Chapter 20: Mitigation

A. INTRODUCTION

As described in Chapter 1, "Project Description," the Applicant, WF Liberty, LLC, is proposing the redevelopment of a 17.72-acre portion of a 33.68-acre property (the Project Site) along the Arthur Kill waterfront in Staten Island that would result in a 589,619-gross-square-foot (gsf) commercial center (the Proposed Project). This chapter considers mitigation measures to address significant adverse impacts generated by the Proposed Project.

The Proposed Project has the potential to result in significant adverse impacts to archaeological resources and transportation. Mitigation measures for these technical areas are identified below.

PRINCIPAL CONCLUSIONS

ARCHAEOLOGICAL RESOURCES

A Phase 1 report prepared for the Proposed Project concluded that there are areas of potential archaeological sensitivity within the Project Site. Therefore, additional Phase 1B and Phase 2 testing was completed and a draft report summarizing such work has been submitted to the New York City Landmarks Preservation Commission (LPC) and the New York State Historic Preservation Office (SHPO). Based on the testing completed to date, the Proposed Project could potentially result in a significant adverse impact on archaeological resources due to site disturbance. Accordingly, the Applicant will complete any required additional investigation and/or mitigation in consultation with the LPC and SHPO. To the extent that mitigation is not completed prior to the issuance of the Final Environmental Impact Statement, the Applicant will record a restrictive declaration obligating it to complete such additional work to the satisfaction of LPC (see Appendix B).

TRANSPORTATION

The Proposed Project would result in significant adverse impacts with respect to traffic. As discussed in Chapter 12, "Transportation," traffic conditions were evaluated at 20 intersections for the weekday AM, midday, PM, and Saturday peak hours. In the 2019 With Action condition (the Proposed Project), there would be the potential for significant adverse traffic impacts at 10 intersections during the weekday AM peak hour, 12 intersections during the weekday midday peak hour, 15 intersections during the weekday PM peak hour, and 15 intersections during the Saturday peak hour.

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¹ Greenhouse Consultants, Inc. (January 2017): "Supplemental Phase 1B and phase 2 Archaeological and Historical Investigations of Riverside Galleria (Formerly Waterfront Commons); Block 7620, Lot 1; Block 7632, Lots 1, 6, 60, 150, and 151; Richmond County, Staten Island, New York; 07PR04902." Prepared for: Robert Konig, Esq., Woodmere, NY

Notes:

As summarized in **Table 20-1**, some of the locations where significant adverse traffic impacts are predicted to occur could be fully mitigated with the implementation of standard traffic mitigation measures (e.g., signal timing changes, approach daylighting, and lane restriping), which are described below. However, the significant adverse impacts at the intersections of Arthur Kill Road at North Bridge Street, Arthur Kill Road at Richmond Valley Road, Page Avenue at Richmond Valley Road, Boscombe Avenue at the Route 440 Ramps, Amboy Road and Page Avenue, Page Avenue at Hylan Boulevard, North Bridge Street at the 440 westbound off-ramp, and Arthur Kill Road at South Bridge Street could not be fully mitigated during one or more analysis peak hours (see Chapter 21, "Unavoidable Adverse Impacts"). At two of these intersections, improvement measures were recommended to partially mitigate the projected impacts at one or more of the impacted movements. No significant adverse impacts were identified for transit, pedestrians, vehicular and pedestrian safety, and parking.

