temporal Distribution	Stude 1. 1. Grad	ents (2) 20 30 uate Stu (3,5) <u>In</u>	1.: 1.: dents <u>Out</u>	20 30 F a <u>Total</u>	(3 1. 1. nculty (3	2) 20 30 ,5) Out	(2 1. 1. Re <u>Total</u>	2) 20 30 esearche (3,5) <u>In</u>	1. 1. ers <u>Out</u>	20 30 Admir <u>Total</u>	<u>In</u>	persons s (3,5) Out	per veh	icle 'isitors (<u>In</u>	5) Out
Taxi/Black Car Temporal Distribution AM Peak Hour	1. 1. Grad Total 7.0%	ents (2) 20 30 uate Stu (3,5) <u>In</u> 95.0%	1.: 1.: dents <u>Out</u> 5.0%	20 30 F a <u>Total</u> 11.0%	1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	2) 20 30 ,5) Out 5.0%	(2 1. 1. Re Total 19.0%	2) 20 30 esearche (3,5) <u>In</u> 95.0%	1. 1. ers <u>Out</u> 5.0%	20 30 Admir <u>Total</u> 21.0%	<u>In</u> 95.0%	persons (3,5) Out 5.0%	per veh	icle 'isitors (<u>In</u> 90.0%	5) Out 10.0%
Auto Taxi/Black Car Femporal Distribution	Stude 1. 1. Grad	ents (2) 20 30 uate Stu (3,5) <u>In</u>	1.: 1.: dents <u>Out</u>	20 30 F a <u>Total</u>	(3 1. 1. nculty (3	2) 20 30 .5) <u>Out</u> 5.0% 50.0%	(3 1. 1. Re Total 19.0% 13.0%	2) 20 30 esearche (3,5) <u>In</u> 95.0% 50.0%	1. 1. ers Out 5.0% 50.0%	20 30 Admir <u>Total</u>	<u>In</u> 95.0% 50.0%	persons (3,5) Out 5.0% 50.0%	per veh	icle 'isitors (<u>In</u>	5) Out 10.0% 50.0%

⁽⁴⁾ Midday percentages derived from various sources and AKRF assumptions

⁽⁵⁾ Weighted percentages and Columbia University/AKRF assumptions

Table 17-4b
Travel Demand Assumptions for Columbia University Components

Daily Trip Rates	Academic Research (1,2)	Academic (3)		ersity ng (4,5)	Retail	(9,11)		
Person Trips			4	.0	47	.42		All rates are per 1,000 gsf, except fo
Truck Trips	0.10	0.03	0.	03	0.3	35		University Housing (per dwelling unit
Modal Split				ersity ing (6)	Reta	iil (9)		
			AM/PM	MD	AM/PM	MD		
Auto			12.0%	12.0%	2.0%	2.0%		
Taxi			3.0%	3.0%	3.0%	3.0%		
Subway			40.0%	40.0%	20.0%	20.0%		
Bus			12.5%	12.5%	5.0%	5.0%		
Shuttle			2.0%	2.0%	0.0%	0.0%		
Commuter Rail			1.5%	1.5%	0.0%	0.0%		
Walk Only			29.0%	29.0%	70.0%	70.0%		
Total			100%	100%	100%	100%		
Vehicle Occupancy				ersity ng (6,7)	Retai	il (10)		
Auto			1.	20	1.	60		All vehicle occupancy figures are
Taxi/Black Car			1.	20	1.:	20		persons per vehicle
Temporal		Unive	ersity Ho (8)	using	ı	Retail (9)	
<u>Distribution</u>		<u>Total</u>	<u>In</u>	Out	Total	<u>ln</u>	Out	
AM Peak Hour		9.1%	20.0%	80.0%	3.1%	50.0%	50.0%	
Midday Peak Hour		4.7%	51.0%	49.0%	19.0%	50.0%	50.0%	
PM Peak Hour		10.7%	65.0%	35.0%	9.6%	50.0%	50.0%	
	Academic Research (1)	Academic (1)		ersity ing (4)	Reta	iil (9)		
		9.7%	9.7	7%	8.0	0%		
Delivery Trip Distribution (In/Out) AM Peak Hour	9.7%	3.7 70			4.4	0%		
Distribution (In/Out)	9.7% 9.1%	9.1%	9.	1%	11.	U 70		

- (5) 4 of 8.075 daily person trips (CEQR) conservatively assumed not to be journey trips to/from University facilities
- (6) 2000 Journey to Work statistics, US Census Bureau; "adjusted percentages used for estimating non-University related trips
- (7) AKRF assumptions
- (8) Urban Space for Pedestrians (1975), Pushkarev & Zupan
- (9) Harlem Park Development EAS (May 2004)
- (10) Harlem Center Project EA (December 1999)
- (11) Retail and Industrial Zoning Text Amendments FGEIS (October 1996)

Table 17-5 2015 Trip Generation Results for Columbia University Components

Analysis Period	Αι	ito	Ta	axi	Sub	way	В	us	Shu	ıttle	Comi	n Rail	Walk	Only		Total	
and Use	In	Out	ln	Out	In	Out	ln	Out	In	Out	In	Out	In	Out	ln	Out	Total
AM Peak Hour																	
Research	56	4	4	0	145	8	25	1	9	0	0	0	75	5	314	18	332
Academic	188	11	23	1	491	26	75	3	19	0	0	0	345	21	1141	62	1203
General Support	6	0	0	0	15	1	3	0	1	0	0	0	6	1	31	2	33
University Housing	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Retail	1	1	1	1	9	9	2	2	0	0	0	0	31	31	44	44	88
Total	251	16	28	2	660	44	105	6	29	0	0	0	457	58	1530	126	1656
Midday Peak Hour																	
Research	6	6	3	3	14	14	6	6	3	3	0	0	99	99	131	131	262
Academic	28	28	11	11	57	57	28	28	17	17	0	0	426	426	567	567	1134
General Support	1	1	0	0	1	1	1	1	0	0	0	0	7	7	10	10	20
University Housing	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Retail	5	5	8	8	54	54	14	14	0	0	0	0	189	189	270	270	540
Total	40	40	22	22	126	126	49	49	20	20	0	0	721	721	978	978	1956
PM Peak Hour																	
Research	6	54	0	4	15	138	2	23	0	8	0	0	10	72	33	299	332
Academic	19	178	3	22	50	466	8	72	1	18	0	0	38	327	119	1083	1202
General Support	1	6	0	0	1	15	0	3	0	1	0	0	1	5	3	30	33
University Housing	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Retail	3	3	4	4	27	27	7	7	0	0	0	0	96	96	137	137	274
Total	29	241	7	30	93	646	17	105	1	27	0	0	145	500	292	1549	1841
Vehicle Trips by	Туре																
Analysis Period	Αι	ıto	Ta	axi	Deli	very	Shu	uttle								Total	
and Use	ln	Out	ln	Out	In	Out	ln	Out			•				ln	Out	Total
AM Peak Hour																	
Research	47	4	4	4	4	4									55	12	67
Academic	156	10	18	18	2	2									176	30	206
General Support	5	0	0	0	0	0									5	0	5
University Housing	0	0	0	0	0	0									0	0	0
Retail	1	1	1	1	2	2									4	4	8
Total	209	15	23	23	8	8	12	12							252	58	310
Midday Peak Hour																	
Research	6	6	3	3	4	4									13	13	26
Academic	24	24	11	11	2	2									37	37	74
General Support	1	1	0	0	0	0									1	1	2
University Housing	0	0	0	0	0	0									0	0	0
Retail	3	3	10	10	2	2									15	15	30
Total	34	34	24	24	8	8	12	12							78	78	156
PM Peak Hour																	
Research	6	45	4	4	2	2									12	51	63
Academic	17	147	18	18	1	1									36	166	202
General Support	1	5	0	0	0	0									1	5	6
University Housing	0	0	0	0	0	0									0	0	0
					1	1	ı	i							1	1	1
Retail	2	2	4	4	0	0									6	6	12

Table 17-6 2030 Trip Generation Results for Columbia University Components

		2030		PO	JIICI (IIC	Julus	101	COIG		u Ch	11 / C1	Sity	COII	роп	
Person Trips by	Mode																
Analysis Period	Αι	uto	Ta	axi	Sub	way	В	us	Shu	uttle	Comr	n Rail	Walk	Only		Total	_
and Use	In	Out	In	Out	In	Out	In	Out	ln	Out	In	Out	In	Out	ln	Out	Total
AM Peak Hour																	
Research	355	20	28	1	859	47	150	8	55	3	0	0	377	21	1824	100	1924
Academic	465	26	50	3	1082	60	182	9	45	1	0	0	650	38	2474	137	2611
General Support	38	2	3	0	92	5	20	1	4	0	0	0	29	2	186	10	196
University Housing	5	20	1	5	16	66	5	21	1	3	1	2	12	47	41	164	205
Retail	2	2	3	3	19	19	5	5	0	0	0	0	67	67	96	96	192
Total	865	70	85	12	2068	197	362	44	105	7	1	2	1135	175	4621	507	5128
Midday Peak Hour																	
Research	35	35	14	14	71	71	35	35	22	22	0	0	523	523	700	700	1400
Academic	57	57	24	24	113	113	57	57	33	33	0	0	841	841	1125	1125	2250
General Support	3	3	1	1	6	6	3	3	2	2	0	0	46	46	61	61	122
University Housing	6	6	2	2	22	21	7	7	1	1	1	1	15	14	54	52	106
Retail	12	12	18	18	117	117	29	29	0	0	0	0	410	410	586	586	1172
Total	113	113	59	59	329	328	131	131	58	58	1	1	1835	1834	2526	2524	5050
PM Peak Hour																	
Research	39	337	2	25	90	817	16	143	5	52	0	0	40	360	192	1734	1926
Academic	49	443	6	47	114	1027	20	174	5	43	0	0	66	614	260	2348	2608
General Support	4	36	0	3	10	87	2	18	0	4	0	0	4	28	20	176	196
University Housing	19	10	5	3	62	34	20	11	3	2	2	1	45	23	156	84	240
Retail	6	6	9	9	59	59	15	15	0	0	0	0	207	207	296	296	592
Total	117	832	22	87	335	2024	73	361	13	101	2	1	362	1232	924	4638	5562
Vehicle Trips by																	
	1	uto	Т:	axi	Deli	ivery	Sh	uttle	1							Total	
Analysis Period and Use	In .	Out	In	Out	In	Out	In	Out							In	Out	Total
AM Peak Hour																	
Research	297	17	22	22	25	25									344	64	408
Academic	389	23	39	39	6	6									434	68	502
General Support	32	2	2	2	0	0									34	4	38
University Housing	4	17	4	4	2	2									10	23	33
Retail	1	1	4	4	4	4									9	9	18
Total	723	60	71	71	37	37	12	12							843	180	1023
Midday Peak Hour				- ' '											040	100	1020
Research	31	31	16	16	24	24									71	71	142
Academic	49	49	27	27	6	6									82	82	164
General Support	3	3	1	1	0	0									4	4	8
University Housing	5	5	3	3	2	2									10	10	20
Retail	8	8	22	22	5	5									35	35	70
Total	96	96	69	69	37	37	12	12							214	214	428
PM Peak Hour	30	30	- 03	- 03	- 31		12	12	1						217	217	720
Research	34	281	20	20	13	13									67	314	381
Academic	42	370	40	40	3	3									85	413	498
General Support	3	370	2	2	0	0									5	32	496 37
University Housing	16	8	5	5	1	1									22	32 14	30
Retail	4	4	12	12	1	1									17	17	34
Total	99	693	79	79	18	18	12	12							208	802	1010
i Utai	99	5 093	19	19	10	10	12	2	1						_∠∪ō	002	: 1010

Academic Research

Based on the reasonable worst-case transportation scenario, for the 2015 analysis year, the population projected for the trip generation analysis includes 263 graduate students, 79 faculty members, 126 post-doctorate and general researchers, 216 administrators, and 45 other general support employees. Approximately 370 person trips were estimated during each of the AM and PM peak hours, and 280 person trips during the midday peak hour. Accounting also for deliveries, the 2015 vehicle trip projections resulted in approximately 70, 30, and 70 vehicle trips during the AM, midday, and PM peak hours, respectively. For the 2030 analysis year, there would be 642 graduate students, 531 faculty members, 855 post-doctorate and general researchers, 1,388 administrators, and 280 other general support employees. The 2030 projected person-trip totals would be approximately 2,100 during the AM and PM peak hours, and 1,500 during the midday peak hour. The vehicle-trip totals were estimated at approximately 450, 150, and 420 during the AM, midday, and PM peak hours, respectively.

Academic

Based on the reasonable worst-case transportation scenario, for the 2015 analysis year, the population projected for the trip generation analysis includes 2,517 graduate students, 534 faculty members, 53 post-doctorate and general researchers, and 651 administrators. Approximately 1,200 person trips were estimated during each of the AM and PM peak hours, and 1,130 person trips during the midday peak hour. Accounting also for deliveries, the 2015 vehicle trip projections resulted in approximately 210, 70, and 200 vehicle trips during the AM, midday, and PM peak hours, respectively. For the 2030 analysis year, there would be 3,680 graduate students, 1,430 faculty members, 140 post-doctorate and general researchers, and 1,740 administrators. The 2030 projected person-trip totals would be approximately 2,610 during the AM and PM peak hours, and 2,250 during the midday peak hour. The vehicle-trip totals were estimated at approximately 500, 160, and 500 during the AM, midday, and PM peak hours, respectively.

General Support

A general support staff would be required to maintain the various operations on the Manhattanville campus, including the below-grade support uses. For the 2015 analysis year, 47 support employees were projected. These employees were estimated to generate less than 20 to 40 person trips and fewer than 10 vehicle trips during each of the AM, midday, and PM peak hours. For the 2030 analysis year, there would be 280 support employees, resulting in 120 to 200 person trips and fewer than 40 vehicle trips during each of the AM, midday, and PM peak hours.

Housing for Graduate Students, Faculty, and Other Employees

It was assumed that most trips made to and from university housing would be linked to other University facilities within Manhattanville. Starting with the standard CEQR residential rate of 8.075 daily trips per dwelling unit, a conservative credit was taken for the linked trip component, resulting in 4.0 non-University related daily trips per dwelling unit. While the housing to/from University trips would mostly be walk only, the non-Manhattanville trips were assumed to follow journey-to-work patterns for all analysis periods, resulting in modal splits of 12.0 percent by auto and 3.0 percent by taxi. Since university housing within Manhattanville would not be available during the 2015 analysis year in the reasonable worst-case transportation scenario, trip projections were developed only for the 2030 future analysis year. The residential units would generate approximately 210, 110 and 240 person trips during the AM, midday, and PM peak

hours, respectively. Accounting also for deliveries, the 2030 vehicle trip projections resulted in 32, 20, and 30 vehicle trips during the AM, midday, and PM peak hours, respectively.

Active Ground-Floor Uses

The ground floors for most of the planned buildings in Subdistrict A are expected to be configured into storefronts to serve both the Columbia University and the adjacent communities. Possible uses include bookstores, exhibits, local retails, and restaurants. For trip generation purposes, all active ground-floor uses were assumed to retain local retail characteristics, serving primarily Columbia University and the nearby community. Since a substantial amount of linkage is expected for these uses, the *Retail and Industrial Zoning Text Amendments GEIS* rate of 47.42 daily trips per 1,000 gsf was used to project future trips attributed to the retail uses planned for Manhattanville. Peak period modal splits were estimated at 2.0 percent auto and 3.0 percent taxi for all analysis hours. For the 2015 analysis year, approximately 90, 540, and 270 person trips were estimated during the AM, midday, and PM peak hours, respectively. Accounting also for deliveries, the 2015 vehicle trip projections resulted in approximately 10, 50, and 20 vehicle trips over the same time periods. For the 2030 analysis year, there would be approximately 190, 1,170, and 590 person trips during the AM, midday, and PM peak hours. The vehicle-trip totals were estimated at approximately 20, 70, and 30 over the same time periods.

Subdistrict B Developments¹

As part of the overall proposed rezoning in Manhattanville, office and retail uses could be developed within available parcels between Twelfth Avenue and Marginal Street, from St. Clair Place to West 135th Street.

The CEQR general office rate of 18.0 daily trips per 1,000 gsf was used to project future trips attributed to the office use. Peak period modal splits were estimated at 26.0 percent auto and 0.5 percent taxi during the morning and evening hours, and are expected to be about half via walk only, with only 14.0 percent auto and 1.0 percent taxi, during the midday hours. In total, approximately 140, 160, and 120 person trips were estimated during the AM, midday, and PM peak hours, respectively. Accounting also for deliveries, the office uses would result in approximately 30, 20, and 30 peak hour vehicle trips over the same time periods. It should be noted that commuting trips associated with the non-Columbia-affiliated commercial uses within Subdistrict B were estimated using the 1990 (instead of 2000) census reverse journey-to-work statistics, which for the Manhattanville area were determined to be more comparable with existing characteristics exhibited in areas surrounding Columbia University's Morningside Heights campus.

Using the *Retail and Industrial Zoning Text Amendments GEIS* rate of 47.42 daily trips per 1,000 gsf, the retail space would generate approximately 180, 420, and 570 person trips during the AM, midday, and PM peak hours, respectively. Unlike other retail developments in the Manhattanville Project Area, this retail space is likely to resemble the more destination-oriented retail uses developed within Hudson River Park. Therefore, rates presented in the *Hudson River Park FEIS* (1998) were used to estimate the modal splits for this waterfront retail use. As a

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¹ CPC is contemplating certain modifications to Subdistrict B. The proposed modifications would rezone Subdistrict B to a modified M1-2 light manufacturing district to support light manufacturing and retail uses. It is anticipated that this modification would not result in any projected development sites in Subdistrict B. The proposed modifications are more fully described in Chapter 29, "Modifications to the Proposed Actions." Chapter 29 also analyzes the potential environmental impacts that could result from the proposed modifications.

result, peak period modal splits were estimated at 35.0 percent auto and 5.0 percent taxi during all analysis hours. Accounting also for deliveries, the retail uses would result in approximately 60, 130, and 160 vehicle trips during the AM, midday, and PM peak hours, respectively.

Subdistrict C Developments

This subdistrict includes the western end of the block bounded by West 133rd Street, West 134th Street, and Twelfth Avenue, and a small portion of the block to the north. Because the existing uses already exceed what the proposed rezoning would permit for this area, no additional development in this area as a result of the Proposed Actions is assumed.

Other Area Developments

Non-Columbia University uses could also be developed within available parcels in the Other Area. Although the Harlem Piers west of Marginal Street is part of this Other Area, no additional development beyond the currently-under-construction waterfront project is assumed <u>for the Future Without the Proposed Actions in association with that project</u>. East of Broadway on the western portion of the block bounded by West 134th and West 135th Streets, residential dwelling units, retail uses, and a 62,000-gsf health center have been identified as potential development as a result of the Proposed Actions.

The CEQR residential rate of 8.075 daily trips per dwelling unit was used to project future trips attributed to the residential use. Peak period modal splits were estimated at 13.0 percent auto and 2.5 percent taxi during all analysis hours. In total, approximately 70, 40, and 90 person trips were estimated during the AM, midday, and PM peak hours, respectively. Accounting also for deliveries, the residential units would result in between 10 and 15 peak hour vehicle trips during the AM and PM peak hours, and less than five vehicle trips during the midday peak hour.

Using the *Retail and Industrial Zoning Text Amendments GEIS* rate of 47.42 daily trips per 1,000 gsf, the retail space would generate approximately 30, 160, and 80 person trips during the AM, midday, and PM peak hours, respectively. Peak period modal splits were estimated at 2.0 percent auto and 3.0 percent taxi during all analysis hours. Accounting also for deliveries, the retail uses would result in negligible traffic in the morning and evening hours and 10 vehicle trips during the midday peak hour.

The community facility component of the planned development was assumed to serve as a 62,000-gsf neighborhood health center. Using information presented in the 506 East 76th Street Rezoning FEIS (1999) and The New York Hospital Program and Facility Development Plan FEIS (1993), trip rates and travel characteristics were developed for the facility's doctors and staff, patients, and visitors. In total, approximately 290, 520, and 490 person trips were estimated during the AM, midday, and PM peak hours, respectively. Accounting also for deliveries, the health center would generate 90, 140, and 150 vehicle trips over the same time periods.

Tables 17-7 and 17-8 provide summaries of the travel demand assumptions and projected person and vehicle trips discussed above for the potential future developments in Subdistrict B and Other Area.

Table 17-7
Travel Demand Assumptions for Subdistrict B and Other Area

Daily Trip Rates								-	ns f								
Daily Trip Rates		SUBDIS	TRICT B					Отн	ER ARE	A							
	Comm		Peta	il (1,4)		dential 5,9)	Pot	ail (1,4)		Doctor	s &	HC: Pat Visi & (12,1)	tors		All rates gsf, exce		1,000
Person Trips	18			.42		075		7.42		10.0	,	59.2			residenti	al (per c	lwelling
Truck Trips	0.2			.35		.03		0.35		0.00		0.3			unit)		
Modal Split	Comm		TRICT B					Отн	ER ARE	A Doctor	-c &	HC: Pat	ionte				
moual opin	Office		Reta	ail (5)	Reside	ential (7)) Re	tail (1)		aff (3,1		Visito					
	AM/PM	MD	AM/PM		AM/PN	• •	AM/PI				•	M/PM	MD				
Auto	26.0%	14.0%	35.0%	35.0%	13.0%	13.0%	2.0%	2.0%	26.0	0% 10	.0% 3	2.0%	32.0%				
Taxi	0.5%	1.0%	5.0%	5.0%	2.5%	2.5%	3.0%	3.09	0.5	% 0.	0% 1	1.0%	11.0%				
Subway	32.5%	21.0%	15.0%	15.0%	52.0%	52.0%	20.0%	6 20.09	% 32.5	5% 0.	0% 2	0.0%	20.0%				
Bus	16.0%	14.0%	10.0%	10.0%	12.5%	12.5%	5.0%	5.0%	16.0	0,	0% 1	7.0%	17.0%				
Shuttle	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%		6.0			0.0%	0.0%				
Commuter Rail	6.0%	0.0%	0.0%	0.0%	1.5%	1.5%	0.0%					0.0%	0.0%				
Walk Only	19.0%	50.0%	35.0%	35.0%		18.5%							20.0%				
Total	100%	100%	100%	100%	100%	100%	100%						100%				
rotai	10070	10070	10070	10070	10070	10070	100 /	100	0 100	70 10	0,0 1	0070	10070				
V-k!-I- 0	_	SUBDIS	TRICT B					Отн	ER ARE								
Vehicle Occupancy	Comm		Date	-:! (a)		dential	р.	4=:1 (2)		Docto		IC: Pat			All vehic	le occur	ancv
Auto	Office			ail (2) .60		,10) .20		tail (2)	3	taff (11 1.00	, .	Visito			figures a		
Taxi/Black Car	1.4			.20		.20		1.60 1.20		1.40				1	vehicle		
Taxi/Black Car	1.4	+0	1.	20	1	.20		1.20		1.40		1.4	U				
		SUBDIS	STRICT B								Отне	R AREA					
Temporal Co	mmercial	Office				Re	sidentia	al				HC:	Doctors a	& Staff		: Patien	
<u>Distribution</u>	(1,2)		R	etail (1,	5)		(8)		F	Retail (1	•		(11)		Vi	isitors (12)
Tota		<u>Out</u>	Total	<u>In</u>	Out	Total	<u>In</u>	Out	Total	<u>In</u>	Out	Tota		Out	Total	<u>In</u>	<u>Out</u>
AM Peak Hour 14.0			3.1%	50.0%			15.0%						6 100.0%		3.7%		15.0%
MD Peak Hour 15.6	% 36.0%	64.0%	7.1%	50.0%	50.0%	4.7%	50.0%	50.0%	19.0%	50.0%	50.0%	17.09	6 50.0%	50.0%	11.3%	65.0%	35.0%
PM Peak Hour 11.8	% 11.0%	89.0%	9.6%	50.0%	50.0%	10.7%	70.0%	30.0%	9.6%	50.0%	50.0%	24.0%	6 50.0%	100.09	6 9.3%	60.0%	40.0%
		SUBDIS	D					0.71	ER ARE								
			TRICIT														
Delivery Trip	Comm		TRICT B					UIF			's & 1	HC: Pat	ients				
Delivery Trip Distribution (In/Out)	Comm	nercial		I (1.5)	Reside	ential (9) Re			Doctor Staff		HC: Pat					
Distribution (In/Out)	Office	nercial e (1,2)	Retai	i I (1,5) 0%		ential (9) 7%		tail (1)		Doctor		Visito	rs (13)				
		nercial e (1,2) 0%	Retai 8.0	i i (1,5) 0% .0%	9.	ential (9) 7% 8%				Doctor			rs (13) %				

Table 17-8 2015 Trip Generation Results for Subdistrict B and Other Area

Person Trips by			1								1						
Analysis Period	Αι	ito		axi	Sub	way		us		uttle	Com	n Rail	Walk	Only		Total	
and Use	ln	Out	In	Out	In	Out	In	Out	In	Out	ln	Out	In	Out	In	Out	Total
AM Peak Hour																	
Subdistrict B																	
Commercial Office	34	2	1	0	43	2	21	1	0	0	8	0	25	2	132	7	139
Retail	32	32	5	5	14	14	9	9	0	0	0	0	32	32	92	92	184
Other Area																	
Residential	1	8	0	2	16	32	1	8	0	0	0	1	3	11	11	62	73
Retail	0	0	0	0	3	3	1	1	0	0	0	0	9	9	13	13	26
Health Center	76	6	14	2	71	4	44	3	0	0	9	0	51	5	265	20	285
Total	143	48	20	9	137	55	76	22	0	0	17	1	120	59	513	194	707
Midday Peak Hour																	
Subdistrict B																	
Commercial Office	8	14	1	1	12	21	8	14	0	0	0	0	27	49	56	99	155
Retail	74	74	11	11	32	32	21	21	0	0	0	0	72	72	210	210	420
Other Area																	
Residential	2	2	0	0	10	10	2	2	0	0	0	0	5	5	19	19	38
Retail	2	2	2	2	16	16	4	4	0	0	0	0	57	57	81	81	162
Health Center	91	51	30	16	54	29	46	25	0	0	0	0	102	77	323	198	521
Total	177	143	44	30	124	108	81	66	0	0	0	0	263	260	689	607	1296
PM Peak Hour																	
Subdistrict B																	
Commercial Office	3	27	0	1	4	34	2	17	0	0	1	6	3	19	13	104	117
Retail	100	100	14	14	43	43	29	29	0	0	0	0	99	99	285	285	570
Other Area																	
Residential	8	3	2	1	31	14	8	3	0	0	1	0	10	5	60	26	86
Retail	1	1	1	1	8	8	2	2	0	0	0	0	29	29	41	41	82
Health Center	66	83	23	16	41	75	35	47	0	0	0	9	40	56	205	286	491
Total	178	214	40	33	127	174	76	98	0	0	2	15	181	208	604	742	1346
-			•		•												
Vehicle Trips by	Type																
Vehicle Trips by		ıto.	Т:	ovi	Deli	verv	Sh	uttle								Total	
Analysis Period	Αι	ıto Out		axi		very		uttle								Total	Total
Analysis Period and Use		ıto Out	Ta In	Out	Del In	very Out	Sh In	uttle Out							ln	Total Out	Total
Analysis Period and Use AM Peak Hour	Αι	1				1									In		Total
Analysis Period and Use AM Peak Hour Subdistrict B	Au In	Out	ln	Out	In	Out										Out	
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office	Au In	Out 2	In	Out 1	In 1	Out 1									28	Out 4	32
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail	Au In	Out	ln	Out	In	Out										Out	
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area	Au In 26 20	Out 2 20	1 6	Out 1 6	1 4	Out 1 4									28 30	Out 4 30	32 60
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential	26 20	Out 2 20 7	1 6 2	1 6 2	1 4 0	Out 1 4 0									28 30 3	Out 4 30 9	32 60
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail	26 20 1	Out 2 20 7 0	1 6 2 0	0ut 1 6 2 0	1 4 0 1	Out 1 4 0 1									28 30 3 1	Out 4 30 9 1	32 60 12 2
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center	26 20 1 0 62	Out 2 20 7 0 4	1 6 2 0 10	0ut 1 6 2 0 10	1 4 0 1 2	Out 1 4 1 1 2	<u>In</u>	Out							28 30 3 1 74	Out 4 30 9 1 16	32 60 12 2 90
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total	26 20 1	Out 2 20 7 0	1 6 2 0	0ut 1 6 2 0	1 4 0 1	Out 1 4 0 1									28 30 3 1	Out 4 30 9 1	32 60 12 2
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour	26 20 1 0 62	Out 2 20 7 0 4	1 6 2 0 10	0ut 1 6 2 0 10	1 4 0 1 2	Out 1 4 1 1 2	<u>In</u>	Out							28 30 3 1 74	Out 4 30 9 1 16	32 60 12 2 90
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B	26 20 1 0 62 109	Out 2 20 7 0 4 33	1 6 2 0 10 19	Out 1 6 2 0 10 19	1 4 0 1 2 8	Out 1 4 0 1 2 8	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60	32 60 12 2 90 196
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office	26 20 1 0 62 109	2 20 7 0 4 33 11	1 6 2 0 10 19	Out 1 6 2 0 10 19	1 4 0 1 2 8 1	Out 1 4 0 1 2 8	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60	32 60 12 2 90 196
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail	26 20 1 0 62 109	Out 2 20 7 0 4 33	1 6 2 0 10 19	Out 1 6 2 0 10 19	1 4 0 1 2 8	Out 1 4 0 1 2 8	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60	32 60 12 2 90 196
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area	26 20 1 0 62 109	7 0 4 33 11 46	1 6 2 0 10 19 1 1 1 3	Out 1 6 2 0 10 19 1 1 13	1 4 0 1 2 8 1 5	Out 1 4 0 1 2 8 1 5	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60 13 64	32 60 12 2 90 196
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential	26 20 1 0 62 109 6 46 2	7 0 4 33 11 46 2	1 6 2 0 10 19 1 13 0	Out 1 6 2 0 10 19 1 13	1 4 0 1 2 8 1 5 0	Out 1 4 0 1 2 8 1 5 0	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60 13 64	32 60 12 2 90 196
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail	26 20 1 0 62 109 6 46 2	Out 2 20 7 0 4 33 11 46 2 1	1 6 2 0 10 19 1 13 0 3	Out 1 6 2 0 10 19 1 13 0 3	1 4 0 1 2 8 1 5 0 1	Out 1 4 0 1 2 8 1 5 0 1	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60 13 64 2 5	32 60 12 2 90 196 21 128 4
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center	26 20 1 0 62 109 6 46 2 1 59	7 0 4 33 11 46 2 1 34	1 6 2 0 10 19 1 13 0 3 21	0ut 1 6 2 0 10 19 1 13 0 3 21	1 4 0 1 2 8 1 5 0 1 2	Out 1 4 0 1 2 8 1 5 0 1 2	0	Out							28 30 3 1 74 136	9 1 16 60 13 64 2 5 57	32 60 12 2 90 196 21 128 4 10 139
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total	26 20 1 0 62 109 6 46 2	Out 2 20 7 0 4 33 11 46 2 1	1 6 2 0 10 19 1 13 0 3	Out 1 6 2 0 10 19 1 13 0 3	1 4 0 1 2 8 1 5 0 1	Out 1 4 0 1 2 8 1 5 0 1	<u>In</u>	Out							28 30 3 1 74 136	Out 4 30 9 1 16 60 13 64 2 5	32 60 12 2 90 196 21 128 4
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour	26 20 1 0 62 109 6 46 2 1 59	7 0 4 33 11 46 2 1 34	1 6 2 0 10 19 1 13 0 3 21	0ut 1 6 2 0 10 19 1 13 0 3 21	1 4 0 1 2 8 1 5 0 1 2	Out 1 4 0 1 2 8 1 5 0 1 2	0	Out							28 30 3 1 74 136	9 1 16 60 13 64 2 5 57	32 60 12 2 90 196 21 128 4 10 139
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B	26 20 1 0 62 109 6 46 2 1 59 114	7 0 4 33 11 46 2 1 34 94	1 6 2 0 10 19 1 13 0 3 21 38	0ut 1 6 2 0 10 19 1 13 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 9	Out 1 4 0 1 2 8 1 5 0 1 2 9 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 57 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail Commercial Office Retail Commercial Office Retail Commercial Office	26 20 1 0 62 109 6 46 2 1 59 1114	2 20 7 0 4 33 11 46 2 1 34 94 21	1 6 2 0 10 19 1 13 0 3 21 38	0ut 1 6 2 0 10 19 1 13 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 0 0	Out 1 4 0 1 2 8 1 5 0 1 2 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	Out 4 30 9 1 16 60 13 64 2 5 57 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail	26 20 1 0 62 109 6 46 2 1 59 114	7 0 4 33 11 46 2 1 34 94	1 6 2 0 10 19 1 13 0 3 21 38	0ut 1 6 2 0 10 19 1 13 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 9	Out 1 4 0 1 2 8 1 5 0 1 2 9 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 57 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail Center Total Office Retail Center Total Office Retail Office Retail Office Retail Other Area	At In 26 20 1 0 62 109 6 46 2 1 59 114 2 63	2 20 7 0 4 33 11 46 2 1 34 94 21	1 6 2 0 10 19 1 13 0 3 21 38 1 18	0ut 1 6 2 0 10 19 1 13 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 0 0	Out 1 4 0 1 2 8 1 5 0 1 2 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 7 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail	26 20 1 0 62 109 6 46 2 1 59 1114	2 20 7 0 4 33 11 46 2 1 34 94 21	1 6 2 0 10 19 1 13 0 3 21 38	0ut 1 6 2 0 10 19 1 13 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 0 0	Out 1 4 0 1 2 8 1 5 0 1 2 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 57 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Retail Health Center Total PM Peak Hour Subdistrict B Commercial Office Retail Center Total Office Retail Center Total Office Retail Office Retail Office Retail Other Area	At In 26 20 1 0 62 109 6 46 2 1 59 114 2 63	2 20 7 0 4 33 11 46 2 1 34 94 21 63	1 6 2 0 10 19 1 13 0 3 21 38 1 18	0ut 1 6 2 0 10 19 1 133 0 3 21 38	1 4 0 1 2 8 1 5 0 1 2 9 0 1 1	Out 1 4 0 1 2 8 1 5 0 1 2 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 7 141	32 60 12 2 90 196 21 128 4 10 139 302
Analysis Period and Use AM Peak Hour Subdistrict B Commercial Office Retail Other Area Residential Health Center Total Midday Peak Hour Subdistrict B Commercial Office Retail Health Center Total Piter Area Residential Retail Health Center Total Commercial Office Retail Other Area Subdistrict B Commercial Office Retail Commercial Office Retail Other Area Residential	At In 26 20 109 6 46 20 114 2 63 7	7 0 4 33 11 46 2 1 34 94 21 63 3	1 1 6 2 0 10 19 13 0 3 21 38 1 18 2	0ut 1 6 2 0 10 19 1 13 0 3 21 38 1 18 2	1 1 4 0 1 2 8 8 1 5 0 1 2 9 0 1 1 0	Out 1 4 0 1 2 8 1 5 0 1 2 9	0	Out							28 30 3 1 74 136 8 64 2 5 82 161	9 1 16 60 13 64 2 5 57 141	32 60 12 2 90 196 21 128 4 10 139 302 25 164

C. EXISTING CONDITIONS

While the DEIS determined that the weekday morning and evening hours would be the most critical time periods for identifying potential traffic impacts, a midday analysis was added to more fully depict current and projected future traffic conditions with and without the Proposed Actions. The sections below detail the data collection efforts performed, current traffic operations for the three analysis time periods, and existing on- and off-street parking conditions.

TRAFFIC VOLUMES

Existing traffic volumes were developed using traffic data collected in April 2004 when New York City public schools and Columbia University were both still in session. The field efforts included manual counts (performed over two weekdays on Tuesday, April 20th and Thursday, April 22nd) at the intersections described in Section B, under Study Area, during the weekday morning (7 AM to 10AM), midday (noon to 2 PM), and evening (4 PM to 7 PM) time periods and continuous automatic traffic recorder (ATR) machine counts for a 10-day period between April 16th and April 25th. These machine counts were taken at 24 locations, as shown below. To reflect the current year of analysis, 2006, traffic volumes collected in 2004 were increased by 0.5 percent per annum to account for background growth.

- Riverside Drive southbound south of 135th Street
- Riverside Drive northbound south of 135th Street
- West 125th Street westbound between Amsterdam Avenue and Convent Avenue
- West 125th Street eastbound between Amsterdam Avenue and Broadway
- Amsterdam Avenue northbound between Lasalle Street and West 125th Street
- Amsterdam Avenue southbound between West 126th Street and West 125th Street
- West 125th Street eastbound between St. Clair Place and Broadway
- West 125th Street westbound between Broadway and Amsterdam Avenue
- Broadway northbound between West 125th Street and Tiemann Place
- Broadway southbound between West 125th Street and West 126th Street
- West 125th Street eastbound between Marginal Street and Twelfth Avenue
- West 125th Street westbound between 129th Street and Twelfth Avenue
- Twelfth Avenue northbound between St. Clair Place and 125th Street
- Twelfth Avenue southbound between West 130th Street and West 125th Street
- West 133rd Street eastbound between Broadway and Twelfth Avenue
- West 133rd Street westbound between Broadway and Twelfth Avenue
- West 138th Street westbound between Broadway and Hamilton Place
- Broadway northbound between West 137th Street and West 138th Street
- Broadway southbound between West 138th Street and West 139th Street
- Henry Hudson Parkway northbound exit ramp at St. Clair Place
- Henry Hudson Parkway southbound entrance ramp at St. Clair Place
- Henry Hudson Parkway northbound entrance ramp at West 133rd Street
- Henry Hudson Parkway southbound exit ramp at West 133rd Street
- Marginal Street southbound between West 132nd Street and West 133rd Street

An inventory of the analyzed intersections was also conducted to determine traffic signal timings, phasing, and cycle lengths, street and curbside signage, pavement markings, and lane

dimensions to be used in the calculation of street capacities. Official signal timing data were collected from NYCDOT for confirmation of field observations.

Figures 17-3, 17-4, and 17-5 show the existing traffic volumes within the primary study area for the weekday 8 to 9 AM, 1 to 2 PM, and 4 to 5 PM peak hours, respectively. The secondary study area peak hour volumes are presented in Figures 17-6, 17-7, and 17-8. The selected peak hours represent periods when the combination of vehicular traffic generated by the Proposed Actions and background traffic would be greatest.

Peak hour volumes on West 125th Street are approximately 220 vehicles per hour (vph) eastbound and 930 vph westbound during the AM peak hour, approximately 370 vph eastbound and 820 vph westbound during the midday peak hour, and approximately 420 vph eastbound and 1,140 vph westbound during the PM peak hour. Along Broadway, AM peak hour volumes are approximately 500 vph northbound and 700 vph southbound. The midday peak hour volumes are approximately 500 vph northbound and 540 vph southbound. The PM peak hour volumes are approximately 830 vph northbound and 550 vph southbound. Along Amsterdam Avenue, northbound volumes are approximately 670 vph, 400 vph, and 860 vph during the AM, midday, and PM peak hours, respectively. Southbound volumes are approximately 500 vph, 450 vph, and 470 vph during the AM, midday, and PM peak hours, respectively. In the vicinity of the Project Area, northbound volumes on Riverside Drive are approximately 380 vph, 320 vph, and 1,920 vph during the AM, midday, and PM peak hours, respectively, while southbound volumes are approximately 1,350 vph, 160 vph, and 220 vph during the same periods. Along Marginal Street, peak hour volumes are approximately 640 vph in the morning, 700 vph in the midday, and 1,050 vph in the evening.

INTERSECTION CAPACITY ANALYSIS

Most intersections in both the primary and secondary study areas operate at overall acceptable levels. Locations with notable service constraints, those operating at mid-LOS D (45.0 seconds of delay for signalized intersections and 30.0 seconds of delay for unsignalized intersections) or worse are described below. Detailed analysis results are summarized in Tables 17-9 and 17-10 for intersections in the primary study area and in Tables 17-11 and 17-12 for intersections in the secondary study area.

PRIMARY STUDY AREA

Twelfth Avenue and West 133rd Street

During the PM peak hour, the westbound left-through-right movement operates at LOS D with an average delay of 48.5 seconds and a v/c ratio of 0.89.

Twelfth Avenue and West 125th Street

During the PM peak hour, the westbound right-turn movement operates at LOS E with an average delay of 55.8 seconds and a v/c ratio of 1.01.

Broadway Northbound and West 135th Street

During the PM peak hour, the eastbound *de facto* left-turn movement operates at LOS D with an average delay of 51.7 seconds and a v/c ratio of 0.77.

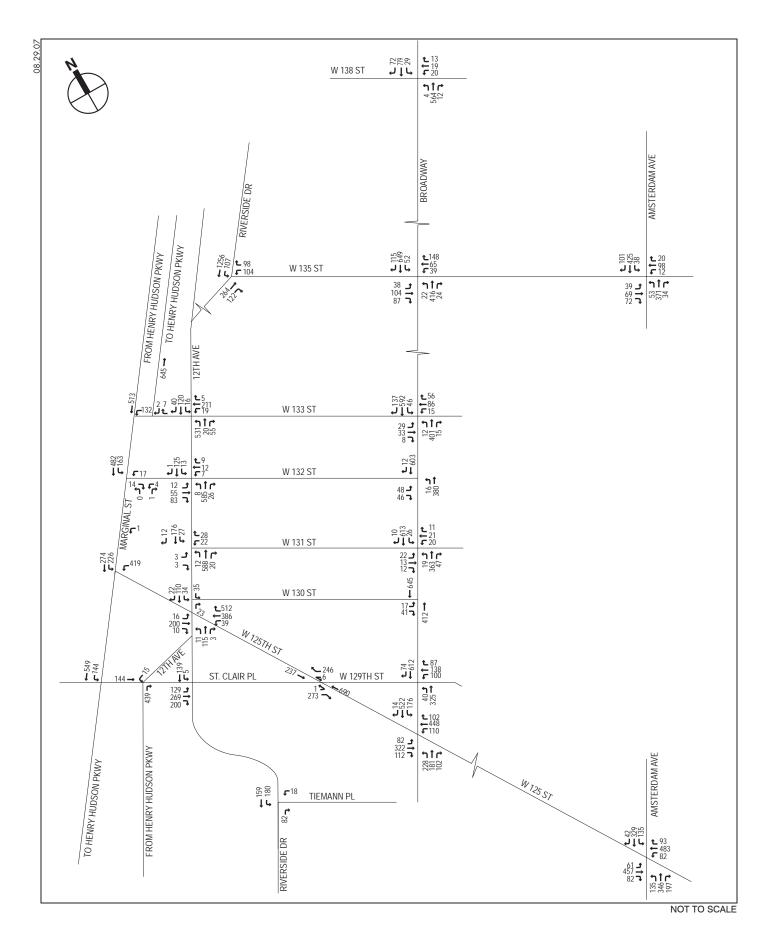


Figure 17-3
Primary Study Area
Existing 2006 Morning Peak Hour

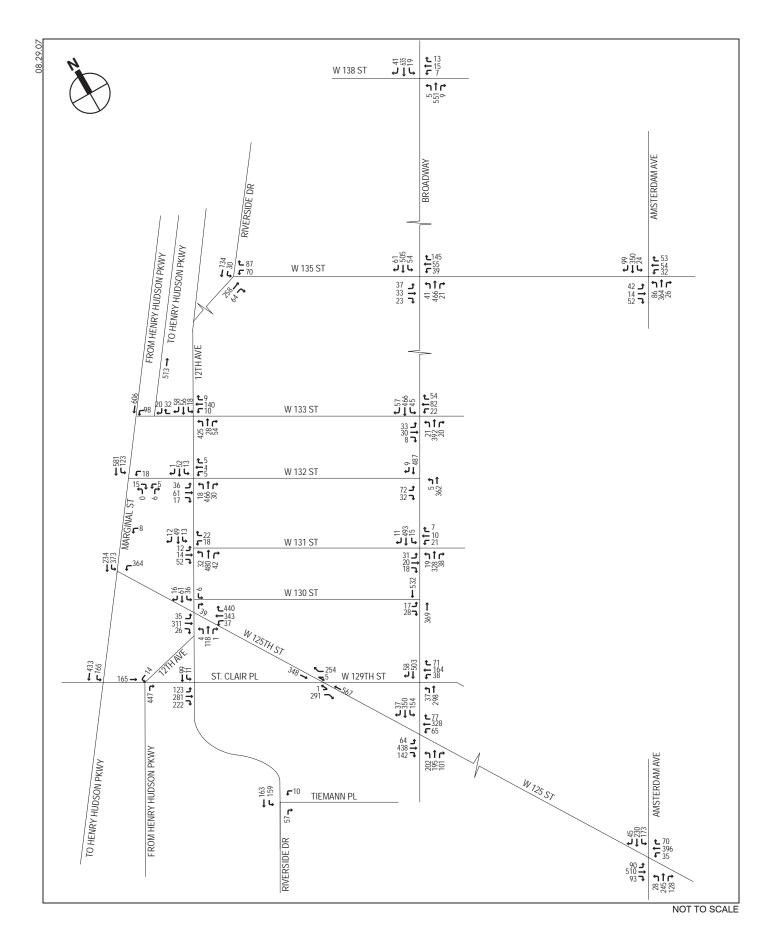


Figure 17-4
Primary Study Area
Existing 2006 Midday Peak Hour

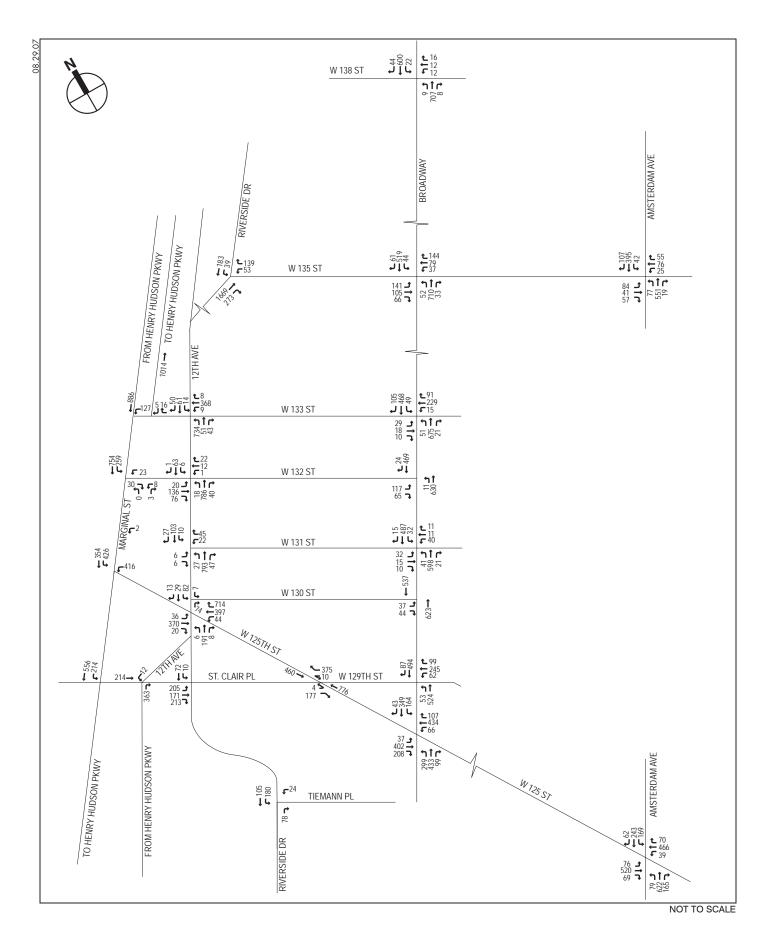


Figure 17-5
Primary Study Area
Existing 2006 Evening Peak Hour

Table 17-9
2006 Existing Conditions
Primary Study Area Signalized Inersection Level-of-Service Analysis

				ruuy .	Area Si							ilalybib
		AM Pe	ak Hour			idday I	Peak Hou	<u>ır</u>		PM Pe	ak Hour	
Interesetion	Lane	V/C	Delay	1.00	<u>Lane</u>	V/C	<u>Delay</u>	1.00	Lane	VIC	Delay	LOS
Intersection		V/C	(spv)	LOS	<u>Group</u>	<u>V/C</u>	<u>(spv)</u>	LOS	Group	V/C	(spv)	LUS
Marginal Stre				_		0.05	00.7	_		0.44	04.5	0
Westbound		0.40	24.3	С	⊨	0.35	23.7	<u>C</u> <u>B</u>	L	0.41	24.5	С
Southbound		0.23	12.1	В	<u> </u>	0.36	13.6	불	L	0.41	14.3	В
	LT Int.	0.25	11.8 17.5	B B	<u>LT</u> Int.	<u>0.25</u>	<u>11.8</u> 16.8	<u>B</u> B	LT Int.	0.35	12.8 17.3	B B
Riverside Dr		loot 12E		Ь	ШЬ		10.0	<u> </u>	IIIL.		17.3	D
Westbound		0.32	28.0	С	1 1	0.21	26.1	_	L	0.16	25.6	С
vvestbouriu	R	0.32	30.9	C	L R	0.34	29.3	<u>C</u> <u>C</u> <u>A</u> A	R	0.10	36.4	D
Northbound		0.40	8.1	A	協	0.19	<u>29.5</u> 7.9	<u>S</u>	TR	0.98	23.3	C
Southbound		0.22	11.4	В	置	0.10	7.4	₽	LT	0.30	8.1	A
Southbound	Int.	0.01	12.6	В	Int.	0.10	<u>1.4</u> 12.4	B	Int.	0.13	22.7	Ĉ
Twelfth Aver		/est 133			1116.		16.1		1116.			
Westbound		0.56	28.9	С	LTR	0.40	25.0	С	LTR	0.89	48.5	D
Northbound		0.91	39.9	D	L <u>= 1115</u>	0.70	<u>23.0</u> 21.8		L	0.79	26.3	C
rtoranboana	LTR	0.18	10.8	В	<u>LTR</u>	0.16	10.7	B	LTR	0.60	17.8	B
Southbound		0.26	11.7	В	LTR	0.20	11.1	B	LTR	0.17	10.8	В
	Int.		28.6	С	Int.		18.9	<u>B</u>	Int.		28.9	С
Twelfth Aver	nue @ W	/est 132										
Eastbound		0.31	23.2	С	LTR	0.24	22.2	<u>C</u>	LTR	0.44	25.2	С
Westbound		0.06	20.0	В	LTR	0.03	19.7	В	LTR	0.07	20.1	С
Northbound		0.45	13.3	В	LTR	0.37	12.4	В	LTR	0.60	15.6	В
Southbound	LTR	0.24	11.6	В	LTR	0.13	10.5		LTR	0.11	10.2	В
	Int.		14.9	В	Int.		14.0	<u>B</u>	Int.		17.3	В
Twelfth Aver	nue @ N	/est 125	th Street									
Eastbound		0.18	12.7	В	LTR	0.29	13.7	<u>B</u>	LTR	0.31	13.9	В
Westbound		0.12	12.6	В	L	0.12	12.7	<u>B</u>	L	0.15	13.1	В
	Т	0.57	18.9	В	<u>T</u> <u>R</u>	0.49	<u>17.1</u>	B	Т	0.57	18.8	В
	R	0.86	34.1	С		0.85	<u>32.5</u>	<u>C</u>	R	1.01	55.8	Е
Northbound		0.23	19.3	В	LTR	0.22	<u>19.2</u>	<u>B</u>	LTR	0.36	21.1	С
Southbound		0.34	21.4	С	<u>LT</u>	<u>0.21</u>	<u>19.3</u>	BIBIBICIBIBIC	LT	0.28	20.5	С
	Int.		23.2	С	<u>Int.</u>		<u>21.6</u>	<u>C</u>	Int.		32.0	С
Broadway @				•		0.40	00.0	_		0.40	00 -	•
Westbound		0.16	24.1	C	LTR	0.10	<u>23.3</u>	<u>C</u> <u>A</u> <u>A</u>	LTR	0.13	23.7	C
Northbound		0.31	6.0	A	탈	0.30	6.0	<u>A</u>	LT	0.37	6.3	A
Couthbours	R	0.02	4.7	A	<u>R</u>	0.02	<u>4.7</u>		R	0.02	4.7	A
Southbound	LTR Int.	0.52	7.7 7.6	A A	LTR Int	<u>0.40</u>	<u>6.7</u> 6.8	<u>A</u> A	LTR Int.	0.38	6.5 6.9	A A
Broadway No		nd @ M			Int.		0.0		IIIL.		0.8	
Eastbound		0.31	25.1	C	LI	0.23	24.3	<u>C</u>	DefL	0.77	51.7	D
Lasiboullu	L'	0.51	۷. ۱	O	===	<u>U.Z.J</u>	47.0	프	T	0.77	27.1	С
Westbound	TR	0.46	28.0	С	IR	0.45	28.0	<u>C</u>	TR	0.38	28.4	C
Northbound		0.39	6.7	A	<u>itk</u>	0.42	<u>7.0</u>	<u><u>B</u></u>	LTR	0.59	8.7	A
. to a local la	Int.	0.00	16.4	В	<u>Int.</u>	<u> </u>	<u>7.0</u> 15.1	<u>B</u>	Int.	0.00	19.1	В
Broadway So		nd @ W						=_				
Eastbound		0.38	26.6	C	<u>TR</u>	0.15	23.5	С	TR	0.48	27.9	С
Westbound		0.21	24.0	Č	LT	0.23	<u>24.1</u>	<u>C</u> <u>C</u>	LT	0.28	24.8	Č
Southbound		0.08	5.0	Ä		0.09	<u>5.1</u>	Ä	L	0.07	5.0	Ä
	TR	0.62	9.5	Α	TR	0.48	7.6	A	TR	0.47	7.5	Α
	Int.		14.4	В	Int.		11.8	В	Int.		15.8	В
Notes: L:	Left Tur	n; T: Thr	ough; R: F	Right Tur	n; DefL: [Defacto I	Left Turn;	Int.: Inte	ersection			
V	/C: Volun	ne to Cap	pacity; spv	: Secon	ds per Ve	hicle; L0	OS: Level	of Servi	ce			