Table 20-1 Summary of Significant Adverse Traffic Impacts Proposed Project

	Weekday AM Weekday M		iddav	1		Saturday Peak			
Interse	ction	Peak Ho		Peak Ho		Peak Ho		Hour	
		Significant		Significant		Significant		Significant	
EB/WB Street	NB/SB Street	Impacts	Mit	Impacts	Mit	Impacts	Mit	Impacts	Mit
Woodrow Road/School Driveway	Bloomingdale Road	Not Impacted	NA	Not Impacted	NA	WB-L	Yes	WB-L	Yes
Englewood Avenue	Veterans Road East	Not Impacted	NA	Not Impacted	NA	EB-LTR	Yes	EB-LTR	Yes
Veterans Road West/Allentown Lane	Arthur Kill Road	WB-LTR	Yes	WB-LTR NB-LTR SB-LTR	Yes Yes Yes	WB-LTR NB-LTR SB-LTR	Yes Yes Yes	WB-LTR NB-LTR SB-LTR	Yes Yes Yes
North Bridge Street	Arthur Kill Road	WB-LR	No	WB-LR	No	WB-LR	No	WB-LR SB-T	No No
Richmond Valley Road	Arthur Kill Road	WB-TR	No	EB-L EB-TR WB-TR	No No No	EB-L EB-TR WB-L WB-TR NB-TR	No No No No	EB-L EB-TR WB-TR NB-TR	No No No No
Richmond Valley Road	Page Avenue	Not Impacted	NA	EB-L EB-TR NB-L	Yes Yes No	EB-L EB-TR WB-LTR NB-L	Yes Yes Yes No	EB-L EB-TR WB-LTR NB-L	No No No No
South Bridge Street	Page Avenue/ Boscombe Avenue	EB-LT	Yes	EB-LT	Yes	EB-LT	Yes	EB-LT	Yes
Boscombe Avenue	Route 440 Ramps	EB-L	No	EB-L	No	EB-L	No	EB-L	No
Veterans Road West	Tyrellan Avenue	WB-LTR	Yes	WB-LTR	Yes	EB-LTR WB-LTR	Yes Yes	EB-LTR WB-LTR	Yes Yes
Veterans Road West	North Bridge Street/Bricktown Way	EB-L WB-L	Yes Yes	EB-L EB-TR WB-L	Yes Yes Yes	EB-L EB-TR WB-L	Yes Yes Yes	EB-L EB-TR WB-T	Yes Yes Yes
Amboy Road	Page Avenue	EB-L	Yes	SB-L	Yes	EB-L NB-T SB-L	No Yes Yes	EB-L NB-L NB-T SB-T	No No Yes No
Hylan Boulevard	Page Avenue	EB-L SB-L	No No	SB-L	No	EB-L SB-L	No No	EB-L SB-L	No No
Amboy Road/Pleasant Plains Avenue	Bloomingdale Road	Not Impacted	NA	Not Impacted	NA	NB-LTR	Yes	WB-L NB-LTR	Yes Yes
Route 440 Off-Ramp	North Bridge Street	WB-L	No	WB-L	No	WB-L	No	WB-L	No
South Bridge Street	Arthur Kill Road	Not Impacted	NA	SB-LT	No	SB-LT	No	SB-LT	No
Total Impacted Interse	ections/Lane Groups	10/12		12/20		15/30		15/32	

SB = Southbound, Mit = Mitigation Provided, NA = Not Applicable

L = Left Turn, T = Through, R = Right Turn, DefL = De facto Left Turn, EB = Eastbound, WB = Westbound, NB = Northbound,

B. HISTORIC AND ARCHAEOLOGICAL RESOURCES

A Phase 1A report prepared for the Proposed Project concluded that there are areas of potential archaeological sensitivity within the Project Site. Therefore, additional Phase 1B and Phase 2 testing was completed and a draft report summarizing such work has been submitted to LPC and SHPO. Based on the testing completed to date, the Proposed Project could potentially result in a significant adverse impact on archaeological resources due to site disturbance. Accordingly, the Applicant will complete any required additional investigation and/or mitigation in consultation with the LPC and SHPO. To the extent that mitigation is not completed prior to the issuance of the Final Environmental Impact Statement, the Applicant will record a restrictive declaration obligating it to complete such additional work to the satisfaction of LPC (see **Appendix B**).

With these mitigation measures in place, it is concluded that the potential significant impacts of the Proposed Project on archaeological resources would be fully mitigated and the Proposed Project would not result in significant adverse impacts to historic archaeological resources.

C. TRANSPORTATION

TRAFFIC

As described in Chapter 12, "Transportation," traffic levels of service (LOS) at signalized and unsignalized intersections are evaluated using average stop control delay, in seconds per vehicle, for individual lane groups (grouping of movements in one or more travel lanes), the approaches, and the overall intersection. According to the criteria presented in the 2014 CEQR Technical Manual, impacts are considered significant and require examination of mitigation if they result in an increase in the With Action condition of five or more seconds of delay in a lane group over No Action levels beyond mid-LOS D. For No Action LOS E, a four-second increase in delay is considered significant. For No Action LOS F, a three-second increase in delay is considered significant. In addition, impacts are considered significant if levels of service deteriorate from acceptable A, B, or C in the No Action condition to marginally unacceptable LOS D (a delay in excess of 45 and 30 seconds, the midpoint of LOS D, for signalized and unsignalized intersections, respectively), or unacceptable LOS E or F in the With Action condition. For unsignalized intersections, for the minor street to trigger significant impacts, at least 90 passenger car equivalents (PCEs) must be identified in the With Action condition in any peak hour. A traffic impact is considered fully mitigated when the resulting degradation in the average control delay per vehicle under the Action-with-Mitigation condition compared to the No Action condition is no longer deemed significant following the impact criteria described above.

PROPOSED PROJECT

Tables 20-2 through **20-5** itemize the recommended mitigation measures that address the identified impacts under the Proposed Project.

Table 20-2 Recommended Mitigation Measures: Proposed Project Weekday AM Peak Hour

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Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Veterans Road East and Englewood Avenue ¹	Phase A: NB Green = 27 s Phase B: EB/WB Green = 9 s	Restripe the EB approach from one 24.5 ft moving lane to one 12 ft left-turn lane and one 12 ft through/right-turn lane.	No change from No-Action
Arthur Kill Road and Veterans Road West/ Allentown Lane	Phase A: NB/SB Green = 25 s Phase B: EB/WB Green = 25 s	1) Shift the centerline of Arthur Kill Road south of Veterans Road West 1 ft to the west. Restripe the NB approach from one 20 ft moving lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Allow right turn on red for the NB right-turn movement; 3) Restripe the WB approach from one 13 ft moving lane and one 9 ft parking lane to one 11 ft travel lane and one 11 ft through/right-turn lane; 4) Increase the signal cycle length from 60 seconds to 90 seconds; 5) Create a protected WB left-turn phase with NB right-turn overlap.	Phase A: NB/SB Green = 38 s Phase B: WB/NBR Green = 24 s Phase C: EB/WB Green = 13 s
Arthur Kill Road and North Bridge Street	Phase A: NB/SB Green = 30 s Phase B: WB Green = 20 s	Unmitigated	No change from No-Action
Arthur Kill Road and Richmond Valley Road	Phase A: NB/SB Green = 34 s Phase B: WB Green = 16 s	Unmitigated	No change from No-Action
Page Avenue and Richmond Valley Road ²	Phase A: NB/SB Green = 49 s Phase B: EB/WB Green = 31 s	1) Restripe the WB approach for 100 ft from one 22.5 ft moving lane to one 11 ft left-turn lane and one 11 ft through/right-turn lane; 2) Install "No Standing Anytime" for 110 ft at the SB approach to create an additional right-turn lane; 3) Increase the signal cycle length from 90 seconds to 120 seconds; 4) Create a leading EB phase with SB right-turn overlap.	Phase A: NB/SB Green = 67 s Phase B: EB/SBR Green = 4 s Phase C: EB/WB Green = 34 s
Page / Boscombe Avenue and South Bridge Street	Phase A: NB/SB Green = 50 s Phase B: EB/WB Green = 30 s	1) Restripe the EB approach from one 18 ft moving lane and buffer zone to one 11 ft left-turn lane and one 10 ft left-turn/through lane; 2) Prohibit WB left-turns; 3) Restripe the SB approach from one 23 ft moving lane to one 12 ft left-turn/through lane and one 11' through lane; 4) Create a leading SB phase and a protected EB left-turn phase.	Phase A: SB/WBR Green = 11 s Phase B: NB/SB Green = 36 s Phase C: EB Green = 28 s
Boscombe Avenue and the Route 440 Ramps	Phase A: EB/WB Green = 35 s Phase B ⁴ : NB/SB Green = 16 s Phase C: EB/SBR Green = 54 s	Unmitigated	No change from No-Action