Table 17-9 (Continued) 2006 Existing Conditions

Primary Study Area Signalized Inersection Level-of-Service Analysis

			ak Hour	 J			Peak Ho				rvice Ai	J 526
	Lane	Amiro	Delay		Lane	lidddy i	Delay	<u> </u>	Lane		Delay	
Intersection		V/C	(spv)	LOS	Group	V/C	(spv)	LOS	Group	V/C	(spv)	LOS
Broadway No						<u> 1/U</u>	(SPV)	LUU	Croup	1/0	(354)	LOU
Eastbound		0.20				0.00	47 E	В	LT	0.18	17.1	В
			17.2	B C	뜵	0.22	<u>17.5</u>	<u>B</u> <u>C</u>				
Westbound		0.34	21.1		TR	0.31	<u>20.6</u>	늗	TR	0.67	29.0	С
Northbound		0.39	11.7	В	트	0.35	<u>11.3</u>	<u>B</u> <u>A</u>	LT	0.66	15.6	В
	R	0.04	9.0	Α	<u>.</u> <u>R</u>	0.05	9.0	≜	R	0.05	9.0	Α
	Int.		14.6	В	<u>Int.</u>		<u>14.4</u>	<u>B</u>	Int.		19.3	В
Broadway So												
Eastbound		0.12	17.9	В	IR	0.12	<u>18.0</u>	<u>B</u>	TR	0.09	17.7	В
Westbound	LT	0.20	17.3	В	<u>LT</u>	0.21	<u>17.3</u>	B	LT	0.47	21.0	С
Southbound	LTR	0.46	12.1	В	LTR	0.33	<u>10.8</u>	<u>B</u>	LTR	0.38	11.2	В
	Int.		13.2	В	Int.		12.6	В	Int.		14.6	В
Broadway No	orthbou	nd @ W	est 132n	d Street								
Eastbound		0.11	16.4	В	L	0.14	<u>16.6</u>	<u>B</u>	L	0.22	17.5	В
Northbound		0.32	10.9	В	<u>LT</u>	0.29	10.6	<u>B</u>	LT	0.49	12.6	В
	Int.		11.5	В	Int.		11.6	B	Int.		13.4	В
Broadway So		nd @ W										
Eastbound		0.22	19.5	В	<u>TR</u>	0.20	<u>19.0</u>	<u>B</u>	TR	0.34	20.9	С
Westbound		0.03	15.5	В	III	0.01	15.4	<u>B</u>	LT	0.02	15.4	В
Southbound		0.44	12.0	В	LTR	0.35	11.1	<u>B</u>	LTR	0.36	11.2	В
Southbound	Int.	0.77	13.1	В	Int.	0.00	12.5	<u>≅</u> B	Int.	0.50	13.8	В
Broadway @		24 -4 C4-		Ь	шь		12.0	<u> </u>	IIIL.		13.0	Ь
				0	LTD	0.44	20.0	0	LITO	0.40	20.6	0
Eastbound		0.10	20.4	С	LTR	<u>0.14</u>	<u>20.9</u>	늦	LTR	0.12	20.6	С
Westbound		0.10	20.5	С	뎐	0.09	<u>20.4</u>	<u>E</u>	LT	0.14	21.1	С
	R	0.03	19.7	В	. <u>R</u>	0.02	<u>19.6</u>	₽	R	0.03	19.7	В
Northbound		0.27	7.9	Α	LTR	0.23	<u>7.7</u>		LTR	0.39	8.8	Α
Southbound		0.39	8.8	Α	<u>LTR</u>	0.30	8.2		LTR	0.33	8.4	Α
	Int.		9.5	Α	<u>Int.</u>		<u>9.3</u>	<u>A</u>	Int.		9.7	Α
Broadway @					i							
Eastbound	LR	0.18	24.4	С	<u>LR</u>	0.13	<u>23.7</u>	<u>C</u>	LR	0.24	25.4	С
Northbound	Т	0.21	5.4	Α	I	0.18	<u>5.3</u>	<u>A</u>	T	0.28	5.8	Α
Southbound	Т	0.31	6.0	Α	<u> </u>	0.25	<u>5.6</u>	<u>A</u>	Т	0.27	5.7	Α
	Int.		6.7	Α	Int.		6.4	A	Int.		7.0	Α
Broadway @		29th Str	eet						•			
Westbound		0.38	18.2	В	<u>LT</u>	0.35	<u>17.8</u>	<u>B</u>	LT	0.46	19.6	В
	R	0.19	16.0	В	R	0.15	15.5	<u>=</u> <u>B</u>	R	0.22	16.4	В
Northbound		0.33	17.0	В	Î	0.30	16.6	<u>B</u>	LT	0.57	20.6	C
Southbound		0.48	15.9	В	ĪR	0.37	17.3	<u>B</u>	TR	0.39	17.6	В
Southbound	Int.	5.45	16.6	В	Int.	<u> </u>	17.1	B	Int.	0.00	19.0	В
Broadway @		25th Str			11.16.		<u> </u>	<u> </u>	1 1111.		10.0	ر
Eastbound		0.55	36.8	D	1	0.35	26.5	C	L	0.27	25.7	С
Lasiboullu	TR	0.55	26.8	C	ĪR	0.33	30.6	<u>C</u> C	TR	0.27	36.7	D
Moothoused				_				_				_
Westbound		0.68	44.6 20.4	D	<u>L</u>	0.54	<u>39.1</u>		L	0.65	50.8	D
Nowth land of	TR	0.71	30.4	C	TR	0.56	<u>26.5</u>	$\stackrel{\smile}{\sim}$	TR	0.68	29.3	С
Northbound		0.43	32.1	С	<u>L</u> <u>LT</u>	0.38	<u>31.1</u>	监	L	0.56	35.4	D
	LT	0.41	30.5	С		0.43	<u>30.7</u>	띁	LT	0.82	42.9	D
	R	0.55	39.8	D	<u>R</u>	<u>0.54</u>	<u>39.6</u>	Ď	R	0.51	37.8	D
Southbound		0.33	30.1	C	. <u>L</u> _	0.28	<u>29.1</u>	<u>c</u>	L_	0.25	28.5	C
	LTR	0.90	49.1	D	LTR	<u>0.70</u>	<u>36.8</u>	<u>D</u>	LTR	0.75	38.9	D
	Int.		35.7	D	<u>Int.</u>		<u>31.6</u>	<u>C</u>	Int.		36.6	D
Notes: L:	Left Turi	n; T: Thre	ough; R: F	Right Tur	n; DefL: [Defacto	Left Turn;	Int.: Inte	ersection			
			pacity; sp									

Table 17-9 (Continued) 2006 Existing Conditions

Primary Study Area Signalized Inersection Level-of-Service Analysi	Primary Stu	dv Area	Signalized	Inersection l	Level-of-Ser	vice Analysis
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			iiiiai y i	J 1 1 2 1 2 J								
		AM Pe	ak Hour		<u>N</u>	lidday F	Peak Ho	<u>ur</u>		PM Pe	ak Hour	
	Lane		Delay		Lane		Delay		Lane		Delay	
Intersection	Group	V/C	(spv)	LOS	Group	V/C	(spv)	LOS	Group	V/C	(spv)	LOS
Amsterdam A	Avenue	@ West	135th S	treet								
Eastbound	LT	0.31	26.4	С	<u>LT</u>	0.20	24.8	<u>C</u>	LT	0.47	30.8	С
	R	0.36	29.0	С	<u>LT</u> <u>R</u>	0.22	25.6	C	R	0.25	26.3	С
Westbound	LTR	0.36	27.1	С	LTR	0.45	29.6	C	LTR	0.48	30.2	С
Northbound	LTR	0.44	7.4	Α	LTR	0.50	8.2	A	LTR	0.63	10.0	Α
Southbound	LTR	0.52	8.2	Α	LTR	0.44	7.4		LTR	0.52	8.3	Α
	Int.		12.3	В	Int.	<u> </u>	12.0	B	Int.		13.8	В
Amsterdam A	Avenue	@ West	125th S	treet								
Eastbound	L	0.56	41.8	D	L	0.72	53.2	D	L	0.58	41.0	D
	TR	0.77	34.1	С	TR	0.82	36.9	D	TR	0.79	34.8	С
Westbound	L	0.67	49.4	D	L	0.38	32.9	C	L	0.42	35.4	D
	TR	0.76	33.3	С	TR	0.68	30.4	C	TR	0.71	31.4	С
Northbound	L	0.31	14.4	В	<u>TR</u> <u>L</u> <u>TR</u>	0.06	9.7	A	L	0.20	15.4	В
	TR	0.67	26.7	С	<u>TR</u>	0.47	22.3	<u>C</u>	TR	0.84	33.4	С
Southbound	L	0.47	22.9	С	<u>L</u>	0.48	18.8	B	L	0.58	33.9	С
	TR	0.41	21.2	С	TR	0.30	19.8		TR	0.73	33.4	С
	Int.		29.3	С	Int.		<u>29.2</u>	<u>C</u>	Int.		33.0	С
Notes: L:	Left Turr	n; T: Thre	ough; R: F	Right Tur	n; DefL: [Defacto I	Left Turn;	Int.: Inte	ersection			
V/	C: Volun	ne to Cap	pacity; spv	r: Secon	ds per Ve	hicle; L0	DS: Level	of Servi	ce			

Broadway and West 125th Street

During the AM peak hour, the southbound shared left-through-right movement operates at LOS D, with an average delay of 49.1 seconds and a v/c ratio of 0.90.

During the PM peak hour, the westbound left-turn movement operates at LOS D, with an average delay of 50.8 seconds and a v/c ratio of 0.65.

Amsterdam Avenue and West 125th Street

During the AM peak hour, the westbound left-turn movement operates at LOS D, with an average delay of 49.4 seconds and a v/c ratio of 0.67.

<u>During the midday peak hour, the eastbound left-turn movement operates at LOS D, with an average delay of 53.2 seconds and a v/c ratio of 0.72.</u>

Marginal Street and West 133rd Street

During the PM peak hour, the southbound approach operates at LOS E, with an average delay of 40.8 seconds and a v/c ratio of 0.96.

West 125th Street and West 129th Street/St. Clair Place

During the AM peak hour, westbound left-turn movement operates at LOS E, with an average delay of 43.7 seconds and a v/c ratio of 0.06.

During the PM peak hour, the eastbound left-turn movement operates at LOS F. The westbound left-turn movement operates at LOS F, with an average delay of 115.4 seconds and a v/c ratio of 0.26 and the westbound right-turn movement operates at LOS F, with an average delay of 92.0 seconds and a v/c ratio of 1.05.

SECONDARY STUDY AREA

Broadway and West 110th Street

During the AM peak hour, the westbound *de facto* left-turn movement operates at LOS E, with an average delay of 55.1 seconds and a v/c ratio of 0.85.

Table 17-10
2006 Existing Conditions
Primary Study Area Unsignalized Incrsection Level-of-Service Analysis

			iary Sii	iuy III					1			iidiy 515
		AM Pe	ak Hour			lidday i	Peak Hou	<u>ur</u>		PM Pe	ak Hour	
	Lane		Delay		<u>Lane</u>		<u>Delay</u>		Lane		Delay	
Intersection		V/C	(spv)	LOS	<u>Group</u>	V/C	(spv)	LOS	Group	V/C	(spv)	LOS
Marginal Stre												
Westbound		0.18	10.5	В	<u>L</u> T	0.13	<u>10.1</u>	<u>B</u> C	L	0.17	10.3	В
Southbound		0.68	18.5	С	I	<u>0.78</u>	<u>23.4</u>	<u>C</u>	Т	0.96	40.8	Е
Marginal Stre		est 132	nd Stree	t								
Westbound		-	9.0	Α	LT	≣	9.3	<u>A</u>	LT	-	9.7	Α
Southbound		-	14.5	В	LT	= =	<u>13.4</u>	B	LT	-	29.2	D
	TR	-	7.9	Α	<u>TR</u>	≣	<u>8.1</u>	<u>A</u>	TR	-	9.5	Α
	Int.		13.0	В	<u>Int.</u>		<u>12.0</u>	B	Int.		22.9	С
Marginal Stre												
Southbound		0.17	9.8	Α	<u>L</u>	0.20	10.0	<u>A</u> C	L	0.26	10.4	В
	Т	0.76	22.0	С	I	0.68	<u>18.5</u>	<u>C</u>	Т	0.81	26.1	D
Twelfth Aver	nue @ W	est 131	st Street									
Northbound		0.01	7.8	Α	<u>LT</u>	0.02	<u>7.6</u>	<u>A</u>	LT	0.02	7.7	Α
Southbound		0.04	9.6	Α	<u>LT</u>	0.02	9.3	<u>A</u>	LT	0.02	10.5	В
Westbound		0.16	16.8	С	<u>LTR</u>	<u>0.13</u>	<u>16.5</u>	<u>A</u> <u>A</u> <u>C</u>	LTR	0.28	23.5	С
Eastbound	LTR	0.01	12.9	В	<u>LTR</u>	<u>0.15</u>	<u>12.6</u>	<u>B</u>	LTR	0.03	13.9	В
Twelfth Aver	nue @ W	est 125'	th Street	South	ound R	ight Tui	rn					
Southbound		0.04	10.4	В	<u>R</u>	0.03	<u>10.0</u>	<u>B</u>	R	0.02	10.1	В
Twelfth Aver	nue @ Si	t. Clair l	Place									
Eastbound		-	9.3	Α	I	Ξ	9.8	<u>A</u>	Т	-	10.2	В
Northbound	R	-	10.6	В	I R L	= =	<u>11.5</u>	<u>B</u>	R	-	10.4	В
Southbound		-	8.8	Α		≣	<u>8.6</u>	Α	L	-	8.6	Α
	Int.		10.2	В	<u>Int.</u>		<u>11.0</u>	<u>B</u>	Int.		10.2	В
Riverside Dr					_							
Eastbound	LTR	0.10	7.6	Α	LTR	0.10	<u>7.6</u>	<u>A</u>	LTR	0.15	7.8	Α
Southbound	LT	0.26	20.6	С	LT	0.25	22.2	<u>A</u> <u>C</u> C	LT	0.20	22.8	С
	Т	0.25	20.1	С	I	0.21	21.1	<u>C</u>	T	0.17	23.2	С
Riverside Dr.	ive @ Ti	emann	Place		_							
Westbound		-	8.2	Α	<u>L</u>	=	<u>8.1</u>	<u>A</u>	L	-	8.3	Α
Northbound		-	7.3	Α		= = = = = = = = = = = = = = = = = = = =	7.1	<u>A</u>	R	-	7.2	Α
Southbound		-	9.9	Α	<u>L</u>	=	<u>9.5</u>	<u>A</u>	L	-	10.0	Α
	Т	-	8.8	Α		=	<u>8.7</u>	<u>A</u> <u>A</u>	Т	-	8.2	Α
	Int.		9.0	Α	<u>Int.</u>		<u>8.8</u>	<u>A</u>	Int.		8.8	Α
West 125th S	Street @	West 1	29th Stre	et/St. C	lair Plac	:e <u>*</u>						
Westbound		0.06	43.7	Е	<u>L</u>	0.02	22.9	<u>C</u>	L	0.26	115.4	F
	R	0.58	22.6	С	<u>R</u>	0.45	14.4	<u>C</u> <u>B</u>	R	1.05	92.0	F
Eastbound	L	0.01	30.0	D	<u>L</u> R	0.01	25.1	<u>D</u> B	L			F
	R	0.44	14.1	В	<u>R</u>	<u>0.40</u>	<u>12.8</u>	<u>B</u>	R	0.48	20.8	С

Notes: L: Left Turn; T: Through; R: Right Turn; Int.: Intersection

V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service

^{*}The unsignalized intersection analysis procedure, which assumes random arrival, breaks down with high major or minor street volumes and reports exaggerated levels of stop delays. As such, its results are not necessarily indicative of actual conditions, where gaps created by adjacent signalized intersections often provide additional throughput capacity and result in lower stop delays.

Table 17-11 2006 Existing Conditions Secondary Study Area Signalized Inersection Level-of-Service Analysis

				iuy Ai	rea Sigi							latysis
		AM Pea	ak Hour			idday F	Peak Ho	<u>ır</u>		PM Pe	ak Hour	
	Lane		Delay		<u>Lane</u>		<u>Delay</u>		Lane		Delay	
Intersection	Group	V/C	(spv)	LOS	<u>Group</u>	V/C	(spv)	LOS	Group	V/C	(spv)	LOS
Broadway @ We												
Eastbound		0.38	24.1	C	LTR	0.17	21.3		LTR	0.20	20.8	С
Westbound		0.85	55.1	E	<u>DefL</u>	0.57	<u>30.4</u>	<u>C</u>	DefL	0.76	44.0	D
	TR	0.50	29.0	С	IR	0.38	<u>25.3</u>	Ë	TR	0.78	43.1	D
Northbound		0.42	12.5	В	LTR	0.44	<u>12.6</u>		LTR	0.58	14.4	В
Southbound	LTR Int.	0.68	20.0 22.4	B C	LTR Int	0.48	13.0	<u>B</u> B	LTR	0.55	17.4	B C
Broadway @ We		Stroot		C	<u>Int.</u>		<u>16.0</u>	<u>D</u>	Int.		21.7	
Eastbound		0.40	27.5	С	LTR	0.30	26.2	C	LTR	0.31	26.2	С
Westbound		1.04	100.2	F	謊	0.77	45.8	<u>C</u> D	LTR	0.87	57.8	Ē
Northbound		0.35	5.8	Ā	LTR	0.33	5.7	<u>B</u>	LTR	0.43	6.4	Ā
Southbound		0.52	7.1	A	LTR	0.41	6.2	Ä	LTR	0.42	6.4	A
	Int.		21.4	C	Int.		13.5	B	Int.		15.3	В
Amsterdam Ave		Vest 12			· <u></u>						-	
Eastbound		0.42	38.4	D	<u>L</u>	0.48	40.7	D	L	0.55	44.4	D
	TR	0.94	72.7	Е	TR	0.67	47.2	D	TR	0.73	49.4	D
Westbound		0.08	29.2	С	L	0.02	27.7		L	0.01	27.3	С
	TR	0.29	32.1	С	<u>TR</u>	<u>0.18</u>	30.0		TR	0.28	31.7	С
Northbound	LTR	0.56	5.0	Α	<u>LTR</u>	0.54	<u>4.7</u>	<u>A</u>	LTR	0.79	9.5	Α
Southbound		0.53	4.6	A	LTR	0.31	3.1	≜	LTR	0.31	3.0	Α
	Int.		18.4	В	<u>Int.</u>		<u>11.9</u>	<u>B</u>	Int.		15.0	В
Frederick Doug							4	-	LITO	0 = 4	00.0	•
Eastbound		0.69	24.9	С	LTR	0.65	<u>17.7</u>	불	LTR	0.74	26.6	С
Westbound		0.60	21.6	С	LTR	0.52	<u>14.9</u>	R	LTR	0.71	25.5	C B
Northbound	LT R	0.17 0.11	15.3 15.0	B B	ᅜ	0.33	22.8 24.9	늗	LTR	0.55	17.7	ь
Southbound		0.11	17.1	В	<u>R</u> LT	0.30 0.41	<u>24.9</u> 24.1	$\stackrel{\smile}{\sim}$	LT	0.41	18.0	В
Southboaria	R	0.20	13.8	В	R	0.36	<u>26.4</u>	Ë	R	0.30	18.4	В
	Int.	0.20	20.0	В	Int.	0.00	<u>20.4</u> 19.3	BIBICICICIOIB	Int.	0.00	22.1	C
Madison Avenu		t 125th			1116		10.0					
Eastbound		0.85	32.5	С	<u>LT</u>	0.71	<u>25.3</u>	С	LT	0.91	38.1	D
Westbound		0.60	21.8	Č	TR	0.45	18.9	<u>C</u> <u>B</u>	TR	0.50	19.7	В
Northbound	LTR	0.55	16.9	В	LTR	0.45	15.5	В	LTR	0.67	18.8	В
	Int.		23.3	С	Int.		19.7	В	Int.		25.1	С
Second Avenue	@ East	125th	Street	·								
Eastbound		1.05	86.0	F	I R	1.05	86.7	<u>E</u> <u>C</u>	Т	1.05	82.3	F
	R	0.23	32.3	С		0.22	<u>31.7</u>	<u>C</u>	R	0.21	31.7	С
Westbound		1.05	103.1	F	LT	0.49	<u>35.6</u>	<u>D</u>	LT	0.51	36.4	D
0	T	1.05	103.3	F		0.01	47.0	_		0.70	00.0	
Southbound		0.55	20.2	C	LTR	0.31	<u>17.6</u>	<u>B</u>	LTR	0.70	22.3	С
Southwestbound		1.00	72.6	E	<u>TR</u>	0.61	36.6 43.5	<u>D</u>	TR	0.84	47.2	D
First Avenue @	Int.	5th Stor	63.2	<u> </u>	<u>Int.</u>		<u>43.5</u>	<u>D</u>	Int.		52.8	D
Eastbound		0.99	66.0	Е	Li	0.67	29.5	<u>C</u>	L	0.87	42.5	D
Lasiboullu	LT	0.99	18.7	В	L <u>L</u>	0.07	<u> 19.5</u>	<u>B</u>	LT	0.87	20.0	С
Northbound		0.10	16.1	В	🚆	0.13	9.6	<u>B</u>	L	0.12	9.5	A
	Ť	0.34	10.8	В		0.39	<u>11.2</u>	<u>B</u>	T	0.61	13.4	В
	R	0.22	10.6	В	<u> </u>	0.23	10.6	<u>=</u> B	R	0.46	13.8	В
	Int.		26.1	C	Int.		15.5	B	Int.		19.0	В
Notes: L: Le		: Throug		ht Turn;	DefL: De	facto Le		nt.: Inter	section			
					per Vehi							

Table 17-11 (Continued) 2006 Existing Conditions

Secondary Study Area Signalized Inersection Level-of-Service Analysis

		AM Pe	ak Hour		<u>_M</u>	idday I	Peak Ho	<u>ur</u>		PM Pea	ak Hour				
	Lane		Delay		<u>Lane</u>		<u>Delay</u>		Lane		Delay				
Intersection	Group	V/C	(spv)	LOS	<u>Group</u>	V/C	(spv)	LOS	Group	V/C	(spv)	LOS			
Broadway @ We	est 145ti	h Stree	t												
Eastbound	Eastbound LTR 0.22 25.2 C LTR 0.26 25.7 C LTR 0.24 25.5 C Westbound LTR 0.78 41.7 D LTR 0.90 56.4 E LTR 0.86 46.0 D														
Westbound	LTR	0.78	41.7	D	LTR	0.90	56.4	E	LTR	0.86	46.0	D			
Eastbound LTR 0.22 25.2 C LTR 0.26 25.7 C LTR 0.24 25.5 C Westbound LTR 0.78 41.7 D LTR 0.90 56.4 E LTR 0.86 46.0 D Northbound LTR 0.53 19.7 B LTR 0.63 21.5 C LTR 0.76 24.8 C Southbound L 0.18 9.3 A L 0.32 11.5 B L 0.37 13.1 B TR 0.42 10.5 B TR 0.50 11.4 B TR 0.44 10.7 B															
Southbound	L	0.18	9.3	Α	L	0.32	11.5	В	L	0.37	13.1	В			
	TR	0.42	10.5	В	IR	0.50	11.4	В	TR	0.44	10.7	В			
	Int.		20.9	С	Int.		23.8	<u>C</u>	Int.		24.3	С			
Notes: L: Le	ft Turn; T	: Throug	gh; R: Rig	ht Turn;	DefL: De	facto Le	eft Turn; I	nt.: Inter	section						
V/C:	Volume t	o Capac	ity; spv: S	Seconds	per Vehi	cle; LOS	S: Level o	f Servic	е						

Table 17-12 2006 Existing Conditions Secondary Study Area Unsignalized Inersection Level-of-Service Analysis

		AM Pea	ak Hour		<u>M</u>	idday l	Peak Hou	<u>ur</u>		PM Pea	ak Hour				
	Lane		Delay		Lane		Delay		Lane		Delay				
Intersection															
First Avenue @	East 12	5th Stre	et Sout	hbound	Right T	urn									
Southbound	R	0.26	11.7	В	<u>R</u>	<u>0.11</u>	<u>9.4</u>	<u>A</u>	R	0.10	9.4	Α			
Notes: R: Ri	ght Turn;	V/C: Vo	lume to 0	Capacity;	spv: Sed	conds p	er Vehicle	; LOS: I	_evel of S	Service					

Broadway and West 120th Street

During the AM peak hour, the westbound approach operates at LOS F, with an average delay of 100.2 seconds and a v/c ratio of 1.04.

<u>During the midday peak hour, the westbound approach operates at LOS D, with an average delay of 45.8 seconds and a v/c ratio of 0.77.</u>

During the PM peak hour, the same approach operates at LOS E, with an average delay of 57.8 seconds and a v/c ratio of 0.87.

Amsterdam Avenue and West 120th Street

During the AM peak hour, the eastbound through-right movement operates at LOS E, with an average delay of 72.7 seconds and a v/c ratio of 0.94.

<u>During the midday peak hour, the eastbound through-right movement operates at LOS D, with an average delay of 47.2 seconds and a v/c ratio of 0.67.</u>

During the PM peak hour, the same movement operates at LOS D, with an average delay of 49.4 seconds and a v/c ratio of 0.73.

Second Avenue and East 125th Street

During the AM peak hour, the eastbound through movement operates at LOS F, with an average delay of 86.0 seconds and a v/c ratio of 1.05. The westbound *de facto* left-turn movement operates at LOS F, with an average delay of 103.1 seconds and a v/c ratio of 1.05 and the westbound through movement operates at LOS F, with an average delay of 103.3 seconds and a

v/c ratio of 1.05. The southwestbound approach operates at LOS E, with an average delay of 72.6 seconds and a v/c ratio of 1.00.

<u>During the midday peak hour, the eastbound through movement operates at LOS E, with an</u> average delay of 86.7 seconds and a v/c ratio of 1.05.

During the PM peak hour, the eastbound through movement operates at LOS F, with an average delay of 82.3 seconds and a v/c ratio of 1.05. The southwestbound approach operates at LOS D, with an average delay of 47.2 seconds and a v/c ratio of 0.84.

First Avenue and East 125th Street

During the AM peak hour, the eastbound left-turn movement operates at LOS E, with an average delay of 66.0 seconds and a v/c ratio of 0.99.

Broadway and West 145th Street

<u>During the midday peak hour, the westbound approach operates at LOS E, with an average delay of 56.4 seconds and a v/c ratio of 0.90.</u>

During the PM peak hour, the same approach operates at LOS D, with an average delay of 46.0 seconds and a v/c ratio of 0.86.

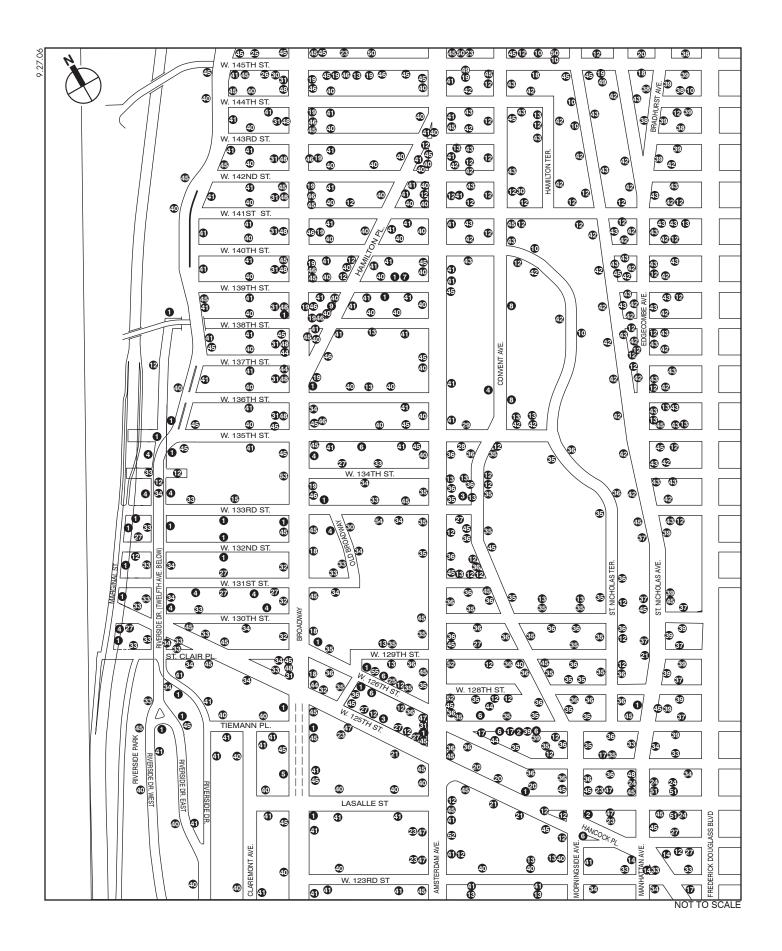
PARKING SUPPLY AND UTILIZATION

Existing study area parking conditions were evaluated via a field inventory of on-street parking regulations and utilization, as well as off-street public parking facilities.

ON-STREET PARKING

On-street parking regulations within ½ mile of the project site are summarized in Figures 17-9a and 17-9b. Parking is regulated by meters on most of Broadway north of West 135th Street and sections of West 145th Street. It is limited to authorized vehicles (8 AM-6 PM Monday-Friday) during school hours or completely restricted on Convent Avenue adjacent to City College, and is not permitted at any time on West 141st Street between Amsterdam and Edgecombe Avenues. No standing regulations prevail on most of West 132nd and 133rd Streets between Broadway and Twelfth Avenue. The remaining streets in the ½-mile radius have on-street parking restricted only during street-cleaning hours.

Parking utilization surveys were conducted in the study area under typical weekday conditions on Wednesday, June 2, 2004 and Tuesday, June 8, 2004 from 7 AM to 7 PM. Individual street capacities and an hourly assessment of on-street parking utilization were collected for each street in the study area. Utilization rates and capacities collected in 2004 were verified and modified, where necessary, in September 2006. Figures 17-10 through 17-12 present summaries of the survey results, in terms of the average percentage of available on-street spaces utilized during the morning, midday, and evening periods. The results indicate that throughout the study area, several of the streets approach capacity sometime during the day, particularly during the midday and evening periods. However, other areas never approach capacity during the survey period. These locations include the area south of West 125th Street between Twelfth and Amsterdam Avenues, as well as the Riverside Drive viaduct. Within ½ mile of the Project Area, on-street parking utilization is 76, 79, and 77 percent during the morning, midday, and evening periods, respectively. Overall, within ½ mile, on-street parking utilization is 72, 78, and 80 percent during the morning, midday, and evening periods, respectively (see Table 17-13).



- NS Anytime
- NS Except Trucks Loading & Unloading (7am-7pm Except Sunday)
- 3 NS Except Trucks Loading & Unloading (7am-3pm Except Sunday)
- NS Except Trucks Loading & Unloading (8am-6pm Monday-Friday)
- 5 NS Anytime Except Trucks Loading & Unloading
- 6 NS Anytime Except Authorized Vehicles
- NS Anytime Except Authorized Vehicles (FDNY)
- 3 NS Except Authorized Vehicles (8am-6pm Monday-Friday)
- NS Except Authorized Vehicles (7am-4pm Except Sunday) MIU
- NS Except Authorized Vehicles (7am-7pm Except Sunday) CUNY
- NS 8am-6pm (Monday-Friday)
- NP Anytime
- NP 7am-4pm (School Days)
- MP 7am-6pm (Monday Thursday Friday)
- I NP 7am-7pm (School Days)
- 16 NP 7am-7pm (Monday-Friday)
- NP 7am-7pm (Except Sunday)
- NP 7:30am-8am (Tuesday & Friday)
- NP 7:30am-8am (Except Sunday)
- NP 8am-8:30am (Monday & Thursday)
- 3 NP 8am-8:30am (Tuesday & Friday)
- NP 8am-8:30am (Wednesday & Friday)
- NP 8am-8:30am (Except Sunday)
- NP 8am-9am (Except Sunday)
- NP 8am-9:30am (Monday & Thursday)
- NP 8am-9:30am (Tuesday & Friday)
- 2 NP 8am-6pm (Monday-Friday)
- NP 8am-6pm (Tuesday & Thursday)

NS = No Standing NP = No Parking

- NP 8am-6pm (Monday Wednesday Friday)
- NP 8am-6pm (Except Sunday)
- 3 NP 8:30am-9am (Except Sunday)
- P NP 8:30am-9am (Monday & Thursday)
- 3 NP 8:30am-10am (Monday & Thursday)
- 4 NP 8:30am-10am (Tuesday & Friday)
- 3 NP 9am-10:30am (Monday & Thursday)
- 3 NP 9am-10:30am (Tuesday & Friday)
- The state of the s
- 3 NP 9:30am-11am (Monday & Thursday)
- NP 9:30am-11am (Tuesday & Friday)
- NP 11am-12:30pm (Monday & Thursday)
- 4 NP 11am-12:30pm (Tuesday & Friday)
- P NP 11:30am-1pm (Monday & Thursday)
- NP 11:30am-1pm (Tuesday & Friday)
- 4 Taxi Stand (1 Hour Limit)
- 49 Bus Stop (NS Anytime)
- 1 Hr Meter Parking 8am-7pm (Except Sunday)
- 1 Hr Meter Parking 8:30am-7pm (Except Sunday)
- 1 Hr Meter Parking 9am-7pm (Except Sunday)
- 49 2 Hr Meter Parking 8am-7pm (Except Sunday)
- 2 Hr Meter Parking 8:30am-7pm (Except Sunday)
- **5** 2 Hr Meter Parking 9am-7pm (Except Sunday)
- Snow Route
- NS 7am-7pm (Monday-Friday)
- NS Except Authorized Vehicles (7am-7pm) HPD
- S NS Except Authorized Vehicles (8am-5pm Monday-Friday)

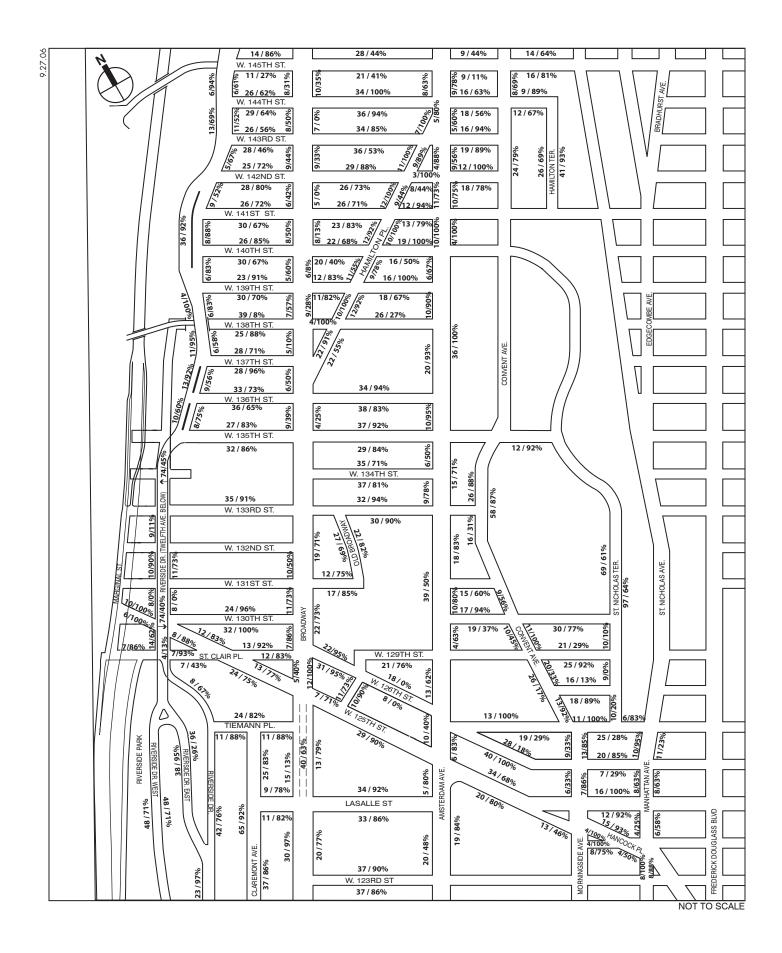


Figure 17-10

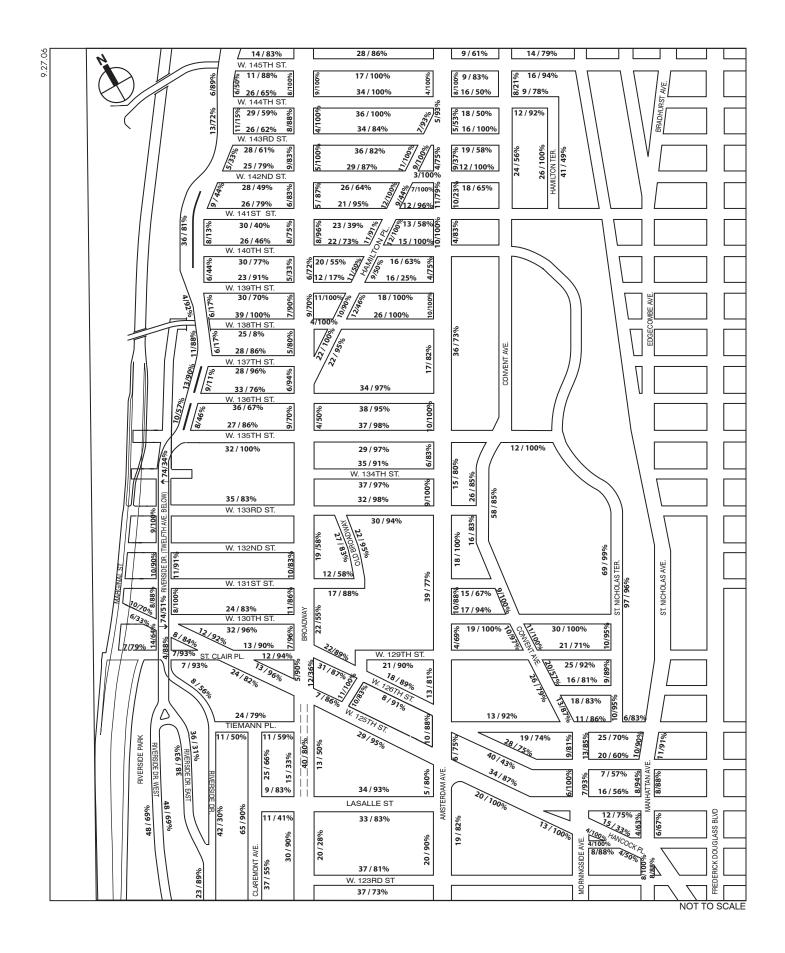


Figure 17-11
Existing 2006 Midday On-Street Parking
Capacity/Occupancy Rate

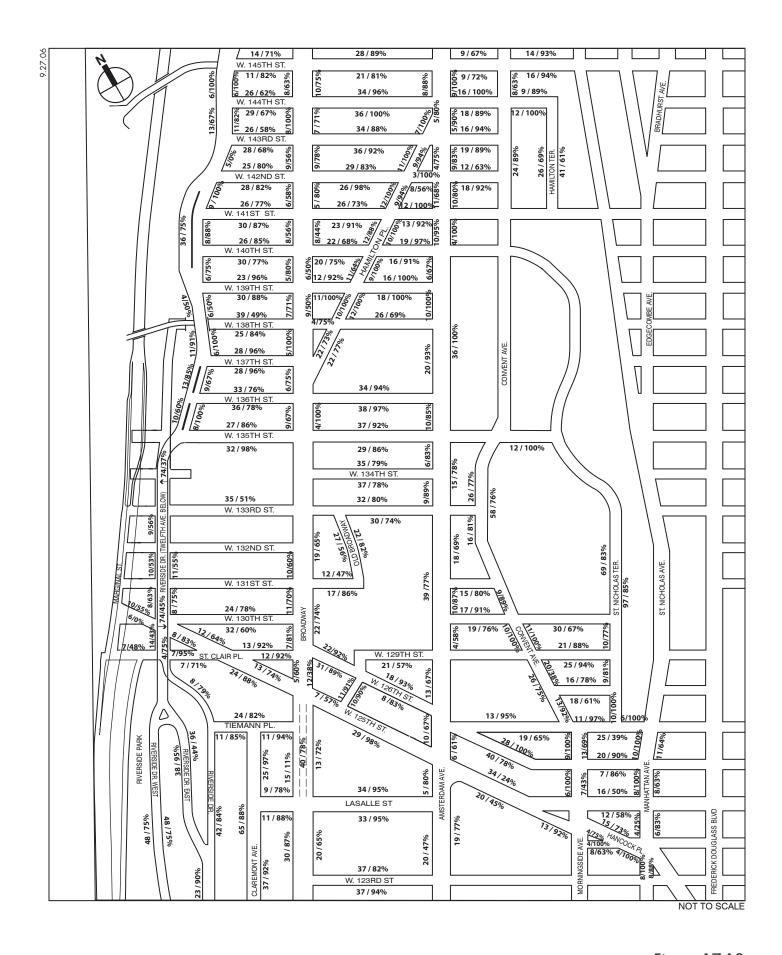


Figure 17-12
Existing 2006 Evening On-Street Parking
Capacity/Occupancy Rate

Table 17-13
2006 Existing On-Street Parking Utilization Summary

2000 Existing On-St	n cet i ai king	Cunzanon	Summary
2006 Existing	AM	MD	PM
1/	4-Mile Radius		
Capacity	1847	1847	1847
Demand	1407	1457	1415
Remaining Spaces	440	390	432
Utilization	76%	79%	77%
1/	∕₂-Mile Radius		
Capacity	4783	4783	4783
Demand	3454	3732	3807
Remaining Spaces	1329	1051	976
Utilization	72%	78%	80%

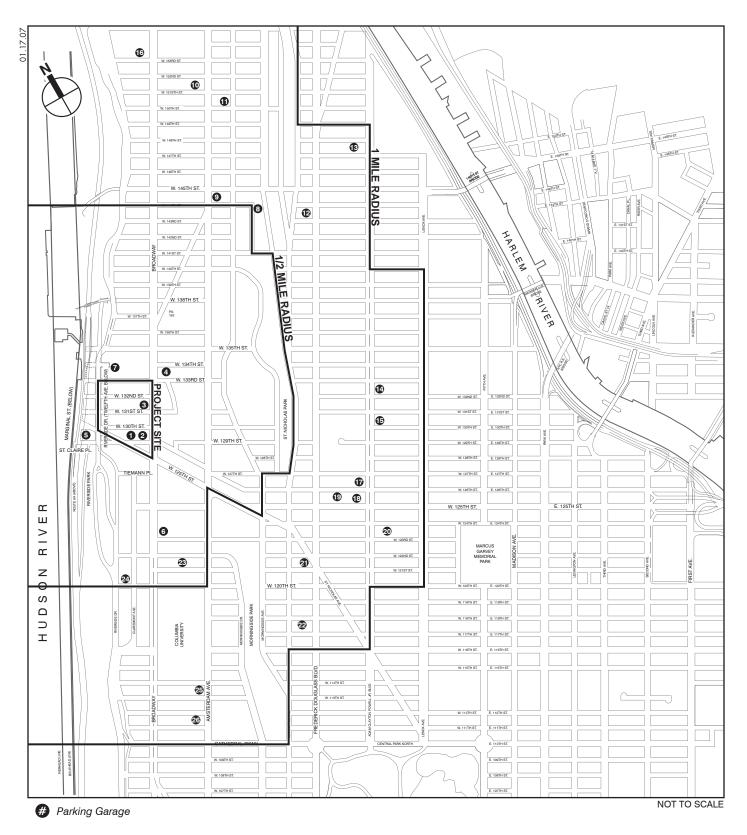
OFF-STREET PARKING

Off-street publicly accessible parking lots and garages within one mile of the project site were surveyed in September 2006 to assess their capacities and approximate weekday utilization during the morning, midday, and evening peak periods. Individual facility licensed capacities and utilization estimates were surveyed and recorded. Results are summarized in Appendix H and displayed in Figures 17-13a and 17-13b.

Seven public parking facilities are located within ¼ mile of the Project Area, with a combined capacity of 1,330 spaces. Currently, these facilities are 88, 81, and 89 percent utilized, with 159, 249, and 142 available spaces during the morning, midday, and evening periods, respectively. Within ½ mile, there are nine public parking facilities, with a combined capacity of 1,710 spaces. Currently, these facilities are 88, 85, and 84 percent utilized, with 209, 249, and 282 available spaces during the morning, midday, and evening periods, respectively. Overall within one mile, there are 26 public parking facilities with a combined capacity of 3,576 spaces. Currently, these facilities are 85, 83, and 67 percent utilized, with 546, 619, and 1,189 available spaces during the morning, midday, and evening periods, respectively (see Table 17-14).

Table 17-14 2006 Existing Off-Street Parking Utilization Summary

2006 Existing	AM	MD	PM
1/4	Mile Radius		
Capacity	1330	1330	1330
Demand	1171	1081	1188
Remaining Spaces	159	249	142
Utilization	88%	81%	89%
1/2*	Mile Radius		
Capacity	1710	1710	1710
Demand	1501	1461	1428
Remaining Spaces	209	249	282
Utilization	88%	85%	84%
1-	Mile Radius		
Capacity	3576	3576	3576
Demand	3030	2957	2387
Remaining Spaces	546	619	1189
Utilization	85%	83%	67%



- For Parking Facility information, please see Fig. 17-13b

- 1 MPT 3300 Broadway Corp. 627 West 129th Street (Capacity: 200)
- 2 West 129th Street LLC 605 West 129th Street (Capacity: 134)
- 3 Uni Facility Corp. 631-635 West 131st Street
- 4 Y & H Enterprises Inc. 526-534 West 134th Street (Capacity: 175)
- 5 Columbia Waterfront LLC 69 St. Clair Pl. (Capacity: 70)
- 6 Morningside Heighs Housing Corp. 3100 Broadway (Capacity: 291)
- Tedison Riverside Corp. 3333 Broadway (Capacity: 360)
- 3 Marvel Parking Corp. 673 St. Nicholas Avenue (Capacity: 180)
- Uptown Parking Corp. 1721-1735 Amsterdam Avenue (Capacity: 53)
- Micholson & Nicholas Park 503 West 151st Street (Capacity: 20)
- 1 LAZ Parking Limited 457 West 150th Street (Capacity: 125)
- @ Giselle Garage Corp. 310 West 144th Street
- 3 Stanns Parking 234-40 West 148th Street (Capacity:110)
- Giselle Garage Corp. 161 West 132nd Street (Capacity: 130)
- EZ Going Park 2201 Seventh Avenue (Capacity: 48)
- 6 Stable Car Parking Inc 616 West 153rd Street (Capacity: 135)
- Impark HSW LLC 2130-38 Seventh Avenue
- (B) Impark HSW LLC 215 West 125th Street (Capacity: 60)
- EZ Going Park 270 West 126th Street (Capacity: 159)
- 175) Uptown Parking Corp. 160 West 124th Street (Capacity: 175)
- 2 Easy Cross Parking Corp. 225 St. Nicholas Avenue (Capacity:160)
- 2 SoLo Parking 316 West 118th Street (Capacity: 130)
- Park GMC Garage Management Corp. 532 West 122nd Street (Capacity: 180)
- 2 Rapid Park Industries 480 Claremont Avenue (Capacity: 200)
- ProPark America 1090 Amsterdam Avenue (Capacity: 135)
- 3 GGMC Garage Corp. 516-20 West 112th Street (Capacity: 75)

D. 2015 FUTURE WITHOUT THE PROPOSED ACTIONS

The analysis of traffic conditions in the 2015 analysis year without the Proposed Actions, or the 2015 No Build condition, serves as the baseline against which the impacts of the Proposed Actions would be compared. The future No Build analysis reflects increases in background traffic volumes and traffic from other planned developments in the area, as well as roadway or signal timing changes scheduled to be implemented by the analysis year. Although the West Harlem Master Plan contemplates the possibility of improved bus service, a new ferry landing, and a potential Metro-North stop at West 125th Street, these transportation improvements are still currently being studied and were not included in the future No Build analysis. Similarly, DCP and the New York City Department of Transportation (NYCDOT) are undertaking a "river-to-river" study for the Harlem section of Manhattan (Harlem Morningside Heights Transportation Study) to develop potential strategies to improve both vehicular and pedestrian travel. The outcomes of this study were also not accounted for in the future No Build analysis.

Future No Build peak hour traffic volumes were estimated by first applying a background growth of 0.5 percent per year (as recommended by the *CEQR Technical Manual*), for a total of 4.6 percent by 2015. As detailed in Section B of this chapter, other projects anticipated to be completed independent of the Proposed Actions were considered, along with the general background growth, to forecast the future baseline traffic volumes.

TRAFFIC VOLUMES AND INTERSECTION CAPACITY ANALYSIS

Peak hour traffic volumes for the 2015 No Build condition are shown in Figures 17-14 through 17-19. Within the primary study area, increases in two-way peak hour traffic over existing conditions are expected to range from 150 vph to 310 vph along Broadway, from 150 vph to 200 vph along Amsterdam Avenue, and from 0 vph to 190 vph along Twelfth Avenue. Two-way peak hour volumes along West 125th Street would increase by 200 vph to 380 vph during the three analysis periods. In the secondary study area, intersections along East 125th Street are expected to experience substantial increases in traffic volumes as a result of the East River Plaza and East 125th Street developments in East Harlem, particularly along First and Second Avenues at East 125th Street, where increases in PM peak hour traffic over existing volumes will exceed 470 vph and 850 vph, respectively. PM peak hour eastbound and westbound volumes along East 125th Street between Second and Fifth Avenues are expected to increase by 330 vph and 200 vph, respectively.

In addition to changes in traffic volumes, the No Build analysis also accounted for the modification of parking lot access at the Fairway supermarket, which would result in the elimination of the eastbound approach at the intersection of Marginal and West 132nd Streets and the addition of an eastbound approach to the intersection of Twelfth Avenue and West 131st Street. The No Build analysis included additional roadway modifications expected to occur as a result of construction of the West Harlem Waterfront park, including lane width and pavement-marking modifications along Marginal Street, and relocation of the bicycle lane, currently southbound on Twelfth Avenue between West 125th and West 134th Streets, to west of Marginal Street. Relocation of the bicycle lane would result in an additional southbound lane on Twelfth Avenue between West 125th and West 133rd Streets. Results of the LOS analysis are presented in Tables 17-15 and 17-16 for the primary study area, and in Tables 17-17 and 17-18 for the secondary study area. As with existing conditions, locations with notable service constraints, those operating at mid-LOS D or worse, are described below.

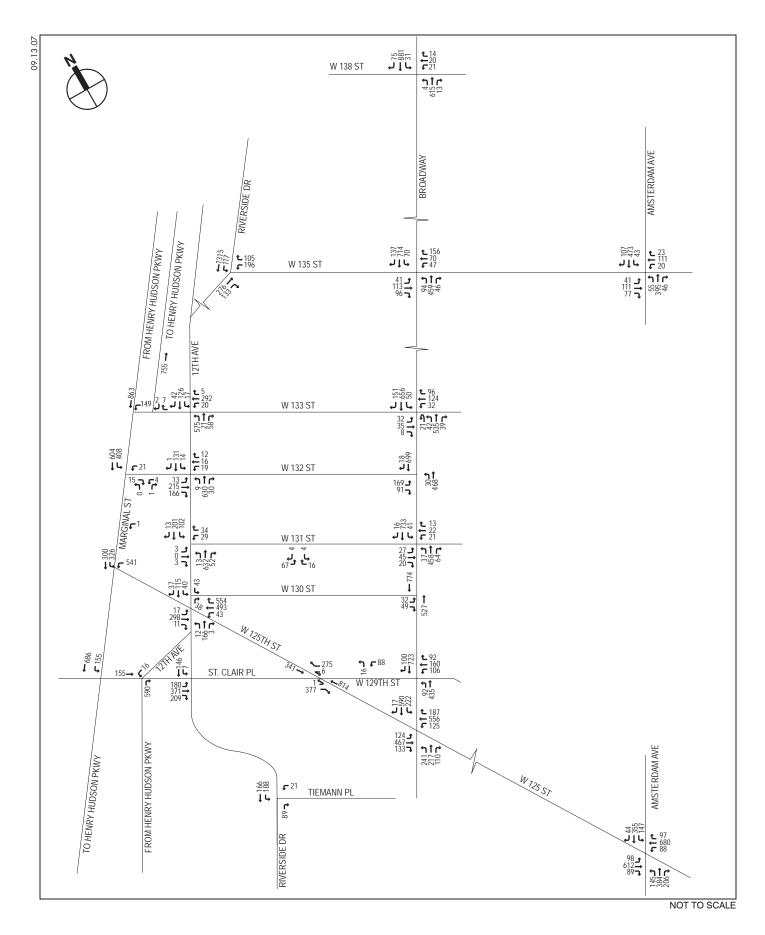


Figure 17-14
Primary Study Area
No Build 2015 Morning Peak Hour

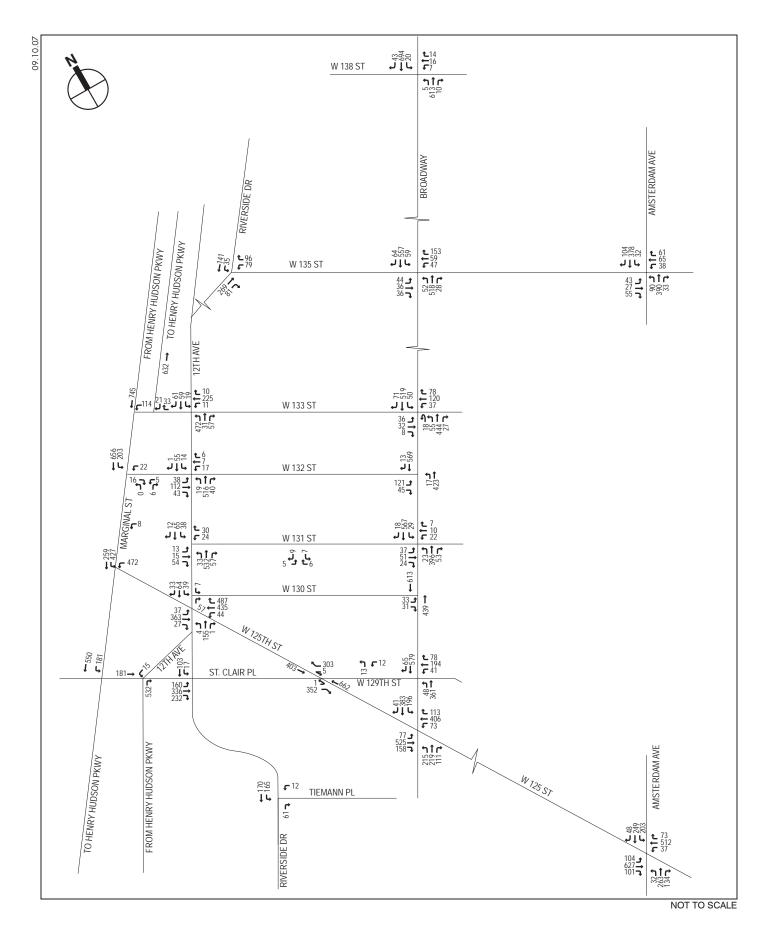


Figure 17-15
Primary Study Area
No Build 2015 Midday Peak Hour

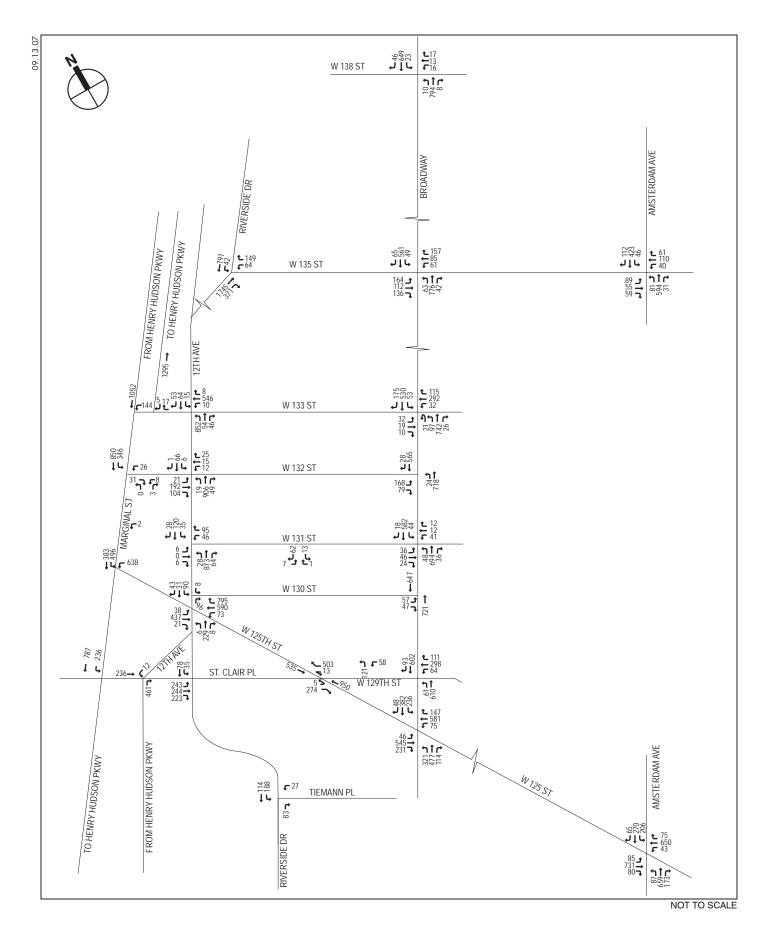
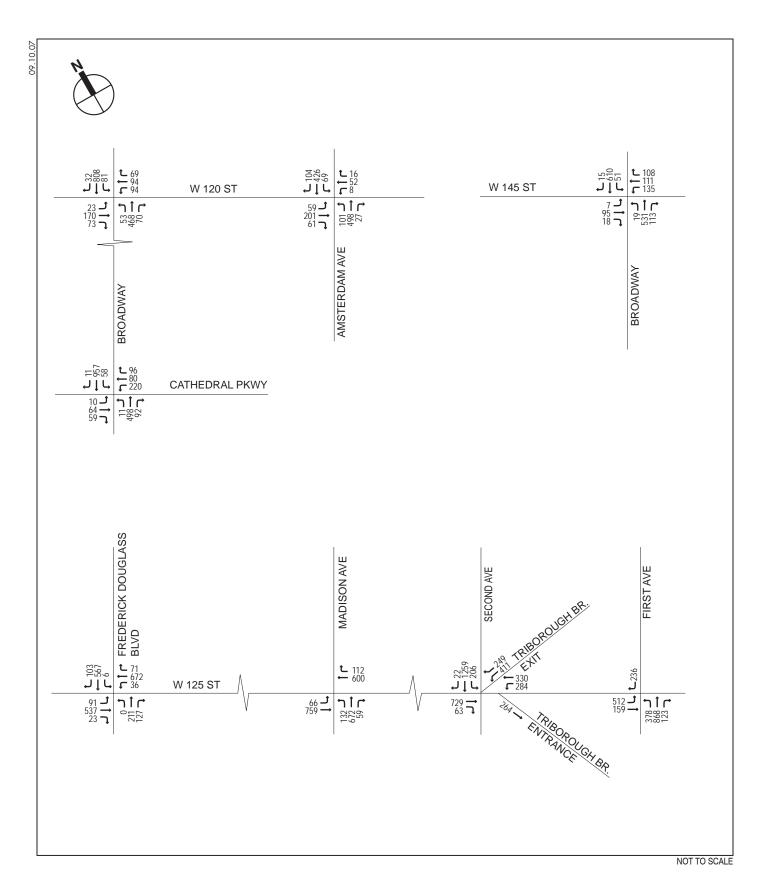


Figure 17-16
Primary Study Area
No Build 2015 Evening Peak Hour



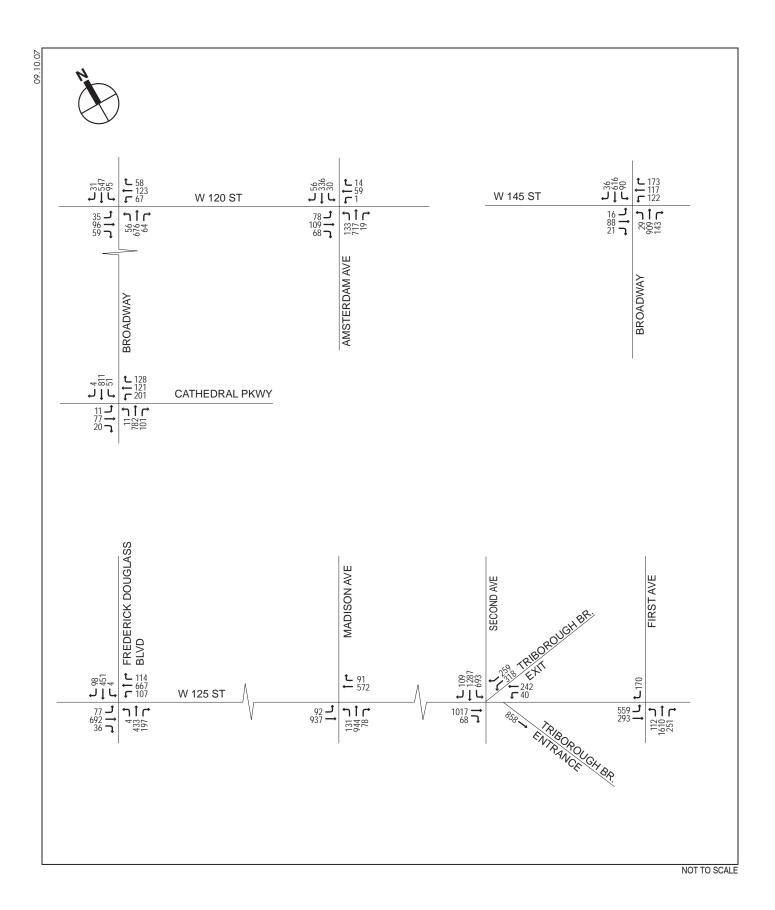


Figure 17-19

Table 17-15 2015 No Build Condition Primary Study Area Signalized Intersection Level-of-Service Analysis

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	Ln		Delay		Ln		Delay		<u>Ln</u>		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
							\cdot	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			et @ W 24.3				26.0	C	Li	0.35	23.7	_	Li	0.45	25.0	C	l i	0 41	24.5	C	Li	0.63	28.2	С
			12.1	В	-	0.31	20.0	C				B	느	<u>U.4J</u>	20.0	<u>u</u>				В	_	0.00	20.2	C
			11.8	В	LT	0.4Z	14.3	В	_	-	11.8		LI	0.50	14.7	В				В	LT	0.64	1 <u>7.1</u>	В
	Int.		17.5		Int.		19. <u>6</u>	В	Int.		<u>16.8</u>	<u>B</u>	Int.		<u>18.9</u>	<u>B</u>	Int.		17.3	В	Int.		2 <u>1.9</u>	С
			e @ W					_		0.04	00.4	_		0.00	00.5	_		0.40	05.0	_		0.00	00.0	•
WB			28.0 30.9	C		0.59	35.0	C			26.1 29.3	<u>C</u>			26.5 30.1	<u>C</u>			25.6 36.4	C D		0.20		C D
NB			8.1	Ä		0.43		A		0.19		Ä		0.30		Ä			23.3	C			51.8	D
			11.4		LT	0.86	13.6	В		0.10		A		0.11		Ā	LT	0.19	8.1	Ā	LT	0.21	8.2	Α
	Int.		12.6	В	Int.		15.5	В	Int.		<u>12.4</u>	<u>B</u>	Int.		<u>12.8</u>	<u>B</u>	Int.		22.7	С	Int.		46.3	D
			ie @ И					_	li TD	0.40	05.0	_	LED	0.00	20.5	_	li TD	0 00	40.5	_	LED	4 20	477.4	_
				C				D E	LIK	0.40	25.0 21.8	2			30.5 27.2	<u>C</u>				D C			177.1 44.2	F D
			10.8					В	LTR	0.16	10.7	B			10.8					В		0.71		C
			11.7				10.4	В			11.1				10.1				10.8	В			10.0	В
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			20.0				20.7				22.2 19.7				24.7 20.3				20.1			0.00		C
			13.3	_			13.8	_			12.4				13.0					-			17.6	В
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ட	Int.		14.9	В	Int.		21.1	С	Int.		14.0	<u>B</u>	Int.		<u>15.6</u>	<u>B</u>	Int.		17.3	В	Int.		19.9	В
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			12.7 12.6	В			1 <u>3.4</u> 1 <u>3.0</u>	В			13.7 12.7	<u>В</u> В			14.1 13.3	<u>В</u> В				B B		=	1 <u>5.1</u> 1 <u>5.3</u>	B B
VVD			18.9				23.8					B			19.9	B				В			30.6	C
			34.1				48.8					C			46.1	D				Ē			100.5	
			19.3	В	LTR	0.32	20.6	С	LTR	0.22	<u> 19.2</u>	<u>B</u>	LTR	0.29	20.1	С				С				С
SB		0.34		С		0.38		С		0.21		<u>B</u>		0.23	<u>19.5</u>	<u>B</u>		0.28		С		0.34	21.8	С
Dras	Int.		23.2 Vest 1	C	Int.	.4	28. <u>7</u>	С	Int.		21.6	<u>C</u>	<u>Int.</u>		26.6	<u>C</u>	Int.		32.0	С	Int.		49. <u>8</u>	D
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				Ä		0.34		Ä		0.30		Ā		0.34		Ā		0.37		Ă		0.41		Ä
			4.7	Α		0.02		Α		0.02		Α		0.02		Α		0.02		Α		0.02		Α
SB	LTR	0.52				0.59		Α		0.40		A	-	<u>0.44</u>		A		0.38				0.41		Α
Pros	Int.	, No.	7.6 thbou	A nd @	Int.	+ 125	8.0	A	<u>Int.</u>		<u>6.8</u>	<u>A</u>	<u>Int.</u>		<u>7.0</u>	<u>A</u>	Int.		6.9	Α	Int.		7.2	Α
			25.1	C		0.36		C	Ιιτ	0.23	24.3	С	Ιιτ	0 27	24.8	C	Defl	0 77	51.7	D	Defl	0.95	83.0	F
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			28.0	С		0.49		С				<u>C</u>				<u>C</u>			28.4	С		0.54		С
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Bros	Int.	/ So:	16.4 Ithbou	B Ind @	Int.	t 125	16.6	B	<u>Int.</u>		<u>15.1</u>	<u>B</u>	<u>Int.</u>		<u>15.4</u>	<u>B</u>	Int.		19.1	В	Int.		23.7	С
				C		0.42		C	IR	0.15	23.5	С	IR	0.19	24.0	С	TR	0.48	27.9	С	TR	0.67	32.8	С
				č		0.34		Č			24.1	č			24.7	Č				Č			26.9	č
SB				Α	L	0.10		Α	L	0.09	5.1	Α	L	0.09	5.1	Α		0.07		Α		0.08		Α
		0.62	9.5	Α		0.70	11.0	В		0.48		A	_	0.52		A		0.47		Α		0.51		Α
Dug :	Int.	. Na:	14.4		Int.	4 4 2 2	16.0	В	Int.		<u>11.8</u>	В	<u>Int.</u>		<u>12.5</u>	<u>B</u>	Int.		15.8	В	Int.		18.8	В
			<i>thbou</i> 17.2	na @ B		0.21		eet B	lт	0.22	17.5	<u>B</u>	lт	0.25	17.9	В	Ιιτ	0.18	17 1	В	lт	0.23	17 8	В
			21.1	C			24.3	C			20.6	C			23.4	C			29.0	C			40.9	D
	LT	0.39	11.7	В	LT	0.56	13.9	В	LT	0.35	11.3	<u>B</u>	LT	0.44	12.2	В	LT	0.66	15.6	В	LT	0.78	18.9	В
		0.04	9.0	Α		0.11		Α	R	0.05	9.0	Α	R	0.08	9.4	A		0.05		Α		0.07		Α
	Int.		14.6	В	Int.		16.8	<u>B</u>	Int.		<u>14.4</u>	<u>B</u>	Int.		<u>15.9</u>	В	Int.		19.3	В	Int.		25.3	С
			ithbou 17.9	nd @ B			18.0	eet B	Ιτρ	0 12	18.0	R	ΙΤΡ	U 13	<u>18.1</u>	В	TP	0 00	17.7	В	ΙΤΡ	0.10	17 7	В
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<u> </u>			//C: Vo	iume	ιο Ca	pacity;	spv: S	econ	us per	venic	ie; LO	s: Le	vei of	Servic	e									

Table 17-15 (Continued) 2015 No Build Condition

Primary Study Area Signalized Intersection Level-of-Service Analysis AM Peak Hour Midday Peak Hour PM Peak Hour 2006 Existing 2015 No Build 2006 Existing 2015 No Build 2006 Existing 2015 No Build I_n Delay Ln Delay Ln Delay Ln Delay Ln Delay Delay Int. Grp V/C (spv) LOS Grp V/C Broadway Northbound @ West 132nd Street EB L 0.11 16.4 B L 0.40 20.5 B B B <u>LT 0.35 11.1</u> NB LT 0.32 10.9 LT 0.29 10.6 В LT 0.57 13.6 В LT 0.41 11.7 LT 0.49 12.6 14.0 11.5 В 11.6 Int. 13.4 Int. 14.6 Int. Int. Int. В Broadway Southbound @ West 132nd Street TR 0.20 19.0 LT 0.01 15.4 LTR 0.35 11.1 TR 0.32 20.7 LT 0.04 15.6 LTR 0.41 11.7 EB TR 0.22 19.5 B I TR 0.61 27.9 TR 0.34 20.9 C TR 046 229 C B B B LT 0.02 15.4 WB IT 0.03 15.5 В IT 0.06 15.8 В В LT 0.04 15.6 В SB LTR 0.44 12.0 В LTR 0.51 12.9 В LTR 0.36 11.2 В LTR 0.43 11.9 В Int. 13.1 В Int. 17.0 Int. Int. 13.8 В Int. В Broadway @ West 131st Street LTR 0.14 20.9 LT 0.09 20.4 R 0.02 19.6 LTR 0.23 7.7 LTR 0.30 8.2 LTR 0.20 21.7 EB LTR 0.10 20.4 С LTR 0.24 22.1 LT 0.10 20.5 R 0.02 19.6 LTR 0.12 20.6 LTR 0.21 21.8 WB LT 0.10 20.5 LT 0.11 20.6 С LT 0.14 21.1 С LT 0.16 21.3 С В R 0.03 19.7 В R 0.04 19.8 R 0.03 19.7 В R 0.04 19.8 В Ā LTR 0.30 8.1 8.7 NB LTR 0.27 7.9 LTR 0.37 8.8 Α LTR 0.39 8.8 LTR 0.47 9.6 Α Α Α LTR 0.37 LTR 0.50 9.9 Α SB LTR 0.39 8.8 LTR 0.33 8.4 Α LTR 0.42 9.1 Α Α 10.6 9.5 Int Int. Α Int. Int. Α Int. Broadway @ West 130th Street LR 0.13 23.7 T 0.18 5.3 T 0.25 5.6 LR 0.25 25.6 <u>LR</u> I I 0.19 24.5 0.21 5.4 EB LR 0.18 24.4 C С LR 0.24 25.4 С LR 0.31 26.5 С <u>C</u> NB LT 0.21 5.4 Α LT 0.26 5.7 Α LT 0.28 5.8 Α LT 0.32 6.0 Α SB LT 0.31 6.0 LT 0.37 6.3 Α 0.29 5.9 LT 0.27 5.7 Α LT 0.32 6.0 Int. 6.7 Α Int. Int. 7.0 Α Int. Broadway @ West 129th Street LT 0.35 17.8 R 0.15 15.5 LT 0.30 16.6 LT 0.41 18.7 R 0.18 15.9 LT 0.38 17.6 WB LT 0.38 18.2 В LT 0.42 19.0 LT 0.46 19.6 В В LT 0.54 21.1 С R 0.19 16.0 R 0.20 16.1 B B B B R 0.22 16.4 R 0.27 17.1 В В В В LT 0.57 20.6 NB LT 0.33 17.0 В DefL 0.61 34.9 С LT 0.69 23.5 С T 048 192 В TR 0.37 17.3 17.1 SB TR 0.48 15.9 В В TR 0.60 17.7 TR 0.39 17.6 TR 0.49 18.8 В В <u>TR</u> <u>0.44</u> <u>18.1</u> В В Int. 16.6 R Int. 19.1 R 18.0 Int 19.0 R Int C Broadway @ West 125th Street EB L 0.55 36.8 D L 1.<u>44</u> 2<u>75.6</u> 0.35 26.5 0.56 37.1 L 0.27 25.7 0.55 45.1 TR 0.57 26.8 TR 0.7<u>9</u> 3<u>4.3</u> TR 0.71 30.6 TR 0.87 39.7 TR 0.83 36.7 D TR 1.08 84.9 L 0.54 39.1 TR 0.56 26.5 WB L 0.68 44.6 D L 1.15 159.2 F 0.81 77.6 0.65 50.8 D L 1.04 140.8 **HICHCHOICH** <u>E</u> TR 0.71 30.4 TR 0.9<u>9</u> <u>57.7</u> TR 0.73 31.6 TR 0.68 29.3 TR 0.<u>92</u> <u>44.9</u> D С LT 0.38 31.1 LT 0.43 30.7 R 0.54 39.6 L 0.28 29.1 2.70 36.8 L 0.43 32.1 С L 0.46 32.7 Ī 0.40 31.6 0.56 35.4 L 0.60 36.7 D LT 0.40 31.6 LT 0.47 31.5 R 0.68 49.5 L 0.35 30.4 LTR 0.79 40.9 LT 0.41 30.5 С LT 0.48 31.6 Ċ LT 0.82 42.9 LT 0.91 50.5 D R 0.51 37.8 R 0.55 39.8 R 0.62 43.8 R 0.61 43.4 D 0.38 30.6 LTR 0.70 36.8 L 0.33 30.1 С С 0.25 28.5 С L 0.36 30.1 С LTR 1.04 76.6 Ē D LTR 0.90 49.1 D LTR 0.75 38.9 LTR 0.86 45.9 D D 35.7 Int. Int. Int. 36.6 D Int. Е Int. 64.6 Int Amsterdam Avenue @ West 135th Street <u>LT 0.20 24.8</u> <u>R 0.22 25.6</u> EB LT 0.31 26.4 С LT 0.42 28.3 <u>LT 0.23 25.2</u> <u>R 0.23 25.8</u> LT 0.47 30.8 LT 0.58 35.1 D R 0.36 29.0 С R 0.38 29.8 С R 0.25 26.3 С R 0.27 26.6 С LTR 0.45 29.6 WB LTR 0.36 27.1 С LTR 0.44 28.9 С LTR 0.53 31.8 LTR 0.48 30.2 С LTR 0.65 36.2 D LTR 0.50 8.2 LTR 0.44 7.4 NB LTR 0.44 7.4 Α LTR 0.48 7.9 Α LTR 0.54 8.8 LTR 0.63 10.0 Α LTR 0.69 11.3 В SB LTR 0.52 8.2 Α LTR 0.58 8.9 Α LTR 0.44 LTR 0.49 7.9 LTR 0.52 8.3 Α LTR 0.55 8.6 Int. 12.3 В Int. 13.6 Amsterdam Avenue @ West 125th Street L 1.10 145.3 TR 1.00 62.1 L 0.51 45.7 L <u>1.06</u> <u>144.6</u> TR 1.0<u>8</u> <u>84.5</u> $\begin{array}{c|c} \underline{L} & \underline{0.72} & \underline{53.2} \\ \underline{TR} & \underline{0.82} & \underline{36.9} \\ \underline{L} & \underline{0.38} & \underline{32.9} \end{array}$ L 0.58 41.0 EB L 0.56 41.8 D L 1.39 264.3 E E D TR 0.77 34.1 TR 0.9<u>9</u> <u>60.5</u> TR 0.79 34.8 С С D D B L 0.67 49.4 L 0.42 35.4 $0.60 \overline{54.1}$ D L <u>1.08</u> <u>148.3</u> D L TR 0.84 38.0 L 0.07 10.1 TR 1.02 66.8 L 0.35 15.7 TR 0.68 30.4 TR 0.96 52.1 TR 0.76 33.3 С Ε TR 0.71 31.4 D C TR 0.07 23.3 0.62 25.0 NB L 0.06 9.7 TR 0.47 22.3 I 0 20 15 4 В I 0 31 14 4 В В В 1 0 23 16 9 TR 0.67 26.7 С TR 0.73 28.6 TR 0.84 33.4 TR 0.90 38.<u>5</u> С CICICID С D L 0.48 TR 0.30 L 0.62 25.0 IR 0.33 20.2 SB С L 0.47 22.9 L 0.53 26.<u>7</u> С 18.8 L 0.58 33.9 С L 0.75 45.0 D TR 0.44 21.6 TR 0.73 33.4 TR 0.81 38.6 TR 0.41 21.2 С С 19.8 С D C Int. 29.3 С Int. 56 1 Int Int. 33.0 Int. 56.2 F Int Notes: L: Left Turn; T: Through; R: Right Turn; DefL: Defacto Left Turn; Int.: Intersection

V/C: Volume to Capacity: spy: Seconds per Vehicle: LOS: Level of Service

Table 17-16 2015 No Build Condition Primary Study Area Unsignalized Intersection Level-of-Service Analysis

			Αl	VI Pea	ak Ho	our			, ~ .	- u- <u>J</u>	Midd		_								ak H			
	2	006 E	xistin	g	2	015 N	o Bui	ld	2	2006 I	Existing	1	2	015 N	o Buil	d	2	006 E	xistin	g	2	015 N	lo Buil	d
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
								LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			et @ V																					
		0.18		В	L	0.20	10.6	В	L ↓	0.13	10.1 23.4	<u>B</u> C	Ī	0.16 0.96	10.3 45.2	<u>В</u> Е	L		10.3	B E	L		10.5	B F
		0.68	18.5 et @ И	C				F	<u> </u>	<u>U.78</u>	<u>23.4</u>	U	Ι <u></u>	0.96	<u>45.2</u>	드		0.96	40.8	E	ļ !	1.15	<u>95.9</u>	F
	giriai I LT	Stree	иши 9.0	A	132110 L	a Stre	9.7	۸	lт		9.3	۸	Li		9.6	۸	LT		9.7	Α	L		9.9	^
	LŤ	_	14.5	В	LT	_	82.1	<u>A</u> F	뜶	=	13.4	B	١ħ	=	20.1	C	LT	-	29.2	Ď	LT	_	70.4	<u>A</u> F
"	TR	_	7.9	Ā	T	_	8.2	A	菔	= = =	8.1	<u>A</u> <u>B</u> <u>A</u>	Ī	= =	8.3	<u>A</u> <u>C</u> <u>A</u>	TR	-	9.5	Ā	T	_	10.0	В
	Int.		13.0	В	Int.		68.3	F	Int.	=	12.0	В	Int.	=	17.0	C	Int.		22.9	С	Int.		52.1	F
			et @ S																					
SB		0.17		Α		0.18		Α	L	0.20	10.0 18.5	<u>A</u> C	L	0.22	<u>10.1</u>	<u>B</u> D				В	L		10.6	В
_		0.76		С		0.94		<u>E</u>	I	0.68	<u>18.5</u>	<u>C</u>	I	0.86	<u>30.4</u>	D	T	0.81	26.1	D	Т	1. <u>15</u>	103.2	F
			ie @ V								7.0			0 00	- -						1	0.00	7.0	
		0.01		A A		0.01 0.15		A B		0.02	<u>7.6</u> 9.3	<u>A</u> <u>A</u> <u>C</u>		0.02	<u>7.5</u> 10.0	<u>A</u> A		0.02	10.5	A B		0.02	7.8 11.9	A B
_			16.8							0.02	<u>9.5</u> 16.5	8											85.7	F
		0.01				0.02		Č		0.15	12.6	B		0.19					13.9				19.2	Ċ
								ıthbo			Turn													
		0.04				0.07				0.03		<u>B</u>	R	0.06	10.8	В	R	0.02	10.1	В	R	0.08	11. <u>6</u>	В
		venu	ie @ S	t. Cl		ace																		
	T	-	9.3	Α	T	-	10.0	Α	I R	≣ ≣	9.8	<u>A</u> <u>B</u> <u>A</u>	Ī	≘	<u>10.4</u>	<u>B</u> <u>B</u> <u>A</u>	T	-	10.2	В	T	-	11. <u>2</u>	В
	R	-	10.6	В	R	-	14.7	В	₽	≘	11.5	<u>B</u>	₽	≘	14.5	<u>B</u>	R	-	10.4	В	R	-	13. <u>0</u>	В
SB	L Int.	-	8.8 10.2	A B	L Int.	-	9.1 13.6	A B	L Int.	=	<u>8.6</u> 11.0	<u>A</u> B		≘	<u>8.8</u> 13.4	<u>A</u> B	L Int.	-	8.6 10.2	A B	L Int.	-	8.9 12. <u>3</u>	A B
Pive		Driv	e @ S			200	13.0	ь	ШЬ		11.0	D	шь		10.4	D	IIIL.		10.2	ь	IIII.		12. <u>J</u>	
		0.10				0.13	7.8	Α	lltr	0.10	7.6	Α	lltr	0.13	7.7	Α	LTR	0.15	7.8	Α	lltr	0.18	7.9	Α
		0.26		C			34.2			0.25	22.2		TT	0.37	31.9	D			22.8	C			3 <u>3.6</u>	D
		0.25		C	Т	0.39		D	I	0.21	21.1	С	Ī	0.29	28.5	D	Т	0.17	23.2	С	Т		32.6	D
			e @ T								-													
WB		-	8.2	Α	L	-	8.3	Α	L	Ξ	8.1	Α	L	Ξ	8.1	<u>A</u> <u>A</u>	L	-	8.3	Α	L	-	8.3	Α
	R	-	7.3	Α	R	-	7.4	Α	₽	=	7.1	A	₽	<u>=</u> =	7.1	A	R	-	7.2	Α	R	-	7.3	Α
SB	L	-	9.9	A	L	-	10.1	В		Ξ	9.5	<u>A</u> <u>A</u> <u>A</u> A		€	9.6	Ā	L	-	10.0	A	L	-	10.2	В
	T Int.	-	8.8 9.0	A A	T Int.	-	8.9 9.1	A A	l <u>⊥</u> Int.	≣	8.7 8.8	A A	l⊥ Int.	≞	8.8 8.8	A	T Int.	-	8.2 8.8	A A	T Int.	-	8.3 9.0	A A
Wes		th St	eet @			Oth St				200*	0.0		шь		0.0		IIIL.		0.0	А	IIIL.		9.0	А
		0.06		E			1 <u>28.0</u>			0.01	25.1	D	Li	0.08	67.0	F	lι	0.26	115.4	F	ΙL	1 75	1242.0	F
I '''		0.58		C		0.83		Ē		0.40	12.8	B	Ē	0.76	32.2	Ď				F			466.9	F
EB		0.01		D	L	0.02		F	Ī	0.02	22.9	<u>D</u> B C	L	0.02	67.0 32.2 61.2 19.7		L			F	L		=	F
	R	0.44	14.1	В	R	0.68	22.1	С		0.45	14.4	В	R	0.61	19.7	С	R	0.48	20.8	С	R	0.83	4 <u>7.2</u>	Ε

Notes:

L: Left Turn; T: Through; R: Right Turn; Int.: Intersection V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service

^{*}The unsignalized intersection analysis procedure, which assumes random arrival, breaks down with high major or minor street volumes and reports exaggerated levels of stop delays. As such, its results are not necessarily indicative of actual conditions, where gaps created by adjacent signalized intersections often provide additional throughput capacity and result in lower stop delays.

Table 17-17
2015 No Build Condition
Secondary Study Area Signalized Intersection Level-of-Service Analysis

				4 -	.1		Seco	nda	ary	Stua					ed In	ters	ecti	on I					Ana	lysis
ŀ		000 5			ak Ho		- D.::1	_	_	000 5			eak I		- D!		_	000 5			k Ho		la Dui	
			xisting	g		_	o Buile				xistin	<u>g</u>			o Buil	<u>a</u>		006 E	xistin	g		U15 N	lo Bui	-
Int	Ln		Delay (spv)	LOS	Ln		Delay (sny)		<u>Ln</u> Grn		Delay (spy)	ıos	<u>Ln</u> Grn		Delay (spy)	LOS	Ln Grn	V/C	Delay (spy)	LOS	Ln Grn	V/C	Delay (sny)	
			Vest 1				(351)		<u>UIP</u>	1/0	(SPY)		<u>UID</u>	1/0	(SPY)		Oip	1,0	(SPV)		Cip	1,0	(351)	
			24.1	C			24.7	С	LTR	0.17	21.3	C	LTR	0.20	21.7	С	LTR	0.20	20.8	С	LTR	0.27	21.8	С
WB	DefL	0.85	55.1	Ε		1.0 <u>1</u>				0.57		<u>C</u>			32.1				44.0	D	DefL	0.87	59.2	Ε
			29.0	С		0.58		С		0.38		С			26.3	C			43.1	D			48.4	D
			12.5	В		0.4 <u>6</u>		В		0.44		В			13.0				14.4	В			15. <u>1</u>	В
SB		0.68	20.0	В		0.72		С		<u>0.48</u>		<u>B</u>		0.51	13.4	_		0.55	17.4			0. <u>59</u>	18.3	В
	Int.		22.4	C	Int.		2 <u>6.9</u>	С	Int.		<u>16.0</u>	<u>B</u>	<u>Int.</u>		<u>16.7</u>	<u>B</u>	Int.		21.7	С	Int.		24. <u>3</u>	С
			Vest 12 27.5	C			27.9	С	lı тр	0.30	26.2	<u>C</u>	lı то	U 33	26.4	<u>C</u>	Іі тр	N 31	26.2	С	lı то	U 33	26.5	С
			100.2					F		0.77		B			51.5				57.8	Ĕ			70.9	Ĕ
		0.35		A		0.39		A		0.33		Ā		0.36				0.43		Ā		0.48		Ā
		0.52		Α		0.59		Α		0.41		A		0.44	6.4			0.42		Α		0.48		Α
	Int.		21.4	С	Int.		28.9	С	Int.		13.5	В	Int.		14.2	В	Int.		15.3	В	Int.		17.0	В
			venue					_				_				_				_				_
EB			38.4	D	L		39.3	D		0.48		Ē			41.7	臣			44.4	D			45.9	D
WB			72.7 29.2	E C	I I K	0.98	82.4 29.4	F C		0.67 0.02	<u>47.2</u> 27.7	<u>D</u> C			48.5 27.8	<u>D</u> C			49.4 27.3	D C			54.4 27.3	D C
VVB			29.2 32.1	C	_	0.09		C		0.02		C		0.03		C			31.7	C			32.5	C
NB		0.56		Ă		0.61		Ä		0.54	4.7	A		0.57	5.1	A		0.79		Ä			12.5	В
		0.53		A		0.57		A		0.31		A		0.34	3.2			0.31		A		0.35		Ā
	Int.		18.4	В	Int.		20.1	С	Int.		11.9	В	Int.		12.3	В	Int.		15.0	В	Int.		17.2	В
			glass l	Boule				5th S	Stree															
			24.9	С		1.01		Ē		0.65		<u>B</u>			<u>25.8</u>				26.6				<u>149.6</u>	
			21.6	C		0.86		C		0.52		<u>B</u>			28.1	C			25.5	C			255.6	
NB				B B		0.19		B B		0.33		흗			23.5	<u>C</u>	LIK	0.55	17.7	В	LIK	0. <u>72</u>	21.9	C
SB			15.0 17.1	В		0. <u>30</u> 0.57		В		0.30	24.9 24.1	늗			36.6 25.3	<u>D</u> C	ıт	0 41	18.0	В	ıт	0 44	18.5	В
OD			13.8	В			14.3			0.36		BICICICIC			27.5	č			18.4	В			19.3	В
	Int.	0.20	20.0	В	Int.	0.2.	33.2	Č	Int.	0.00	19.3	B	Int.	<u>v. 10</u>	26.8	č	Int.	0.00	22.1	Č	Int.	0.0 .	125.5	
Madi	son	Aven	ue @ L	East	125th	Stre	et														•			
			32.5	С			<u>102.5</u>	E		0.71		C			60.1	<u>E</u>			38.1	D			234.8	
			21.8	С			28.9	С		0.45		B			22.3	<u>c</u>			19.7	В			24.7	С
NB	LIR Int.	0.55	16.9 23.3	B C	ILIR Int.	0.59	17.4 50.4	B D		0.45		<u>B</u> B		0.48	<u>15.9</u>		ILIR Int.	0.67	18.8 25.1	B C	LIR Int.	0.74	20. <u>4</u>	C
Soco		lvoni	∠ა.ა ie @ E	-		Stroo		U	<u>Int.</u>		<u>19.7</u>	D	<u>Int.</u>		<u>34.7</u>	C	IIIL.		25.1	C	IIIL.		99.2	<u>E</u>
				asi i F				F	ΙI	1.05	86.7	F	I	1 47	257.3	Ε	Т	1 05	82.3	F	ΙТ	1 34	1 <u>96.3</u>	F
			32.3	c		0.37		D.	<u>Ē</u>	0.22	31.7	Ċ	Ē	0.60		D			31.7	Ċ			3 <u>1.7</u>	c
WB			103.1	F			128.5	F	Ϊ́	0.49		E C D			67.2	Ē			36.4	Ď			63.8	Ĕ
			103.3	F	Т	1. <u>35</u>	217.2	F				_				_								_
			20.2	С		0. <u>59</u>		C		0.31		<u>B</u>			<u>18.3</u>	<u>B</u>			22.3	С			3 <u>0.5</u>	C
SW		1.00	72.6	E		1. <u>22</u>	149.2	F		0.61	36.6	D		<u>0.76</u>	42.3	臣		0.84	47.2	D		<u>1.06</u>	88.6	Ē
Eiro 1	Int.	nus /	63.2	E 1254	Int.	'00t	1 <u>19.2</u>	F	Int.		43.5	D	Int.		103.1	<u>E</u>	Int.		52.8	D	Int.		81.8	<u>E</u>
			@ East 66.0	125t	th Str		102.9	F	Li	0.67	20 5	C	Li	0.82	38.4	D	l i	በ 87	42.5	D	L	1 06	82.4	F
			18.7	В			20.2	C	_	0.07		<u>С</u> В	븁		21.8	C			20.0	С			25.4	E C
NB			16.1	В	L		17. <u>4</u>	B	జ	0.13		Ā	붙		10.0	Ă		0.12		Ä		0.17		A
	Ŧ		10.8	В	Ŧ		11.0	В	Ī	0.39		В	Ī		12.4	В			13.4	В			1 <u>4.0</u>	В
	R	0.22	10.6	В	R	0.23	10.7	В	R	0.23	10.6	В	R	0.26	11.0	В	R	0.46	13.8	В	R	0.48	14.2	В
	Int.		26.1	С	Int.		3 <u>6.1</u>	D	Int.		<u>15.5</u>	<u>B</u>	Int.		<u>18.2</u>	<u>B</u>	Int.		19.0	В	Int.		2 <u>8.0</u>	С
			Vest 14		Stree		05.4	_	lı ze	0.00	05.7	_	lı ze	0 0-	05.0	_	l. 75	001	05.5	_	lı zc	0.00	05.7	_
			25.2	C D			25.4			0.26		Ē			<u>25.9</u>	뎥			25.5	C D			25.7	C D
		0.78	41.7 19.7	В		0.88 0.58		D C		0.90 0.63		E C		0.97	69.3 23.0	<u>E</u> C			46.0 24.8	С			53.0 28.5	С
SB		0.53		A	L	0.30		A	LIK	0.32	11.5	B	L	0.09		B			13.1	В			14.7	В
OD			10.5	В		0.48		В		0.50	11.4	B	_	0.54	12.0	B			10.7	В			11.1	В
	Int.		20.9	C	Int.		23.4	C	Int.	==	23.8	C	Int.		26.8	C	Int.		24.3	Č	Int.		27.3	Č
Notes	s:		.: Left T													า								
		١.	//C: Vol	lume i	to Car	pacity:	spv: S	econ	ds pei	Vehic	cle; LOS	S: Le	vel of	Servi	ce									

Table 17-18 2015 No Build Condition Secondary Study Area Unsignalized Intersection Level-of-Service Analysis

			Α	M Pea	ak Ho	ur					Midd	ау Ре	ak F	lour					Р	M Pe	ak Ho	our		
	2	2006 E	xistir	ig	20)15 N	o Bui	ld		2006	Existing	1	2	015 N	lo Bui	ld	2	006 I	Existir	ig	2	015 N	lo Buil	ld
	Ln																							
Int.	Grp	Grp V/C (spv) LOS Grp V/C (spv																						
First	<i>Ave</i>	enue	@ Eas	t 125	th St	reet S	South	oound	l Rig	ht Tu	rn													
SB	R	0.26	11.7	В	R	0.37	13.0	В	R	0.11	9.4	<u>A</u>	R	0.18	9.9	<u>A</u>	R	0.10	9.4	Α	R	0.21	10.2	<u>B</u>
Note	s:		R: Rigl	nt Turr	n; V/C	: Volu	me to	Capac	ity; s	pv: Se	conds p	er Vel	nicle;	LOS:	Level	of Ser	vice							

PRIMARY STUDY AREA

Twelfth Avenue and West 133rd Street

During the AM peak hour, the northbound left-turn movement would deteriorate from LOS D to LOS E, with delay increasing from 39.9 to 65.9 seconds and v/c ratio increasing from 0.91 to 1.02.

During the PM peak hour, the westbound approach would deteriorate from LOS D to LOS F, with delay increasing from 48.5 to 177.1 seconds and v/c ratio increasing from 0.89 to 1.30.

Twelfth Avenue and West 125th Street

During the AM peak hour, the, the westbound right-turn movement would deteriorate from LOS C to LOS D, with delay increasing from 34.1 to 48.8 seconds and a v/c ratio increasing from 0.86 to 0.96.

During the midday peak hour, the westbound right-turn movement would deteriorate from LOS C to beyond mid-LOS D, with delay increasing from 32.5 to 46.1 seconds and a v/c ratio increasing from 0.85 to 0.95.

During the PM peak hour, the westbound right-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 55.8 to 100.5 seconds and a v/c ratio increasing from 1.01 to 1.14.

Broadway Northbound and West 135th Street

During the PM peak hour, the eastbound *de facto* left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 51.7 to 83.0 seconds and v/c ratio increasing from 0.77 to 0.95.