Table 20-2, cont'd **Recommended Mitigation Measures: Proposed Project** Weekday AM Peak Hour

			ekday AM Peak Hour
Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Veterans Road West and Tyrellan Avenue	Phase A: WB Green = 23 s Phase B: EB Green = 23 s Phase C: NB/SB Green = 29 s	1) Restripe the EB approach from one 13 ft travel lane, one 11 ft travel lane and 10 ft median to one 13 ft travel lane, one 11 ft travel lane and one 12 ft left-turn only lane; 2) Restripe the WB approach from one 14 ft travel lane, one 10 ft travel lane and 10 ft median to one 14 ft travel lane, one 10 ft travel lane, one 12 ft left-turn only lane; Change EB and WB split phasing to permitted phasing with an EB/WB protected only left-turn phase.	Phase A: NB/SB Green = 30 s Phase B: EB/WB Green = 10 s Phase C: WB Green = 12 sec Phase D: EB/WB Green = 23 s
Veterans Road West and North Bridge Street/ Bricktown Way	Phase A: EB/WB Green = 30 s Phase B: NB Green = 25 s Phase C: SB Green = 20 s	1) Restripe the NB approach from one 23 ft moving lane to one 11 ft left-turn lane and one 12 ft through/right-turn lane; 2) Restripe the WB approach 12 ft right-turn lane to one 12 ft through/right-turn lane; 3) Restripe the WB receiving lanes from one 12.5 ft through lane and one 9.5 ft right-turn only/parking lane to two 11 ft through lanes to Arthur Kill Road; 4) Increase the signal cycle length from 90 seconds to 120 seconds; 5) Change NB and SB split phasing to permitted and create protected NB, SB and EB left-turn phases.	Phase A: NBL/SBL Green = 4 s Phase B: SB Green = 4 s Phase C: NB/SB Green = 42 s Phase D: EB Green = 7s Phase E: EB/WB Green = 43 s
Page Avenue and Amboy Road	Phase A: EB/WB Green = 27 s Phase B: NB/SB Green = 35 s Phase C: EB Green = 13 s	1) Restripe the WB approach from one 10 ft left-turn lane and one 11 ft through/right-turn lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Restripe the NB approach from one 10 ft left-turn lane, one 11 ft through lane and one 11 ft right-turn lane to one 10 ft left-turn lane, one 11 ft through lane and one 11 ft through lane and one 11 ft through/right-turn lane; 3) Shift 1 second each from Phase B (NB/SB) to Phase A (EB/WB) and to Phase C (EB)	Phase A: EB/WB Green = 28 s Phase B: NB/SB Green = 33 s Phase C: EB Green = 14 s
Page Avenue and Hylan Boulevard	Phase A: EB/WB Green = 69 s Phase B: LPI Green = 7 s Phase C: NB/SB Green = 33 s	Unmitigated	No change from No-Action
Bloomingdale Road and Amboy Road and Pleasant Plains Avenue ¹	Phase A: EB/WB Green = 40 s Phase B: NB/SB Green = 40 s	Shift the centerline of Amboy Road, south of Pleasant Plains Avenue 1 ft to the west creating a 12 ft NB moving lane; Create NB and WB leading phases.	Phase A: NB Green = 4 s Phase B: NB/SB Green = 32 s Phase C: WB Green = 4 s Phase D: EB/WB Green = 30 s
North Bridge Street and the Route 440 Off-Ramp	Unsignalized	Unmitigated	Unsignalized
Arthur Kill Road and South Bridge Street	Unsignalized	Unmitigated	Unsignalized

Notes: EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound; L = Left; T = Through; R = Right.

(1) Mitigation measures not required for the weekday AM and midday peak hours.

(2) Mitigation measures not required for the weekday AM peak hour.

(3) Any time not used in Phase C will be added to Phase A.

(4) If NB approach doesn't get called, WBR will operate with SB phase during Phase B.