Broadway and West 125th Street

During the AM peak hour, the southbound shared left-through-right movement would deteriorate from LOS D to LOS E, with delay increasing from 49.1 to <u>76.6</u> seconds and v/c ratio increasing from 0.90 to 1.04. The eastbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 36.8 to <u>275.6</u> seconds and v/c ratio increasing from 0.55 to <u>1.44</u>. The westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 44.6 to <u>159.2</u> seconds and v/c ratio increasing from 0.68 to <u>1.15</u>. At the same time, the westbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 30.4 to <u>57.7</u> seconds and v/c ratio increasing from 0.71 to <u>0.99</u>.

During the midday peak hour, the northbound right-turn movement would deteriorate within LOS D, with delay increasing from 39.6 to 49.5 seconds and v/c ratio increasing from 0.54 to 0.68. The westbound left-turn movement would deteriorate from LOS D to LOS E, with delay increasing from 39.1 to 77.6 seconds and v/c ratio increasing from 0.54 to 0.81.

During the PM peak hour, the northbound left-through movement would continue to operate at LOS D, with delay increasing from 42.9 to 50.5 seconds and v/c ratio increasing from 0.82 to 0.91. The southbound left-through-right movement would continue to operate at LOS D, with delay increasing from 38.9 to 45.9 seconds and v/c ratio increasing from 0.75 to 0.86. The eastbound left-turn movement would deteriorate from LOS C to beyond mid-LOS D, with delay increasing from 25.7 to 45.1 seconds and v/c ratio increasing from 0.27 to 0.55. The eastbound through-right movement would deteriorate from LOS D to LOS F, with delay increasing from 36.7 to 84.9 seconds and v/c ratio increasing from 0.83 to 1.08. The westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 50.8 to 140.8 seconds and v/c ratio increasing from 0.65 to 1.04.

Riverside Drive and West 135th Street

During the PM peak hour, the northbound approach would deteriorate from LOS C to LOS D, with delay increasing from 23.3 to 51.8 seconds and v/c ratio increasing from 0.98 to 1.07.

Amsterdam Avenue and West 125th Street

During the AM peak hour, the eastbound left-turn movement would deteriorate from LOS D to LOS F, with the delay increasing from 41.8 to 264.3 seconds and v/c ratio increasing from 0.56 to 1.39, and the eastbound through-right movement would deteriorate from LOS C to LOS \underline{E} , with the delay increasing from 34.1 to $\underline{60.5}$ and v/c ratio increasing from 0.77 to $\underline{0.99}$. At the same time, the westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 49.4 to $\underline{148.3}$ seconds and v/c ratio increasing from 0.67 to $\underline{1.08}$ and the westbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 33.3 to $\underline{66.8}$ and v/c ratio increasing from 0.76 to $\underline{1.02}$.

During the midday peak hour, the eastbound left-turn movement would deteriorate from LOS D to LOS F, with the delay increasing from 53.2 to 145.3 seconds and v/c ratio increasing from 0.72 to 1.10, and the eastbound through-right movement would deteriorate from LOS D to LOS E, with the delay increasing from 36.9 to 62.1 and v/c ratio increasing from 0.82 to 1.00. At the same time, the westbound left-turn movement would deteriorate from LOS C to beyond mid-LOS D, with delay increasing from 32.9 to 45.7 seconds and v/c ratio increasing from 0.38 to 0.51.

During the PM peak hour, the southbound left-turn movement would deteriorate from LOS C to LOS D, with delay increasing from 33.9 to 45.0 seconds and the v/c ratio increasing from 0.58 to 0.75. The eastbound left-turn movement would deteriorate from LOS D to LOS $\underline{\underline{\Gamma}}$, with delay increasing from 41.0 to $\underline{144.6}$ seconds and v/c ratio increasing from 0.58 to $\underline{1.06}$. The eastbound through-right movement would deteriorate from LOS C to $\underline{\underline{\Gamma}}$, with delay increasing from 34.8 to $\underline{84.5}$ seconds and v/c ratio increasing from 0.79 to $\underline{1.08}$. At the same time, the westbound left-turn movement would continue to operate at LOS D, with delay increasing from 35.4 to 54.1 seconds and v/c ratio increasing from 0.42 to 0.60 and the westbound through-right movement would deteriorate from LOS C to D, with delay increasing from 31.4 to 52.1 seconds and v/c ratio increasing from 0.71 to 0.96.

Marginal Street and West 133rd Street

During the AM peak hour, the southbound approach would deteriorate from LOS C to LOS F, with delay increasing from 18.5 to $\underline{101.8}$ seconds and v/c ratio increasing from 0.68 to $\underline{1.15}$.

During the midday peak hour, the southbound approach would deteriorate from LOS C to LOS E, with delay increasing from 23.4 to 45.2 seconds and v/c ratio increasing from 0.78 to 0.96.

During the PM peak hour, the southbound approach would deteriorate from LOS E to LOS F, with delay increasing from 40.8 to <u>95.9</u> seconds and v/c ratio increasing from 0.96 to <u>1.15</u>.

Marginal Street and West 132nd Street

During the AM peak hour, the southbound left-through movement would deteriorate from LOS B to LOS F, with delay increasing from 14.5 to 82.1 seconds.

During the PM peak hour, the same movement would deteriorate from LOS D to LOS F, with delay increasing from 29.2 to 70.4 seconds.

Marginal Street and St. Clair Place

<u>During the AM peak hour, the southbound through movement would deteriorate from LOS C to LOS E, with delay increasing from 22.0 to 42.4 seconds and v/c ratio increasing from 0.76 to 0.94.</u>

<u>During the midday peak hour, the same movement would deteriorate from LOS C to LOS D, with delay increasing from 18.5 to 30.4 seconds and v/c ratio increasing from 0.68 to 0.86.</u>

During the PM peak hour, the same movement would deteriorate from LOS D to LOS F, with delay increasing from 26.1 to 103.2 seconds and v/c ratio increasing from 0.81 to 1.15.

Twelfth Avenue and West 131st Street

<u>During the AM peak hour, the westbound approach would deteriorate from LOS C to LOS D, with delay increasing from 16.8 to 30.3 seconds and v/c ratio increasing from 0.16 to 0.33.</u>

During the PM peak hour, the westbound approach would deteriorate from LOS C to LOS F, with delay increasing from 23.5 to 85.7 seconds and v/c ratio increasing from 0.28 to 0.86.

Riverside Drive and St. Clair Place

<u>During the AM peak hour, the southbound left-through movement would deteriorate from LOS C</u> to LOS D, with delay increasing from 20.6 to 34.2 seconds and v/c ratio increasing from 0.26 to 0.42.

<u>During the midday peak hour, the same movement would deteriorate from LOS C to LOS D, with delay increasing from 22.2 to 31.9 seconds and v/c ratio increasing from 0.25 to 0.37.</u>

<u>During the PM peak hour, the same movement would deteriorate from LOS C to LOS D, with delay increasing from 22.8 to 33.6 seconds and v/c ratio increasing from 0.20 to 0.40.</u>

West 125th Street and West 129th Street/St. Clair Place

During the AM peak hour, the eastbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 30.0 to $\underline{92.8}$ seconds and v/c ratio increasing from 0.01 to 0.02. At the same time, the westbound left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 43.7 to $\underline{128.0}$ seconds and v/c ratio increasing from 0.06 to $\underline{0.17}$. The westbound right-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 22.6 to $\underline{47.4}$ seconds and v/c ratio increasing from 0.58 to $\underline{0.83}$.

During the midday peak hour, the eastbound left-turn movement would deteriorate from LOS C to LOS F, with delay increasing from 22.9 to 61.2 seconds and v/c ratio would remain at 0.02. At the same time, the westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 25.1 to 67.0 seconds and v/c ratio increasing from 0.01 to 0.08 and

the westbound right-turn movement would deteriorate from LOS B to LOS D, with delay increasing from 12.8 to 32.2 seconds and v/c ratio increasing from 0.40 to 0.76

During the PM peak hour, the eastbound left-turn movement would continue to operate at LOS F, and the eastbound right-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 20.8 to <u>47.2</u> seconds and v/c ratio increasing from 0.48 to 0.83. At the same time, the westbound left-turn movement would continue to operate at LOS F, with delay increasing from 115.4 to <u>1,242.0</u> seconds and v/c ratio increasing from 0.26 to <u>1.75</u>, and the westbound right-turn movement would continue to operate at LOS F, with delay increasing from 92.0 to <u>466.9</u> seconds and v/c ratio increasing from 1.05 to <u>1.94</u>.

SECONDARY STUDY AREA

Broadway and West 110th Street

During the AM peak hour, the westbound *de facto* left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 55.1 to $\underline{89.5}$ seconds and v/c ratio increasing from 0.85 to $\underline{1.01}$.

During the PM peak hour, the westbound *de facto* left-turn movement would deteriorate from LOS D to LOS E, with delay increasing from 44.0 to <u>59.2</u> seconds and v/c ratio increasing from 0.76 to <u>0.87</u>, and the westbound through-right movement would continue to operate at LOS D, with delay increasing from 43.1 to 48.4 seconds and v/c ratio increasing from 0.78 to 0.83.

Broadway and West 120th Street

During the AM peak hour, the westbound approach would continue to operate at LOS F, with delay increasing from 100.2 to 161.5 seconds and v/c ratio increasing from 1.04 to 1.21.

<u>During the midday peak hour, the westbound approach would continue to operate at LOS D,</u> with delay increasing from 45.8 to 51.5 seconds and v/c ratio increasing from 0.77 to 0.82.

During the PM peak hour the same approach would continue to operate at LOS E, with delay increasing from 57.8 to 70.9 seconds and v/c ratio increasing from 0.87 to 0.94.

Amsterdam Avenue and West 120th Street

During the AM peak hour, the eastbound through-right movement would deteriorate from LOS E to LOS F, with delay increasing from 72.7 to 82.4 seconds and v/c ratio increasing from 0.94 to 0.98.

During the PM peak hour, the eastbound through-right movement would continue to operate at LOS D, with delay increasing from 49.4 to 54.4 seconds and v/c ratio increasing from 0.73 to 0.78.

Frederick Douglass Boulevard and West 125th Street

<u>During the AM peak hour, the eastbound approach would deteriorate from LOS C to LOS E,</u> with the delay increasing from 24.9 to 61.0 seconds and v/c ratio increasing from 0.69 to 1.01.

During the PM peak hour, the eastbound approach would deteriorate from LOS C to LOS \underline{F} , with the delay increasing from 26.6 to $\underline{149.6}$ seconds and v/c ratio increasing from 0.74 to $\underline{1.25}$. The westbound approach would deteriorate from LOS C to LOS F, with delay increasing from 25.5 to 255.6 and v/c ratio increasing from 0.71to 1.49

Madison Avenue and East 125th Street

During the AM peak hour, the eastbound approach would deteriorate from LOS C to LOS \underline{F} , with the delay increasing from 32.5 to $\underline{102.5}$ seconds and v/c ratio increasing from 0.85 to $\underline{1.14}$.

<u>During the midday peak hour, the eastbound approach would deteriorate from LOS C to LOS E, with the delay increasing from 25.3 to 60.1 seconds and v/c ratio increasing from 0.71 to 1.02.</u>

During the PM peak hour, the eastbound approach would deteriorate from LOS D to LOS F, with delay increasing from 38.1 to 234.8 and v/c ratio increasing from 0.91 to 1.45.

Second Avenue and East 125th Street

During the AM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 86.0 to $\underline{252.1}$ seconds and v/c ratio increasing from 1.05 to $\underline{1.46}$. The westbound *de facto* left-turn movement would continue to operate at LOS F, with delay increasing from 103.1 to $\underline{128.5}$ seconds and v/c ratio increasing from 1.05 to 1.13, and the westbound through movement would continue to operate at LOS F, with delay increasing from 103.3 to $\underline{217.2}$ seconds and v/c ratio increasing from 1.05 to $\underline{1.35}$. The southwestbound approach would deteriorate from LOS E to LOS F, with delay increasing from 72.6 to $\underline{149.2}$ seconds and v/c ratio increasing from 1.00 to $\underline{1.22}$.

During the midday peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 86.7 to 257.3 seconds and v/c ratio increasing from 1.05 to 1.47. The westbound approach would deteriorate from LOS D to LOS E, with delay increasing from 35.6 to 67.2 seconds and v/c ratio increasing from 0.49 to 0.91.

During the PM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 82.3 to $\underline{196.3}$ seconds and v/c ratio increasing from 1.05 to $\underline{1.34}$. The westbound approach would deteriorate from LOS D to LOS E, with delay increasing from $\underline{36.4}$ to $\underline{63.8}$ seconds and v/c ratio increasing from 0.51 to 0.91. At the same time, the southwestbound approach would deteriorate from LOS D to LOS \underline{F} , with delay increasing from 47.2 to $\underline{88.6}$ and v/c ratio increasing from 0.84 to $\underline{1.06}$.

First Avenue and East 125th Street

During the AM peak hour, the eastbound left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 66.0 to $\underline{102.9}$ seconds and v/c ratio increasing from 0.99 to $\underline{1.11}$.

During the PM peak hour, the same movement would deteriorate from LOS D to LOS \underline{F} , with delay increasing from 42.5 to 82.4 and v/c ratio increasing from 0.87 to 1.06.

Broadway and West 145th Street

During the AM peak hour, the westbound approach would continue to operate at LOS D, with delay increasing from 41.7 to 51.9 seconds and v/c ratio increasing from 0.78 to 0.88.

<u>During the midday peak hour, the westbound approach would continue to operate at LOS E, with delay increasing from 56.4 to 69.3 seconds and v/c ratio increasing from 0.90 to 0.97.</u>

During the PM peak hour, the same approach would continue to operate at LOS D, with delay increasing from 46.0 to 53.0 seconds and v/c ratio increasing from 0.86 to 0.91.

PARKING SUPPLY AND UTILIZATION

On- and off-street parking utilization rates for the 2015 No Build condition were based on an increase in background parking demand of 0.5 percent per year for a total of 4.6 percent by 2015.

ON-STREET PARKING

Table 17-19 shows expected on-street parking utilization in 2015, assuming a 0.5 percent per year growth in background parking demand. As discussed below under "Off-Street Parking," vehicles generated by No Build projects within the on-street parking study area were not expected to materially add to the area's on-street parking demand, and therefore were assumed to be part of the background growth in the on-street parking analysis. On-street parking utilization within ¼ mile of the Project Area would increase to 80, 83, and 80 percent during the morning, midday, and evening periods, respectively. Overall, within ½ mile, on-street parking utilization would increase to 76, 82, and 83 percent during the morning, midday, and evening periods, respectively.

Table 17-19 2015 No Build On-Street Parking Utilization Summary

2015 No Build	AM	MD	PM		
		IVID	L IAI		
1/4-Mile Radius					
Capacity	1847	1847	1847		
2006 Existing Demand	1407	1457	1415		
Background Growth (0.5% per year)	65	67	65		
2015 No Build Demand	1471	1524	1480		
Remaining Spaces	376	323	367		
Utilization	80%	83%	80%		
½-Mile Radius					
Capacity	4783	4783	4783		
2006 Existing Demand	3454	3732	3807		
Background Growth (0.5% per year)	159	171	175		
2015 No Build Demand	3613	3903	3982		
Remaining Spaces	1170	880	801		
Utilization	76%	82%	83%		

OFF-STREET PARKING

A growth rate of 0.5 percent per year was also assumed for off-street parking facilities within the one-mile radius of the Project Area. In addition, the projected peak hour parking accumulation of the following No Build projects (identified in Section B of this chapter) are expected to utilize off-street parking facilities within the Project Area in the 2015 No Build condition:

- No Build Project 18: new Columbia University academic building
- No Build Project 21: new Columbia University administrative building (the Studebaker Building)
- No Build Project 22: science, math, and engineering secondary school
- No Build Project 23: new Columbia University office building
- No Build Project 24: new Columbia University office building

Other No Build projects within the off-street parking study area would mostly accommodate onsite ancillary parking spaces per New York City zoning requirements. As demonstrated in the analysis results summarized in Appendix H, the number of ancillary parking spaces anticipated in accordance with the applicable zoning regulations would be adequate in accommodating the demand projected from the parking accumulation analysis of relevant No Build projects.

The seven public parking facilities located with ¼ mile of the Project Area would be 113, 115, and 107 percent utilized, with expected shortfalls of 167, 195, and 99 spaces during the morning, midday, and evening periods, respectively. The nine public parking facilities located within ½ mile would be 108, 112, and 98 percent utilized, with expected shortfalls of 132 and 212 spaces during the morning and midday periods, respectively, and 30 available spaces during the evening period. Overall, within the one mile, the 26 public parking facilities would be 96, 98, and 75 percent utilized, with 135, 89, and 893 available spaces during the morning, midday, and evening periods, respectively (see Table 17-20).

Table 17-20 2015 No Build Off-Street Parking Utilization Summary

2015 No Build Off-St	reet raikiiiş	g Omizanon	Summar y	
2015 No Build	AM	MD	PM	
1/4-Mile Radius				
Capacity	1330	1330	1330	
2006 Existing Demand	1171	1081	1188	
Background Growth (0.5% per year)	54	50	55	
No Build Project Generated Parking Demand	272	394	186	
New Columbia Academic Building	48	118	69	
Science, Math, and Engineering Secondary School	13	13	0	
New Columbia Administrative Building (Studebaker)	86	148	89	
New Columbia Office Building (No. 23)	77	70	17	
New Columbia Office Building (No. 24)	48	45	11	
Total 2015 No Build Demand	1497	1525	1429	
Remaining Spaces	-167	-195	-99	
Utilization	113%	115%	107%	
½-Mile Radius				
Capacity	1710	1710	1710	
2006 Existing Demand	1501	1461	1428	
Background Growth (0.5% per year)	69	67	66	
No Build Project Generated Parking Demand	272	394	186	
Total 2015 No Build Demand	1842	1922	1680	
Remaining Spaces	-132	-212	30	
Utilization	108%	112%	98%	
1-Mile Radius				
Capacity	3576	3576	3576	
2006 Existing Demand	3030	2957	2387	
Background Growth (0.5% per year)	139	136	110	
No Build Project Generated Parking Demand	272	394	186	
Total 2015 No Build Demand	3441	3487	2683	
Remaining Spaces	135	89	893	
Utilization	96%	98%	75%	
Note: See Appendix H for detailed parking accumulation analysis.				

E. 2015 PROBABLE IMPACTS OF THE PROPOSED ACTIONS

OVERVIEW

The 2015 with the Proposed Actions condition, or the 2015 Build condition, reflects increases in traffic volumes, as well as, roadway and signal timing improvements that are part of the Proposed Actions¹. The 2015 Build condition was developed by first removing from the 2015 No Build condition those No Build projects that would not occur if the Proposed Actions are approved (No Build project numbers 23 and 24), modifying No Build trips that would be distributed differently with the Proposed Actions (No Build project numbers 18 and 22), incorporating traffic diversions due to roadway network changes, and finally adding the 2015 reasonable worst-case transportation scenario project-generated trips, as described below.

STUDY AREA STREET NETWORK

The 2015 Build analysis accounted for several roadway and signal timing improvements that would occur as part of the Proposed Actions. These improvements were included as part of the design process, since they would be integral to the overall planning of site access and traffic circulation, and reflect a more typical street grid configuration that addresses access and circulation needs of the community and the Manhattanville project. The project-related roadway improvements include the following (details provided in Appendix H):

- Conversion of West 133rd Street between Broadway and Twelfth Avenue from two-way to one-way westbound;
- Conversion of West 132nd Street between Broadway and Marginal Street from two-way to one-way eastbound;
- Conversion of West 131st Street between Broadway and Twelfth Avenue from two-way to one-way westbound;
- Conversion of West 125th Street between Twelfth Avenue and Marginal Street from twoway to one-way westbound; and
- Installation of traffic signals (and associated pavement markings) at the following intersections, which are currently stop-controlled:
 - Twelfth Avenue and West 131st Street
 - Twelfth Avenue and St. Clair Place/Riverside Drive
 - West 125th Street and St. Clair Place/West 129th Street
 - Marginal Street and West 133rd Street
 - Marginal Street and West 132nd Street
 - Marginal Street and St. Clair Place

In addition, a roadway improvement alternative for West 125th Street between Broadway and Marginal Street is currently being considered as part of a streetscapes project conducted by the

¹ The probable impact analysis presented in this section includes only operational impacts which would occur in 2015 and does not include cumulative impacts due to construction. Cumulative impacts are discussed in Chapter 21, "Construction."

New York City Economic Development Corporation (EDC). This alternative marginally differs from components of the Proposed Actions in sidewalk widths and roadway alignment. However, from a traffic analysis standpoint, because roadway geometry and operational assumptions would be identical under both alternatives, the analysis findings discussed below would be the same regardless of the alternative chosen.

VEHICLE TRIP ASSIGNMENT

To develop an appropriate baseline traffic network from which potential project impacts could be evaluated, the 2015 No Build traffic network volumes were adjusted to account for traffic diversions resulting from the project-related street direction changes, No Build projects that would not be constructed if the Proposed Actions are approved, and changes in how certain No Build trips would be distributed. The primary and secondary study area net 2015 diverted peak hour traffic volumes are shown in Appendix H.

In the 2015 Build condition, the Columbia University development component of the Proposed Actions is projected to generate a total of 310 (252 entering and 58 exiting), 156 (78 entering and 78 exiting), and 307 (67 entering and 240 exiting) vehicle trips during the weekday AM, midday, and PM peak hours, respectively. The non-Columbia University development component is expected to generate a total of 196 (136 entering and 60 exiting), 302 (161 entering and 141 exiting), and 355 (157 entering and 198 exiting) vehicle trips during the AM, midday, and PM peak hours, respectively. The 2015 project-generated peak hour traffic volumes within the primary and secondary study areas are shown in Appendix H.

COLUMBIA UNIVERSITY TRIPS

Trips associated with Columbia University uses were assigned to the traffic network based on area travel patterns and information accumulated from the on-line survey of Columbia University personnel. Separate trip assignments were prepared separately for student cars, student taxis, faculty/other employees cars, faculty/other employees taxis, and trucks. Auto and taxi trips generated by the Columbia University component of the Proposed Actions were assigned to study area roadways based on likely pathways between their origins and destinations. The primary routes expected to carry the majority of project-generated traffic include Broadway, 125th Street, the Triborough Bridge, the FDR Drive, and the Henry Hudson Parkway. Truck trips to and from the Project Area would traverse designated local truck routes in Manhattan, which include 125th and 145th Streets, Broadway, and Amsterdam Avenue.

It was also assumed that a shuttle bus service would be provided by Columbia University to transport students and faculty/other employees within and between the Manhattanville and the Morningside and Health Sciences campuses. Shuttle bus trips would traverse a set route through the campuses, with stops at specific locations. Trips associated with this service were incorporated into the Build analysis under the assumption that they would travel at 10-minute headways, resulting in 12 (six entering and six exiting) bus trips during each peak hour to/from Morningside or Health Sciences for a total of 24 shuttle bus trips during each peak hour.

Trips approaching the primary study area were assigned to the area's parking facilities, loading areas, or at or near proposed development sites. All Columbia University auto trips are expected to use off-street parking in four facilities in the primary study area, described in more detail in the "Parking Supply and Utilization" section below. Ingress/egress points for these facilities are located on West 133rd Street between Broadway and Amsterdam Avenue, West 131st Street between Twelfth Avenue and Broadway, and Broadway at West 132nd Street.

NON-COLUMBIA UNIVERSITY TRIPS

Future Build peak hour non-Columbia University trips were also assigned to the study area traffic network. The most recent Census reverse journey-to-work data were used to develop trip origin and destination patterns for office and residential uses.

TRAFFIC VOLUMES AND INTERSECTION CAPACITY ANALYSES

The 2015 project-generated traffic volumes were added to the 2015 traffic diversion network volumes to obtain the 2015 Build traffic network. Figures 17-20, 17-21, and 17-22 depict the 2015 Build condition traffic volumes within the primary study area for the weekday AM. midday, and PM peak hours, respectively. The secondary study area peak hour 2015 Build condition volumes are presented in Figures 17-23 to 17-25.

Within the primary study area, increases in two-way peak hour traffic over 2015 No Build volumes are expected to range from 20 vph to 400 vph along Broadway, from 10 vph to 100 vph along Amsterdam Avenue, and from 30 vph to 400 vph along Twelfth Avenue. Two-way peak hour volumes along West 125th Street would increase by 50 vph to 150 vph during the two analysis periods.

Tables 17-21 and 17-22 compare the 2015 No Build and Build peak hour conditions for the primary study area intersections. The secondary study area 2015 No Build and Build peak hour conditions are presented in Tables 17-23 and 17-24. Based on the thresholds established in the CEQR Technical Manual, as described in Section B of this chapter, significantly impacted movements are denoted with a + sign in the tables and detailed below.

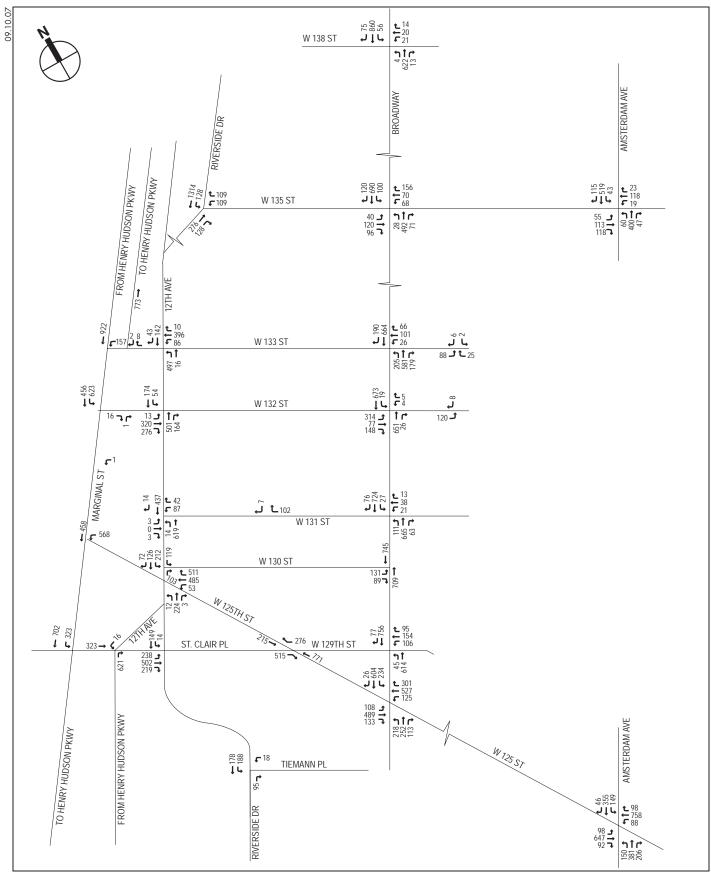
PRIMARY STUDY AREA

Amsterdam Avenue and West 125th Street

During the AM peak hour, the westbound left-turn movement would continue to operate at LOS F, with delay increasing from $\underline{148.3}$ to $\underline{194.1}$ seconds and v/c ratio increasing from $\underline{1.08}$ to $\underline{1.20}$, and the westbound through-right movement would deteriorate from LOS E to LOS F, with delay increasing from $\underline{66.8}$ seconds to $\underline{100.5}$ seconds and v/c ratio increasing from $\underline{1.02}$ to $\underline{1.12}$. The eastbound through-right movement would continue to operate at LOS \underline{E} , with delay increasing from $\underline{60.5}$ to $\underline{75.6}$ seconds with v/c ratio increasing from $\underline{0.99}$ to $\underline{1.05}$.

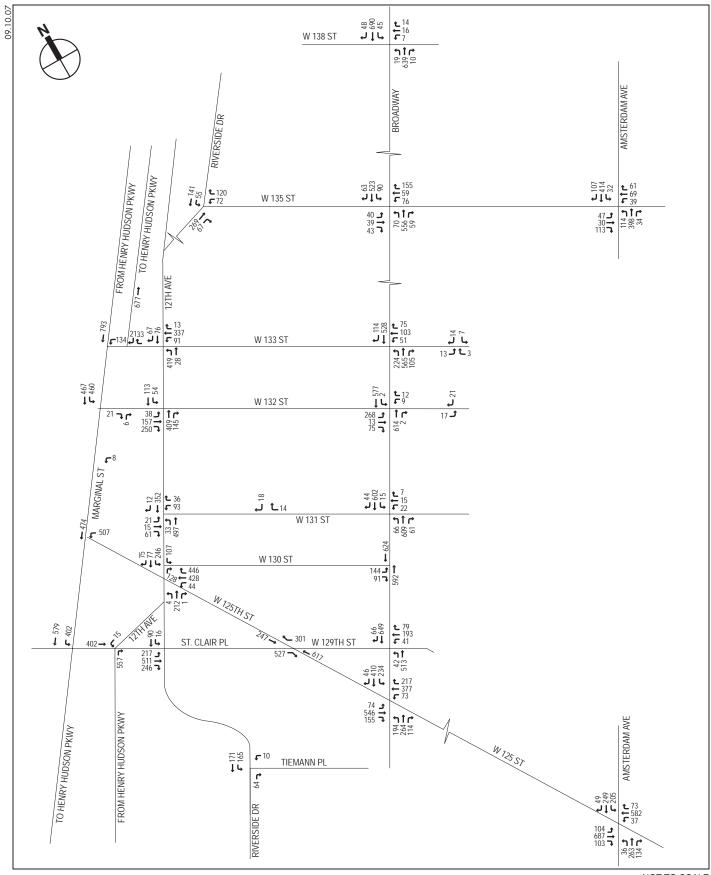
During the midday peak hour, the eastbound left-turn movement would continue to operate at LOS F, with delay increasing from 145.3 to 235.8 seconds and v/c ratio increasing from 1.10 to 1.33, and the eastbound through-right movement would deteriorate from LOS E to LOS F, with delay increasing from 62.1 to 87.3 seconds and v/c ratio of 1.00 to 1.08. The westbound through-right movement would deteriorate within LOS D, with delay increasing from 38.0 to 48.3 seconds and v/c ratio increasing from 0.84 to 0.93.

During the PM peak hour, the eastbound left-turn movement would <u>continue to operate at LOS E</u>, with delay increasing from <u>144.6</u> to <u>168.5</u> seconds and v/c ratio increasing from <u>1.06</u> to <u>1.13</u>, and the eastbound through-right movement would <u>also continue to operate at LOS E</u>, with delay increasing from <u>84.5</u> to <u>125.2</u> seconds and v/c ratio of <u>1.08</u> to <u>1.18</u>. The westbound through-right movement would <u>deteriorate from LOS D to LOS E</u>, with delay increasing from <u>52.1</u> to <u>75.0</u> seconds and v/c ratio increasing from <u>0.96</u> to <u>1.05</u>.



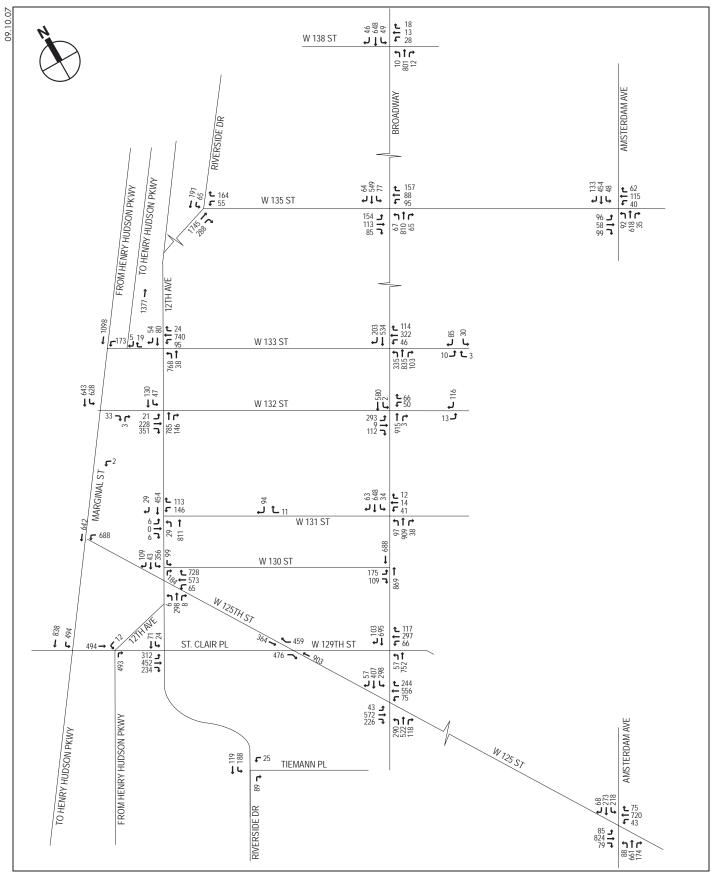
NOT TO SCALE

Figure 17-20
Primary Study Area
Build 2015 Morning Peak Hour



NOT TO SCALE

Figure 17-21
Primary Study Area
Build 2015 Midday Peak Hour



NOT TO SCALE

Figure 17-22
Primary Study Area
Build 2015 Evening Peak Hour

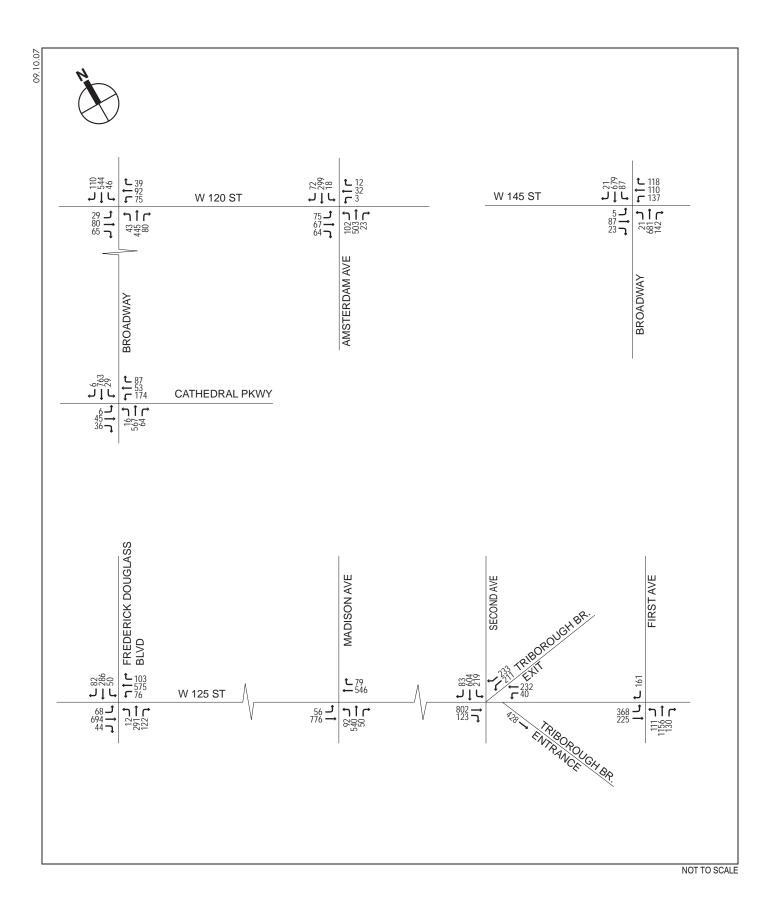


Table 17-21 Comparison of 2015 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

			A B	/ Day	ale Lle		- 11	11116	ary i	Jiuu					ed In	tCI b	·	011 1					xiiai	yor
	2(015 N	lo Buil	/IPea	ak HC		Build		2	015 N	<u>wilac</u> lo Buil	_	eak	<u> 10ur</u> 2015	Build		20	015 N	lo Buil		ak Ho		Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
_					_			LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
<i>Mar</i> g WB		Stree	t @ W	est 1			t* 32.3	C	ĺ				Li	0.38	30.5	С	ı				Li	0.56	37.6	D
SB						0.90		Ċ						0.67		B						0.91		C
					Int.		27.2	С					Int.		16.1	В					Int.		26.1	С
<i>Marg</i> SB		Stree	t @ W	est 1				В	ĺ				Lit	0.50	14.6	D	ı				LIT	0.82	21.0	_
SB					Int.	0. <u>73</u>	1 <u>7.6</u>	В					Int.	0.56	14.6	<u>В</u> В					Int.	U. <u>02</u>	21.0	<u>C</u>
Marg	inal	Stree	t @ W	est 1		Stree										_								
			2 <u>6.0</u>			0.63		С			<u>25.0</u>	<u>C</u>			<u>27.3</u>	<u>C</u>			2 <u>8.2</u>	C		0.80		C
SB	Int.	0.4 <u>7</u>	1 <u>4.3</u> 19. <u>6</u>		Int.	0.3 <u>2</u>	12. <u>0</u> 2 <u>1.4</u>	B C	Int.	<u>U.5U</u>	<u>14.7</u> 18.9	<u>В</u> В	⊥ Int.	0.34	20.3	<u>B</u> C	Int.	U.6 <u>4</u>	1 <u>7.1</u> 2 <u>1.9</u>	B C	Int.	0.4 <u>6</u>	14. <u>1</u> 24.7	B C
Marg	_	Stree	t @ St																					
SB								В							<u>14.6</u>	<u>B</u>						0.32		В
					LI Int.	0.5 <u>3</u>	13. <u>8</u> 1 <u>3.3</u>	B B					LT Int.	0.40	12.1 13.0	<u>B</u> B					I L I Int.	0. <u>72</u>	1 <u>7.4</u> 1 <u>6.4</u>	B B
Rive	rside	Driv	e @ W	est 1		Stree		Ь					шь		13.0	D	l				IIIt.		10.4	ь
	L	0.59	35.0	С	L	0.33	28.2				26.5	C		0.21		C			26.0	С	L	0.17	25.6	С
ND			31.7	C		0.45		C		0.38		<u>C</u>		0.46		<u>C</u>			38.4	D		0.67		D
		0.23		A B		0.23 0.88		A B		<u>0.21</u> 0.11		<u>A</u> A		0.20 0.13		<u>A</u> A		0.21	51.8 8.2	D A		1.02		C F+
		0.00		_		0.00		_	_			=	==			=		0.2.	0.2			0.20		Α
<u></u>	Int.			В	Int.		14.9	В	Int.		<u>12.8</u>	<u>B</u>	Int.		<u>13.4</u>	<u>B</u>	Int.		46.3	D	Int.		33.9	С
Rive:		Driv	e @ Si	t. Cla		o.21	45	Α	I				ΙL	0.21	45	Α	ı				Li	0.28	4 Q	Α
						0.66		Ä						0.68		A						0.62		Ä
SB						0.37		D						0.25		C						0.22		C
T	ELL A		- @ 14	14	Int.		12.2	В					Int.		11.1	<u>B</u>					Int.		9. <u>8</u>	Α
			е @ И 37.1				e t 21.9	С	lltr	0.62	30.5	С	ΙL	0.23	22.3	C	lltr	1.30	177.1	F	ΙL	0.23	22.2	С
		00	• • • • • • • • • • • • • • • • • • • •			0.49		Č				=		0.44		С				•		0.89		Ď
NB			65.9			0.71		С		0.79		C	_	0.75		C			44.2	D		0.91		D
S B			11.0 10.4	В		0.29		B B			10.8 10.1	<u>В</u> В		0.09	10.1 10.1				21.6 10.0	C B		0.60 0.10		В <u>В</u>
SB	Int.	0.15	42.1	D	Int.	0.15	20.2	C	Int.	<u>U.12</u>	23.2	E C	Int.	<u>U.11</u>	21.4	E C	Int.	0.10	81.7	F	Int.	0.10	32.5	E
			e @ W			Stre																		
			37.2		LTR	0. <u>63</u>	2 <u>7.5</u>	С				<u>C</u>	<u>LTR</u>	0.51	<u>25.3</u>	C			28.9	С	LTR	0.6 <u>2</u>	2 <u>7.2</u>	С
			20.7 13.8	C B	TR	0.50	14 1	В			20.3 13.0	<u>С</u> В	TR	0.40	12.8	В			20.6 17.6	C B	TR	0.66	16 9	В
			10.3			0.24		В		0.07		Ā		0.20		_	LTR			A		0.18		В
	Int.		21.1	С	Int.		1 <u>9.2</u>	В	Int.		<u>15.6</u>	В	Int.		17.3	В	Int.		19.9	В	Int.		19. <u>9</u>	В
Twell WB		venu	e @ W	est 1		Stree 0.23		С	1				1 1	U 28	23.2	C	ı				Li	0.38	24 0	С
VVD						0.23		В					_	0.28 0.00		<u>С</u> В						0.00		В
					R	0.11	20.6	С					R	0.11	20.6	С					R	0.32	24.0	Č
NB						0.50		В							13.9	B							17. <u>6</u>	В
SB					IR Int.	0.3 <u>4</u>	1 <u>2.1</u>	В						0.27	11.4	<u>B</u>					Int.	0.3 <u>4</u>	1 <u>2.U</u>	В
Twel	fth A	venu	е @ И	est 1		Stree	t*																	
NB							15.7								<u>15.8</u>	<u>B</u>						0.72		В
SB							30.3 12. <u>8</u>								28.0 12.7	<u>C</u> B							4 <u>0.3</u> 10. <u>5</u>	D В
					Int.	0. <u>51</u>	16.2	В					Int.	0.00	15.9	B					Int.	0.5 <u>0</u>	1 <u>5.8</u>	В
			e @ W			Stree																		
			1 <u>3.4</u>	В							14.1	B							1 <u>5.1</u>	В				
WB			1 <u>3.0</u> 2 <u>3.8</u>	B C	ıт	0.59	2 <u>5.6</u>	C	Ī		<u>13.3</u> 19.9	<u>В</u> В	lт	0.50	23.8	C			1 <u>5.3</u> 30.6	B C	lт	0.83	3 <u>8.3</u>	D
			48.8	Ď		0.77		В			46.1	D		0.78		С			100.5			1.01		D
			20.6	С			28.2	С			20.1	<u>C</u>		0.32		C			22.2	С		0.45		С
SB	LI	0.38	22.1	С			2 <u>9.2</u> 11.6	C B	ഥ	0.23	<u>19.5</u>	<u>B</u>			23.2 11.1	<u>С</u> В	LI	υ.34	21.8	С		0.8 <u>7</u> 0.12	32.5 8.4	C A
	Int.		28. <u>7</u>	С	Int.	0.10	2 <u>3.2</u>	C	Int.		26.6	C	Int.	<u> </u>	22.0	C	Int.		49. <u>8</u>	D	Int.	J. 12	3 <u>8.1</u>	D
Notes			: Left T	urn;	T: Thr		R: Righ	t Tur	n; Det		facto Le	eft Tu	rn; Int		rsection	1								
1			//C: Vo - Signifi						ds per	Vehic	cle; LOS	S: Le	vel of	Servi	ce									

 ^{*} Volume to Capacity, spv. Seconds per Venicle, ECS. Level of Service
 * Significant Adverse Traffic Impact
 * indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition

Table 17-21 (Continued) Comparison of 2015 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

			ΑN	/I Pea	ak Ho	our	11	11116	ur y i	Jiuu	•		_	Hour	ea In	ter s	, cu	VII I			ak Ho		ı xııaı	y 313
	20	15 N	o Buil				Build		2	015 N	o Buil				Build		20	015 N	o Buil		L		Build	
l	Ln		Delay		Ln		Delay		Ln		Delay		<u>Ln</u>		Delay		Ln		Delay		Ln		Delay	
			(spv) e @ S				(spv)	LOS	Grp	V/C	(spv)	LOS	<u>Grp</u>	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
EB		venu	e @ 3	t. Cia	T		22.9	С	1				Ιт	0.37	23.7	С	I				Ιт	0.48	25.2	С
NB							40. <u>9</u>							0.82		D							32. <u>1</u>	Č
SB						0.08	32.4						L	0.07	32.1	C						0.05	31.9	
14/	4 405	4- 04-		14/	Int.	44- C4-	34. <u>5</u>	C	i D/-	+			Int.		<u>31.4</u>	D					Int.		28. <u>6</u>	С
EB		n su	eet w	wes			eet/St. 43.5		ir Pia	ce			l R	0.75	35.4	D	l				l R	0.84	42.7	D
WB							26.7						R	0.44	27.0	С							35.6	
NB							3 <u>8.8</u>								27.9								43. <u>8</u>	
SB					Int.	0.2 <u>4</u>	2 <u>1.0</u> 36.1	D					Int.	0.23	20.8 29.0	<u>C</u>					T Int.	0.38	21. <u>3</u> 38.2	D
Broa	dwa	/ @ V	Vest 1	38th		et	J <u>U. 1</u>	<u> </u>	<u> </u>				11114		20.0	<u> </u>	l				III.		J <u>U.Z</u>	D
WB	LTR	0.17	24.3	С	LTR	0.17	24.3		LTR	0.11	23.4	С			23.4				23.9	С	LTR	0.18	24.4	С
NB			6.2				6.2					A		0.37		A		0.41		Α		0.42		Α
SB	LTR		4.7 8.4			0.04	4.9 8.0	A A		0.02 0.44		<u>A</u>		0.02 0.50		<u>A</u> A	LTR	0.02		A A		0.04 0.47		A A
OB	Int.	0.00	8.0	Α	Int.	0.02	8.3	Α	Int.	<u> </u>	7.0	Α	Int.	0.00	7.4	Α	Int.	0.41	7.2	Α	Int.	0.47	7.6	A
			thbou				th Stre	et								_				_				_
EB	LT	0.36	25.8	С	LT	0.41	26.5	С	ഥ	0.27	<u>24.8</u>	<u>C</u>	ഥ	0.31	<u>25.2</u>	<u>C</u>								F C
				С				С				C	IR	0.53	29.4	C	TR	0.54	29.7	С	TR	0.58	30.4	С
NB		0.51		A				A		0.47				0.55		_		0.65				0.70		
Bros		/ Sou							ınt.		15.4	ㅂ	ınt.		10.2	ㅂ	ınt.		23.1	Ü	ınt.		23.9	Ü
									IR	0.19	24.0	С	IR	0.20	24.1	С	TR	0.67	32.8	С	TR	0.54	29.3	С
WB	LT	0.34	25.6	С	LT	0.32	25.5	С	LI	0.27	24.7	С				С	LT	0.40	26.9	С				
C D		0 10	E 1	^	١,	0.15	E 1	٨	١.	0.00	E 1	^		0.14	E 4	^	١.	0.00	E 0	٨				
90																								
	B L 0.10 5.1 A L 0.15 5.4 A L 0.09 5.1 A L 0.14 5.4 B LT 0.20 7.8 A L 0.15 5.4 B LT 0.21 17.4 B B LT 0.56 13.9 B L 0.37 15.5 B LT 0.44 12.2 B L 0.39 12.4 B LT 0.78 18.9 B LT 0.56 13.9 B L 0.56 15.4 B LT 0.56 13.9 B L 0.56 15.4 B LT 0.56 15.9 B LT 0.56 15.9 B LT 0.56 15.4 B LT 0.56 15.4 B LT 0.56 15.4 B LT 0.56 15.4 B LT 0.56 15.9 B LT 0.56 15.4 B LT 0.56 15.9 B LT 0.56 15.4 B LT															В								
	TR 0.49 28.7 C TR 0.52 29.2 C TR 0.48 28.6 C TR 0.53 29.4 C TR 0.65 9.7 A LTR 0.70 10.6 B Int. 16.6 B Int. 17.5 B Int. 15.4 B Int. 16.2 B Int. 23.7 C Int. 23.9 Int. 2																							
	Int. 16.6 B Int. 17.5 B Int. 15.4 B Int. 16.2 B Int. 23.7 C Int. 23.9 Octobrology Southbound @ West 135th Street															D								
			9.6	Α			16.7	В			9.4	Α			13.9				9.2	Α				C
	Northbound @ West 133rd Street B LT 0.21 17.4 B															С								
					y we:	st 133	rd Str	eet	l TR	ი 13	18 1	R	1				l tr	0 10	177	R	1			
					LT	0.20	7.5	Α					LT	0.22	7.6	Α					LT	0.35	5.2	Α
SB	LTR	0.52	12.7	В	TR	0.77		С				В				С		0.48	12.2		TR	0.87	41.6	
		•							<u>Int.</u>		<u>14.0</u>	<u>B</u>	<u>Int.</u>		<u>18.5</u>	<u>B</u>	Int.		18.2	В	Int.		23.8	С
									Li	0.23	17 7	В	Ιιτ	0.61	22 8	С	lι	0.31	18 7	В	Ιιτ	0.81	40 2	D
WB		2.10	_0.0	٠			15.1	В	=						15.3	B	-	3.51		_				В
NB		0.41	11.7			0.47	14.4			0.35	11.1	<u>B</u>	_	0.40	13.6			0.57		В		0.55	13.1	В
Broo	Int.	, Sa.	14.0	B nd @	Int.	cf 122	25.2 nd St r	C	<u>Int.</u>		<u>12.6</u>	<u>B</u>	<u>Int.</u>		<u>16.5</u>	<u>B</u>	Int.		14.6	В	Int.		19.8	В
			тпрои 27.9				na Str 20.9		ΙR	0.32	20.7	C	IR	0.27	17.6	В	TR	0.46	22.9	С	TR	0.30	19.7	В
					R	0.25	18.2	В					R	0.24	18.3	В					R	0.34	21.8	С
			15.8		L		15.2				<u>15.6</u>				13.4 15.0				15.6				19.3	
28	LTR Int.	U.51	12.9 17.0	B B	Int.	0.59	16.6	B B	Int.	0.41	11.7 13.8	<u>B</u> B	Int.	<u>u.49</u>	15.0 16.0	<u>В</u> В	LTR Int.	0.43	11.9 15.2	B B	Int.	0.47	12.4 15.9	B B
Broa		/ @ V	Vest 1			et	10.0		11114		10.0		2116		10.0				10.2				10.0	
EB	LTR	0.20	21.7	С						0.24		<u>C</u>							21.8	C				
WB			20.6								20.5	<u>E</u>							21.3	С				
NR	LTR		19.8 8.8	B A						0.02	<u>19.6</u> 8.1	<u>B</u>						0.04	19.8 9.6	B A				
	LTR			Α						0.37		Δ						0.42		Α				
	Int.		10.6	В					Int.		10.1	В					Int.		10.7	В				
Notes	s:						R: Righ spv: S									n								
							spv. 3 ffic Imp		us pei	A CITIC	AU, LU	J. LC	v GI UI	OEI VIC										
							vhich w		ınsign	alized	in the I	Νο Βι	uild co	nditio	n and s	ignal	ized ir	the E	Build co	nditio	n			

Table 17-21 (Continued) Comparison of 2015 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

			ΔΙ	VI Pea	ak Ho	ıır		rıma	11 y k	Jiuu	•		eak		ea In	tCI S	ccu	OII I		l Pea			Alle	i y bib
	20	015 N	o Buil		_		Build		20	015 N	o Buil	_	Car		Build		20	015 N	lo Buil		IK IIC		Build	
	Ln	-	Delay		Ln		Delay		Ln		Delay		Ln	-	Delay		Ln	-	Delay	_	Ln		Delay	
Int.	Grp		(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
		/ Nor	thbou	nd @			st Stre									ı	ii			ı	i			
EB					T	0.06		С					ΙĒ	0.03		B						0.07		С
WB NB						0.09		C B					_	0.08 0.43	20.1 9.2	<u>C</u>						0.09		C B
IND					Int.	0.51	11.1	B					Int.	0.43	9.2 10.0	B					Int.	0.59	11.6	В
Broa	dwa	/ Sou	thbou	ınd @		st 131	st Str						11111		10.0									
WB		,				0.19		С	ĺ				LI	0.13	20.6	С					LT	0.17	20.9	С
SB						0.64		В						0.51		<u>B</u>						0.56		В
					Int.		13.7	В					Int.		<u>11.6</u>	<u>B</u>					Int.		12.6	В
		y @ v 0.25	Vest 1	30th	Stree		24.4	С	Iть	0.10	24.5	_		0.24	24.6	C	IВ	0.21	26.5	С	ΙL	0.27	25.0	С
EB	LK	0.25	25.0	C	R		27.3	C	<u>LR</u>	0.19	<u> 24.5</u>	<u>C</u>	R	0.24	27.6	C	LK	0.31	20.5	C	R	0.40		C
NB	LT	0.26	5.7	Α	T	0.38	6.4	Ä	I	0.21	5.4	Α	Ī	0.31	6.0	Ä	LT	0.32	6.0	Α	T	0.42	6.7	A
SB	LT	0.37	6.3	Α	LT	0.50	7.6	Α	Ī	0.29	5.9	Ā	Ī	0.42	6.9	Ā		0.32		Α	LT	0.49	7.4	Α
	Int.		7.2	Α	Int.		9.5	Α	Int.		6.8	Α	Int.		9.6	Α	Int.		7.5	Α	Int.		10.1	В
			Vest 1		Stree		40.0	_	l	0.44	40.7	_		0.40	40.0	_		۰.	04.4	_		۰	04.6	
WB		0.4 <u>2</u> 0.20		B B	LT R	0.43	19. <u>3</u> 16.7	B B		0.41 0.18	18.7 15.9	<u>В</u> В	H R	0.42 0.19	18.9 16.2	<u>B</u> B			21. <u>1</u> 17.1	C B		0. <u>57</u> 0.31		C B
NB		0.20		C	LT	0.59		C		0.18	17.6	B	Ϊ́Ι	0.13		В			23.5	C		0.84		С
		0.48		В				-	==			=	==			=				-				-
SB	TR	0.60	17.7	В	TR	0.86	27.8	С	IR	0.44	18.1	В	IR	0.69	23.4	C	TR	0.49	18.8	В	TR	0.80	27.4	С
	Int.		19.1	В	Int.		23.6	С	Int.		<u>18.0</u>	<u>B</u>	Int.		<u>20.8</u>	<u>C</u>	Int.		2 <u>0.9</u>	С	Int.		2 <u>6.9</u>	С
	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$															_								
FB	Broadway @ West 125th Street EB L 1.44 275.6 F L 0.93 92.4 E L 0.56 37.1 D L 0.37 24.3 C L 0.55 45.1 D L 0.45 38.2 E TR 0.79 34.3 C T 0.62 29.8 C TR 0.87 39.7 D T 0.49 22.3 C TR 1.08 84.9 E T 0.76 34.4 C																							
	IK	0.7 <u>9</u>	3 <u>4.3</u>	C					112	0.07	39.1	U	_	-		జ	IK	1.00	04.9	드		_		
WB	L	1.15	159.2	F	Ĺ			F	L	0.81	77.6	E	L			Č	L	1.04	140.8	F	Ĺ			Ē
		0.99		Ē	T	0.68	31.2	C	ĪR	0.73		C	Ī	0.42	21.2	C	TR		44.9	D	T	0.69	31.9	Ĉ
		_			R	0.76	27. <u>7</u>	С					R	0.79	41.8	D					R	0.64	21.8	С
NB		0.46		С	L		36.3	D		0.40		<u>C</u>	L	0.47	38.3	D		0.60		D	L	0.61		D
		0.48		С	T		22.6	С		0.47	<u>31.5</u>		Ī	0.40	<u>27.4</u>	C			50.5	D	T	0.61		С
SB		0.6 <u>2</u> 0.38		D	R L	0.50	30. <u>5</u> 36.5	C D	片	0.68 0.35	49.5 30.4	<u>D</u>	R L	0.68 0.59	46.4 41.1	<u>D</u>	K L		4 <u>3.4</u> 30.1	D	R L	0.5 <u>0</u> 0.55	3 <u>0.1</u> 37. <u>3</u>	C
SB		1.04		E	T		27.8	C	lı∄ĸ	0.79	40.9	D	Ī	0.56	30.0	C	_		4 <u>5.9</u>	Ď	Ť	0.3 <u>3</u>		С
			. =	_	R		22.4	Č				=	Ē	0.23				0.0		_	R	0.29		Č
	Int.		64.6	Е	Int.		3 <u>5.6</u>	D	Int.		37.7	D	Int.		28.8	C	Int.		5 <u>6.9</u>	<u>E</u>	Int.		3 <u>1.5</u>	С
			enue					_	L	0.00	05.0	_		0.05	05.5	_		0.50	25.4	_		001	20.4	_
EB		0.42		СС	LT R		29.8 36.3	C D		0.23	25.2 25.8		H	0.25		alala		0.58 0.27		D	LI R	0.64	38.4	D C
WR		0.36		C		0.56		C		0.23	<u>25.6</u> 31.8	<u>C</u>		0.46 0.55	31.3 32.3	S.			36.2	D		0.43		D
	LTR		7.9	A		0.52	8.3	A		0.54	8.8	Ä		0.62	10.3	B		0.69	11.3	В			13.5	В
	LTR		8.9	Α		0.62	9.6	Α		0.49	7.9	Ā		0.53	8.4	Ā		0.55	8.6	Α	LTR	0.61	9.5	Α
	Int.		13.6	В	Int.		15.1	В	Int.		13.0	B	Int.		14.6	В	Int.		16.1	В	Int.		18.0	В
			enue					_		4 40	445.0	_		4.00	005.0	_		4.00	444.0	_		4.40	400 5	_
EB			264.3	F	L TR		264.3	F.	I IR	1.10 1.00	145.3	Ē	ҍ		235.8 97.3		L		144.6 84.5	Ē	L		1 <u>68.5</u>	F+ F+
WB	L	0.9 <u>9</u> 1.08	<u>60.5</u> 148.3	<u>E</u> F	L		75.6 194.1	<u>E</u> + F+		0.51	62.1 45.7	E D	IR L	1.08 0.51	87.3 45.7	탄 D	L	1.0 <u>8</u>	54.5	<u>E</u>	L	0.60	1 <u>25.2</u> 54 1	F+ D
""		1.02	66.8	Ë			100.5	F+	_	0.84	38.0	D	ĪŘ	0.93	48.3	D+		0.96		D		1.05	75.0	E+
NB		0.35	15.7	В	L		16.1	В		0.07	10.1	В	Ī	0.08	10.2	В	L	0.23	16.9	В	L	0.24	17.5	B
	TR	0.73		С		0.73		С	ĪR	0.52	23.3	<u>C</u>	ĪR	0.51	23.2	CIC	TR	0.90	=	D		0.90		D
SB		0.53		С	L	0.54		С	<u> </u>	0.62	25.0	C	<u>L</u>	0.62	<u>25.0</u>	C	L		45.0	D	L	0.78		D
		0.44		-		0.44			_	<u>0.33</u>				<u>0.34</u>				0.81				0.83		
Note	TR 0.44 21.6 C TR 0.44 21.7 C TR 0.33 20.2 C TR 0.34 20.2 C TR 0.81 38.6 D TR 0.83 40.7 D Int. 56.1 E Int. 70.7 E Int. 44.0 D Int. 58.4 E Int. 56.2 E Int. 74.0 E																							
notes	Int. 56.1 E Int. 70.7 E Int. 44.0 D Int. 58.4 E Int. 56.2 E Int. 74.0 E otes: L: Left Turn; T: Through; R: Right Turn; Deft.: Defacto Left Turn; Int.: Intersection V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service																							
1							ffic Im		ao poi	VOIN	, LO	J. <u>L</u> U	. 0. 01	JO: VIC										
		*	indicat	tes int	ersec	tions v	vhich v	ere u	nsign	alized	in the I	Νο Βι	uild co	nditio	n and s	ignali	zed ir	the E	Build co	nditio	n			

Table 17-<u>22</u> Comparison of 2015 No Build and Build Conditions Primary Study Area Unsignalized Intersection Level-of-Service Analysis

			A۱	/ Pea	ak Ho	ur					Midd	ay Pe	ak H	our					PΝ	/I Pea	ak Ho	ur		
	2	015 N	o Buil	d			Build		2	2015 1	No Buil	d			Build		2		lo Buil			2015	Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
			(spv)					LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			et @ W		133rd	Stre	et*																	
WB		0.20		В					Ī	0.16	10.3 45.2	<u>В</u> Е							10.5	В				
SB			101.8						LΤ	0.96	<u>45.2</u>	<u> </u>					Т	1.1 <u>5</u>	<u>95.9</u>	F				
		Stree	et @ W		132nc	d Stre	eet*				0.0		ı			ı			0.0		ı			
WB		-	9.7	<u>A</u> F						= = =	9.6 20.1						L	-	<u>9.9</u> 7 <u>0.4</u>	<u>A</u> F				
SB	LT	-	82.1 8.2						I₩	Ξ	20.1	노					T	-						
	Int.	-	8. <u>∠</u> 68.3	A F					L⊒	=	<u>8.3</u> 17.0	A					Int.	-	10. <u>0</u> 52.1	B F				
1/10		Ctro	et @ S		i. Di	*			ШЬ		17.0	<u> </u>					IIIL.		3 <u>Z. I</u>		l			
		0.18		A	III Pic	ace"			Li	0 22	10 1	В	1			ı		0.20	10.6	В	ı			
SD		0.10		Ê					L T	0.22 0.86	<u>10.1</u> 30.4	<u>B</u> D							103.2					
Two			<u> 32.3</u> ie @ V		13101	Stro	of*			0.00	<u> </u>	므						1.10	100.2	-				
		0.01		ΑΙ	10130	000			LI	0.02	7.5	Δ	ı			1	ΙT	0.02	7.8	Α	l			
		0.15		В						0.05	10.0	A						0.07		В				
		0.33		D					_	0.26	24.6	Ċ						0.86		F				
		0.02	16.4	Č						0.19	14.5	<u>A</u> <u>A</u> <u>C</u> B						0.05		Ċ				
			ie @ V	Vest	125th	Stre	et Sou	ıthbo			t Turn*													
			11. <u>6</u>								10.8	В					R	0.08	11. <u>6</u>	В	ĺ			
Twe	Ifth /	Avenu	ıe @ S	t. Cl	air Pla	ace*															•			
EB	Т	-	10.0	Α					I	Ξ	10.4	В					Τ	-	11. <u>2</u>	В				
NB		-	14.7	В					R	= =	14.5	В					R	-	13.0	В				
SB	L	-	9.1	Α						=	8.8	<u>B</u> <u>A</u> B					L	-	8.9	Α				
	Int.		13.6	В					Int.		13.4	<u>B</u>					Int.		12. <u>3</u>	В				
			re @ S		air Pla	ace*																		
		0.13		Α						0.13	7.7	<u>A</u> <u>D</u> D						0.18	7.9	Α				
SB		0.42		D					LI	0.37	31.9	D							3 <u>3.6</u>	D				
		0.3 <u>9</u>		D					I	0.29	<u> 28.5</u>	D					Т	0.25	3 <u>2.6</u>	D				
			re @ T			lace																		
WB		-	8.3	Α	L	-	8.3	Α		Ξ	8.1 7.1	A	<u> </u>	= = =	<u>8.1</u>	<u>A</u> <u>A</u> <u>A</u> A	L	-	8.3	Α	l L	-	8.3	Α
NB		-	7.4	Α	R	-	7.4	Α	R	Ξ		A	R	Ξ	<u>7.1</u>	A	R	-	7.3	Α	R	-	7.3	Α
SB		-	10.1	В	L	-	10.1	В	I≟	= = =	9.6	A	L	Ξ	9.6	≜	L	-	10.2	В	L	-	10.1	В
	T	-	8.9	Α	Т	-	9.0	Α		≣	8.8	<u>A</u> <u>A</u> <u>A</u> <u>A</u> A		≣	8.1 7.1 9.6 8.8 8.8	A	T	-	8.3	Α	T	-	8.3	Α
147	Int.		9.1	A	Int.		9.1	Α	Int.		8.8	<u>A</u>	int.		8.8	<u>A</u>	Int.		9.0	Α	Int.		8.9	Α
			reet @		st 129	th S	treet/S	t. Cla	1 .			_	ı					1 75	4040.0	_				
WB			1 <u>28.0</u>	F					L R	0.08	67.0	늗							1 <u>242.0</u>					
EB		0.83		E F					🛱	0.76	32.2	븓					ĸ	ı. <u>94</u>	4 <u>66.9</u>	F				
FR		0.02 0.6 <u>8</u>		C					L R	0.02 0.61	61.2 19.7						L	0.83	47.2	F E				
	ĸ	U.0 <u>8</u>	2 <u>2.1</u>	U					K	<u>U.D.I</u>	19.7	U					ĸ	U.03	4 <u>1.2</u>					

Notes:

L: Left Turn; T: Through; R: Right Turn; Int.: Intersection
V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service
+ Significant Adverse Traffic Impact
* indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition

^{**} The unsignalized intersection analysis procedure, which assumes random arrival, breaks down with high major or minor street volumes and reports exaggerated levels of stop delays. As such, its results are not necessarily indicative of actual conditions, where gaps created by adjacent signalized intersections often provide additional throughput capacity and result in lower stop

Table 17-<u>23</u> Comparison of 2015 No Build and Build Conditions Secondary Study Area Signalized Intersection Level-of-Service Analysis

			ΔN	/I Pe	ak Ho	ur	Seco	·IIU	41 y ,	Juu	Midd				ea In	CIB	CCLI	VII I			ak Ho		1 1114	J 515
	20)15 N	lo Buil				Build		2	015 N	lo Buil				Build		20	015 N	lo Buil		l l		Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
							(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			Vest 1				047	_	li TO	0 00	04.7	_	li TD	0 00	04.7	_	lı TD	0.07	04.0	С	li TD	0.07	04.0	0
				-			24.7 91.7				21.7 32.1	C			21.7 32.1	<u>C</u>				E			21.8 61.2	C E
'''			31.3				31.3				26.3	C			26.3	č			48.4	D	_		48.4	D
NB			12.9	В	LTR	0.46	12.9	В				В			13.2	В	LTR	0.62	15. <u>1</u>	В			15. <u>2</u>	В
SB		0.72	21.1			0.73	21.2			0.51	<u>13.4</u>	<u>B</u>		0.52	<u>13.5</u>	<u>B</u>		0. <u>59</u>	18.3			0.60	18. <u>3</u>	В
Dras	Int.	. @ 1	2 <u>6.9</u>	C	Int.	.4	27.2	С	Int.		<u>16.7</u>	<u>B</u>	Int.		<u>16.7</u>	<u>B</u>	Int.		24. <u>3</u>	С	Int.		24.6	С
		_	Vest 12 27.9				27.8	C	lıтр	U 33	26.4	C	lıтр	N 32	26.4	<u>C</u>	Ιтр	U 33	26.5	С	lıтр	U 33	26.5	С
			161.5				127.9				51.5	<u>C</u> D			51.5	D			70.9	Ē			61.2	Ē
	LTR					0.40		A		0.36		Ā		0.38		Ā	LTR			Ā		0.49		Ā
SB	LTR	0.59	7.9	Α	LTR	0.60	8.0	Α	LTR	0.44		Α	LTR	0.46		Α	LTR	0.48	6. <u>8</u>	Α	LTR	0.49	7.0	Α
لــــا	Int.		28.9	С	Int.		24. <u>4</u>	С	Int.		<u>14.2</u>	<u>B</u>	Int.		<u>14.1</u>	<u>B</u>	Int.		17.0	В	Int.		15.7	В
			enue					_		0.50	44.7	_		0.50	44.7	_		0.57	45.0	_	1.	0.57	45.0	_
EB			39.3 82.4	D			42.1 62.8	D E	뉴	0.50	41.7 48.5	<u>D</u>			41.7 48.5	<u>D</u>			45.9 54.4	D D			45.9 53.6	D D
WB			29.4				29.4				27.8	C			27.8	Ċ			27.3	C			27.3	C
				С				С			30.3	C			30.3	C			32.5	С	TR	0.32	32.5	C
	LTR			Α		0.61		Α			5.1	Α		0.58		Α			12.5	В			12.7	В
SB	LTR	0.57		A C		0.57		A B		0.34	3.2 12.3	<u>A</u> B		0.34	<u>3.2</u> 12.2	<u>A</u> B		0.35	3.3 17.2	A B		0.36		A B
Erod	Int.	Dou	20.1	_	Int.	I @ W	17.1 /est 12		Int.		12.3	ь	Int.		12.2	D	Int.		17.2	ь	Int.		17.2	ь
							84.2		ILTR	0.84	25.8	С	lltr	0.91	32.4	<u>C</u>	LTR	1.25	149.6	F	lltr	1.40	214.5	F+
			32.4	C				D	LTR	0.86	25.8 28.1 23.5	C			37.9	D	LTR	1.49	255.6	Ē			334.2	
NB			15.5					В	LI	0.38	23.5	С			23.5	D C	LTR	0. <u>72</u>	255.6 21.9	С	LTR	0. <u>73</u>	22.3	C
				В				В	<u>R</u>	0.63	36.6	D			<u>36.6</u>	D				_			40.0	_
SB			17.6 14.3	B				B B			25.3	<u>C</u>			25.3 27.5	<u>C</u>			18.5	B B			18. <u>6</u>	B B
	Int.	0.24	33.2	С	Int.	0.24	41.5	D	Int.	<u>0.40</u>	27.5 26.8		Int.	0.40	32.1	c	Int.	0.34	19.3 125.5	E	Int.	0.34	19.3 170.7	
Madi		4 ven	ue @ l			Stre			шк		<u> </u>		11154		<u>V=</u>				120.0		11116.		117.1	
							<u>134.4</u>	F+	LI	1.02	60.1	<u>E</u>	LI	<u>1.10</u>	85.4	E±	LT	1. <u>45</u>	234.8	F	LT	1. <u>61</u>	303.3	F+
			2 <u>8.9</u>	С			32.0				22.3	C			23.8	<u>C</u>			2 <u>4.7</u>				2 <u>6.8</u>	C
NB		0.59	17.4	В		0.59	17.4	В		0.48	<u>15.9</u>	<u>B</u>		0.48	<u>15.9</u>	<u>B</u>		0.74	20. <u>4</u>	C		0.74	20. <u>5</u>	Ç
Saac	Int.	· · ·	<u>50.4</u> ie @ E	D	Int.	Ctroo	62.6	<u>E</u>	Int.		<u>34.7</u>	<u>C</u>	Int.		<u>45.4</u>	D	Int.		99.2	<u>E</u>	Int.		128.0	F
EB			252.1				t 274.2	F±	lт	1 47	257.3	F	Ι	1 55	291.3	F±	Ιт	1 34	1 <u>96.3</u>	F	Ιт	1 43	233.2	F+
			3 <u>6.1</u>	D.			36.8				43.8	D			46.7	D			31.7	Ċ			33.5	C
WB	DefL	1.13	1 <u>28.5</u>	F	DefL	1.13	1 <u>28.5</u>	F			67.2	E			85.4	E+	LT	0. <u>91</u>	63.8	E	LT	0. <u>97</u>	76.1	E+
			217.2				2 <u>48.2</u>				40.0	_			40.0	_				_				_
			20. <u>6</u> 1 <u>49.2</u>				20. <u>6</u> 1 <u>78.7</u>				18.3 42.3	<u>В</u> D			18.3	<u>B</u> D			3 <u>0.5</u> 88.6	C E			3 <u>0.5</u> 112.4	C F+
SVV	Int.		1 <u>49.2</u>	F	Int.	1.29	134.0	F	Int.	0.70	103.1	Ë	Int.		<u>45.8</u> 118.0	Ë	Int.	1.00	81.8	Ē	Int.	1.13	97.3	F
First			@ East			eet		•	,				,											-
	L	1. <u>11</u>	102.9	F	L	1. <u>11</u>	102.9				38.4	D			38.4	D			82.4	E			82.4	E
اا			20.2	<u>C</u>			20.4				21.8	C			22.3	<u>C</u>			2 <u>5.4</u>	Ċ			26.5	C
NB				B B	L		17. <u>5</u> 1 <u>1.0</u>				10.0 12.4	<u>A</u> B				<u>В</u> В			9. <u>9</u>	A B	L		10.0	A B
			1 <u>1.0</u> 10.7	В			10.7	В			12.4 11.0	B			<u>12.4</u> 11.0	B			1 <u>4.0</u> 14.2	В			1 <u>4.0</u> 14.2	В
	Int.	J. <u>2</u> U	36.1	D	Int.	0.20	36.1	D	Int.	<u> </u>	18.2	B	Int.	<u> </u>	18.3	B	Int.	5.40	28.0	C	Int.	5.40	28.1	C
	dway		Vest 1	45th	Stree																			
EB	LTR	0.23	25.4	С	LTR	0.23					25.9		LTR			<u>C</u>							25.7	С
								D			<u>69.3</u>	Ē		1.00		Ē+			53.0	D			56.3	E
	LIR		20.6	C A		0.60	21.0	C A		0.69	23.0 12.4	달			23.9 12.7	<u>С</u> В				C B			29.2 14.8	C B
SB			9.6 11.2				9.7	В		0.36 0.54		<u>B</u> B		0.56	<u>12.7</u> 12.3	B			14.7	В			14.8	В
	Int.	2. 10	23.4	C	Int.	0.10	23.5	C	Int.	<u></u>	26.8	C	Int.	2.22	28.2	C	Int.	2	27.3	C	Int.	0.10	28.2	C
Notes	s:	L	.: Left T	urn;	T: Thr	ough;	R: Righ	t Tur	n; Det	L: De	facto Le	eft Tu	ırn; Int	.: Inte	rsection	1								

L: Left Turn; 1: Infough; K: Right Turn; Delacto Left Turn; Int.: Intersection
V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service
+ Significant Adverse Traffic Impact
* indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition

Table 17-<u>24</u>
Comparison of 2015 No Build and Build Conditions
Secondary Study Area Unsignalized Intersection Level-of-Service Analysis

						~			~~									V			~			J 222
			Α	M Pea	ak Ho	ur					Midd	ay Pe	ak F	our					P	M Pea	ak Ho	our		
	2	015 N	lo Bui	ld		2015	Build		2	2015	No Buil	p		2015	Build		20	015 N	lo Bui	ld		2015	Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
Int.	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
Firs	t Ave	enue	@ Eas	t 125	th St	reet S	South	ounc	l Rig	ht Tu	ırn													
SB	R	0.37	13.0	В	R	0.39	13. <u>3</u>	В	<u>R</u>	0.18	9.9	<u>A</u>	R	0.20	<u>10.1</u>	<u>B</u>	R	0.21	10.2	<u>B</u>	R	0.23	10.3	<u>B</u>
Note	s:		R: Rigl	nt Turr	1; V/C	: Volu	me to (Capac	ity; s	ov: Se	conds p	er Vel	nicle;	LOS:	Level	of Ser	vice							

Riverside Drive and West 135th Street

During the PM peak hour, the southbound *de facto* left-turn movement would deteriorate from LOS A to LOS F with delay increasing from 8.2 to 90.7 seconds and v/c ratio increasing from 0.21 to 0.90.

SECONDARY STUDY AREA

Frederick Douglass Boulevard and West 125th Street

During the AM peak hour, the eastbound approach would <u>deteriorate from LOS E to LOS F</u>, with delay increasing from <u>61.0</u> to <u>84.2</u> seconds and v/c ratio increasing from <u>1.01</u> to <u>1.08</u>.

During the PM peak hour, the eastbound approach would <u>continue to operate at LOS F</u>, with delay increasing from $\underline{149.6}$ to $\underline{214.5}$ seconds and v/c ratio decreasing from $\underline{1.25}$ to $\underline{1.40}$. The westbound approach would <u>also continue to operate at LOS F</u>, with delay increasing from $\underline{255.6}$ to $\underline{334.2}$ seconds and v/c ratio increasing from $\underline{1.49}$ to $\underline{1.67}$.

Madison Avenue and East 125th Street

During the AM peak hour, the eastbound approach would <u>continue to operate at LOS F</u>, with delay increasing from $\underline{102.5}$ to $\underline{134.4}$ seconds and v/c ratio increasing from $\underline{1.14}$ to $\underline{1.22}$.

During the midday peak hour, the eastbound approach would deteriorate from LOS E to LOS F, with delay increasing from 60.1 to 85.4 seconds and v/c ratio increasing from 1.02 to 1.10.

During the PM peak hour, the eastbound approach would continue to operate at LOS F, with delay increasing from 234.8 to 303.3 seconds and v/c ratio increasing from 1.45 to 1.61.

Second Avenue and East 125th Street

During the AM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from $\underline{252.1}$ to $\underline{274.2}$ seconds and v/c ratio increasing from $\underline{1.46}$ to $\underline{1.51}$. The westbound through movement would continue to operate at LOS F, with delay decreasing from $\underline{217.2}$ to $\underline{248.2}$ seconds and v/c ratio increasing from $\underline{1.35}$ to $\underline{1.43}$. The southwestbound approach would continue to operate at LOS F, with delay increasing from $\underline{149.2}$ to $\underline{178.7}$ seconds and v/c ratio increasing from $\underline{1.22}$ to $\underline{1.29}$.

During the midday peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 257.3 to 291.3 seconds and v/c ratio increasing from 1.47 to 1.55. The westbound approach would deteriorate from LOS E to LOS F, with delay increasing from 67.2 to 85.4 seconds and v/c ratio increasing from 0.91 to 0.99.

During the PM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from $\underline{196.3}$ to $\underline{233.2}$ seconds and v/c ratio increasing from $\underline{1.34}$ to $\underline{1.43}$.

The westbound approach would continue to operate at LOS E, with delay increasing from 63.8 to 76.1 seconds and v/c ratio increasing from 0.91 to 0.97. The southwestbound approach would also continue to operate at LOS F, with delay increasing from 88.6 to 112.4 seconds and v/c ratio increasing from 1.06 to 1.13.

Broadway and East 145th Street

During the midday peak hour, the westbound approach would continue to operate at LOS E, with delay increasing from 69.3 to 75.2 seconds and v/c ratio increasing from 0.97 to 1.00.

SIGNIFICANT IMPACTS

Within the primary study area, the Proposed Actions would result in significant adverse impacts at the follow intersections:

AM Peak Hour

• Amsterdam Avenue and West 125th Street

Midday Peak Hour

Amsterdam Avenue and West 125th Street

PM Peak Hour

- Riverside Drive and West 135th Street
- Amsterdam Avenue and West 125th Street

Within the secondary study area, the Proposed Actions would result in significant adverse impacts on the following intersections:

AM Peak Hour

- Frederick Douglass Boulevard and West 125th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

Midday Peak Hour

- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street
- Broadway and West 145th Street

PM Peak Hour

- Frederick Douglass Boulevard and West 125th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

It should be noted that the projected traffic impacts were identified primarily at intersections along 125th Street. These intersections were identified to already operate at congested levels during peak hours in the future without the Proposed Actions. As mentioned earlier, NYCDCP and NYCDOT are currently conducting an areawide transportation study in the effort to improve vehicular and pedestrian travel in the Harlem section of Manhattan. While this study, when completed, may

recommend corridor-wide strategies in easing congestion along 125th Street, potential measures to mitigate the significant adverse impacts identified above are presented in Chapter 23.

BUILD WITHOUT PROJECT-RELATED ROADWAY IMPROVEMENTS

An analysis was performed for a Build scenario that does not include the project-related improvements listed previously. This analysis compared traffic impacts for 2015 Build conditions assuming the operation of the roadway network under the existing geometric configuration to traffic impacts for 2015 Build conditions assuming the roadway network incorporating improvements recommended as part of the Proposed Actions. The analysis demonstrates how the recommended improvements, including the conversion of two-way streets to one-way couples, as described above, would substantially improve overall network operations and circulation, and not yield significant adverse impacts at the Project Area intersections. Furthermore, the comparison analysis confirmed that these improvements are both logical and practical as part of the comprehensive, community-based planning approach of the design process. In the alternative analysis, West 133rd Street between Broadway and Twelfth Avenue, West 132nd Street between Broadway and Marginal Street, West 131st Street between Broadway and Twelfth Avenue, and West 125th Street between Twelfth Avenue and Marginal Street would continue to operate as two-way, east-west streets. All project-generated traffic was routed to conform to this existing street network. In addition, traffic generated by the Harlem Piers project was also routed to conform to the existing two-way street network. Significantly impacted intersections within the Project Area are indicated below in Table 17-25. The detailed findings of this alternative analysis are presented in Appendix M. Since traffic volumes and operating characteristics at the analysis locations outside of the Project Area would be the same with or without the improvements, conditions at these locations have been previously discussed and are not summarized below or addressed in Appendix M.

Table 17-<u>25</u>
Significantly Impacted Intersections within the Project Area in 2015 Build Condition without Improvements

		Duna C	71141111011	***************************************	mpro ,	
	Build wit	hout Impro	vements	Build w	ith Improv	ements
Project Area Intersections	AM Peak	MD Peak	PM Peak	AM Peak	MD Peak	PM Peak
Signalized Intersections						
Marginal Street @ West 125th Street						
Twelfth Avenue @ West 133rd Street						
Twelfth Avenue @ West 132nd Street						
Twelfth Avenue @ West 125th Street						
Broadway NB @ West 133rd Street						
Broadway SB @ West 133rd Street						
Broadway NB @West 132nd Street						
Broadway SB @ West 132nd Street						
Broadway @ West 131st Street						
Broadway @ West 130th Street						
Broadway @ West 129th Street						
Broadway @ West 125th Street						
Unsignalized Intersections						
Marginal Street @ West 133rd Street						
Marginal Street @ West 132nd Street						
Marginal Street @ St. Clair Place						
Twelfth Avenue @ West 131st Street						
Twelfth Avenue @ St. Clair Place						
Riverside Drive @ St. Clair Place						
West 125th St. @ West 129th St./St. Clair Pl.						
Notes: Significantly Impacted In	tersections					

PARKING SUPPLY AND UTILIZATION

COLUMBIA UNIVERSITY PARKING

Four interim parking facilities, comprising a total of 652 spaces, would be provided to accommodate demand generated by Columbia University uses, as follows.

- 635 West 131st Street (122 spaces)
- 603-09 West 131st Street (75 spaces)
- 553-09 West 133rd Street (205 spaces)
- U-Haul Site (250 spaces)

A parking accumulation analysis was performed to ensure that the off-street parking supply provided as part of the Proposed Actions would be adequate to meet the projected demand. This analysis shows that the peak weekday Columbia University parking demand of 494 vehicles could be adequately accommodated by the above supply.

ON-STREET PARKING

Table 17-26 shows the expected on-street parking utilization in the 2015 Build condition. The 2015 Build analysis included additional demand for on-street parking from non-Columbia University project-generated vehicles (Subdistrict B and "Other Area"). Roadway modifications that were planned as part of the Proposed Actions would create additional on-street parking spaces on certain blocks and remove spaces from others, resulting in a net loss of 57 on-street parking spaces in the 2015 Build condition. The project-related modifications to on-street parking include:

- Addition of a parking lane on the south side of West 133rd Street between Broadway and Twelfth Avenue:
- Addition of a parking lane on the south side of West 132nd Street between Broadway and Twelfth Avenue;
- Addition of a parking lane on the north side of West 131st Street between Broadway and Twelfth Avenue;
- Removal of the parking lane on the north side of West 130th Street between Broadway and Twelfth Avenue:
- Removal of parking lanes on the north and south sides of West 129th Street between Broadway and West 125th Street;
- Removal of a parking lane on the west side of Broadway between West 129th Street and West 125th Street; and
- Removal of a parking lane on the north side of West 125th Street between Old Broadway and Broadway.

Table 17-<u>26</u> 2015 Build On-Street Parking Utilization Summary

2015 Duliu Oli-Su	cet I al Killg	Cunzation	Summar y
2015 Build	AM	MD	PM
1/4-Mile Radi	us		
Capacity	1847	1847	1847
Spaces Removed due to Geometric Modifications	57	57	57
Effective 2015 Build Capacity	1790	1790	1790
2015 No Build Demand	1471	1524	1480
Subdistrict B and "Other Area" Demand	136	201	142
2015 Build Demand	1607	1725	1622
Remaining Spaces	183	65	168
Utilization	90%	96%	91%
% No Build Availability Utilized in Build Condition	51%	80%	54%
½-Mile Radi	us		
Capacity	4783	4783	4783
Spaces Removed due to Geometric Modifications	57	57	57
Effective 2015 Build Capacity	4726	4726	4726
2015 No Build Demand	3613	3903	3982
Subdistrict B and "Other Area" Demand	136	201	142
2015 Build Demand	3749	4104	4124
Remaining Spaces	977	622	602
Utilization	79%	87%	87%
% No Build Availability Utilized in Build Condition	16%	29%	25%
Note: See Appendix H for detailed parking accumulat	ion analysis.	_	

Within ¼ mile of the Project Area, on-street parking would be 90, 96, and 91 percent utilized during the morning, midday, and evening periods, respectively, in the 2015 Build condition (compared with 80, 83, and 80 percent utilization for the same time periods under the 2015 No Build condition). Spaces available in the 2015 No Build condition would be 51, 80, and 54 percent utilized in the 2015 Build condition during the morning, midday, and evening periods, respectively. Since these utilization levels exceed 50 percent, a larger ½-mile on-street area was evaluated. Overall, within ½ mile, on-street parking would be 79, 87, and 87 percent utilized during the morning, midday, and evening periods, respectively, in the 2015 Build condition (compared with 76, 82, and 83 percent utilization for the same time periods under the 2015 No Build condition). Spaces available in the 2015 No Build condition would be 16, 29, and 25 percent utilized in the 2015 Build condition during the morning, midday, and evening periods, respectively. Since less than half of the on-street parking capacity available in the 2015 No Build condition would be utilized in the Build condition, no significant adverse impacts to on-street parking conditions would occur.

OFF-STREET PARKING

A survey was performed at three off-street public parking facilities within the Project Area in June 2005. Several different groups of users were identified parking at these facilities, including local residents, workers at local businesses, patrons of local businesses, and park-and-commute motorists. Approximately 10 to 15 percent of the parking demand at the surveyed facilities was generated by existing businesses or residences that would be displaced by the Proposed Actions. Another 15 to 20 percent was generated by commuters parking in Manhattanville and then riding the subway to work, and by motorists who neither work nor live in or near Manhattanville, but who choose to park there due to the availability of relatively inexpensive parking. Therefore, it is

expected that the Proposed Actions would not only result in the displacement of several area parking facilities, but would also result in a reduction of parking demand that would otherwise exist in the future without the Proposed Actions. For a conservative parking analysis, a credit of 16 percent was applied to parking facilities within the primary study area to account for the displacement of existing generators in the Project Area or the relocation of existing parking demand to other areas. An additional deduction of 143, 160, and 53 spaces during the morning, midday, and evening periods, respectively, in area public parking demand was taken to account for the elimination of parking demand generated by certain Columbia University No Build projects, which would not be constructed or would be reduced in size under the Build condition.

Five parking facilities, as shown in Table 17-27, would be displaced by the proposed development in the 2015 Build condition. The remaining two public parking facilities located within ¼ mile of the Project Area would be 192, 194, and 196 percent utilized, with expected shortfalls of 601, 612, and 623 spaces during the morning, midday, and evening periods, respectively. The four public parking facilities that would remain within ½ mile would be 155, 161, and 148 percent utilized, with expected shortfalls of 566, 629, and 494 spaces during the morning, midday, and evening periods, respectively. Overall, within one mile, the remaining 21 public parking facilities would be 110, 111, and 87 percent utilized during the morning, midday, and evening periods, respectively. The resulting parking shortfalls would be 299 spaces during the morning and 328 spaces during the midday.

Because there would be a surplus of 64 spaces from the four Columbia University interim parking facilities (after accommodating the projected construction worker parking demand, 94 vehicles, in 2015), some of the parking demand from Columbia University's other area uses could be accommodated by this surplus. Hence, while there would still be an off-street parking shortfall resulting from the Proposed Actions, this shortfall would be limited to 235 spaces in the morning period and 264 spaces in the midday period. It is possible that, as future parking demand increases, market forces could result in the construction of new pubic parking facilities in the area. However, since there is no certainty in these facilities being constructed, in accordance with guidance of the CEQR Technical Manual, the projected parking shortfall, while relative small compared with the total supply in the area, would constitute a significant adverse parking impact. Potential measures to mitigate this impact are discussed in Chapter 23, "Mitigation."

F. 2030 FUTURE WITHOUT THE PROPOSED ACTIONS

The analysis of traffic conditions in the 2030 analysis year without the Proposed Actions, or the 2030 No Build condition, considers increases in background traffic volumes, and as with the 2015 No Build analysis, incorporates traffic from other planned developments in the area, as well as roadway or signal timing changes scheduled to be implemented. These changes include the mitigation measures proposed as part of the 125th Street Corridor Rezoning and Related Actions DEIS, some of which would result in prohibiting left turns from 125th Street onto Amsterdam Avenue, Frederick Douglass Boulevard, and Madison Avenue. As discussed in Section D, "2015 Future Without the Proposed Actions," DCP and NYCDOT are currently undertaking a "river-to-river" study to develop potential strategies for improving vehicular and pedestrian travel. The outcomes of this study, however, were not accounted for in the future No Build analysis.

Future No Build peak hour traffic volumes were estimated by first applying a background growth of 0.5 percent per year for 24 years (as recommended by the *CEQR Technical Manual*). As with the 2015 No Build analysis, trips generated by No Build Projects were incorporated into the 2030 No Build analysis.

Table 17-<u>27</u> 2015 Build Off-Street Parking Utilization Summary

2015 Build Off-St	reet Parking	g Utilization	Summary
2015 Build	AM	MD	PM
1/4-Mile Radiu	s		
2015 No Build Capacity	1330	1330	1330
Displaced Garages	679	679	679
MTP 3300 Broadway Corp.	200	200	200
West 129th Street LLC	134	134	134
Uni Facility Corp.	100	100	100
Y&H Enterprises Inc.	175	175	175
Columbia Waterfront LLC	70	70	70
Effective 2015 Build Capacity	651	651	651
2015 No Build Demand	1497	1525	1429
Credit for Removal and Reduction of Projects in Build	143	160	53
New Columbia Academic Building	18	45	25
New Columbia Office Building (No. 23)	77	70	17
New Columbia Office Building (No. 24)	48	45	11
16% Credit for Displaced Parking Demand*	102	102	102
Total 2015 Build Demand	1252	1263	1274
Remaining Spaces	-601	-612	-623
Utilization	192%	194%	196%
Added Supply from Columbia On-Site Parking	64	64	64
Remaining Spaces	-537	-548	-559
Effective Utilization	182%	184%	186%
½-Mile Radiu			
2015 No Build Capacity	1710	1710	1710
Displaced Garages	679	679	679
Effective 2015 Build Capacity	1031	1031	1031
2015 No Build Demand	1842	1922	1680
Credit for Removal and Reduction of Projects in Build	143	160	53
16% Credit for Displaced Parking Demand*	102	102	102
Total 2015 Build Demand	1597	1660	1525
Remaining Spaces	-566	-629	-494
Utilization	155%	161%	148%
Added Supply from Columbia On-Site Parking	64	64	64
Remaining Spaces	-502	-565	-430
Effective Utilization	149%	155%	142%
1-Mile Radius		0570	0570
2015 No Build Capacity	3576	3576	3576
Displaced Garages	679	679	679
Effective 2015 Build Capacity	2897	2897	2897
2015 No Build Demand	3441	3487	2683
Credit for Removal and Reduction of Projects in Build	143	160	53
16% Credit for Displaced Parking Demand*	102	102	102
Total 2015 Build Demand	3196	3225	2528
Remaining Spaces	-299	-328	369
Utilization	110%	111%	87%
Added Supply from Columbia On-Site Parking	64	64	64
Remaining Spaces	-235	-264	433
Effective Utilization	108%	109%	85%

Note: Credit accounts for 16% of the existing demand at the five displaced parking garages grown to 2015 levels. See Appendix H for detailed parking accumulation analysis.

TRAFFIC VOLUMES AND INTERSECTION CAPACITY ANALYSES

Peak hour traffic volumes for the 2030 No Build condition are shown in Figures 17-26 through 17-31. Within the primary study area, increases in two-way peak hour traffic over existing volumes are expected to range from 270 vph to 400 vph along Broadway, from 250 vph to 300 vph along Amsterdam Avenue, and from 30 vph to 260 vph along Twelfth Avenue. Two-way peak hour volumes along West 125th Street would increase by 300 vph to 480 vph during the two analysis periods. As with the 2015 No Build analysis, secondary study area traffic volumes at intersections along East 125th Street are projected to increase substantially as a result of the East River Plaza and East 125th Street developments in East Harlem. Increases in PM peak hour traffic over existing volumes at the intersections of First and Second Avenues and 125th Street are projected to exceed 900 vph and 1.300 vph, respectively. Two-way volumes along East 125th Street between Second and Fifth Avenues are projected to increase by 650 vph.

As with the 2015 No Build analysis, the 2030 No Build analysis also included the roadway modifications planned in connection with the Fairway parking lot relocation and the West Harlem Waterfront park development. Results of the LOS analysis are presented in Tables 17-28 and 17-29 for the primary study area, and in Tables 17-30 and 17-31 for the secondary study area. As with existing, locations with notable service constraints, those operating at mid-LOS D or worse, are described below.

PRIMARY STUDY AREA

Riverside Drive and West 135th Street

During the PM peak hour, the northbound through-right movement would deteriorate from LOS C to LOS F, with delay increasing from 23.3 to 83.7 seconds and v/c ratio increasing from 0.98 to 1.15.

Twelfth Avenue and West 133rd Street

During the AM peak hour, the northbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 39.9 to 94.3 seconds and v/c ratio increasing from 0.91 to 1.11.

During the PM peak hour, the northbound left-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 26.3 to 60.1 seconds and v/c ratio increasing from 0.79 to 1.01. At the same time, the westbound approach would deteriorate from LOS D to LOS F, with delay increasing from 48.5 to 206.1 seconds and v/c ratio increasing from 0.89 to 1.36.

Twelfth Avenue and West 125th Street

During the AM peak hour, the westbound right-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 34.1 to 68.0 seconds and a v/c ratio increasing from 0.86 to 1.03.

<u>During the midday peak hour, the westbound right-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 32.5 to 61.4 seconds and a v/c ratio increasing from 0.85 to 1.01.</u>

During the PM peak hour, the same movement would deteriorate from LOS E to LOS F, with delay increasing from 55.8 to 135.7 seconds and a v/c ratio increasing from 1.01 to 1.23.

Broadway Northbound and West 135th Street

During the PM peak hour, the eastbound *de facto* left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 51.7 to 108.8 seconds and v/c ratio increasing from 0.77 to 1.04.