Table 20-3 Recommended Mitigation Measures: Proposed Project Weekday Midday Peak Hour

Intersection	No Action Signal Timing	Recommended Mitigation	Recommended Signal Timing
Intersection	No Action Signal Timing	Measures Restripe the EB approach from one	Recommended Signal Timing
Veterans Road East and Englewood Avenue ¹	Phase A: NB Green = 27 s Phase B: EB/WB Green = 9 s	24.5 ft moving lane to one 12 ft left- turn lane and one 12 ft through/right- turn lane.	No change from No-Action
Arthur Kill Road and Veterans Road West/ Allentown Lane	Phase A: NB/SB Green = 25 s Phase B: EB/WB Green = 25 s	1) Shift the centerline of Arthur Kill Road south of Veterans Road West 1 ft to the west. Restripe the NB approach from one 20 ft moving lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Allow right turn on red for the NB right-turn movement; 3) Restripe the WB approach from one 13 ft moving lane and one 9 ft parking lane to one 11 ft travel lane and one 11 ft through/right-turn lane; 4) Increase the signal cycle length from 60 seconds to 90 seconds; 5) Create a protected WB left-turn phase with NB right-turn overlap.	Phase A: NB/SB Green = 43 s Phase B: WB/NBR Green = 18 s Phase C: EB/WB Green = 14 s
Arthur Kill Road and North Bridge Street	Phase A: NB/SB Green = 30 s Phase B: WB Green = 20 s	Unmitigated	No change from No-Action
Arthur Kill Road and Richmond Valley Road	Phase A: NB/SB Green = 34 s Phase B: WB Green = 16 s	Unmitigated	No change from No-Action
Page Avenue and Richmond Valley Road ²	Phase A: NB/SB Green = 49 s Phase B: EB/WB Green = 31 s	1) Restripe the WB approach for 100 ft from one 22.5 ft moving lane to one 11 ft left-turn lane and one 11 ft through/right-turn lane; 2) Install "No Standing Anytime" for 110 ft at the SB approach to create an additional right-turn lane; 3) Increase the signal cycle length from 90 seconds to 120 seconds; 4) Create a leading EB phase with SB right-turn overlap.	Phase A: NB/SB Green = 57 s Phase B: EB/SBR Green = 18 s Phase C: EB/WB Green = 30 s
Page / Boscombe Avenue and South Bridge Street	Phase A: NB/SB Green = 50 s Phase B: EB/WB Green = 30 s	1) Restripe the EB approach from one 18 ft moving lane and buffer zone to one 11 ft left-turn lane and one 10 ft left-turn/through lane; 2) Prohibit WB left-turns; 3) Restripe the SB approach from one 23 ft moving lane to one 12 ft left-turn/through lane and one 11' through lane; 4) Create a leading SB phase and a protected EB left-turn phase.	Phase A: SB/WBR Green = 11 s Phase B: NB/SB Green = 36 s Phase C: EB Green = 28 s
Boscombe Avenue and the Route 440 Ramps	Phase A: EB/WB Green = 35 s Phase B ⁴ : NB/SB Green = 16 s Phase C: EB/SBR Green = 54 s	Unmitigated	No change from No-Action

Table 20-3, cont'd Recommended Mitigation Measures: Proposed Project Weekday Midday Peak Hour