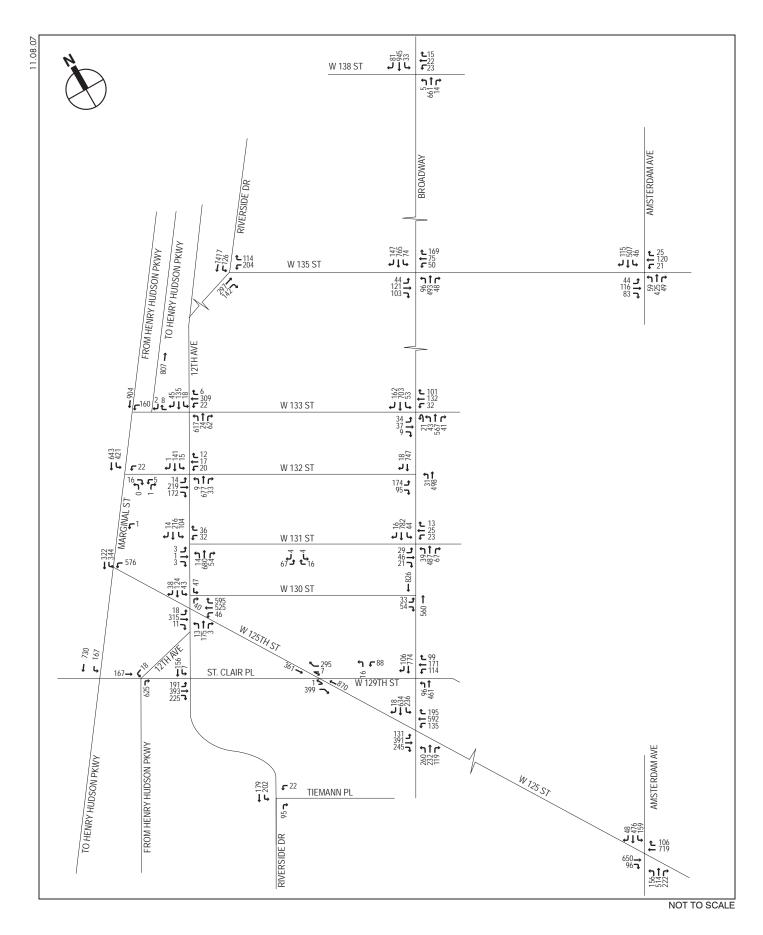


Figure 17-26
Primary Study Area
No Build 2030 Morning Peak Hour

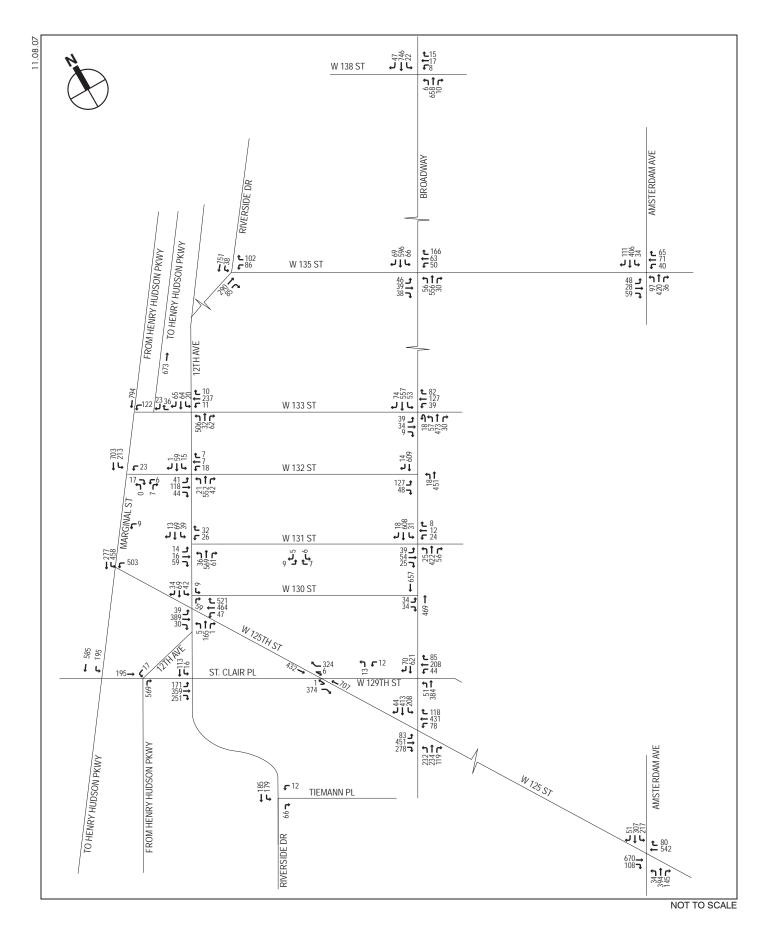


Figure 17-27
Primary Study Area
No Build 2030 Midday Peak Hour

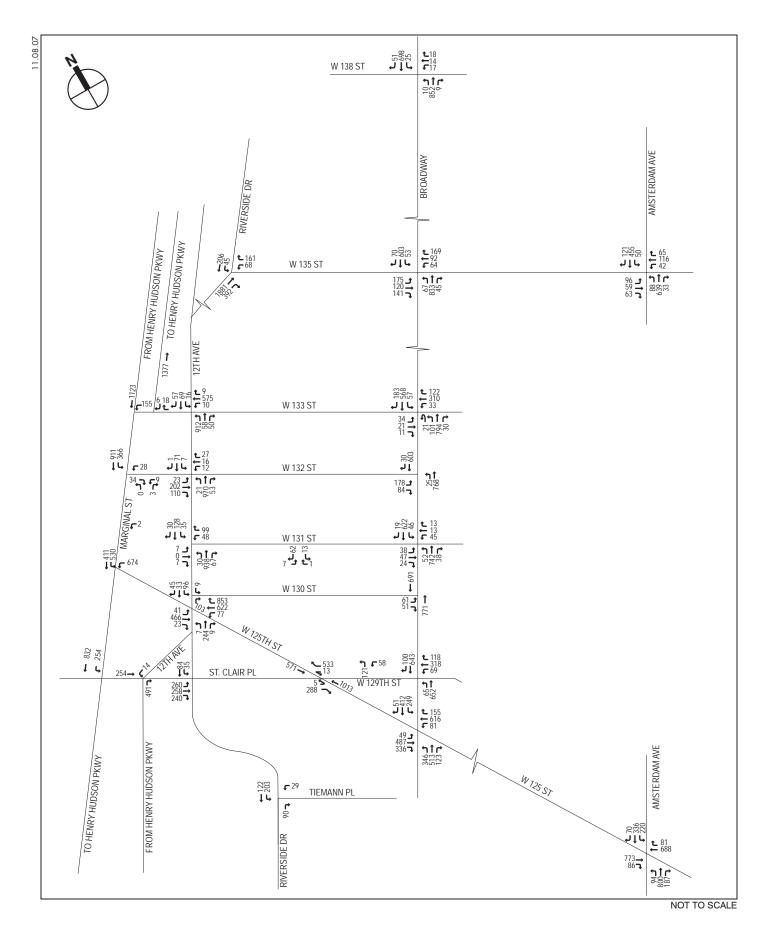


Figure 17-28

Primary Study Area

No Build 2030 Evening Peak Hour

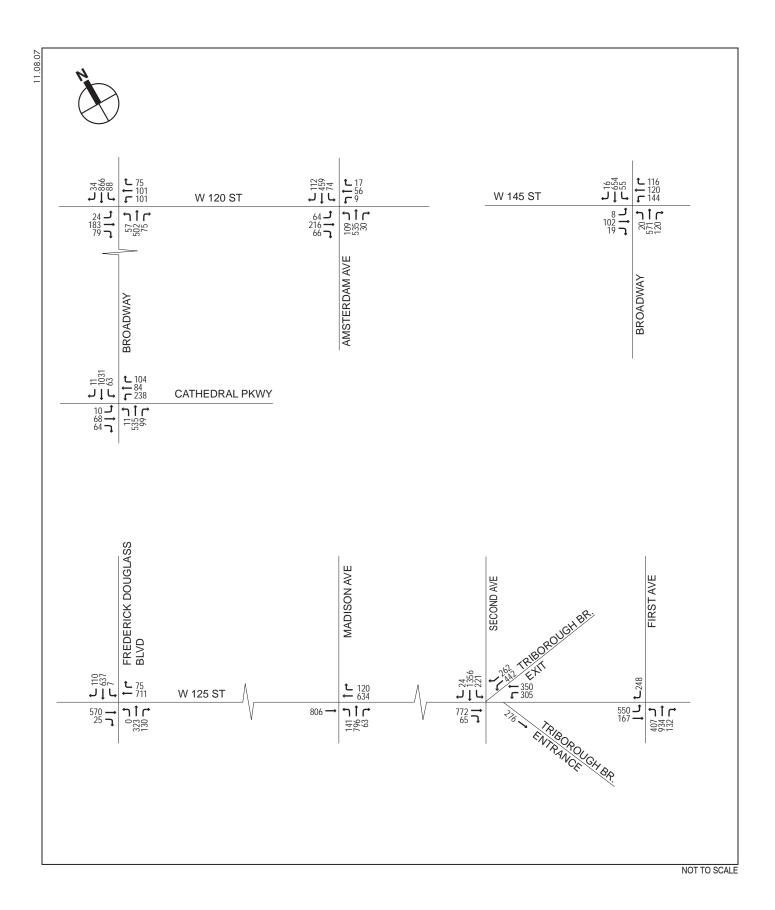
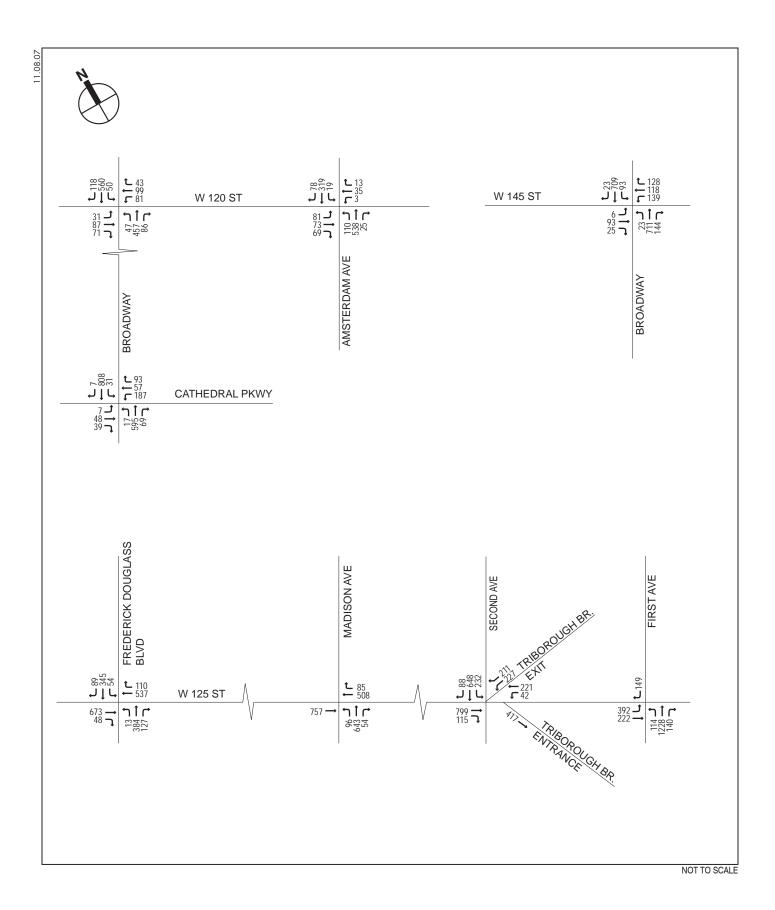


Figure 17-29



No Build 2030 Midday Peak Hour

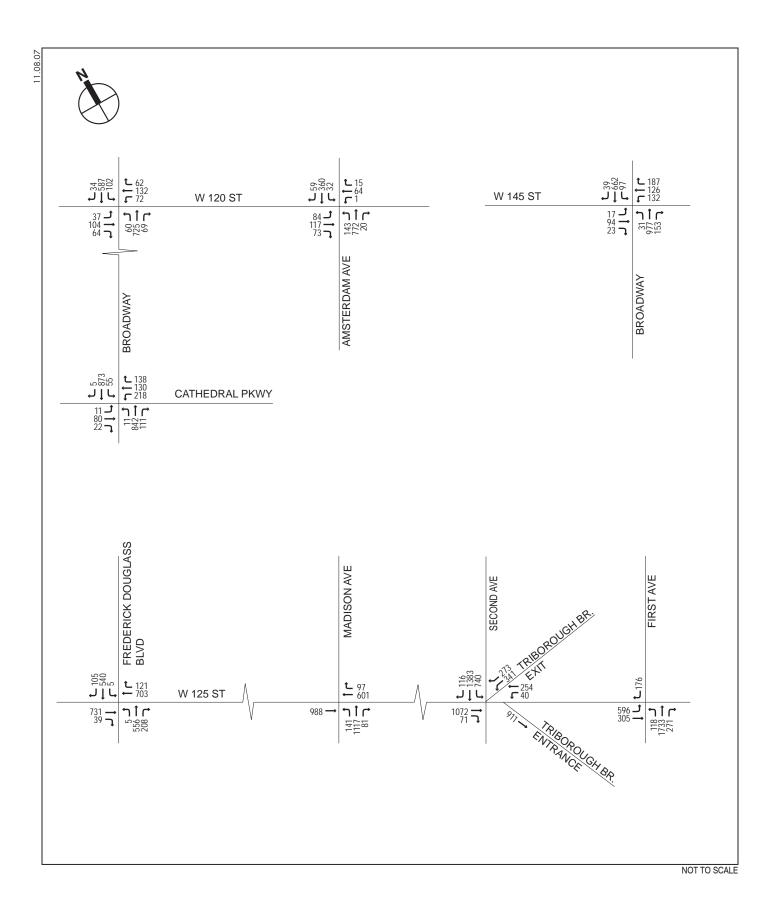


Table 17-<u>28</u>
2030 No Build Condition
Primary Study Area Signalized Intersection Level-of-Service Analysis

				4.5			Pr	ıma	iry :	Stud					ed In	ters	ecti	on I	Level				Ana	lysis
		006 5			ak Ho		- D.::I		-	006 5			eak l		- D:I	٠	_	006 5			ak Ho		la Duil	14
	Ln	-	xisting Delay	g	Ln	-	o Buil Delay	u	Ln	_	xistin Delay	_	Ln		o Buil Delay		Ln		xistin Delay	_	Ln	USU N	lo Bui Delay	_
Int.			(spv)	LOS				Los														V/C		
Marg	jinal	Stree	t @ W	est 1	25th	Stree	t																	
WB SB	L	0.40 0.23 0.25	12.1	C B B			<u>26.5</u> 14.7		L	0.36		<u>C</u> <u>B</u> B	-		<u>25.5</u> <u>15.2</u>	<u>С</u> В	L	0.41	24.5 14.3 12.8	C B B			<u>29.0</u> 18.1	СВ
	Int.		17.5	В	Int.		20.1	C	Int.		16.8	В	Int.		19.4	В	Int.		17.3	В	Int.		22.7	С
			e @ W						1 .			_				_								
WB		0.32		C		0.62		D C	F	0.21 0.34	<u>26.1</u>	<u>C</u> C		0.25 0.40		<u>C</u>			25.6 36.4	C D			26.3 40.9	C D
NB		0.40		Ä		0.47		A		0.19		A		0.40		A			23.3	С			83.7	F
			11.4			0.94		В		0.10		Ā		0.12		Ā		0.19		Ă		0.14		A
	Int.		12.6	В	Int.		19.7	В	Int.		12.4	В	Int.		12.9	В	Int.		22.7	С	Int.		72.6	E
			e @ W					_	l			_	l. =5			_	l. =5			_	l			_
		0.56 0.91		C D		0.80		D F		0.40 0.70	25.0 21.8	<u>C</u>			31.6 32.8	<u>C</u>			48.5 26.3	D C		1.36	206.1	F E
IND			10.8				11.2					B			11.0				17.8	В		0.76		C
SB			11.7				10.5		_	0.20		В		0.12	_				10.8	В			10.1	B
	Int.		28.6	С	Int.		55.4	Ε	Int.		18.9	B	Int.		25.9	С	Int.		28.9	С	Int.		96.8	F
			e @ W					_	lı TD	0.04	22.2	_	lı TD	0.40	25.0	_	lı TD	0.44	25.0	_	lı TD	0.00	20.0	_
			23.2 20.0	В		0.81		D C		0.24		<u>С</u> В		0.43		_			25.2 20.1	C			29.9 20.7	C C
			13.3				14.4			0.03		B			13.3				15.6	В		0.74		В
			11.6			0.14				0.13		B		0.08								0.06		A
	Int.		14.9	В	Int.		21.7	С	Int.		14.0	В	Int.		15.9	В	Int.		17.3	В	Int.		21.1	С
			e @ W					_	lı TD	0.00	40.7	_	lı TD	0.07	44.5	_	lı TD	0.04	40.0	_	lı TD	0.45	45.0	_
MB			12.7 12.6			0.27	13.5 13.3			0.29		<u>В</u> В	FIR		14.5 13.6	<u>В</u> В			13.9 13.1	B B		-	15.8 16.0	B B
VVD			18.9				26.1		_	0.12	-	B	_	0.66		C			18.8	В		-	35.1	D
			34.1			1.03		Ĕ		0.85		C	Ē	1.01	61.4	Ē			55.8	Ē			135.7	F
NB	LTR	0.23				0.34		С	LTR	0.22		В	LTR	0.31	20.4	С			21.1	С	LTR	0.45	22.7	С
SB		0.34		С		0.42		C		0.21		В		0.25	<u>19.9</u>	<u>B</u>		0.28	20.5	С		0.39	22.8	C
Dras	Int.	@ 1/	23.2 Vest 13	C	Int.	.4	<u>35.4</u>	D	<u>Int.</u>		<u>21.6</u>	С	<u>Int.</u>		<u>31.9</u>	C	Int.		32.0	С	Int.		<u>63.7</u>	<u>E</u>
			24.1				24.5	С	lı TR	0 10	23.3	С	lı TR	0 12	23.5	C	lı TR	0 13	23.7	С	lı TR	0.16	24 1	С
		0.31		Ä		0.37		Ä		0.30		Ā		0.36		Ā		0.37		Ä		0.44		Ä
		0.02		Α		0.03		Α		0.02		Α		0.02		Α		0.02		Α		0.03	4.8	Α
SB		0.52				0.63		Α	_	0.40		A		0.48		À		0.38		Α		0.45		Α
Proc	Int.	ı, Nor	7.6 thbour	A @	Int.	4 12E	8.5	A	<u>Int.</u>		<u>6.8</u>	<u>A</u>	<u>Int.</u>		<u>7.3</u>	<u>A</u>	Int.		6.9	Α	Int.		7.4	Α
		0.31		C	1	0.39		C	Ιιτ	0.23	24.3	C	Ιιτ	0 29	25.1	С	Defl	0 77	51.7	D	Defl	1 04	108.8	F
				_			-	-				_	l ==			_			27.1	C			28.3	Ċ
		0.46		C		0.53		С		0.45		C		0.52		C			28.4	C			30.6	С
NB		0.39		Α		0.54		Α	_	0.42	<u>7.0</u>	A		0.51		_		0.59		Α		0.70	10.5	В
Bros	Int.	v So.	16.4 thbou	B nd @	Int.	t 125	17.1	B	Int.		<u>15.1</u>	<u>B</u>	<u>Int.</u>		<u>15.9</u>	<u>B</u>	Int.		19.1	В	Int.		27.2	С
		y 30u 0.38		C		0.45		C	IR	0.15	23.5	С	IR	0.20	24.1	С	TR	0.48	27.9	С	TR	0.71	34.2	С
			24.0			0.36		Č		0.23	24.1	<u>C</u>		0.29		<u>C</u>			24.8	Č			27.6	č
SB		0.08		Α		0.11		Α			5.1	Α	_	0.11		Α		0.07		Α				Α
		0.62		A		0.75		В		0.48		<u>A</u> B		0.56		A		0.47		A		0.55		A
D::a	Int.	N	14.4	B nd @	Int.	4 4 2 2	16.9	В	Int.		<u>11.8</u>	ㅂ	<u>Int.</u>		<u>12.9</u>	ㅂ	Int.		15.8	В	Int.		19.5	В
			<i>thboui</i> 17.2				17.6		LT	0.22	17.5	В	LT	0.27	18 2	В	LT	0.18	17.1	В	LT	0.26	18.3	В
		0.34		C		0.53		C		0.31		č		0.49		Č			29.0	C			50.9	Ď
	LT	0.39	11.7	В	LT	0.59	14.5	В	LI	0.35	11.3	<u>C</u> B	LT	0.47	12.5	В	LT	0.66	15.6	В	LT	0.83	21.1	С
		0.04		Α		0.12		Α		0.05		<u>A</u>		0.09		<u>A</u>		0.05		Α		0.06		Α
D:::= :	Int.		14.6	B	Int.	4 4 9 9	17.3	B	<u>Int.</u>		<u>14.4</u>	<u>B</u>	<u>Int.</u>		<u>16.3</u>	<u>B</u>	Int.		19.3	В	Int.		29.6	С
		y Sou 0.12	thbou 17 9	nd @ B		0.13		eet B	TR	0.12	18 N	<u>B</u>	l TR	0.14	18 2	В	l TR	n na	17.7	В	l TR	0 10	17.8	В
		0.20		В		0.13		C	置	0.12	17.3	B			20.5	C			21.0	C			30.4	C
			12.1	В		0.56		В		0.33		В			11.6				11.2	В			12.6	В
	Int.		13.2	В	Int.		15.0	В	Int.		12.6	<u>B</u>	Int.		<u>14.2</u>	<u>B</u>	Int.		14.6	В	Int.		19.0	В
Notes	s:		: Left T													า	_			-	_			_
Щ_		\	//C: Vol	ume	io Cap	pacity;	spv: S	econ	ıs pei	venic	ie; LO	o: Le	vei of	Servic	е									

Table 17-<u>28</u> (Continued) 2030 No Build Condition

Primary Study Area Signalized Intersection Level-of-Service Analysis

			ΑN	/l Pea	ak Ho	ur		*****	11 5 5) tua	Mido		_		.u III	tel s	,	OH I			ak Ho		Anai	ysis
	2	006 E	xistin				o Buil	d	2	006 E	xistin			_	lo Buil	d	2		xistin				o Buil	d
Int	Ln		Delay		Ln		Delay (spv)	201	<u>Ln</u> Grn		Delay		Ln		Delay		Ln		Delay	108	Ln		Delay	ا م
							nd Stre		<u> vib</u>	110	(SAA)		<u> VIP</u>	110	(Sp.)		O.P	170	(SPV)		Cip	1,0	(301)	
			16.4	В		0.41		С				<u>B</u>			<u>17.8</u>				17.5			0.33		В
NB	LT Int.	0.32	10.9 11.5	B B	LT Int.	0.43	12.0 14.2	B B	LT Int.	0.29	10.6 11.6	<u>В</u> В	Int.	0.37	11.3 12.7	B B	LT Int.	0.49	12.6 13.4	B B	LT Int.	0.61	14.3 15.1	B B
Broa		/ Sou				st 132	nd Str		11114.		11.0	브	11114		14.1	브	IIIL.		10.7		IIIC.		10.1	
EB	TR	0.22	19.5					С			19.0	<u>B</u>				C	TR	0.34	20.9	С	TR	0.49	23.5	С
				В				В			<u>15.4</u>	B				B				В		0.04		В
28	Int.		12.0 13.1	В	Int.	0.55	13.3 17.4	B B	Int.	0.35	11.1 12.5	<u>B</u> B	Int.	0.44	12.0 14.1	<u>B</u> B	Int.	0.36	11.2 13.8	B B	Int.	0.46	12.2 15.5	
Broa			Vest 1			et																		_
				С		0.21		С	_		20.9				22.3	<u>C</u>			20.6			0.22		С
			20.5 19.7	C B		0.13	20.8 19.8	C B			20.4 19.6	<u>С</u> В			20.7 19.6	<u>C</u> B			21.1 19.7	C B		0.17 0.04		C B
		0.27		A		0.40		A		0.23				0.32				0.39				0.51		B
		0.39		Α		0.54	10.3	В	_	0.30				0.40				0.33				0.45		A
Bros	Int.	/ @ I/	9.5 Vest 1 :	A 30th	Int.	of.	10.9	В	<u>Int.</u>		9.3	<u>A</u>	<u>Int.</u>		<u>10.3</u>	<u>B</u>	Int.		9.7	Α	Int.		11.0	В
			24.4	C		0.27	26.0	С	LR	0.13	23.7	<u>C</u>	<u>LR</u>	0.20	24.6	<u>C</u>	LR	0.24	25.4	С	LR	0.34	27.0	С
		0.21		Α		0.28		Α		0.18		A		0.23		A		0.28		Α		0.35		Α
SB	L I Int.	0.31	6.0 6.7	A A	L I Int.	0.40	6.5 7.4	A A	Int.	0.25	<u>5.6</u> 6.4	<u>A</u> A	LI Int.	0.31	6.0 6.9	<u>A</u> A	LI Int.	0.27	5.7 7.0	A A	LI Int.	0.35	6.2 7.7	A A
Broa		y @ V	Vest 1			et			11111		<u> </u>		1115.		<u> </u>		1116.		7.0				···	
WB			18.2	В			<u>19.5</u>	В			<u>17.8</u>	<u>B</u>				<u>B</u>			19.6	В		0.59		С
NR			16.0 17.0	B B		0.22		B D			15.5 16.6	<u>В</u> В			<u>16.1</u> 18.0	<u>В</u> В			16.4 20.6	B C		0.27 0.74		B C
IND		0.55	17.0		-		19.7	В		0.00	10.0	₽		<u>v.+ 1</u>	10.0	₽		0.57	20.0	O		0.74	20.0	C
SB		0.48	15.9	В		0.64		В		0.37	<u>17.3</u>	<u>B</u>	_	0.47	<u>18.6</u>	<u>B</u>		0.39		В		0.52		В
Droo	Int.		16.6 Vest 1 2	B	Int.		<u>20.1</u>	С	<u>Int.</u>		<u>17.1</u>	<u>B</u>	<u>Int.</u>		<u>18.4</u>	<u>B</u>	Int.		19.0	В	Int.		22.0	С
				D	L		415.5	F	L	0.35	26.5	C	L	0.64	43.8	D	L	0.27	25.7	С	L	0.41	31.2	C
				С			42.5	D			30.6	С			73.3	Ē			36.7	D		-	112.9	
WB			44.6 30.4	D C	L			F E			39.1 26.5	<u>D</u> C			136.4	<u>E</u> C			50.8 29.3	D C			167.5 27.4	
	IK	0.71	30.4	C	IK	1.05	<u>74.0</u>	_	112	0.50	20.5	<u>u</u>	112	<u>U.76</u>	<u>33.5</u>	<u>u</u>	IK	0.00	29.3	C			32.7	<u>C</u>
NB		0.43		С		0.50		С			<u>31.1</u>	<u>C</u>			32.3	C			35.4	D		0.64		D
			30.5 39.8	C D		0.52	32.2 <u>47.7</u>	C D			30.7 39.6				32.2 54.4	<u>C</u> D			42.9 37.8	D D		0.97 0.67		E D
SB			30.1	C		0.67		С			29.1	C			30.7	C			28.5	С		0.39	_	C
	LTR		49.1	D	LTR		<u>101.4</u>	F	LTR		36.8		LTR		44.9	D	LTR		38.9	D	LTR	0.97	62.6	E
4	Int.	4.	35.7	D	Int.	254k 1	85.1	E	<u>Int.</u>		<u>31.6</u>	<u>C</u>	Int.		<u>50.3</u>	D	Int.		36.6	D	Int.		<u>65.5</u>	Е
			26.4	@ W				С	LT	0.20	24.8	<u>C</u>	LT	0.25	25.5	C	LT	0.47	30.8	С	LT	0.65	39.0	D
	R	0.36	29.0	С	R	0.41	30.6	С	R	0.22	25.6	C	R	0.25	26.2	C	R	0.25	26.3	С	R	0.28	26.9	С
				C				C			<u>29.6</u>	<u>C</u>			33.2				30.2			0.70		D
		0.44		A A		0.53 0.62		A A		0.50 0.44		<u>A</u> A		0.60	9.7 8.4			0.63	10.0 8.3			0.76 0.61		B A
	Int.		12.3	В	Int.		14.2	В	Int.		12.0	B	Int.		13.7	B	Int.	J.JL	13.8	В	Int.	5.51	18.0	В
			enue	_	est 1	25th 3	Street			0.70	E2 2	ь.	- I					0 E0	41.0	ь.	l			
EB			41.8 34.1	D C	TR	1.18	125.8	F	TR	0.72	53.2 36.9	<u>D</u>	TR	1.19	130.8	F			41.0 34.8	D C	TR	1.14	108.5	F
WB	L	0.67	49.4	Ď		-		=			32.9	C	===			=	L	0.42	35.4	Ď				·
ND			33.3	С				E		0.68	30.4	<u>C</u> <u>A</u>			64.3	<u>E</u>			31.4	С		1.02		트
NB			14.4 26.7	B C	L TR		16.7 30.4	B C		0.06	9 <u>.7</u> 22.3	<u>A</u> C		0.08 0.62	9.2 23.4	<u>A</u> C			15.4 33.4	B C		0.27 1.07		<u>C</u> <u>E</u>
SB			22.9	Č	L		31.6	Č			18.8	B		0.69		<u>C</u>			33.9	Č	L		54.2	D
		0.41	21.2	С		0.53	21.1	C		0.30	19.8	В	IR		18.7	В		0.73	33.4	С		0.98	63.8	<u>E</u>
Notes	Int.	-	29.3	C C	Int.	onap.	<u>79.4</u> R: Righ	E t Tur	Int.	I · Def	29.2	C off Tu	Int.	· Into	65.4	<u>E</u>	Int.		33.0	С	Int.		<u>77.1</u>	Е
NOTE	٥.						spv: S									1								
	_			_				_				_		_		_			_	_		_	_	_

Table 17-<u>29</u> 2030 No Build Condition

Primary Study Area Unsignalized Intersection Level-of-Service Analysis

			Αľ	VI Pe	ak Ho	our					Midd	ay Pe	ak H	our					PI	VI Pe	ak H	our		
	2	006 E	xistin	g	2				1	2006 I	Existin	g	20	030 N	lo Buil	d	2				2	030 N	lo Bui	ld
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
								LOS	<u>Grp</u>	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			et@ V		133rc	l Stre	et																	
			10.5		L	0.22	10.8	В	L	0.13	<u>10.1</u> <u>23.4</u>	<u>B</u>	L	0.17	10.3 60.0	<u>B</u>				В			10.6	В
				С	Т	<u>1.20</u>	<u>122.4</u>	F	I	<u>0.78</u>	<u>23.4</u>	<u>C</u>	I	1.02	<u>60.0</u>	<u>E</u>	Т	0.96	40.8	E	T	<u>1.22</u>	125.9	F
			et @ V			d Stre																		
	LT	-		Α	L	-	9.2	Α	LI	≡ Ξ	9.3	<u>A</u> <u>B</u> <u>A</u> B	ΓŢ	= =	<u>9.7</u> 23.8	A C A	LT	-	9.7	Α	L	-	10.0	<u>A</u> F
SB	LT	-		В	LT	-	<u>96.1</u>	F	LI		13.4	₿	₩		<u>23.8</u>	Ċ	LT	-	29.2	D	LT	-	96.5	
	TR	-		A	Т	-	8.3	A F	IR	≘	<u>8.1</u> 12.0	A	Ī	Ξ	<u>8.4</u> 19.8	A	TR	-	9.5	Α	Т	-	10.5	B F
I	Int.		13.0	В			79.5	<u> </u>	Int.		12.0	R	Int.		19.8	<u>C</u>	Int.		22.9	С	Int.		70.2	F
		0.17	et @ S			ace 0.20	0.0	٨		0.20	10.0	^		0 24	10.0	ь		0.06	10.4	В	1.	0.24	10.8	В
98		0.17		C			9.9 55.8	F	I⊨	0.68	10.0 18.5	<u>A</u> <u>C</u>	⊨	0.24	10.2 37.7	<u>D</u>			10.4 26.1	D			128.5	
Ture			22.0 1e @ V					드	<u>_</u>	0.00	10.3	<u>u</u>		0.92	31.1	드	-	0.01	20.1	U		1.22	120.5	Г
		0.01				0.01		Δ	LI	U U3	7.6	Δ	lıт	0.03	7.5	Α	lт	0.02	77	Α	Lit	0.02	7.8	Α
		0.01						В		0.02	9.3	₽			10.2					В			12.1	В
			16.8								16.5	C								_			105.9	_
			12.9					Ċ			12.6				15.5								19.9	Ċ
							et Sou					_=				_=_		0.00				0.00		Ť
			10.4				11.9	В			10.0	В	R	0.06	10.9	В	R	0.02	10.1	В	R	0.08	11.7	В
Twe	Ifth /	venu	ie @ S	St. Cl																				
EB	Т	-	9.3	Α	Т	-	10.4	В	I	Ξ	9.8	Α	I	=	10.9	В	Т	-	10.2	В	Т	-	11.9	В
NB	R	-	10.6	В	R	-	16.8	С	R	≡ Ξ	11.5	В	R	= =	16.9	C	R	-	10.4	В	R	-	14.4	В
SB	L	-	8.8	Α	L	-	9.2	Α	I R L	Ξ	8.6	<u>A</u> <u>B</u> <u>A</u>	I R L	Ξ	9.0	BCA	L	-	8.6	Α	L	-	9.0	Α
	Int.		10.2	В	Int.		<u>15.3</u>	С	Int.		11.0	В	Int.		15.3	C	Int.		10.2	В	Int.		<u>13.5</u>	В
			re @ S						_				_								_			
		0.10				0.14					7.6		<u>LTR</u>		7.8	Δ	LTR	0.15	7.8			0.19		Α
SB		0.26					<u>41.6</u>	Е	LI	0.25	22.2 21.1	<u>C</u>	ഥ	0.45	39.4	E	LT	0.20		С			<u>41.4</u>	Е
		0.25		С	Т		39.0	Е	I	0.21	<u>21.1</u>	<u>C</u>	Ï	0.36	34.6	D	Т	0.17	23.2	С	Т	0.30	<u>39.0</u>	Е
			re @ T																					
		-		Α		-	8.4	Α	L R L I	≣ ≣	8.1 7.1	A	F	= = = = = = = = = = = = = = = = = = = =	8.2 7.2	<u>A</u> <u>A</u> <u>A</u>	L	-	8.3	A	L	-	8.4	A
	R	-	7.3	Α	R	-	7.4	Α	片	≘		À	片	Ξ	<u>1.2</u>	A	R	-	7.2	Α	R	-	7.3	A
SB	L	-	9.9	A	L	-	10.4	В		≘	9.5	A		Ξ	9.8	A	L T	-	10.0	Α	L	-	10.4	В
1		-	8.8	A		-	9.0	A	<u> </u>	≘	8.7 8.8	<u>A</u> <u>A</u> <u>A</u> <u>A</u> A		Ξ		A	-	-	8.2	A		-	8.4	A
14/6	Int.	46.04	9.0	A	Int.	046 04	9.3 reet/S	A C/	Int.	*	8.8	<u>A</u>	Int.		9.0	<u>A</u>	Int.		8.8	Α	Int.		9.1	Α
EB		otn St 0.01		ש we : D			reet/S 401.7				22.0	C	1 1	n na	117 F	_				F	Li			F
I EB		0.01		В			26.7		F	0.02 0.45	22.9 14.4	븜	H	0.03	117.5 23.5	E C	L	0.49	20.8	C	R	0.06	 74.8	F
WB				E			208.0	<u>D</u> F	<u>=</u>	0.45	25.1	D D	<u> </u>	0.09 0.13	<u>23.5</u> 96.9	볼	L		20.6 115.4		L		<u>/4.0</u> 12119	
VVD		0.06		C			73.3	F	F	0.40	<u>25.1</u> 12.8	C B D B	F	0.13 0.86	96.9 46.1	Ē E	R		92.0	F				
—	1.	0.00	44.0	U	- 17	<u>U.30</u>	10.0		17	<u>v.40</u>	14.0	D	\Box	<u>v.00</u>	1 U. I	느	г	1.00	JZ.U		- 17	4.49	<u> </u>	

Notes:

L: Left Turn; T: Through; R: Right Turn; Int.: Intersection V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service

^{*} The unsignalized intersection analysis procedure, which assumes random arrival, breaks down with high major or minor street volumes and reports exaggerated levels of stop delays. As such, its results are not necessarily indicative of actual conditions, where gaps created by adjacent signalized intersections often provide additional throughput capacity and result in lower stop delays.

Table 17-<u>30</u>

2030 No Build Condition

condary Study Area Signalized Intersection Level-of-Service Analysis

									ry Study Area Signalized Inters															
	AM Peak Hour						Midday Peak Hour								PM Peak Hour									
	_	-	xisting	g			o Buile	d	_		xisting	g	_		o Buil	d			xisting	9			lo Buil	
Int	Ln		Delay	LOS	Ln		Delay	LOS	<u>Ln</u> Grn		Delay (spy)	ı os	Ln		Delay (spy)	ı os	Ln Grn		Delay (spv) l	ı os	Ln		Delay (sny)	
			Vest 1				(001)		<u> </u>	1112	(ODT)		<u> VIV</u>	114	(OD T)		Ο. Ρ	*,0	(0)1/		О.Р	.,0	(001)	
			24.1				25.4	С	LTR	0.17	21.3	С	LTR	0.21	21.9	С	LTR	0.20	20.8	С	LTR	0.28	22.0	С
WB							118.6			0.57		С			34.3	С			44.0		-	0.95		Е
L ND				C		0.62		C		0.38		<u>c</u>			<u>27.0</u>	<u>ç</u>			43.1	D		0.90		E
				B B		0.49	13.3 23.4	B C		-	12.6 13.0	<u>В</u> В			13.5 14.0	<u>В</u> В			14.4 17.4	B B			16.0 19.4	B B
OD	Int.	0.00	22.4	C	Int.	0.73	31.3	C	Int.	<u>U.70</u>	16.0	B	Int.	0.00	17.4	B	Int.	0.55	21.7	C	Int.	0.00	27.3	Č
Broa		y @ l	Vest 12			et																		_
				С	LTR	0.46	28.5	С		0.30		С	LTR	0.34	26.8	С	LTR	0.31	26.2	С		0.36		С
			100.2				220.2			0.77		D			<u>70.4</u>	Ē			57.8	E			104.1	F
		0.35		A A		0.42		A A		0.33	<u>5.7</u>	<u>A</u> A		0.40 0.48		Ā		0.43		A A		0.52		A A
ЗБ	Int.	0.52	21.4	C	Int.	0.65	36.6	D	Int.	<u>0.41</u>	13.5	B	Int.	0.40	<u>0.8</u> 17.0	<u>A</u> B	Int.	0.42	15.3	В	Int.	0.52	7.3 21.7	Ĉ
Ams		am A	venue			20th S			шц.		10.0		1 11111.		ш.ч		mic.		10.0		mic.		<u> </u>	
			38.4	D	L	0.48	41.0	D		0.48		D	L	0.54	43.7	D			44.4	D		0.61		D
			72.7	Ε				F	_	0.67		D			53.0	D			49.4	D		0.84		Ε
WB			29.2	С	L		30.3	C		0.02		<u>C</u>			<u>27.8</u>	<u>C</u>			27.3	С			27.4	C
NR		0.29	32.1 5.0	C A		0.33		C A	_	0.18 0.54	<u>30.0</u> <u>4.7</u>	A		0.63	30.5 5.8	<u>C</u> A		0.28	31.7	C A		0.34	19.6	C B
		0.53		Â		0.62		A		0.31			-	0.36				0.73				0.39		Ā
	Int.		18.4	В	Int.		24.3	С	Int.		11.9	<u>A</u> B	Int.		13.3	В	Int.		15.0	В	Int.		21.9	С
			glass i																					
				С			21.0	<u>C</u>		-	<u>17.7</u>	B			<u>17.1</u>	B			26.6	С	_	-	28.4	<u>c</u>
			21.6 15.3	C B	뜭	0.77 0.28		<u>С</u> В		0.52 0.33	14.9 22.8	<u>B</u> C			16.3 25.4	<u>В</u> С			25.5 17.7	C B	ૠ		32.7 28.8	C
IND			15.0	В		0.30		В	_	$\frac{0.33}{0.30}$	24.0	c		0.65		D	LIK	0.55	17.7	Ь	LIK	0.00	20.0	<u>∨</u>
SB			17.1	В			19.0	В			24.1	C			27.2	C	LT	0.41	18.0	В	LT	0.53	19.9	В
	R	0.20	13.8	В	R	0.26	14.5	В	R	0.36	26.4	<u>C</u>	R	0.44	28.6	С	R	0.30	18.4	В	R	0.37	19.8	В
	Int.		20.0	В	Int.		21.0	<u>C</u>	Int.		<u>19.3</u>	<u>B</u>	Int.		21.6	<u>C</u>	Int.		22.1	С	Int.		<u>27.6</u>	C
			ue @ I 32.5	E ast C				_	Lit	0.71	25.2	_	1 -	0.05	21.1	C	lıт	0.01	20 1	П	ıт	1 02	E6 6	_
			21.8	C	TR		32.7 32.0	<u>C</u>		0.71 0.45	<u>23.3</u> 18.9	<u>C</u> B			31.1 23.3	<u>C</u>			38.1 19.7	D B	TR	0.75	<u>56.6</u> 26.0	<u>E</u>
			16.9	В			19.0	В		0.45		B			17.0	B			18.8	В		-	24.7	č
	Int.		23.3	С	Int.		27.1	C	Int.		19.7	В	Int.		23.7	С	Int.		25.1	С	Int.		35.5	<u>D</u>
			ıе @ Е																	_				
EB			86.0	F	T		290.0			1.05		Ē			228.1	Ē			82.3	F	T	-	<u>175.0</u>	
W/B			32.3 103.1	C			<u>36.4</u> 158.7	<u>D</u> F		0.22 0.49	31.7 35.6	<u>C</u> D			39.3 59.0	<u>D</u> E			31.7 36.4	C D	R		<u>29.6</u> 53.4	C D
IVVD			103.1	F	T		251.3	F	==	0.43	55.0	U	==	0.07	33.0	=		0.51	JU. -	D		0.00	55.4	=
SB			20.2	Ċ	Ĺ		26.2	Ċ	LTR	0.31	17.6	В	L	0.56	27.7	C	LTR	0.70	22.3	С	L	1.87	431.8	E
اا				_		0.69		С					ĪR	0.39	20.1	С				_			31.4	<u>C</u>
SW		1.00	72.6	E		<u>1.18</u>	129.1	F		<u>0.61</u>		Ē		0.81	45.3	Ē		0.84	47.2	D		-	90.8	F
Fire	Int.	nuo 1	63.2 @ East	125	Int.	'00*	129.8	F	<u>Int.</u>		43.5	D	Int.		94.2	Ė	Int.		52.8	D	Int.		146.3	F
EB			⊕ Eas t 66.0	1251 E	ın sır L		133.8	F	Li	0.67	29 5	С	L	0.87	43.7	D	lι	0.87	42.5	D	ΙL	1.13	106.7	F
-			18.7	В		0.31		Ċ		0.24	19.5	B		0.42	22.2	C			20.0	C		0.59	26.0	Ċ
NB			16.1	В	L		18.9	В	Ī	0.13	9.6	Α	Ī	0.18		В		0.12		Α	L		10.0	<u>B</u>
	T		10.8	В	T		11.2	В		0.39	11.2	<u>B</u>			12.8	<u>B</u>			13.4	В	T	0.70		В
		0.22	10.6	B C	R Int.	0.25	10.9	В		0.23		<u>В</u> В		<u>0.28</u>	11.2 19.5	B		0.46	13.8	В		0.52	15.0 33.1	B C
Broa	Int.	v @ L	26.1 Vest 1 4			of.	<u>44.3</u>	U	<u>Int.</u>		<u>15.5</u>	D	<u>Int.</u>		19.5	D	Int.		19.0	В	Int.		<u> </u>	U
			25.2	C		0.25	25.6	С	LTR	0.26	<u>25</u> .7	C	LTR	0.29	26.2	С	LTR	0.24	25.5	С	LTR	0.28	26.0	С
			41.7	Ď	LTR	0.95	62.8	Ĕ	LTR	0.90	56.4	Ε	LTR	1.05	89.1	E	LTR	0.86	46.0	Ď			71.7	Ĕ
			19.7	В			21.5	С	LTR	0.63	21.5	C B			24.7	C	LTR	0.76	24.8	С	LTR	0.91	33.5	С
SB			9.3	Α			10.1	В			11.5	B			13.5	<u>B</u>			13.1	В			16.9	В
	IR Int.	0.42	10.5 20.9	B C	IR Int.	0.51	11.7 26.0	B C	III.	<u>u.50</u>	11.4 23.8	<u>B</u> C	III.	<u>U.58</u>	<u>12.6</u> 31.1	<u>B</u>	IR Int.	U.44	10.7 24.3	B C	IR Int.	0.51	11.6 33.1	B C
Notes		- 1	_: Left T			onap.				l · Det				· Inte		<u>v</u>	IIIL.		۷4.3	U	IIIL.		JJ. I	U
.40.6	٠.															'								
V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service																								

Table 17-<u>31</u>
2030 No Build Condition
Secondary Study Area Unsignalized Intersection Level-of-Service Analysis

	AM Peak Hour							Midday Peak Hour							PM Peak Hour									
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1								IVIIUUAY FEAK FIOUL							FIVI FEAK HOUI								
	2006 Existing 2030 No Build						2006 Existing				2030 No Build				2006 Existing				2030 No Build			ld		
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	,	Ln		Delay	
Int.	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
Firs	First Avenue @ East 125th Street Southbound Right Turn																							
SB	R	0.26	11.7	В	R	0.40	13.6	В	R	0.11	9.4	<u>A</u>	<u>R</u>	0.19	10.0	<u>B</u>	R	0.10	9.4	Α	R	0.22	10.3	<u>B</u>
Note	Notes: R: Right Turn; V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service																							

Broadway Northbound and West 133rd Street

During the PM peak hour, the westbound through-right movement would deteriorate from LOS C to LOS D, with delay increasing from 29.0 to 50.9 seconds and v/c ratio increasing from 0.67 to 0.93.

Broadway and West 125th Street

During the AM peak hour, the northbound right-turn movement would continue to operate at LOS D, with delay increasing from 39.8 to <u>47.7</u> seconds and v/c ratio increasing from 0.55 to 0.67. The southbound shared left-through-right movement would deteriorate from LOS D to LOS F, with delay increasing from 49.1 to <u>101.4</u> seconds and v/c ratio increasing from 0.90 to 1.11. The eastbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 36.8 to <u>415.5</u> seconds and v/c ratio increasing from 0.55 to <u>1.76</u>. The westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 44.6 to <u>235.1</u> seconds and v/c ratio increasing from 0.68 to <u>1.35</u>. At the same time, the westbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 30.4 to 74.0 seconds and v/c ratio increasing from 0.71 to 1.05.

During the midday peak hour, the northbound right-turn movement would continue to operate at LOS D, with delay increasing from 39.6 to 54.4 seconds and v/c ratio increasing from 0.54 to 0.73. The eastbound shared through and right-turn movement would deteriorate from LOS C to LOS E, with delay increasing from 30.6 to 73.7 seconds and v/c ratio increasing from 0.71 to 1.04. The westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 39.1 to 136.4 seconds and v/c ratio increasing from 0.54 to 1.02.

During the PM peak hour, the northbound left-through movement would deteriorate from LOS D to LOS E, with delay increasing from 42.9 to 62.1 seconds and v/c ratio increasing from 0.82 to 0.97. The northbound right-turn movement would continue to operate at LOS D, with delay increasing from 37.8 to <u>47.2</u> seconds and v/c ratio increasing from 0.51 to <u>0.67</u>. The southbound left-through-right movement would <u>deteriorate from LOS D to LOS E</u>, with delay increasing from 38.9 to <u>62.6</u> seconds and v/c ratio increasing from 0.75 to <u>0.97</u>. The eastbound through-right movement would deteriorate from LOS D to LOS F, with delay increasing from 36.7 to <u>112.9</u> seconds and v/c ratio increasing from 0.83 to <u>1.16</u>. At the same time, the westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 50.8 to <u>167.5</u> seconds and v/c ratio increasing from 0.65 to <u>1.13</u>.

Amsterdam Avenue and West 125th Street

During the AM peak hour, the eastbound through-right movement would deteriorate from LOS C to LOS \underline{F} , with delay increasing from 34.1 to $\underline{125.8}$ seconds and v/c ratio increasing from 0.77 to $\underline{1.18}$. At the same time, the westbound through-right movement would deteriorate from LOS

C to LOS \underline{F} , with delay increasing from 33.3 to $\underline{139.4}$ seconds and v/c ratio increasing from 0.76 to $\underline{1.21}$.

During the midday peak hour, the eastbound through-right movement would deteriorate from LOS D to LOS F, with delay increasing from 36.9 to 130.8 seconds and v/c ratio increasing from 0.82 to 1.19. At the same time, the westbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 30.4 to 64.3 seconds and v/c ratio increasing from 0.68 to 1.00.

During the PM peak hour, the northbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 33.4 to 74.9 seconds and v/c ratio increasing from 0.84 to 1.07. The southbound left-turn movement would deteriorate from LOS C to LOS D, with delay increasing from 33.9 to 54.2 seconds and v/c ratio increasing from 0.58 to 0.81 and the southbound through-right movement from LOS C to LOS E, with delay increasing from 33.4 to 63.8 seconds and v/c ratio increasing from 0.73 to 0.98. The eastbound through-right movement would deteriorate from LOS C to LOS F, with delay increasing from 34.8 to 108.5 seconds and v/c ratio increasing from 0.79 to 1.14. At the same time, the westbound through-right movement would deteriorate from LOS C to LOS E, with delay increasing from 31.4 to 66.3 seconds and v/c ratio increasing from 0.71 to 1.02.

Marginal Street and West 133rd Street

During the AM peak hour, the southbound approach would deteriorate from LOS C to LOS F, with delay increasing from 18.5 to 122.4 seconds and v/c ratio increasing from 0.68 to 1.20.

<u>During the midday peak hour, the southbound approach would deteriorate from LOS C to LOS</u> F, with delay increasing from 23.4 to 60.0 seconds and v/c ratio increasing from 0.78 to 1.02.

During the PM peak hour, the southbound approach would deteriorate from LOS E to LOS F, with delay increasing from 40.8 to 125.9 seconds and v/c ratio increasing from 0.96 to 1.22.

Marginal Street and West 132nd Street

During the AM peak hour, the southbound left-through movement would deteriorate from LOS B to LOS F, with delay increasing from 14.5 to 96.1 seconds.

During the PM peak hour, the same movement would deteriorate from LOS D to LOS F, with delay increasing from 29.2 to 96.5 seconds.

Marginal Street and St. Clair Place

During the AM peak hour, the southbound through movement would deteriorate from LOS C to LOS F, with delay increasing from 22.0 to <u>55.8</u> seconds and v/c ratio increasing from 0.76 to <u>1.01</u>.

<u>During the midday peak hour, the southbound through movement would deteriorate from LOS C to LOS E, with delay increasing from 18.5 to 37.7 seconds and v/c ratio increasing from 0.68 to 0.92.</u>

During the PM peak hour, the same movement would deteriorate from LOS D to LOS F, with delay increasing from 26.1 to <u>128.5</u> seconds and v/c ratio increasing from 0.81 to <u>1.22</u>.

Twelfth Avenue and West 131st Street

<u>During the AM peak hour, westbound approach would deteriorate from LOS C to LOS E, with delay increasing from 16.8 to 40.8 seconds and v/c ratio increasing from 0.16 to 0.44.</u>

During the PM peak hour, westbound approach would deteriorate from LOS C to LOS F, with delay increasing from 23.5 to 105.9 seconds and v/c ratio increasing from 0.28 to 0.94.

Riverside and St. Clair Place

During the AM peak hour, the southbound left-through movement would deteriorate from LOS C to LOS E, with delay increasing from 20.6 to 41.6 seconds and v/c ratio increasing from 0.26 to 0.49, and the southbound through movement would deteriorate from LOS C to LOS E, with delay increasing from 20.1 to 39.0 seconds and v/c ratio increasing from 0.25 to 0.46.

During the midday peak hour, the southbound left-through movement would deteriorate from LOS C to LOS E, with delay increasing from 22.2 to 39.4 seconds and v/c ratio increasing from 0.25 to 0.45, and the southbound through movement would deteriorate from LOS C to LOS D, with delay increasing from 21.1 to 34.6 seconds and v/c ratio increasing from 0.21 to 0.36.

During the PM peak hour, the southbound left-through movement would deteriorate from LOS C to LOS E, with delay increasing from 22.8 to 41.4 seconds and v/c ratio increasing from 0.20 to 0.47, and the southbound through movement would deteriorate from LOS C to LOS E, with delay increasing from 23.2 to 39.0 seconds and v/c ratio increasing from 0.17 to 0.30.

West 125th Street and West 129th Street/St. Clair Place

During the AM peak hour, the eastbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 30.0 to $\underline{401.7}$ seconds and v/c ratio increasing from 0.01 to $\underline{0.10}$. At the same time, the westbound left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 43.7 to $\underline{208.0}$ seconds and v/c ratio increasing from 0.06 to $\underline{0.29}$. The westbound right-turn movement would deteriorate from LOS C to LOS F, with delay increasing from 22.6 to $\underline{73.3}$ seconds and v/c ratio increasing from 0.58 to $\underline{0.96}$.

During the midday peak hour, the eastbound left-turn movement would deteriorate from LOS C to LOS F, with delay increasing from 22.9 to 117.5 seconds and v/c ratio increasing from 0.02 to 0.03. At the same time, the westbound left-turn movement would deteriorate from LOS D to LOS F, with delay increasing from 25.1 to 96.9 seconds and v/c ratio increasing from 0.01 to 0.13. The westbound right turn movement would deteriorate from LOS B to LOS E, with delay increasing from 12.8 to 46.1 seconds and v/c ratio increasing from 0.40 to 0.86.

During the PM peak hour, the eastbound left-turn movement would continue to operate at LOS F and the eastbound right-turn movement would deteriorate from LOS C to LOS F, with delay increasing from 20.8 to <u>74.8</u> seconds and v/c ratio increasing from 0.48 to 0.96. At the same time the westbound left-turn movement would continue to operate at LOS F, with delay increasing from 115.4 to <u>12,119.0</u> seconds and v/c ratio increasing from 0.26 to <u>14.00</u>. The westbound right-turn movement would also continue to operate at LOS F, with delay increasing from 92.0 to <u>621.2</u> seconds and v/c ratio increasing from 1.05 to <u>2.29</u>.

SECONDARY STUDY AREA

Broadway and West 110th Street

During the AM peak hour, the westbound *de facto* left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 55.1 to 118.6 seconds and v/c ratio increasing from 0.85 to 1.10.

During the PM peak hour the westbound *de facto* left-turn movement would deteriorate from LOS D to LOS E, with delay increasing from 44.0 to 72.4 seconds and v/c ratio increasing from

0.76 to 0.95, and the westbound through-right movement would deteriorate from LOS D to LOS E, with delay increasing from 43.1 to 57.1 seconds and v/c ratio increasing from 0.78 to 0.90.

Broadway and West 120th Street

During the AM peak hour, the westbound approach would continue to operate at LOS F, with delay increasing from 100.2 to 220.2 seconds and v/c ratio increasing from 1.04 to 1.36.

During the midday peak hour, the westbound approach would deteriorate from LOS D to LOS E, with delay increasing from 45.8 to 70.4 seconds and v/c ratio increasing from 0.77 to 0.93.

During the PM peak hour the same approach would deteriorate from LOS E to LOS F, with delay increasing from 57.8 to 104.1 seconds and v/c ratio increasing from 0.87 to 1.06.

Amsterdam Avenue and West 120th Street

During the AM peak hour, the eastbound through-right movement would deteriorate from LOS E to LOS F, with delay increasing from 72.7 to 103.8 seconds and v/c ratio increasing from 0.94 to 1.06.

<u>During the midday peak hour, the eastbound through-right movement would continue to operate at LOS D, with delay increasing from 47.2 to 53.0 seconds and v/c ratio increasing from 0.67 to 0.75.</u>

During the PM peak hour, the eastbound through-right movement would deteriorate from LOS D to LOS E, with delay increasing from 49.4 to 60.9 seconds and v/c ratio increasing from 0.73 to 0.84.

Madison Avenue and East 125th Street

During the PM peak hour, the <u>eastbound</u> approach would deteriorate from LOS D to LOS \underline{E} , with delay increasing from 38.1 to <u>56.6</u> and v/c ratio increasing from 0.91 to <u>1.02</u>.

Second Avenue and East 125th Street

During the AM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 86.0 to $\underline{290.0}$ seconds and v/c ratio increasing from 1.05 to $\underline{1.55}$. The westbound *de facto* left-turn movement would continue to operate at LOS F, with delay increasing from 103.1 to 158.7 seconds and v/c ratio increasing from 1.05 to 1.21, and the westbound through movement would continue to operate at LOS F, with delay increasing from 103.3 to $\underline{251.3}$ seconds and v/c ratio increasing from 1.05 to $\underline{1.44}$. The southwestbound approach would deteriorate from LOS E to LOS F, with delay increasing from 72.6 to $\underline{129.1}$ seconds and v/c ratio increasing from 1.00 to $\underline{1.18}$.

During the midday peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 86.7 to 228.1 seconds and v/c ratio increasing from 1.05 to 1.41. The westbound left-through movement would deteriorate from LOS D to LOS E, with delay increasing from 35.6 to 59.0 seconds and v/c ratio increasing from 0.49 to 0.87. At the same time, the southwestbound approach would continue to operate at LOS D, with delay increasing from 36.6 to 45.3 and v/c ratio increasing from 0.61 to 0.81.

During the PM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 82.3 to 175.0 seconds and v/c ratio increasing from 1.05 to 1.30. The westbound left-through movement would continue to operate at LOS D, with delay increasing from 36.4 to 53.4 seconds and v/c ratio increasing from 0.51 to 0.86. The southbound left-turn movement would deteriorate from LOS C to LOS F, with delay increasing from 22.3 to 431.8 seconds and v/c ratio increasing from 0.70 to 1.87. At the same time, the southwestbound

approach would deteriorate from LOS D to LOS F, with delay increasing from 47.2 to $\underline{90.8}$ and v/c ratio increasing from 0.84 to $\underline{1.07}$.

First Avenue and East 125th Street

During the AM peak hour, the eastbound left-turn movement would deteriorate from LOS E to LOS F, with delay increasing from 66.0 to 133.8 seconds and v/c ratio increasing from 0.99 to 1.20.

During the PM peak hour, the same movement would deteriorate from LOS D to LOS F, with delay increasing from 42.5 to 106.7 and v/c ratio increasing from 0.87 to 1.13.

Broadway and West 145th Street

During the AM peak hour, the westbound approach would deteriorate from LOS D to LOS E, with delay increasing from 41.7 to 62.8 seconds and v/c ratio increasing from 0.78 to 0.95.

During the midday peak hour, the westbound approach would deteriorate from LOS E to LOS F, with delay increasing from 56.4 to 89.1 seconds and v/c ratio increasing from 0.90 to 1.05.

During the PM peak hour, the same approach would deteriorate from LOS D to LOS E, with delay increasing from 46.0 to 71.7 seconds and v/c ratio increasing from 0.86 to 1.00.

PARKING SUPPLY AND UTILIZATION

As with the 2015 No Build analysis, on- and off-street parking utilization rates for the 2030 No Build condition were based on an increase in background parking demand of 0.5 percent per year for a total of 12.7 percent by 2030.

ON-STREET PARKING

Table 17-32 shows the expected on-street parking utilization in 2030, assuming a 0.5 percent per year growth in background parking demand. On-street parking utilization within ¼ mile of the Project Area would increase to 86, 89, and 86 percent during the morning, midday, and evening periods, respectively. Overall, within ½ mile, on-street parking utilization would increase to 81, 88, and 90 percent during the morning, midday, and evening periods, respectively.

OFF-STREET PARKING

A growth rate of 0.5 percent per year was also assumed for off-street parking facilities within the one-mile radius of the Project Area. In addition, the projected peak hour parking accumulation of the same No Build projects incorporated into the 2015 No Build analysis are expected to utilize off-street parking facilities within the Project Area in the 2030 No Build condition.

As shown in Table 17-33, the seven public parking facilities located within ¼ mile of the Project Area would be 120, 121, and 115 percent utilized, with expected shortfalls of 262, 282, and 195 spaces during the morning, midday, and evening periods, respectively. The nine public parking facilities located within ½ mile would be 115, 119, and 105 percent utilized, with expected shortfalls of 254, 331, and 86 spaces during the morning, midday, and evening periods, respectively. Overall, within one mile, the 26 public parking facilities would be 103, 104, and 80 percent utilized, with shortfalls of 111 and 151 spaces during the morning and midday periods, respectively, and a surplus of 699 spaces in the evening period.

Table 17-<u>32</u> 2030 No Build On-Street Parking Utilization Summary

2030 No Dulla Oll-St	i eet 1 ai kiiig	, Cunzauon	Summary								
2030 No Build	AM	MD	PM								
1/4-Mile Radius											
Capacity	1847	1847	1847								
2006 Existing Demand	1407	1457	1415								
Background Growth (0.5% per year)	179	185	180								
2015 No Build Demand	1586	1642	1595								
Remaining Spaces	261	205	252								
Utilization	86%	89%	86%								
½-Mil	e Radius										
Capacity	4783	4783	4783								
2006 Existing Demand	3454	3732	3807								
Background Growth (0.5% per year)	439	475	484								
2015 No Build Demand	3893	4207	4291								
Remaining Spaces	890	576	492								
Utilization	81%	88%	90%								

Table 17-<u>33</u> 2030 No Build Off-Street Parking Utilization Summary

2030 No Build	AM	MD	PM									
1/4-Mile Radius												
Capacity	1330	1330	1330									
2006 Existing Demand	1171	1081	1188									
Background Growth (0.5% per year)	149	137	151									
No Build Project Generated Parking Demand	272	394	186									
New Columbia Academic Building	48	118	69									
Science, Math, and Engineering Secondary School	13	13	0									
New Columbia Administrative Building (Studebaker)	86	148	89									
New Columbia Office Building (No. 23)	77	70	17									
New Columbia Office Building (No. 24)	48	45	11									
Total 2030 No Build Demand	1592	1612	1525									
Remaining Spaces	-262	-282	-195									
Utilization	120%	121%	115%									
½-Mile Radius												
Capacity	1710	1710	1710									
2006 Existing Demand	1501	1461	1428									
Background Growth (0.5% per year)	191	186	182									
No Build Project Generated Parking Demand	272	394	186									
Total 2030 No Build Demand	1964	2041	1796									
Remaining Spaces	-254	-331	-86									
Utilization	115%	119%	105%									
1-Mile Radiu	s		•									
Capacity	3576	3576	3576									
2006 Existing Demand	3030	2957	2387									
Background Growth (0.5% per year)	385	376	304									
No Build Project Generated Parking Demand	272	394	186									
Total 2030 No Build Demand	3687	3727	2877									
Remaining Spaces	-111	-151	699									
Utilization	103%	104%	80%									
Note: See Appendix H for detailed parking accumulation analysis.												

G. 2030 PROBABLE IMPACTS OF THE PROPOSED ACTIONS

OVERVIEW

As with the 2015 Build condition, the 2030 Build condition reflects increases in traffic volumes, as well as roadway and signal timing improvements that are part of the Proposed Actions. The 2030 Build condition was developed by adjusting for the anticipated changes to No Build traffic as a result of the Proposed Actions and adding the 2030 reasonable worst-case transportation scenario project-generated trips to the 2030 No Build network, as described below.

STUDY AREA STREET NETWORK

The 2030 Build analysis accounted for the same roadway and signal timing improvements described for the 2015 analysis, including the possibility of implementing EDC's streetscapes alternative along West 125th Street between Broadway and Marginal Street. These improvements were included as part of the design process, since they would be integral to the overall planning of site access and traffic circulation, and reflect a more typical street grid configuration that addresses access and circulation needs of the community and the Manhattanville project. In addition, to accommodate midblock crosswalks along West 130th, West 131st, and West 132nd Streets, new traffic signals would be installed at these locations. Details of the project-related roadway improvements are summarized in Appendix H.

VEHICLE TRIP ASSIGNMENT

To develop an appropriate baseline traffic network from which potential project impacts could be evaluated, the 2030 No Build traffic network volumes were adjusted to account for the project-related street direction changes, No Build projects that would not be constructed if the Proposed Actions are approved, and changes in how certain No Build trips would be distributed. The primary and secondary study area net 2030 diverted peak hour traffic volumes are shown in Appendix H.

In the 2030 Build condition, the Columbia University development component of the Proposed Actions is projected to generate a total of 1,023 (843 entering and 180 exiting), 428 (214 entering and 214 exiting), and 1,010 (208 entering and 802 exiting) vehicle trips during the weekday AM, midday, and PM peak hours, respectively. The non-Columbia University development component is expected to generate a total of 196 (136 entering and 60 exiting), 302 (161 entering and 141 exiting), and 355 (157 entering and 198 exiting) vehicle trips during the AM, midday, and PM peak hours, respectively. The 2030 project-generated peak hour traffic volumes within the primary and secondary study areas are shown in Appendix H.

COLUMBIA UNIVERSITY TRIPS

As with the 2015 Build analysis, the 2030 project-generated trips associated with Columbia University uses were assigned to the study area traffic network. Ingress/egress points for the onsite parking facilities would be located on West 130th Street between Broadway and Twelfth Avenue, West 131st Street between Broadway and Old Broadway, West 132nd Street between Broadway and Twelfth Avenue, West 133rd Street between Broadway and Amsterdam Avenue, and West 133rd Street between Broadway and Twelfth Avenue. Truck access would be located on West 131st Street between Broadway and Twelfth Avenue. A Columbia University shuttle bus service was also incorporated into the peak hour traffic network.

NON-COLUMBIA UNIVERSITY TRIPS

Future Build peak hour non-Columbia University trips were also assigned to the study area traffic network following the same procedure set forth in the 2015 analysis.

TRAFFIC VOLUMES AND INTERSECTION CAPACITY ANALYSES

The 2030 project-generated traffic volumes were added to the 2030 traffic diversion network volumes to obtain the 2030 Build traffic network. Figures 17-32, 17-33, and 17-34 depict the 2030 Build condition traffic volumes within the primary study area for the weekday AM, midday, and PM peak hours, respectively. The secondary study area peak hour 2030 Build condition volumes are presented in Figures 17-35 to 17-37.

Within the primary study area, increases in two-way peak hour traffic over 2030 No Build volumes are expected to range from 100 vph to 600 vph along Broadway, from 50 vph to 150 vph along Amsterdam Avenue, and from 30 vph to 700 vph along Twelfth Avenue. Two-way peak hour volumes along West 125th Street would increase by 20 vph to 310 vph during the two analysis periods.

Tables 17-<u>34 and</u> 17-<u>35</u> compare the 2030 No Build and Build peak hour conditions for the primary study area intersections. The secondary study area No Build and Build peak hour conditions are presented in Tables 17-<u>36 and</u> 17-<u>37</u>. Based on the thresholds established in the *CEQR Technical Manual*, as described in Section B of this chapter, significantly impacted movements are denoted with a + sign in the tables and detailed below.

PRIMARY STUDY AREA

Amsterdam Avenue and West 125th Street

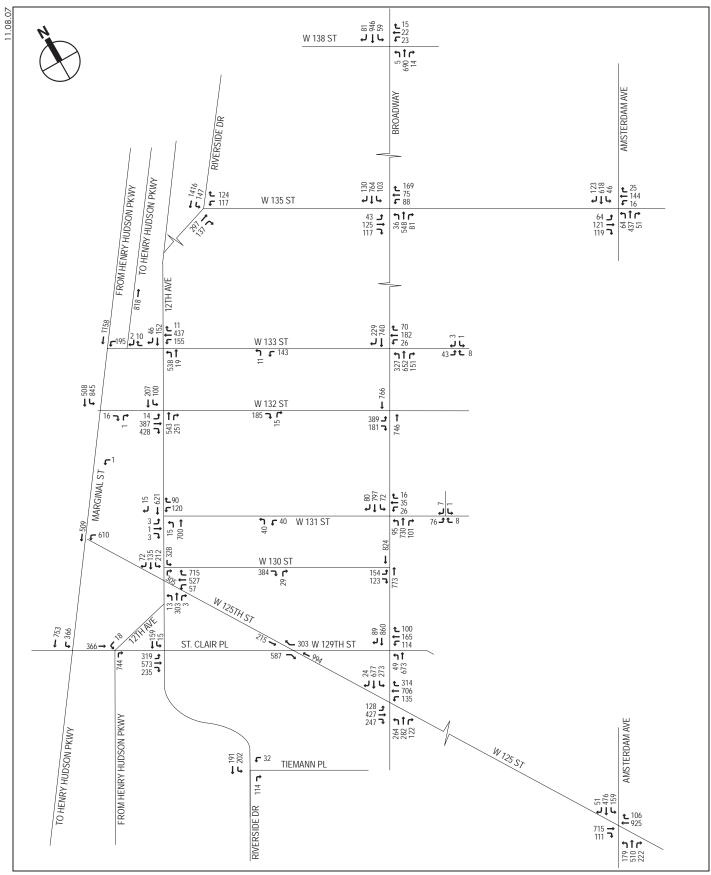
During the AM peak hour, the eastbound through-right movement would <u>continue to operate at LOS F</u>, with delay increasing from $\underline{125.8}$ to $\underline{188.6}$ seconds and v/c ratio increasing from $\underline{1.18}$ to $\underline{1.33}$. The westbound through-right movement would <u>continue to operate at LOS F</u>, with delay increasing from $\underline{139.4}$ to $\underline{260.7}$ seconds and v/c ratio increasing from $\underline{1.21}$ to $\underline{1.49}$.

During the midday peak hour, the eastbound through-right movement would continue to operate at LOS F, with delay increasing from 130.8 to 201.1 seconds and v/c ratio increasing from 1.19 to 1.36. The westbound through-right movement would deteriorate from LOS E to LOS F, with delay increasing from 64.3 to 113.3 seconds and v/c ratio increasing from 1.00 to 1.15.

During the PM peak hour, the eastbound through-right movement would continue to operate at LOS F, with delay increasing from $\underline{108.5}$ to $\underline{235.8}$ seconds and v/c ratio increasing from $\underline{1.14}$ to $\underline{1.44}$. The westbound through-right movement would deteriorate from LOS $\underline{\underline{E}}$ to LOS $\underline{\underline{F}}$, with delay increasing from $\underline{66.3}$ to $\underline{109.4}$ seconds and v/c ratio increasing from $\underline{1.02}$ to $\underline{1.15}$.

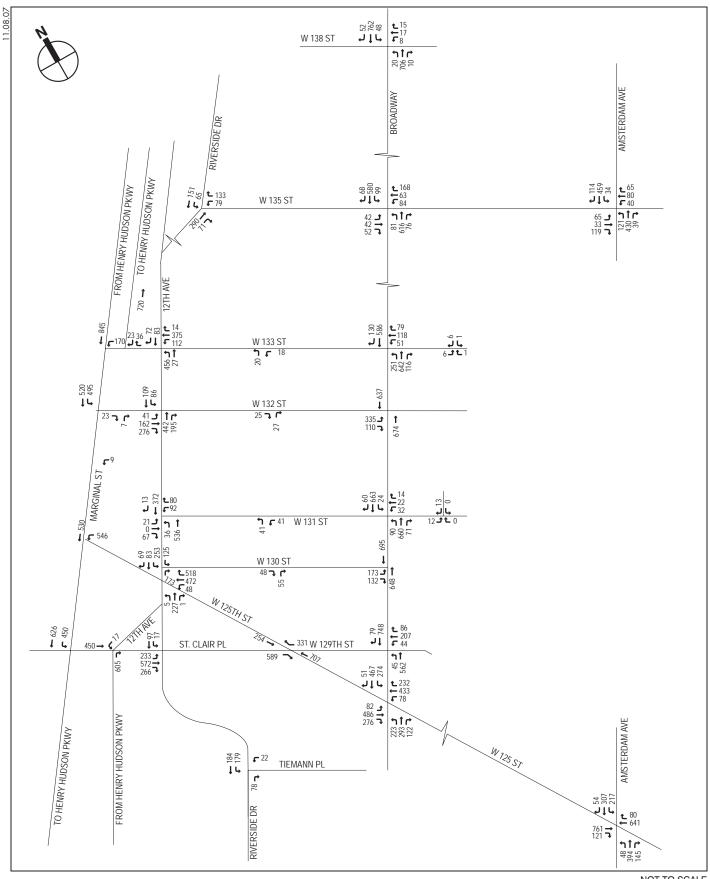
Riverside Drive and West 135th Street

During the PM peak hour, the southbound *de facto* left-turn movement would deteriorate from LOS A to LOS F, with delay increasing from 7.6 to 110.7 to seconds and v/c ratio increasing from 0.14 to 0.98. The westbound right-turn movement would continue to operate at LOS D, with delay increasing from 40.9 to 46.3 seconds and v/c ratio increasing from 0.66 to 0.74.



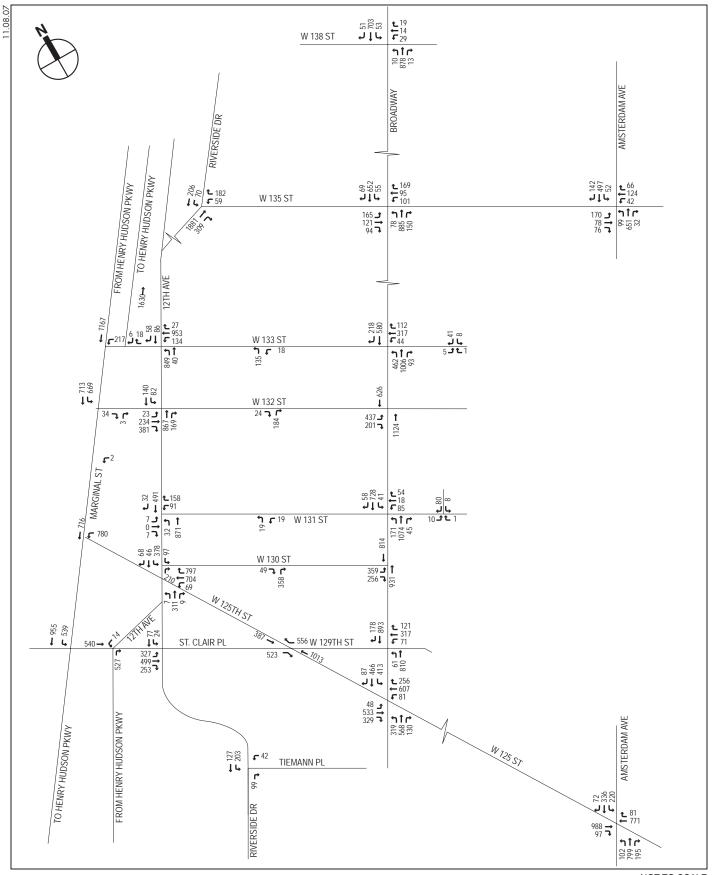
NOT TO SCALE

Figure 17-32
Primary Study Area
Build 2030 Morning Peak Hour



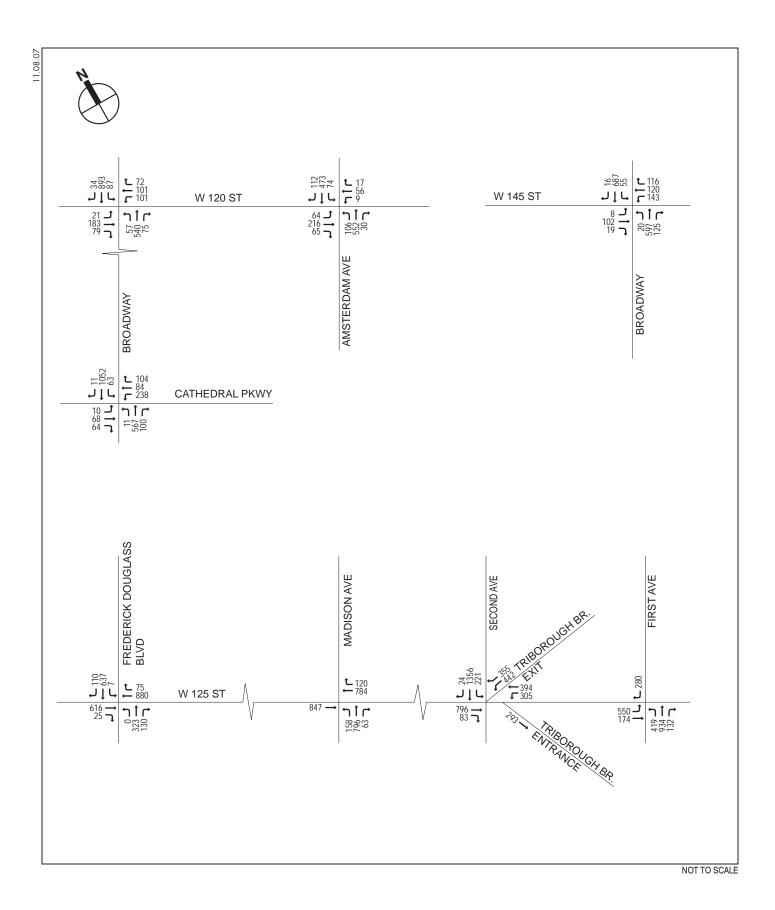
NOT TO SCALE

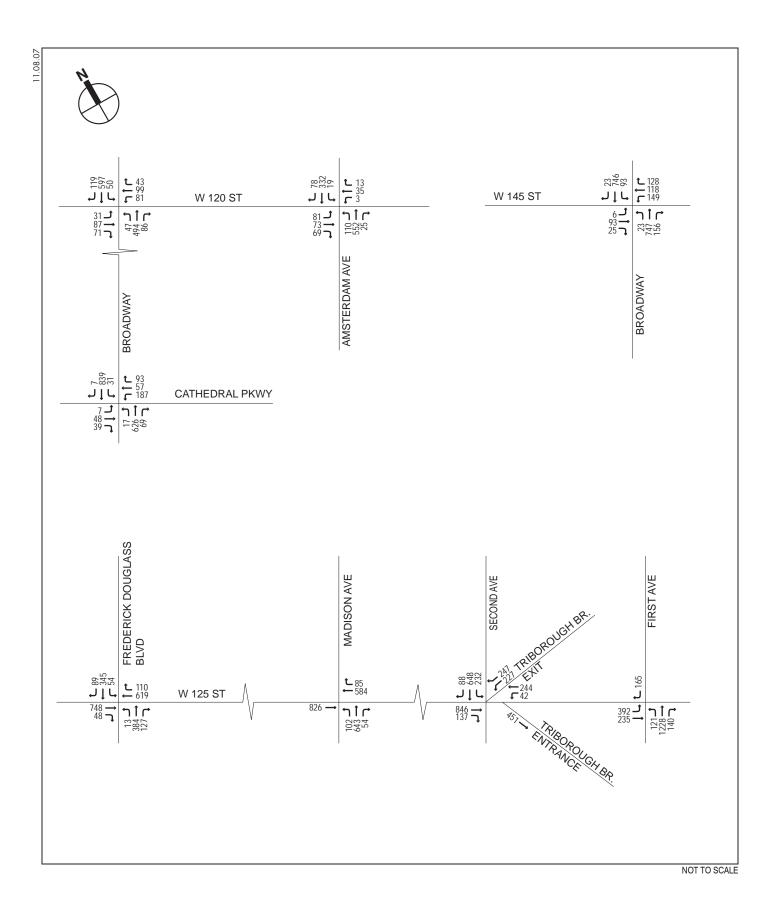
Figure 17-33 **Primary Study Area Build 2030 Midday Peak Hour**



NOT TO SCALE

Figure 17-34
Primary Study Area
Build 2030 Evening Peak Hour





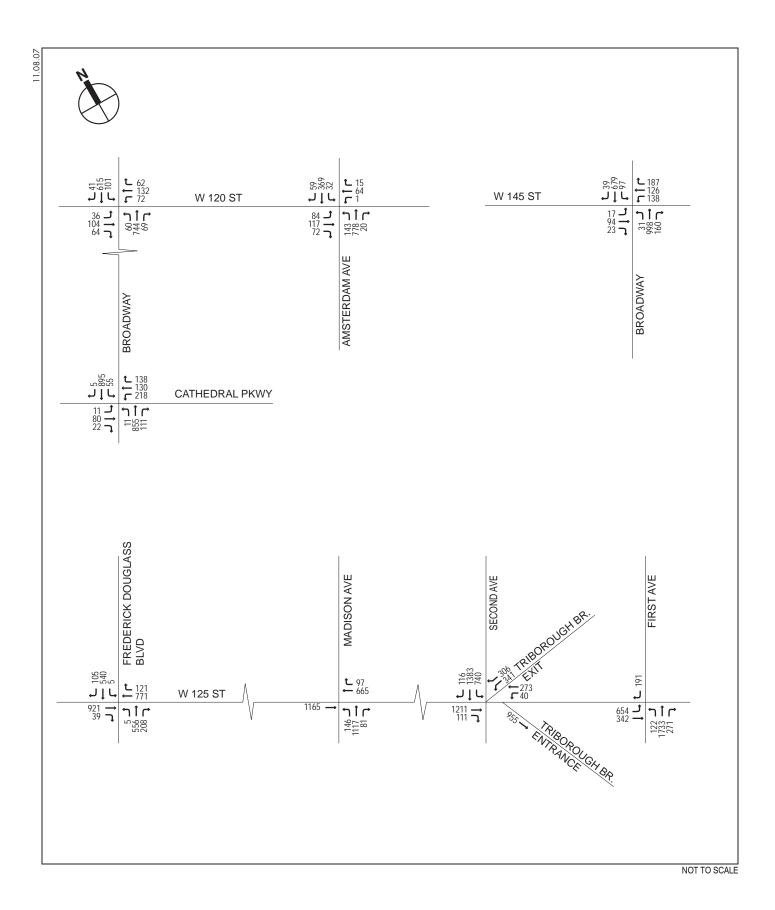


Table 17-<u>34</u>
Comparison of 2030 No Build and Build Conditions
Primary Study Area Signalized Intersection Level-of-Service Analysis

,			nary Study Area S	-		•
	AM Pea			eak Hour	PM Pea	
	2030 No Build	2030 Build	2030 No Build	2030 Build	2030 No Build	2030 Build
Int	Ln Delay	Ln Delay	Ln Delay S Grp V/C (spv) LOS	Ln Delay	Ln Delay	Ln Delay
	ginal Street @ West 1		S G P V/C (SPV) LOS	Gip V/C ((Spv)/LOS	Gip vic (spv)[LOS	Gipi v/C ((Spv))LOS
WB		L <u>0.56</u> 35.1 D	1	L 0.48 32.7 C		L 0.62 39.0 D
SB		T 1.17 102.9 F		<u>I 0.72 14.7 B</u>		T <u>0.97</u> <u>33.1</u> <u>C</u>
		Int. <u>92.9</u> E		<u>Int.</u> <u>17.9</u> B		Int. <u>34.1</u> C
<i>Marg</i> SB	ginal Street @ West 1		1	LIT 004 450 D	i	IT 000 0F0 0
28		LT <u>0.93</u> <u>29.4</u> C Int. <u>29.4</u> C		LT 0.64 15.6 B Int. 15.6 B		LT <u>0.89</u> <u>25.3</u> C Int. <u>25.3</u> C
Marc	ginal Street @ West 1			<u> 10.0 D</u>		IIIt. <u>20.0</u> 0
WB		L <u>0.68</u> 29.9 C	L 0.48 25.5 C	L 0.62 28.3 C	L <u>0.67</u> <u>29.0</u> C	L <u>0.90 41.7</u> D
SB	LT <u>0.50</u> <u>14.7</u> B	T <u>0.36</u> <u>12.9</u> B	<u>LT 0.53 15.2 B</u>	<u>I 0.38 13.1 B</u>	LT <u>0.68</u> <u>18.1</u> B	T <u>0.51</u> <u>14.9</u> B
لـــا	Int. <u>20.1 C</u>	Int. <u>22.1</u> C	<u>Int. 19.4 B</u>	<u>Int.</u> <u>20.8</u> <u>C</u>	Int. <u>22.7</u> C	Int. <u>28.9</u> C
	ginal Street @ St. Clai		1	l	1	L 022 44 0 D
SB		L <u>0.22</u> <u>10.6</u> B LT <u>0.57</u> <u>14.4</u> B		<u>L 0.57 15.8 B</u> LT 0.43 12.5 B		L <u>0.33</u> <u>11.8</u> B LT <u>0.80</u> <u>20.1</u> C
		Int. 13.8 B		Int. 13.7 B		Int. 18.6 B
Rive	rside Drive @ West 1	35th Street	•			
WB		L 0.30 27.4 C		L 0.23 26.5 C	L 0.21 26.3 C	L 0.18 25.8 C
NB	R 0.47 32.8 C TR 0.25 8.3 A	R 0.51 33.9 C	<u>R</u> <u>0.40</u> <u>30.6</u> <u>C</u>	<u>R</u> 0.52 33.9 C	R 0.66 40.9 D TR 1.15 83.7 F	R 0.74 46.3 D+ TR 1.10 62.2 E
	LT 0.94 19.8 B	LT 0.97 24.6 C		<u>TR 0.21 8.1 A</u> LT 0.15 7.7 A		DefL 0.98 110.7 F+
	21 0.01 10.0 B	21 0.07 21.0	<u>== === == == ==</u>		21 0.11 7.0 7	T 0.21 8.2 A
	Int. 19.7 B	Int. 22.0 C	<u>Int. 12.9 B</u>	<u>Int. 13.9 B</u>	Int. 72.6 E	Int. 57.3 E
	erside Drive @ St. Cla		1	1		
EB		L 0.27 4.9 A TR 0.73 11.3 B		L 0.23 4.6 A		L <u>0.29</u> <u>5.0</u> A
SB		TR <u>0.73</u> <u>11.3</u> B LT 0.39 <u>35.7</u> D		<u>TR</u> <u>0.75</u> <u>11.8</u> <u>B</u> LT <u>0.26</u> 33.9 C		TR <u>0.68</u> <u>9.9</u> A LT 0.23 33.6 C
OD		Int. 13.0 B		Int. 12.5 B		Int. 10.6 B
Twel	Ifth Avenue @ West 1					
WB	LTR 0.80 40.5 D	L 0.37 24.4 C		<u>L 0.28 23.0 C</u>	LTR 1.36 206.1 F	L 0.33 23.7 C
ND		TR 0.53 25.9 C		<u>IR 0.49 25.0 C</u>		TR 1.13 100.2 F
INB	L 1.11 94.3 F LTR 0.21 11.2 B	L 0.77 26.9 C LT 0.43 15.2 B	<u>L</u> <u>0.85</u> <u>32.8</u> <u>C</u> LTR 0.20 11.0 B	<u>L 0.81 29.3 C</u> LT 0.13 10.6 B	L 1.01 60.1 E LTR 0.76 24.1 C	L 1.00 60.1 E LT 0.92 44.9 D
SB	LTR 0.21 11.2 B	TR 0.14 10.3 B		TR 0.13 10.2 B	LTR 0.70 24.1 C	TR 0.11 10.1 B
	Int. 55.4 E	Int. 22.2 C		Int. 23.4 C	Int. 96.8 F	Int. 70.4 E
		32nd Street				
	LTR 0.81 39.0 D	LTR <u>0.86</u> <u>36.4</u> <u>D</u>		<u>LTR 0.58</u> <u>26.7</u> <u>C</u>		LTR <u>0.68</u> <u>28.7</u> C
	LTR 0.12 20.8 C LTR 0.52 14.4 B	TR 0.67 17.5 B	LTR 0.09 20.4 C LTR 0.45 13.3 B	TR 0.51 14.2 B	LTR 0.12 20.7 C LTR 0.74 19.1 B	TR 0.75 19.5 B
	LTR 0.14 10.4 B	L <u>0.52</u> 21.7 C		L 0.45 19.3 B	LTR 0.06 9.8 A	L <u>0.51</u> <u>25.3</u> C
		LT <u>0.34</u> <u>12.8</u> B		<u>LT 0.18 11.0 B</u>		LT <u>0.26</u> <u>11.8</u> B
	Int. 21.7 C	Int. <u>25.4</u> C	<u>Int. 15.9 B</u>	<u>Int. 18.9 B</u>	Int. 21.1 C	Int. <u>22.2</u> C
	Ifth Avenue @ West 1		1	1 1 1 1 2 2 2 2 2 2		L 0.24 22.4 C
WB		L 0.32 23.6 C LTR 0.00 19.3 B		<u>L</u> <u>0.28</u> <u>23.2</u> <u>C</u> LTR 0.00 19.3 B		L 0.24 22.4 C LTR 0.00 19.3 B
		R 0.30 23.7 C		R 0.31 24.1 C		R 0.44 26.2 C
NB		LT 0.55 <u>14.9</u> B		LT 0.52 14.4 B		LT <u>0.72</u> 18.4 B
SB		TR <u>0.49</u> <u>13.9</u> B		<u>TR 0.28 11.5 B</u>		TR <u>0.38</u> <u>12.5</u> B
T	Ifth Avenue @ Me : 4	Int.		Int.		Int.
<i>I wel</i> NB	Ifth Avenue @ West 1	30th Street* T 0.53 1.6 A	I	I 0.76 23.7 C	ļ	T 0.68 2.9 A
IND		R 0.84 19.2 B		<u>I 0.76 23.7 C</u> R 0.79 39.6 D		R 0.67 10.2 B
SB		L 0.87 22.3 C		L 0.82 51.6 D		L 0.33 2.8 A
		T <u>0.27</u> <u>8.1</u> A		<u>I</u> <u>0.31</u> <u>12.8</u> <u>B</u>		T <u>0.37</u> <u>11.0</u> B
\square		Int. 10.0 B		<u>Int. 25.1 C</u>		Int. <u>6.1</u> A
Notes			urn; DefL: Defacto Left Tu nds per Vehicle; LOS: Le			
		Adverse Traffic Impact				

Significant Adverse Traffic Impact
 indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition

Table 17-<u>34</u> (Continued) Comparison of 2030 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

			Al	VI Pea	ak Ho	our			1 ,		Midd		_								ak Ho			-J 515
	20	30 N	lo Buil				Build		2	030 N	o Buil	_			Build		20	30 N	o Buil				Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
								LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
EB WB NB	LTR L T R LTR	0.27 0.16 0.78 1.03 0.34	13.3 26.1 68.0 20.8	B B C E C	LT R LTR	0.70 1.04 0.54	31.0 59.8 33.5	<u>C</u> E C	I I R LTR	0.18 0.66 1.01 0.31	21.2 61.4 20.4		R LTR	0.93 0.34	24.1 35.9 27.8	CIDICI	L T R LTR	0.30 0.89 1.23 0.45	22.7	В <u>D</u> F С	R LTR	0.91 1.02 0.62	49.7 37.2	
SB	LI	0.42	22.9	С		0.86 0.17		<u>D</u> A	ഥ	0.25	<u>19.9</u>	<u>B</u>	L TR	-	39.0 11.1	<u>D</u> B	LI	0.39	22.8	С		0.94 0.09		D A
	Int.		35.4	D	Int.	<u>v.11</u>	40.0	D	Int.		31.9	С	Int.	<u> </u>	29.6	C	Int.		63.7	Ε	Int.	0.00	43.6	D
		venu	e @ S	t. Cla	ir Pla	асе*																		
EB					Ţ		26.8	C							24.5	Ë						0.53		С
NB SB					L	0.91	32.6	D C					<u>R</u> L	-	42.5 32.2	<u>D</u>					L	0.77	32.0	СС
					Int.	0.00	36.4	Ď					Int.	<u> </u>	34.9	Č					Int.	0.00	30.0	Č
		h Sti	reet @	Wes			reet/St		ir Pla	ce*														
EB WB						0.88	<u>44.2</u> 27.3	D C					_		<u>37.8</u>	D						1.00	-	E
NB						0.43		D							27.7 30.6	<u>C</u>						0.99 0.96		E D
SB					T		21.1	C					_	-	21.0	C					Т	-	19.3	B
		0.11		00 . /	Int.		<u>39.0</u>	D					<u>Int.</u>		<u>30.9</u>	<u>C</u>					Int.		<u>51.4</u>	D
EB		: @ и	Vest 1	32na		et 0.63	22.3 22.3	C					<u>I</u> Int.	<u>0.41</u>	18.1 18.1	<u>В</u> В					T Int.	0.59	21.0 21.0	СС
Mid-	block	@ V	Vest 1	31st 3		t																		
WB					T Int.	0.22	15.9 15.9	B B					<u>Ι</u> Int.	0.17	15.3 15.3	<u>B</u> B					T Int.	0.24	16.1 16.1	B B
Mid-	block	@ V	Vest 1	30th :		t	10.0						11114		10.0	브	l				IIII.		10.1	D
EB						0.63	21.9	C					I	0.28	<u>16.4</u>	В	1				Т	0.29	<u>16.5</u>	<u>B</u>
					Int.		21.9	<u>C</u>					Int.		<u>16.4</u>	<u>B</u>					Int.		<u>16.5</u>	<u>B</u>
		_	Vest 1 24.5				24.5	С	lı TR	0 12	23.5	С	lı TR	0 12	23.5	С	LTR	0 16	24 1	С	lı TR	0.19	24 6	С
	LT			Ä		0.39		Ä		0.36		Ă		0.42		Ă		0.44		Ä			7.0	Ä
0.0		0.03		Α		0.03		Α		0.02	4.7	A		0.02		A		0.03		Α		0.04		Α
SB	LTR Int.	0.63	9.0 8.5	A A	Int.	0.70	10.4 9.3	B A	Int.	0.48	7.2 7.3	<u>A</u> A	Int.	0.56	8.1 7.9	A	LTR Int.	0.45	7.0 7.4	A A	Int.	0.52	7.7 7.9	A A
Broa		Nor				t 135	th Stre		11114		1.0	Δ	11116		1.0		nit.		7.7	/ \	mic.		7.0	/\
			26.2	С		0.44		С	LI	0.29	<u>25.1</u>	<u>C</u>	LI	<u>0.34</u>	<u>25.7</u>	<u>C</u>			108.8 28.3	F C		1.05 0.44	111.7 28.1	<u>E</u> C
	TR			C		0.59		Ç			<u>29.5</u>	Č				Č				С		0.62		С
NB	LTR Int.	0.54	8.2 17.1	A B	Int.	0.59	8.9 18.3	A B	Int.	0.51	<u>7.8</u> 15.9	<u>A</u> B	Int.	0.63	9.6 17.1	<u>A</u> B	Int.	0.70	10.5 27.2	B C	Int.	0.86	16.5 29.4	B C
Broa		Sou				st 135	th Str				12.2		*****											,
	TR LT			СС	DefL	0.48 0.50 0.28	34.6	CCC	_	-	<u>24.1</u> <u>25.0</u>	<u>C</u>			<u>24.4</u> <u>27.2</u>	<u>C</u>			34.2 27.6	C	DefL	0.59 0.63 0.44	42.9	СОС
SB	L			A	L	0.15	5.4	Α	_	0.11		A		0.16		A		0.08 0.55		A	L	0.08	5.0	Α
	IR Int.	U./5	12.3 16.9		Int.	0.72	11.3	B B	Int.	0.56	<u>8.5</u> 12.9	<u>A</u> B	Int.	0.56	<u>8.5</u> 14.0	<u>A</u> B	Int.	U.35	8.3 19.5	A B	Int.	0.59	8.8 19.1	A B
Broa		Nor				t 133	rd Stre						*****		<u>v</u>									1
			17.6			0 ==	00 -				<u>18.2</u>	B		0.55	04.0		LT			В		0.00	40.7	_
	TR LT			C B	TR L	0.58	26.5 19.4	<u>C</u> <u>B</u>		0.49 0.47		C B	區上		24.9 13.1	<u>C</u> B		0.93	50.9 21.1	D C	TR L	0.88	43.5 21.0	D C
מאי		0.59		A		0.86		C		0.47		A			15.5	B		0.06		A		1.00		<u>D</u>
	Int.		17.3	В	Int.		23.4	С	Int.		16.3	В	Int.		16.9	В	Int.		29.6	С	Int.		37.5	D
					⊉ Wes	st 133	rd Str	eet	Ιτο	0 14	10.2	Þ	1				Ιтο	0.40	17 0	D	1			
	TR LT		18.1 20.9	B C	LT	0.34	8.4	Α		0.14 0.43	18.2 20.5	<u>В</u> С	LT	0.25	<u>7.</u> 8	Α			17.8 30.4	B C	LT	0.43	6.8	Α
	LTR		13.2	В	TR	0.91	39.2	D	LTR		11.6	В	ĪR		26.4	С	LTR		12.6	В	TR	0.89	41.9	D
	Int.		15.0	В	Int.		28.0	C	Int.		14.2	<u>B</u>	Int.		<u>19.4</u>	<u>B</u>	Int.		19.0	В	Int.		24.1	С
Notes	s:										facto Le cle; LOS					1								
		4	- Signif	icant /	Adver	se Tra	iffic Imp	act																
		*	indicat	tes int	ersec	tions v	vhich w	ere u	nsign	alized	in the N	No Bi	uild co	nditio	n and s	ignal	ized ir	the E	Build co	nditio	on			

Table 17-<u>34</u> (Continued) Comparison of 2030 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

			ΛR	/ Por	ak Ho	NIIP.		11116	41 y 1	Jiu	Midd		_		cu III	ter s	ccu	OII I	Level		ak Ho		Anai	ysis
	20)30 N	AN Io Buil		an mo		Build		20	030 N	wiidd Io Buil	_			Build		2	030 N	lo Buil		או חס		Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
		V/C	(spv)			V/C	(spv)	LOS	Grp				Grp		(spv)		Grp				Grp	V/C	(spv)	LOS
			thbou						Li	U 34	170	D		0 20	15.6	Þ	Li	U 33	19.0	ъΙ		0.49	20.5	C
			20.7 12.0	C B	L		19.8 12.2	B B			17.8 11.3	<u>В</u> В			<u>15.6</u> 13.9	<u>В</u> В			19.0 14.3	B B		0.49		C B
	Int.		14.2		Int.		14.8	В	Int.		12.7	B	Int.		14.5	<u>B</u>	Int.		15.1	В	Int.		16.4	В
			ıthbou								04 -	_	·		40 :	_				_				_
FB	IR	0.63	28.7	С		0.58 0.58		C C	TR	0.34	<u>21.0</u>	<u>C</u>			18.1 19.5	<u>В</u> В	IR	0.49	23.5	C		0.65 0.38		C C
WB	LT	0.06	15.8	В	I.V.	0.56	32.0	C	LT	0.04	15.6	В	₽	0.52	13.0	D	LT	0.04	15.6	В	I.	0.50	24.4	C
			13.3	В	LT	0.62	14.7	В				В	LI	0.56		В			12.2	В	LT	0.51	12.9	В
	Int.		17.4	В	Int.		19.8	В	Int.		<u>14.1</u>	<u>B</u>	Int.		<u>17.0</u>	<u>B</u>	Int.		15.5	В	Int.		19.4	В
			Vest 1 : 21.8	31 st (Stree	et			lı TR	0 25	22.3	C	ı				lı TR	0 22	21.9	С	ı			
			20.8	Ċ							20.7	C							21.6	Ċ				
				В							19.6	В							19.8	В				
			9.0	Α						0.32		A							10.0	<u>B</u>				
SB	Int.	U.54	10.3 10.9	B B					LIK Int.	0.40	<u>9.0</u> 10.3	<u>A</u> B					Int.	0.45	9.4 11.0	A B				
Broa		/ Nor	thbou		Wes	st 131:	st Stre	et				<u> </u>	·						11.0	٦	·			
EB				Ī	Т	0.17	21.4	С							19.9	<u>B</u>						0.09		С
WB						0.11		C B							<u>20.4</u>	C						0.21		C B
NB					Int.	0.62	12.8	В					Int.	0.51	10.0 11.1	<u>В</u> В					Int.	0.76	15.1	В
Broa	dway	/ Soi	ıthbou	nd @		st 131			1				1116										10.1	
WB	•			Ì	LT	0.19	21.2	С					_	-	<u>21.7</u>	<u>C</u>						0.31		С
SB						0.79		В						0.59	11.3	B						0.65		В
Bros	dwa	/ @ L	Vest 1	30th	Int.	of.	17.0	В	L				Int.		<u>13.0</u>	<u>B</u>					Int.		14.8	В
			26.0				24.6	С	LR	0.20	24.6	С	L	0.23	24.3	C	LR	0.34	27.0	С	L	0.48	25.8	С
					R	0.45	30.3	С				_	R	0.47	30.8	С					R	0.79	42.9	D
	LT			A	T	0.42		A		0.23		A		0.34		A		0.35		A	-	0.48		A
SB	LT Int.	υ. 4 0	6.5 7.4	A A	LI Int.	0.56	8.2 10.4	A B	LI Int.	0.31	6.0 6.9	<u>A</u> A	Int.	0.46	<u>7.2</u> 10.6	<u>A</u> B	LI Int.	0.35	6.2 7.7	A A	Int.	0.60	10.7 15.9	B B
Broa		/ @ I	Vest 12			et																		
	LT	0.45	19.5	В	LT	0.49	20.6				<u>19.2</u>	<u>B</u>			<u>19.6</u>	<u>B</u>			22.2	С		0.67		С
NP			16.4 43.5	B D		0.27 0.68		B C		0.19	<u>16.1</u> 18.0	<u>В</u> В			<u>17.1</u> 20.4	<u>В</u> С			17.1 25.3	B C		0.40		C C
מאו			19.7	В	_ '	0.00	<u> </u>	U	===	<u>v.+ 1</u>	10.0	브	===	<u> </u>	<u> 20.4</u>	$\stackrel{\smile}{=}$		5.74	<u> </u>	0	- '	0.50	55.0	U
SB			18.5	В	TR	0.97	40.7	D	IR	0.47	18.6	В	IR	0.81	28.3	<u>C</u>	TR	0.52	19.3	В	TR	0.98	44.2	D
	Int.		20.1	С	Int.		30.6	С	Int.		18.4	В	Int.		23.7	С	Int.		22.0	С	Int.		<u>36.6</u>	D
			<i>Vest 12</i> 415.5		Stree L		286.4	F	Li	0.64	43.8	D	L	0 43	26.2	C	l i	0.41	31.2	С	Li	0.53	44.5	D
			42.5	D		0.51		C			73.3	<u>D</u> E			21.6	C			<u>31.2</u> 112.9	F		0.53		
				_	R	0.73	26.3	C			·		R	0.75	23.4	C					R	1.08	93.1	<u>C</u> <u>E</u>
WB			235.1		L		115.2	F			136.4	Ē	L		30.4	<u>C</u>			<u>167.5</u>		L		109.4	Ē
	iΚ	1.05	<u>74.0</u>	Е	T R	0.84	36.5 32.4	<u>D</u>	TR	<u>u./8</u>	<u>33.5</u>	<u>C</u>			22.1 16.7	<u>С</u> В			27.4 32.7	<u>C</u>		<u>0.75</u> 0.74		<u>C</u>
NB	L	0.50	33.5	С	L		38.2	D	L	0.43	32.3	С	L	0.53	39.7	D		-	38.4	D	L	0.67		D
	LT	0.52	32.2	С	Т	0.37	24.8	С	LΤ	0.51	32.2		I	0.44	28.1 52.6	С	LT	0.97	62.1	Ε	Т	0.66	28.3	<u>C</u>
0.0			47.7	D		0.71		D			54.4	D	R	0.74	52.6	D			47.2	D		0.66		
SB			31.1 <u>101.4</u>	C F	L T		38.3 33.1	D C		0.37	30.7 44 9	<u>C</u> D	Ī	-	<u>44.1</u> 31.8	<u>D</u> C			31.5 62.6	C E	L	0.76 <u>0.54</u>		D C
		1.11	101.7	•	R		24.7	C		<u> </u>	<u> </u>	_	B		30.3	<u>C</u>		<u>J.J.</u>	<u>02.0</u>	=	R	0.46		č
	Int.		<u>85.1</u>	Ε	Int.		<u>44.8</u>	Ď	Int.		50.3	D	Int.		28.7	Ĉ	Int.		65.5	Ε	Int.		39.9	Ď
			venue					0	lı-	0.05	25.5	_	Lit	0.05	27.0	0	lı÷	0.05	20.0	ь і	Lit	1.40	100.0	г.
FR			28.8 30.6	C		0.55	32.1 37.1	C D		0.25	25.5 26.2	<u> </u>			27.6 32.3	<u>C</u>			39.0 26.9	D C		1.12 0.46	122.9 31 4	F + C
WB			29.6	C		0.50		C			33.2	Č			34.4	C			39.1	D			60.9	
NB	LTR	0.53	8.4	Α	LTR	0.58	9.4	Α		0.60		<u>A</u>			12.0	В	LTR	0.76	13.4	В	LTR	0.82	16.3	В
SB	LTR	0.62		Α		0.72		В		0.53		Δ		0.59		A		0.61				0.67		В
Note	Int.	-	14.2	B	Int.	ouch:	16.6	B + Tur	Int.	I · De	13.7	B of Tu	Int.	t · Int-	16.0 rsection	<u>B</u>	Int.		18.0	В	Int.		33.5	С
Notes	5.		⊥: Leπ τ //C: Vol													1								
		+	- Signifi	cant /	Adver	se Tra	ffic Imp	act	•															
		*	indicat	es int	ersec	tions v	vhich w	ere u	nsign	alized	in the 1	Νο Βι	uild co	nditio	n and s	ianal	zed ir	n the E	Build co	nditio	n			

^{*} indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition

Table 17-<u>34</u> (Continued) Comparison of 2030 No Build and Build Conditions Primary Study Area Signalized Intersection Level-of-Service Analysis

									• -	Juan	ıy Aı	· ·	/1511	WIII.	- TI	CLI	CCU	UII I	30 1 01	01	JUI .	100	1 11141	Jose
			A۱	/I Pea	ık Ho	ur					Mido	lay P	eak l	lour					PN	/ Pea	k Ho	ur		
	20	030 N	lo Buil	d		2030	Build		2	030 N	lo Buil	d		2030	Build		20	030 N	lo Buil	d		2030	Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
Int.	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
Ams	terda	ım A	/enue	@ W	est 12	25th \$	Street																	
EB																								
		<u>1.18</u>	125.8	F	IR	1.33	188.6	F+	IR	<u>1.19</u>	130.8	E	IR	1.36	201.1	E±	IR	<u>1.14</u>	108.5	F	<u>IR</u>	1.44	235.8	F+
WB																								
	_	=	139.4	F	IR	=	260.7	F+	IR	1.00	64.3	<u>E</u>	IR	1.15	113.3	E±		1.02	66.3	<u>E</u>	<u>IR</u>	=	109.4	<u>E</u> +
NB		0.39		В	L	0.47	18.9	В	L	0.08	9.2	Α		0.11	9.6	<u>A</u> C		0.27		<u>C</u>	L	0.31	21.6	<u>C</u> <u>E</u>
		0.82		С	TR	0.82		С	<u>TR</u>	0.62		<u>A</u> C			23.3			<u>1.07</u>		<u>E</u>	TR	<u>1.07</u>		<u>E</u>
SB		0.60	<u>31.6</u>	С	L	0.60		С	L	0.69	30.1	<u>C</u>	_	0.68		C		0.81	54.2	D	L	0.81	<u>54.3</u>	D
		0.53		С		0.53		С	_	0.37		В		0.37	<u>18.8</u>	В		0.98	<u>63.8</u>	<u>E</u>		0.99		Ē
	Int.		<u>79.4</u>	Е	Int.		<u>134.9</u>	F	<u>Int.</u>		<u>65.4</u>	<u>E</u>	<u>Int.</u>		<u>102.4</u>	<u>E</u>	Int.		<u>77.1</u>	Е	Int.		<u>126.5</u>	F
Note	s:		.: Left T												rsectio	n; V/C	: Volu	ıme to	Capa	city; s	pv: S	econd	ls per	
			/ehicle;																					
		*	indicat	es int	ersect	ions v	vhich w	rere u	nsign	alized	in the I	Νο Βι	ıild co	nditio	n and s	signali	zed ir	the E	Build co	nditio	n			

Table 17-<u>35</u>
Comparison of 2030 No Build and Build Conditions
Primary Study Area Unsignalized Intersection Level-of-Service Analysis

							Prii	nar	y St	udy	Area	Uns	sign	aliz	ed Ir	ter	sect	ion l	Level	l-of-	Serv	vice	Anal	lysis
			ΑN	/l Pea	ak Ho						Midd										ak Ho			
	2		lo Buil	d			Build			2030 1	No Buil				Build		2		lo Buil			2030	Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
								LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
			et @ N		133rd	Stre	et*			o 4=	40.0	_							40.0	_				
			10.8 122.4						l E	<u>0.17</u> 1.02	10.3 60.0	<u>В</u> Е							10.6 125.9	B F				
			<u>122.4</u> et @ И		12222	1 0400	no+*		LT	1.02	00.0	ᆫ					ı	1.22	125.9	Г				
WB		- -	9.2	A	132110	ı sue	eı		Li	_	9.7	Δ	i				ΙL	_	10.0	Δ	i			
	LT	_	96.1	F					블	=	23.8						LT	_	96.5	<u>A</u> F				
00	T T	_	8.3	A					〒	≘ ≘	8.4	Ä					T T	-	10.5	В				
	Int.		79.5	F					Int.	=	19.8	C					Int.		70.2	F				
Mar	ginal	Stree	et @ S	t. Cla	air Pla	ісе*							•											
SB		0.20		Α					L T	0.24	10.2 37.7	<u>B</u> E							10.8	В				
		1.01		Ε					I	0.92	<u>37.7</u>	<u>E</u>					Т	1.22	<u>128.5</u>	F				
			ıe @ V		131st	Stre	et*																	
		0.01		Α						0.03	<u>7.5</u>	A						0.02		Α				
		0.18		В						0.06	10.2	B						0.07		В				
I NAR	LIK	0.44	40.8 20.3	E						0.30	28.2 15.5	<u>D</u> C							105.9 19.9	F C				
					125th	Stro	of So	ıthhı			t Turn*		l				LIK	0.03	19.9	C				
			11.9		12301	Sue	el 30i	JUIDO		0.06	10.9	В	I				R	0.08	11.7	В	ı			
			ie @ S		air Pla	ace*			11	0.00	10.5		<u> </u>				- 1 \	0.00	1111		1			
	ΙΤ	-	10.4	В		100			ΙT	=	10.9	<u>B</u>	l				ΙТ	-	11.9	В	ı			
	Ŕ	_	16.8	C					I R	=	16.9	C					Ŕ	-	14.4	В				
SB	L	-	9.2	Α					L	=	9.0	<u>C</u>					L	-	9.0	Α				
	Int.		<u>15.3</u>	С					Int.		15.3	С					Int.		13.5	В				
			re @ S		air Pla	асе*																		
		0.14		Α						0.14	7.8	<u>A</u> <u>E</u>						0.19		Α				
SB		0.49		E						0.45	<u>39.4</u>	Ē							<u>41.4</u>	E				
<u></u>		0.46		E					I	0.36	<u>34.6</u>	<u>D</u>					Т	0.30	<u>39.0</u>	Ε	<u> </u>			
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	R	-	0.4 7.4	A	L R	-	7.6	A	E	Ξ	<u>8.2</u> <u>7.2</u>	<u>*</u>	I⊨	Ξ	8.3 7.3 9.8	<u>A</u> <u>A</u>	L R	-	8.4 7.3	A A	L R	-	7.5	A
	L	-	10.4	A B	Ĺ	-	10.5	A B	<u> </u>	= =	9.8	<u> </u>	7	=	<u>7.3</u>	<u>A</u>	L	-	10.4	В	L	-	10.6	A B
	-	-	9.0	A	T	-	9.3	A		=	9.0	<u>A</u> <u>A</u> <u>A</u> <u>A</u> A	‡	= = =	9.0	A	T	-	8.4	A	Ϊ́Τ	_	8.5	A
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Wes		th St	reet @			th St		t. Cl	air Pl	ace*/	**								-					
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			26.7	D							23.5	C	l				R		74.8	F				
WB			208.0	F						0.13	96.9		l				L		12119	F				
	R		73.3	F							<u>46.1</u>	_=							621.2	F	<u>L</u>			
Note	s:										ction; V/0													

^{**} The unsignalized intersection analysis procedure, which assumes random arrival, breaks down with high major or minor street volumes and reports exaggerated levels of stop delays. As such, its results are not necessarily indicative of actual conditions, where gaps created by adjacent signalized intersections often provide additional throughput capacity and result in lower stop delays.

Table 17-<u>36</u>
Comparison of 2030 No Build and Build Conditions
Secondary Study Area Signalized Intersection Level-of-Service Analysis

			ΔN	l Pes	ık Ho	ur	beco	iiu	11 y K	Jiuu	•		eak l		ed In	ici s		OII I			ak Ho		1 MIICE	Jord
l	20	30 N	o Buile				Build		20	030 N	o Buil	_	JUN I		Build		20	030 N	lo Buil		an IIC		Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	
							(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
	•		Vest 1 1 25.4				25.4	C	Іть	n 21	21.0	C	lı тр	0.21	21.9	<u>C</u>	lı тр	U 28	22.0	С	lı тр	U 28	22.0	С
			118.6				118.6	F	DefL	0.66	21.9 34.3	င			34.3	C			72.4	E			72.4	Ē
	TR	0.62	33.1	С			33.1	С	IR	0.45	27.0	<u>C</u>	IR	0.45	27.0	С	TR	0.90	57.1	Ε			57.1	Ε
								В			<u>13.5</u>				<u>13.8</u>				<u>16.0</u>	В			<u>16.2</u>	В
SB	LIR Int.	0.79	23.4 31.3	C	Int.	0.81	24.1 31.4	C		0.56	14.0 17.4	<u>B</u>	LIK Int.	0.58	<u>14.3</u> 17.5	<u>В</u> В	Int.	0.65	19.4 27.3	B C	Int.	0.67	19.7 27.4	B C
Broa		/ @ V	Vest 12			t	31.4	<u> </u>	<u>Int.</u>		17.4	Ь	шь		17.5	D	IIIL.		21.3	C	IIIL.		21.4	C
		0.46					28.3	С	LTR	0.34	26.8	C	LTR	0.34	26.8	<u>C</u>	LTR	0.36	26.9	С	LTR	0.36	27.0	С
			220.2				209.4	F	<u>LTR</u>	0.93	<u>70.4</u>	<u>E</u>			<u>70.4</u>	<u>E</u>				F			102.9	F
		0.42				0.45		A		0.40		A			6.3			0.52		A		0.54		A
28		0.65				0.67		A		0.48		AR		0.51		_	LTR	0.52	7.3 21.7	A		0.55		A C
Ams		m Av				20th \$			11116		<u> 11.0</u>	_=_	<u> 1111C.</u>		10.1	<u> </u>	i iii.c.		<u> </u>		mic.		<u> </u>	
				D				D	L	0.54	43.7		L	0.54	43.7	D	L	0.61	48.5	D	L	0.61	48.5	D
WB															Ε									
WB															С									
NB LTR 0.68 6.8 A LTR 0.69 7.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.36 3.3 A LTR 0.37 3.4 A LTR 0.39 3.5 A LTR 0.39 3.5 Int. 24.3 C Int. 19.9 B Int. 13.3 B Int. 13.2 B Int. 21.9 C Int. 22.3																								
NB LTR 0.68 6.8 A LTR 0.69 7.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.37 3.4 A LTR 0.37 3.5 LTR 0.39 3.5 A LTR 0.39 3.5 LTR															Ā									
SB LTR 0.62 5.8 A LTR 0.64 6.0 A LTR 0.36 3.3 A LTR 0.37 3.4 A LTR 0.39 3.5 A LT															С									
Int. 24.3 C Int. 19.9 B Int. 13.3 B Int. 13.2 B Int. 21.9 C Int. 22.3 C																								
			21.0 26.4					뜯				불								읃				片
								胺				E	_											
110												D						<u> </u>		Ŭ		<u>v.vv</u>	<u>=0.0</u>	Ŭ
SB					LT	0.64	19.0	В				C				С	LT	0.53	19.9	В	LT	0.53	19.9	В
		0.26				0.26			_	0.44		C	_	0.44		<u>C</u>		0.37		В		0.37		В
Madi	NB LTR 0.68 6.8 A LTR 0.69 7.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.36 3.3 A LTR 0.37 3.4 A LTR 0.39 3.5 A L														<u>C</u>									
	NB LTR 0.68 6.8 A LTR 0.69 7.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR														F+									
TR 1.06 1.08 1.0 L 0.52 4.3.7 D L 0.54 43.7 D L 0.61 48.5 D L 0.61 48.5 D L 0.61 48.5 D TR 0.66 76.5 E TR 0.75 53.0 D TR 0.84 60.9 E TR 0.61 48.5 D TR 0.84 60.3 E TR 0.84 60.9 E TR 0.84 60.3 E TR 0.84 60.9 E TR 0.84 60.3 E TR 0.84 E TR																								
NB	NB LTR 0.68 6.8 A LTR 0.69 7.0 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.63 5.8 A LTR 0.63 5.8 A LTR 0.64 6.0 A LTR 0.63 6.3 3.4 A LTR 0.63 3.5 A LTR 0.64 1.5 A LTR 0.64 1.5 B LTR 0.64 1.5 B LTR 0.64 1.5 B LTR 0.64 1.5 B LTR 0.65 1.7 B LTR 0.66 1.8 B LTR 0.67 1.8 B																							
NB LT 0.28 16.5 B LT 0.28 16.5 B LT 0.50 25.4 C LT 0.50 25.4 C LTR 0.86 28.8 C LTR 0.86 28.8 C R 0.30 17.7 B R 0.30 17.7 B R 0.65 38.1 D R 0.6														<u>E</u>										
								F±	ΙT	1 41	228 1	F	Т	1 48	258 N	F±	ΙT	1 30	175 O	F	ΙT	1 45	242 4	F±
												₽												
WB												Ē												_
									l .				١			_	١.		-	_	١.		40:-	_
SB									_			<u>C</u>				<u>C</u>								
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3,,										<u> </u>				<u> </u>										
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NID			20.4 18.9	<u>C</u> B				<u>С</u> В				<u>C</u>												С
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			10.9	В	Ŕ		10.9	В			11.2	B	Ē		11.2	B			15.0	В			15.0	В
	Int.		<u>44.3</u>		Int.		44.2	D	<u>Int.</u>		19.5	В	Int.		19.6	В	Int.		33.1	С	Int.		<u>42.2</u>	D
	•		Vest 14				05.0	_	L ==	0.00	00.0	_	l		00.0	_	L	0.00	00.0	_	l	0.00	00.0	_
			25.6 62.8	C E			25.6 64.2	C E			26.2 89.1	<u>C</u> <u>E</u>			<u>26.2</u> 102.0				26.0 71.7	C E			26.0 79.1	C E±
			21.5	C			22.4	C			24.7	C			27.0				33.5	C			37.0	⊏ <u>±</u> D
			10.1	В	L		10.2	В			13.5	B	Ī	0.41	14.1	B			16.9	В			17.5	В
		0.51	11.7	В		0.55	12.2	В		0.58	12.6	В		0.62	13.4	B		0.51	11.6	В		0.53	11.8	В
Nett	Int. 26.0 C Int. 26.5 C Int. 31.1 C Int. 34.2 C Int. 33.1 C Int. 36.0 D Notes: L: Left Turn; T: Through; R: Right Turn; DefL: Defacto Left Turn; Int.: Intersection V/C: Volume to Capacity; spv: Seconds per Vehicle; LOS: Level of Service																							
Notes	: :															1								
Ī	+ Significant Adverse Traffic Impact																							
1	* indicates intersections which were unsignalized in the No Build condition and signalized in the Build condition																							

Table 17-<u>37</u>
Comparison of 2030 No Build and Build Conditions
Secondary Study Area Unsignalized Intersection Level-of-Service Analysis

			Α	M Pea	ak Ho	ur					Midd	ay Pe	eak H	lour					Р	M Pe	ak Ho	our		
	2	030 N	lo Bui	ld		2030	Build		- 2	2030	No Buil	d		2030	Build		20	030 N	lo Bui	ild		2030	Build	
	Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay		Ln		Delay	7	Ln		Delay	
Int.	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS	Grp	V/C	(spv)	LOS
Firs	Ave	enue	@ Eas	t 125	th St	reet S	South	bound	l Rig	ht Tu	rn													
SB	R	0.40	13.6	В	R	0.45	14.5	В	R	0.19	10.0	<u>B</u>	<u>R</u>	0.21	10.2	<u>B</u>	R	0.22	10.3	<u>B</u>	R	0.24	10.4	<u>B</u>
Note	s:		R: Rigl	nt Turr	n; V/C	: Volu	me to	Capac	ity; s	pv: Se	conds p	er Vel	hicle;	LOS:	Level	of Ser	vice							

Amsterdam Avenue and 135th Street

During the PM peak hour, the eastbound left-through movement would deteriorate from LOS D to LOS F, with delay increasing from 39.0 to 122.9 seconds and v/c ratio increasing from 0.65 to 1.12. The westbound left-through-right movement would deteriorate from LOS D to LOS E, with delay increasing from 39.1 to 60.9 and v/c ratio increasing from 0.70 to 0.89.

SECONDARY STUDY AREA

Madison Avenue and East 125th Street

During the AM peak hour, the westbound approach would deteriorate from LOS C to beyond mid-LOS D, with delay increasing from 32.0 to 47.8 seconds and v/c ratio increasing from 0.85 to 0.97.

During the PM peak hour, the eastbound approach would <u>deteriorate from LOS E to LOS F</u>, with delay increasing <u>56.6</u> to <u>114.1</u> seconds and v/c ratio increasing from <u>1.02</u> to <u>1.18</u>.

Second Avenue and East 125th Street

During the AM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from $\underline{290.0}$ to $\underline{310.7}$ seconds and v/c ratio increasing from $\underline{1.55}$ to $\underline{1.59}$. The westbound through movement would continue to operate at LOS F, with delay increasing from $\underline{251.3}$ to $\underline{329.3}$ and v/c ratio increasing from $\underline{1.44}$ to $\underline{1.62}$. The southwestbound throughright movement would continue to operate at LOS F, with delay increasing from $\underline{129.1}$ to $\underline{186.0}$ seconds and v/c ratio increasing from $\underline{1.18}$ to $\underline{1.31}$.

During the midday peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from 228.1 to 258.0 seconds and v/c ratio increasing from 1.41 to 1.48, and the eastbound right-turn movement would deteriorate beyond mid-LOS D, with delay increasing from 39.3 to 45.1 and v/c ratio increasing from 0.56 to 0.67. The westbound approach would continue to operate at LOS E, with delay increasing from 59.0 to 71.8 and v/c ratio increasing from 0.87 to 0.95. The southwestbound through-right movement would continue to operate at LOS D, with delay increasing from 45.3 to 50.6 seconds and v/c ratio increasing from 0.81 to 0.88.

During the PM peak hour, the eastbound through movement would continue to operate at LOS F, with delay increasing from $\underline{175.0}$ to $\underline{242.4}$ seconds and v/c ratio increasing from $\underline{1.30}$ to $\underline{1.45}$. The westbound approach would deteriorate from LOS D to LOS E, with delay increasing from $\underline{53.4}$ to $\underline{62.1}$ and v/c ratio increasing from $\underline{0.86}$ to $\underline{0.92}$. The southwestbound through-right movement would continue to operate at LOS F, with delay increasing from $\underline{90.8}$ to $\underline{109.9}$ seconds and v/c ratio increasing from $\underline{1.07}$ to $\underline{1.12}$.

Broadway and West 145th Street

<u>During the midday peak hour, the westbound approach would continue to operate at LOS F, with delay increasing from 89.1 to 102.0 seconds and v/c ratio increasing from 1.05 to 1.09.</u>

During the PM peak hour, the westbound approach would continue to operate at LOS E, with delay increasing from 71.7 to 79.1 seconds and v/c ratio increasing from 1.00 to 1.03.

First Avenue and East 125th Street

During the PM peak hour, the eastbound left-turn movement would continue to operate at LOS F, with delay increasing from $\underline{106.7}$ to $\underline{145.4}$ seconds and v/c ratio increasing from $\underline{1.13}$ to $\underline{1.23}$.

SIGNIFICANT IMPACTS

Within the primary study area, the Proposed Actions would result in significant adverse impacts on the follow intersections:

AM Peak Hour

Amsterdam Avenue and West 125th Street

Midday Peak Hour

• Amsterdam Avenue and West 125th Street

PM Peak Hour

- Riverside Drive and West 135th Street
- Amsterdam Avenue and West 135th Street
- Amsterdam Avenue and West 125th Street

Within the secondary study area, the Proposed Actions would result in significant adverse impacts on the following intersections:

AM Peak Hour

- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street

Midday Peak Hour

- Second Avenue and East 125th Street
- Broadway and West 145th Street

PM Peak Hour

- Broadway and West 145th Street
- Madison Avenue and East 125th Street
- Second Avenue and East 125th Street
- First Avenue and East 125th Street

As with the 2015 Build analysis, the projected traffic impacts were identified primarily at intersections along 125th Street. These intersections were identified to already operate at congested levels during peak hours in the future without the Proposed Actions. As mentioned earlier, NYCDCP and NYCDOT are currently conducting an areawide transportation study in

the effort to improve vehicular and pedestrian travel in the Harlem section of Manhattan. While this study, when completed, may recommend corridor-wide strategies in easing congestion along 125th Street, potential measures to mitigate the significant adverse impacts identified above are presented in Chapter 23.

BUILD WITHOUT PROJECT-RELATED ROADWAY IMPROVEMENTS

Similar to the 2015 Build condition, an analysis was performed for a Build scenario that does not include the project-related improvements. This analysis compared traffic impacts for 2030 Build conditions assuming the operations of the roadway network with the existing geometric configuration with traffic impacts for 2030 Build conditions assuming the roadway network incorporating improvements recommended as part of the Proposed Actions. The comparison demonstrated how the recommended improvements, including the conversion of two-way streets to one-way couples, would substantially improve overall network operations and circulation, and not yield significant adverse impacts at the Project Area intersections. Furthermore, the comparison analysis confirmed that these improvements are both logical and practical as part of the comprehensive, community-based planning approach of the design process. In the alternative analysis, West 133rd Street between Broadway and Twelfth Avenue, West 132nd Street between Broadway and Marginal Street, West 131st Street between Broadway and Twelfth Avenue, and West 125th Street between Twelfth Avenue and Marginal Street would continue to operate as two-way, east-west streets. All project-generated traffic was routed to conform to this existing street network. In addition, traffic generated by the Harlem Piers project was also routed to conform to the existing two-way street network. Significantly impacted intersections within the Project Area are indicated in Table 17-38.

Table 17-<u>38</u>
Significantly Impacted Intersections within the Project Area in 2030 Build Condition without Improvements

	1					
	Build wit	hout Impro	vements	Build w	ith Improv	ements
Project Area Intersections	AM Peak	MD Peak	PM Peak	AM Peak	MD Peak	PM Peak
Signalized Intersections						
Marginal Street @ West 125th Street						
Twelfth Avenue @ West 133rd Street						
Twelfth Avenue @ West 132nd Street						
Twelfth Avenue @ West 125th Street						
Broadway NB @ West 133rd Street						
Broadway SB @ West 133rd Street						
Broadway NB @West 132nd Street						
Broadway SB @ West 132nd Street						
Broadway @ West 131st Street						
Broadway @ West 130th Street						
Broadway @ West 129th Street						
Broadway @ West 125th Street						
Unsignalized Intersections						
Marginal Street @ West 133rd Street						
Marginal Street @ West 132nd Street						
Marginal Street @ St. Clair Place						
Twelfth Avenue @ West 131st Street						
Twelfth Avenue @ St. Clair Place						
Riverside Drive @ St. Clair Place						
West 125th St. @ West 129th St./St. Clair Pl.						
Notes: Significantly Impacted Ir	itersections					

The detailed findings of this alternative analysis are presented in Appendix M. Since traffic volumes and operating characteristics at the analysis locations outside of the Project Area would be the same with or without the improvements, conditions at these locations have been previously discussed and are not summarized below or addressed in Appendix M.

PARKING SUPPLY AND UTILIZATION

COLUMBIA UNIVERSITY PARKING

Four below-grade parking facilities, two located east of Broadway between West 131st and West 134th Streets and two located west of Broadway between West 125th and West 133rd Streets, would be constructed to accommodate Columbia University parking demand. Up to approximately 2,300 total spaces, comprising a mix of self-park and valet spaces, would be provided within these parking facilities. A parking accumulation analysis was performed to ensure that the off-street parking supply provided as part of the Proposed Actions would be adequate to meet the projected demand. This analysis shows that the peak weekday parking demand of 1,997 spaces could be adequately accommodated by the above supply and there would also be an on-site surplus of approximately 300 spaces.

ON-STREET PARKING

Table 17-39 shows the expected on-street parking utilization in the 2030 Build condition. The 2030 Build analysis included additional demand for on-street parking from non-Columbia University project-generated vehicles (Subdistrict B and "Other Area"). Roadway modifications that would occur as part of the Proposed Actions would create additional on-street parking spaces on certain blocks and remove spaces from others, resulting in a net loss of 67 on-street parking spaces in the 2030 Build condition. The project-related modifications to on-street parking include:

- Addition of a parking lane on the south side of West 133rd Street between Broadway and Twelfth Avenue;
- Addition of a parking lane on the south side of West 132nd Street between Broadway and Twelfth Avenue;
- Addition of a parking lane on the north side of West 131st Street between Broadway and Twelfth Avenue;
- Removal of the parking lane on the north side of West 130th Street between Broadway and Twelfth Avenue;
- Removal of parking lanes on the north and south sides of West 129th Street between Broadway and West 125th Street;
- Removal of a parking lane on the west side of Broadway between West 129th Street and West 125th Street;
- Removal of a parking lane on the north side of West 125th Street between Old Broadway and Broadway;
- Removal of parking lanes on the east and west sides of Twelfth Avenue between West 125th Street and St. Clair Place; and

Table 17-<u>39</u> 2030 Ruild On-Street Parking Utilization Summary

2030 Build On-Str	eet Parking	g Utilization	Summary
2030 Build	AM	MD	PM
1/4-Mile Radiu	S		
Capacity	1847	1847	1847
Spaces Removed due to Geometric Modifications	67	67	67
Effective 2030 Build Capacity	1780	1780	1780
2030 No Build Demand	1586	1643	1595
Subdistrict B and "Other Area" Demand	136	201	142
2030 Build Demand	1722	1844	1737
Remaining Spaces	58	-64	43
Utilization	97%	104%	98%
% No Build Availability Utilized in Build Condition	78%	131%	83%
½-Mile Radiu	s		
Capacity	4783	4783	4783
Spaces Removed due to Geometric Modifications	67	67	67
Effective 2030 Build Capacity	4716	4716	4716
2030 No Build Demand	3893	4207	4291
Subdistrict B and "Other Area" Demand	136	201	142
2030 Build Demand	4032	4418	4437
Remaining Spaces	684	298	280
Utilization	86%	94%	94%
% No Build Availability Utilized in Build Condition	23%	47%	42%
Note: See Appendix H for detailed parking accumulation	on analysis.		

 Addition of a parking lane on Twelfth Avenue between West 133rd Street and West 132nd Street.

Within ¼ mile of the Project Area, on-street parking would be 97, 104, and 98 percent utilized during the morning, midday, and evening periods, respectively, in the 2030 Build condition (compared with 86, 89, and 86 percent utilization for the same time periods under the 2030 No Build condition). Spaces available in the 2030 No Build condition would be 78, 131, and 83 percent utilized in the 2030 Build condition during the morning, midday, and evening periods, respectively. Since these utilization levels exceed 50 percent, a larger ½-mile on-street area was evaluated. Overall, within ½ mile, on-street parking would be 86, 94, and 94 percent utilized during the morning, midday, and evening periods, respectively, in the 2030 Build condition (compared with 81, 88, and 90 percent utilization for the same time periods under the 2030 No Build condition). Spaces available in the 2030 No Build condition would be 23, 46, and 42 percent utilized in the 2030 Build condition during the morning, midday, and evening periods, respectively. Since less than half of the on-street parking capacity available in the 2030 No Build condition would be utilized in the Build condition, no significant adverse impacts to on-street parking conditions would occur.

OFF-STREET PARKING

As with the 2015 parking analysis, a credit of 16 percent was applied to parking facilities within the Project Area to account for the displacement of existing generators in the Project Area or the relocation of existing parking demand to other areas. In addition, the replacement of No Build projects with components of the Proposed Actions and the reassignment of parking demand from No Build Columbia University projects within the Project Area to the on-site below-grade parking facilities would result in the parking demand reduction of 259 spaces in the morning

peak period, 381 spaces in the midday peak period, and 186 spaces in the evening peak period at area off-street parking facilities.

Five parking facilities, as shown in Table 17-40, would be displaced by the proposed development in the 2030 Build condition. The remaining two public parking facilities located within ¼ mile of the Project Area would be 188, 172, and 189 percent utilized, with expected shortfalls of 572, 470, and 578 spaces during the morning, midday, and evening periods, respectively. The four public parking facilities that would remain within ½ mile would be 155, 150, and 145 percent utilized, with expected shortfalls of 564, 519, and 469 spaces during the morning, midday, and evening periods, respectively. Overall, within one mile, the remaining 21 public parking facilities would be 115, 112, and 89 percent utilized during the morning, midday, and evening periods, respectively. The resulting parking shortfalls would be 421 spaces during the morning and 339 spaces during the midday.

A portion of the above off-street parking shortfalls is attributable to the parking demand generated by existing Columbia personnel. Based on information provided by Columbia University, two of the six parking facilities (numbers 1 and 3) that are expected to be displaced by the proposed development currently accommodate monthly parking for approximately 160 vehicles belonging to Columbia University affiliates. With these spaces no longer available as a result of the Proposed Actions, Columbia University would seek other off-street facilities to replenish the lost supply. This relocation of existing parking locations by Columbia University affiliates was included in the above parking analysis results. In addition to these 160 spaces, another 140 vehicles belonging to Columbia University affiliates are currently accommodated on a monthly basis at a parking garage (number 24) a few blocks south of the Project Area. In total, there are approximately 300 off-street parking spaces near or within the Project Area that are currently secured with leases by Columbia University for some of its students and employees. As discussed above, upon completion of the below-grade parking facilities at Manhattanville, there would be a surplus of approximately 500 parking spaces, which allows Columbia University to relinquish its leases for these 300 off-site spaces and to instead accommodate this demand within the Manhattanville campus. Therefore, while there would still be an off-street parking shortfall resulting from the Proposed Actions, this shortfall would be just over 120 spaces during the morning period and less than 50 spaces during the midday period. It is possible that, as future parking demand increases, market forces could result in the construction of new pubic parking facilities in the area. However, since there is no certainty in these facilities being constructed, in accordance with guidance of the CEQR Technical Manual, the projected parking shortfall, while relatively small compared with the total supply in the area, would constitute a significant adverse parking impact. Potential measures to mitigate this impact are discussed in Chapter 23, "Mitigation."

Table 17-<u>40</u> 2030 Build Off-Street Parking Utilization Summary

2030 Build Off-St	reet Parking	g Utilization	Summary
2030 Build	AM	MD	PM
1/4-Mile Radius	S		
2030 No Build Capacity	1330	1330	1330
Displaced Garages	679	679	679
MTP 3300 Broadway Corp.	200	200	200
West 129th Street LLC	134	134	134
Uni Facility Corp.	100	100	100
Y&H Enterprises Inc.	175	175	175
Columbia Waterfront LLC	70	70	70
Effective 2030 Build Capacity	651	651	651
2030 No Build Demand	1592	1612	1525
Parking Demand Shifted to Columbia Garages	134	266	158
New Columbia Academic Building	48	118	69
New Columbia Administrative Building (Studebaker)	86	148	89
Credit for Removal and Reduction of Projects in Build	125	115	28
New Columbia Office Building (No. 23)	77	70	17
New Columbia Office Building (No. 24)	48	45	11
16% Credit for Displaced Parking Demand*	110	110	110
Total 2030 Build Demand	1223	1121	1229
Remaining Spaces	-572	-470	-578
Utilization	188%	172%	189%
Added Supply from Columbia On-Site Parking	300	300	300
Remaining Spaces	-272	-170	-278
Effective Utilization	142%	126%	143%
½-Mile Radius	S		
2030 No Build Capacity	1710	1710	1710
Displaced Garages	679	679	679
Effective 2030 Build Capacity	1031	1031	1031
2030 No Build Demand	1964	2041	1796
Parking Demand Shifted to Columbia Garages	134	266	158
Credit for Removal and Reduction of Projects in Build	125	115	28
16% Credit for Displaced Parking Demand*	110	110	110
Total 2030 Build Demand	1595	1550	1500
Remaining Spaces	-564	-519	-469
Utilization	155%	150%	145%
Added Supply from Columbia On-Site Parking	300	300	300
Remaining Spaces	-264	-219	-169
Effective Utilization	126%	121%	116%
1-Mile Radius			
2030 No Build Capacity	3576	3576	3576
Displaced Garages	679	679	679
Effective 2030 Build Capacity	2897	2897	2897
2030 No Build Demand	3687	3727	2877
Parking Demand Shifted to Columbia Garages	134	266	158
Credit for Removal and Reduction of Projects in Build	125	115	28
16% Credit for Displaced Parking Demand*	110	110	110
Total 2030 Build Demand	3318	3236	2581
Remaining Spaces	-421	-339	316
Utilization	115%	112%	89%
Added Supply from Columbia On-Site Parking	300	300	300
Remaining Spaces	-121	-39	616
Effective Utilization	104%	101%	79%

Note: Credit accounts for 16% of the existing demand at the five displaced parking garages grown to 2030 levels. See Appendix H for detailed parking accumulation analysis.

H. TRAFFIC SAFETY

According to the CEQR Technical Manual, locations within close proximity to sensitive land uses, such as hospitals, schools, parks, nursing homes, or elderly housing, which could be affected by traffic volumes generated by the Proposed Actions, require a detailed analysis of safety impacts. Roadways with high accident rates or a design that makes it difficult for pedestrians to traverse safely also require analysis. The CEQR Technical Manual (page 3O-4) considers an intersection to be a high-accident location if there are five or more pedestrian/bicycle accidents in any year in the most recent three-year period for which data is available.

Accident records for intersections within the study area were obtained from the NYCDOT for the three-year period from January 1, 2001 to December 31, 2003 (see Tables 17-41 through 17-43). Table 17-44 summarizes the data to present pedestrian and bicycle accidents for the three-year period. A review of these records revealed that there were four pedestrian and one bicycle related accidents at the intersection of Amsterdam Avenue and West 125th Street in 2003. As described for the 2030 No Build conditions and part of the mitigation measures proposed for the 125th Street Corridor Rezoning and Related Actions DEIS, eastbound and westbound left-turn movements would be prohibited at this intersection. Furthermore, as discussed in Chapter 23, "Mitigation," measures to mitigate significant adverse traffic impacts at this intersection were also proposed. Collectively, these measures are expected to improve traffic flow and reduce vehicular-pedestrian conflicts at this intersection. No other locations within the primary study area would be considered high accident locations based on the definition in the CEQR Technical Manual. Therefore, it is unlikely that project-generated volumes would result in significant safety impacts. The potential implementation of traffic calming measures at locations with low accident rates would further reduce the likelihood of pedestrian and vehicular conflicts.

Table 17-<u>41</u> 2001 Study Area Intersection Accident Data

							2()U	1	S	tu	ld	y .	A	re	a	L	nt	er	S	ec	ti	or	1	4(cc	id	eı	nt	D	a	ta
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8 9 T	3	2				1					1				2								2			1						
A 9T			1												1		1		1				1			1						1
х ат																		1														
ЯЭНТО	2	-	-	-	-	-	-	0	0	3	0	2	0	2	2	0	1	1	0	0	2	0	7	0	-	3	0	-	0	0	0	3
SIDESWIPE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0
нерр ои	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
ияит тныя	1	0	0	0	1	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	1	l.	0	1	0	0	0	0	0	0
ЭЛЭИА ТНЭІЯ	0	0	1	0	1	0	1	3	0	2	0	3	0	0	0	1	0	0	2	1	0	1	1	0	0	9	0	0	0	0	0	3
ОЛЕВ ТАКІИ	0	1	0	1	0	0	0	1	0	0	0	1	0	3	1	0	0	1	0	0	2	0	2	0	0	0	0	0	1	0	0	0
верк еир	0	0	2	0	0	0	0	0	0	2	0	4	0	-	4	0	0	0	0	0	0	1	1	1	0	4	0	-	0	1	-	-
ияит тээл	0	0	1	2	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	3	l	0	1	0	0	0	0	0	0
ээа тнэіи	1	0	2	1	0	0	1	1	0	4	0	7	0	2	3	1	0	2	0	0	1	1	9	7	0	7	0	0	0	1	0	4
иет вр	1	1	0	0	0	0	1	0	0	3	0	3	0	1	3	1	1	0	1	0	0	0	3	l	1	2	0	2	0	0	0	3
EIXED OB1.	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
BICYCLIST	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	1
NAIRTEDESTRIAN	1	0	0	0	0	0	1	0	0	2	0	1	0	0	2	0	0	0	0	0	2	0	4	0	0	1	0	1	0	0	0	1
УЯПСИ	4	2	9	1	1	2	3	2	0	6	0	13	0	6	11	4	2	0	7	4	4	1	18	2	2	20	0	3	3	1	-	11
JATA	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ТЧЯ-ИОИ	7	9	9	3	10	4	10	1	7	4	0	11	4	11	12	7	7	4	l	4	3	3	24	7	9	32	2	2	2	2	2	12
тчя	3	2	2	4	3	2	3	4	0	7	0	11	0	8	10	7	1	7	7	7	4	2	11	4	1	18	0	3	2	2	-	6
JATOT	2	8	11	7	13	9	13	5	7	11	0	23	4	19	22	4	3	9	3	9	7	5	41	9	7	20	2	8	7	4	3	21
та seояэ	W 135th St	W 133rd St	W 132nd St	W 131st St	W 125th St	St. Clair PI	29th	W 131st St	W 133rd St	35th	W 138th St	W 145th St	W 129th St	25th	W 125th St	W 130th St	W 131st St	W 132nd St	W 133rd St	W 135th St	W 138th St	W 145th St	W 125th St	W 135th St	W 125th St	E 125th St	W 120th St	Cathedral Pk	Cathedral Pk	20th	20th	Madison Ave
TS NIAM	Riverside Dr	12th Ave	Broadway NB	Broadway SB	Broadway SB	Broadway NB	Broadway SB	Amsterdam Ave	Amsterdam Ave	St. Clair PI	1st Ave	Broadway SB	Broadway SB	Broadway NB	Broadway NB	Amsterdam Ave	E 125th St															
NODE#	1070	1072	1073	1074	1075	1076	1084	1085	1086	1088	1091	1098	1147	1148	1149	1150	1151	1152	1153	1154	1158	1165	1187	1195	1418	8968	9085	6063	9192	9200	9205	9943
ЯЕАЯ	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001	2001

Table 17-<u>42</u> 2002 Study Area Intersection Accident Data

							<i>2</i> (JU	Z	2	tu	l a j	У.	A	re	a	L	nt	eı	S	\mathbf{ec}	tic	or	l A	4	cc	ld	eı	nt	D	a	ta
яи чт																																
Э 4Т	4		1	2			3	1	2			1	1	8	3			1	2		2		9	1	2	12	4	1			1	4
8 9T					1										l.											2	1				1	2
A 9T					1										l.									l l		1						
х ат																																
ЯЭНТО	0	0	0	0	1	0	0	l	0	0	1	0	1	2	4	0	0	0	0	0	1	0	0	l	0	0	0	0	1	0	-	2
SIDESMIPE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
нерр ои	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ия∪т тнэія	0	0	Į.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ВІСНТ АИСЕ	0	0	0	0	0	0	0	0	7	1	1	0	0	1	0	0	0	7	1	0	0	0	0	0	0	4	1	0	0	1	0	7
ОЛЕВ ТАКІИ	1	0	0	0	0	0	0	0	0	0	0	0	0	1	Į.	0	0	0	0	0	0	0	l	0	0	0	0	0	0	0	0	0
REAR END	l	0	0	0	l	0	l	0	0	1	0	0	0	1	1	0	0	0	0	0	0	0	7	l	7	4	0	0	1	0	1	0
ияит тээл	0	0	0	1	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	7	0	0	0	2	0	0	0	0	0
ээа тнэіи	7	0	0	1	1	0	0	1	1	1	1	-	l	1	2	0	0	0	0	0	0	0	1	7	-	က	1	1	0	0	-	7
мет вр	0	0	0	0	0	0	0	0	0	2	0	-	0	7	2	0	0	0	-	0	0	0	2	0	0	က	0	٦	0	0	-	7
FIXED OBJ.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
BICYCLIST	0	0	0	0	0	0	0	ļ	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	٦	0
NAIRTEDESTRIAN	0	0	0	0	0	0	0	0	0	0	1	0	1	0	3	0	0	0	0	0	1	0	0	1	0	0	0	0	_	0	0	1
иливу	4	0	٦	7	7	0	3	ļ	2	4	2	~	1	80	2	0	0	1	7	0	1	0	9	7	2	15	2	ļ	2	2	2	9
JATAŦ	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
тая-иои	2	٦	٦	0	2	0	1	ļ	3	2	0	လ	9	3	2	3	1	4	3	٦	0	2	10	3	2	7	3	0	2	2	٦	0
тчя	2	0	1	1	3	0	7	1	2	4	2	-	٦	2	8	0	0	2	-	2	1	0	2	7	2	12	4	1	2	1	2	2
JATOT		1	2									4			•			9		3		7	•	9	4	19	7	1	4	9	3	2
та ггояс	W 135th St	W 133rd St	W 132nd St	W 131st St	W 125th St	St. Clair PI	W 129th St	W 131st St	W 133rd St	W 135th St	W 138th St	W 145th St	W 129th St	W 125th St	W 125th St	W 130th St	W 131st St	W 132nd St	W 133rd St	W 135th St	W 138th St	W 145th St	W 125th St	W 135th St	W 125th St	E 125th St	W 120th St	Cathedral Pk	Cathedral Pk	W 120th St	W 120th St	Madison Ave
TS NIAM	Riverside Dr	12th Ave	Broadway NB	Broadway SB	Broadway SB	Broadway NB	Broadway SB	Broadway SB	Amsterdam Ave	Amsterdam Ave	St. Clair PI	1st Ave	Broadway SB	Broadway SB	Broadway NB	Broadway NB	Amsterdam Ave	E 125th St														
NODE#	1070	1072	1073	1074	1075	1076	1084	1085	1086	1088	1091	1098	1147	1148	1149	1150	1151	1152	1153	1154	1158	1165	1187	1195	1418	8968	9085	£606	9192	9200	9205	9943
ЯАЭХ	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002	2002

Table 17-<u>43</u> 2003 Study Area Intersection Accident Data

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яи чт																																
Э ЧТ	7	1	3	7	1		2		1		3	9	1	14	3			2	2		1	3	25	2	9	34	2	1	l			7
8 9 T													-	3	1			7			7		2		l	2	-					
A 9T					1				1		_	-	_											1		2						
х ат																										3						
ЯЭНТО	1	0	0	0	0	0	0	0	1	0	0	3	0	2	0	0	0	0	0	0	0	0	2	3	1	3	0	2	0	0	2	2
SIDESMIPE	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0	0	0
невр ои	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ияит тныя	0	0	0	0	0	0	0	0	0	0	0	0	0	1	7	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0
ВІВНТ АИВLE	0	0	-	0	0	0	3	0	1	7	1	2	1	2	0	0	0	1	1	0	1	0	2	0	0	17	1	0	0	1	0	2
ОЛЕВ ТАКІИ	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0
диз ядзя	0	-	0	0	1	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	2	3	1	1	2	0	0	0	0	1	2
ияит тээл	0	0	-	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	3	0	0	0	1	0	0	0	0	0
ээа тнэіи	0	7	-	0	0	0	1	0	0	1	0	0	_	2	3	0	0	0	0	1	1	0	9	3	7	11	0	-	0	1	2	2
ая тэм	0	_	0	0	1	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	3	7	0	7	_	0	1	1	2	1
FIXED OBJ.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
BICYCLIST	l	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1	1
ИАІЯТЕЗЕВНИ	0	0	0	0	0	0	0	0	1	0	0	7	0	1	0	0	0	0	0	0	0	0	4	7	1	0	0	2	0	0	1	-
иливу	1	1	က	2	2	0	2	0	2	2	7	7	3	17	4	0	0	4	10	4	2	3	27	8	9	42	9	2	1	3	2	7
JATA	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0
тая-иои	0	7	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	7	0	1	0	0	0
ТЧЯ	1	1	4	2	3	0	3	1	3	2	7	2	1	10	4	0	0	2	2	2	1	3	21	2	2	23	3	2	1	2	3	9
JATOT	1	2	4	7	3	0	3	1	3	3	1	2	1	11	4	0	0	7	7	7	1	3	21	2	7	23	2	7	7	2	3	9
та геоя	W 135th St	W 133rd St	W 132nd St	W 131st St	W 125th St	St. Clair Pl	W 129th St	W 131st St	W 133rd St	W 135th St	W 138th St	W 145th St	W 129th St	W 125th St	W 125th St	W 130th St	W 131st St	W 132nd St		W 135th St	W 138th St	W 145th St	W 125th St	W 135th St	W 125th St	E 125th St	W 120th St	Cathedral Pk	Cathedral Pk	W 120th St	W 120th St	Madison Ave
TS NIAM	Riverside Dr	12th Ave	Broadway NB	Broadway SB	Broadway SB	Broadway NB	Broadway SB	Amsterdam Ave	Amsterdam Ave	St. Clair PI	1st Ave	Broadway SB	Broadway SB	Broadway NB	Broadway NB	Amsterdam Ave	E 125th St															
NODE#	1070	1072	1073	1074	1075	1076	1084	1085	1086	1088	1091	1098	1147	1148	1149	1150	1151	1152	1153	1154	1158	1165	1187	1195	1418	8968	9085	6063	9192	9200	9205	9943
ЯАЭХ	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003	2003

Table 17-<u>44</u> Study Area Intersection Accident Data Summary

				2001			2002	2	2003			
NODE#	MAIN ST	CROSS ST	TOTAL	PEDESTRIAN	BICYCLIST	TOTAL	PEDESTRIAN	BICYCLIST	TOTAL	PEDESTRIAN	BICYCLIST	
1070	Riverside Dr	W 135th St	1	1	0	0	0	0	1	0	1	
1072	12th Ave	W 133rd St	0	0	0	0	0	0	0	0	0	
1073	12th Ave	W 132nd St	0	0	0	0	0	0	0	0	0	
1074	12th Ave	W 131st St	0	0	0	0	0	0	0	0	0	
1075	12th Ave	W 125th St	0	0	0	0	0	0	0	0	0	
1076	12th Ave	St. Clair Pl	1	0	1	0	0	0	0	0	0	
1084	Broadway NB	W 129th St	1	1	0	0	0	0	0	0	0	
1085	Broadway NB	W 131st St	0	0	0	1	0	1	0	0	0	
1086	Broadway NB	W 133rd St	0	0	0	0	0	0	1	1	0	
1088	Broadway NB	W 135th St	2	2	0	0	0	0	0	0	0	
1091	Broadway NB	W 138th St	0	0	0	1	1	0	0	0	0	
1098	Broadway NB	W 145th St	1	1	0	0	0	0	2	1	1	
1147	Broadway SB	W 129th St	0	0	0	1	1	0	0	0	0	
1148	Broadway SB	W 125th St	0	0	0	0	0	0	1	1	0	
1149	Broadway NB	W 125th St	2	2	0	3	3	0	0	0	0	
1150	Broadway SB	W 130th St	0	0	0	0	0	0	0	0	0	
1151	Broadway SB	W 131st St	0	0	0	0	0	0	0	0	0	
1152	Broadway SB	W 132nd St	1	0	1	0	0	0	0	0	0	
1153	Broadway SB	W 133rd St	0	0	0	0	0	0	0	0	0	
1154	Broadway SB	W 135th St	0	0	0	0	0	0	0	0	0	
1158	Broadway SB	W 138th St	2	2	0	1	1	0	0	0	0	
1165	Broadway SB	W 145th St	0	0	0	0	0	0	0	0	0	
1187	Amsterdam Ave	W 125th St	4	4	0	0	0	0	5	4	1	
1195	Amsterdam Ave	W 135th St	0	0	0	1	1	0	2	2	0	
1418	St. Clair Pl	W 125th St	0	0	0	0	0	0	1	1	0	
8968	1st Ave	E 125th St	2	1	1	0	0	0	1	0	1	
9085	Broadway SB	W 120th St	0	0	0	0	0	0	0	0	0	
9093	Broadway SB	Cathedral Pk	1	1	0	0	0	0	2	2	0	
9192	Broadway NB	Cathedral Pk	0	0	0	1	1	0	0	0	0	
9200	Broadway NB	W 120th St	0	0	0	0	0	0	0	0	0	
9205	Amsterdam Ave	W 120th St	0	0	0	1	0	1	2	1	1	
9943	E 125th St	Madison Ave	2	1	1	1	1	0	2	1	1	

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