			lay Midday Peak Hour
Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Veterans Road West and Tyrellan Avenue	Phase A: WB Green = 23 s Phase B: EB Green = 23 s Phase C: NB/SB Green = 29 s	1) Restripe the EB approach from one 13 ft travel lane, one 11 ft travel lane and 10 ft median to one 13 ft travel lane, one 11 ft travel lane and one 12 ft left-turn only lane; 2) Restripe the WB approach from one 14 ft travel lane, one 10 ft travel lane and 10 ft median to one 14 ft travel lane, one 10 ft travel lane and one 12 ft left-turn only lane; Change EB and WB split phasing to permitted phasing with an EB/WB protected only left-turn phase.	Phase A: NB/SB Green = 35 s Phase B: EB/WB Green = 11 s Phase C: WB Green = 6 sec Phase D: EB/WB Green = 23 s
Veterans Road West and North Bridge Street/ Bricktown Way	Phase A: EB/WB Green = 30 s Phase B: NB Green = 25 s Phase C: SB Green = 20 s	1) Restripe the NB approach from one 23 ft moving lane to one 11 ft left-turn lane and one 12 ft through/right-turn lane; 2) Restripe the WB approach 12 ft right-turn lane to one 12 ft through/right-turn lane; 3) Restripe the WB receiving lanes from one 12.5 ft through lane and one 9.5 ft right-turn only/parking lane to two 11 ft through lanes to Arthur Kill Road; 4) Increase the signal cycle length from 90 seconds to 120 seconds; 5) Change NB and SB split phasing to permitted and create protected NB, SB and EB left-turn phases.	Phase A: NBL/SBL Green = 4 s Phase B: SB Green = 4 s Phase C: NB/SB Green = 42 s Phase D: EB Green = 7s Phase E: EB/WB Green = 43 s
Page Avenue and Amboy Road	Phase A: EB/WB Green = 29 s Phase B: NB/SB Green = 35 s Phase C: EB Green = 11 s	1) Restripe the WB approach from one 10 ft left-turn lane and one 11 ft through/right-turn lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Restripe the NB approach from one 10 ft left-turn lane, one 11 ft through lane and one 11 ft right-turn lane to one 10 ft left-turn lane, one 11 ft through/right-turn lane; 3) Shift 4 seconds from Phase A (EB/WB) to Phase C (EB).	Phase A: EB/WB Green = 25 s Phase B: NB/SB Green = 35 s Phase C: EB Green = 15 s
Page Avenue and Hylan Boulevard	Phase A: EB/WB Green = 69 s Phase B: LPI Green = 7 s Phase C: NB/SB Green = 33 s	Unmitigated	No change from No-Action
Bloomingdale Road and Amboy Road	Phase A: EB/WB Green = 40 s Phase B: NB/SB Green = 40 s	Shift the centerline of Amboy Road, south of Pleasant Plains Avenue 1 ft to the west creating a 12 ft NB moving lane; Create NB and WB leading phases.	Phase A: NB Green = 4 s Phase B: NB/SB Green = 32 s Phase C: WB Green = 4 s Phase D: EB/WB Green = 30 s
North Bridge Street and the Route 440 Off-Ramp	Unsignalized	Unmitigated	Unsignalized
Arthur Kill Road and South Bridge Street	Unsignalized	Unmitigated	Unsignalized

Notes: EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound; L = Left; T = Through; R = Right.

(1) Mitigation measures not required for the weekday AM and midday peak hours.

(2) Mitigation measures not required for the weekday AM peak hour.

(3) Any time not used in Phase C will be added to Phase A.

(4) If NB approach doesn't get called, WBR will operate with SB phase during Phase B.

Table 20-4
Recommended Mitigation Measures: Proposed Project
Weekday PM Peak Hour

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Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Bloomingdale Road and Woodrow Road	Phase A: NB/SB Green = 34 s Phase B: EB/WB Green = 24 s Phase C ³ : EB (Woodrow Rd) Green = 17 s	Shift 6 seconds from Phase C (EB Woodrow Road) to Phase B (EB/WB)	Phase A: NB/SB Green = 34 s Phase B: EB/WB Green = 30 s Phase C ³ : EB (Woodrow Rd) Green = 11 s
Veterans Road East and Englewood Avenue ¹	Phase A: NB Green = 27 s Phase B: EB/WB Green = 9 s	Restripe the EB approach from one 24.5 ft moving lane to one 12 ft left-turn lane and one 12 ft through/right-turn lane.	No change from No-Action
Arthur Kill Road and Veterans Road West/ Allentown Lane	Phase A: NB/SB Green = 25 s Phase B: EB/WB Green = 25 s	1) Shift the centerline of Arthur Kill Road south of Veterans Road West 1 ft to the west. Restripe the NB approach from one 20 ft moving lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Allow right turn on red for the NB right-turn movement; 3) Restripe the WB approach from one 13 ft moving lane and one 9 ft parking lane to one 11 ft travel lane and one 11 ft through/right-turn lane; 4) Increase the signal cycle length from 60 seconds to 90 seconds; 5) Create a protected WB left-turn phase with NB right-turn overlap.	Phase A: NB/SB Green = 36 s Phase B: WB/NBR Green = 23 s Phase C: EB/WB Green = 16 s
Arthur Kill Road and North Bridge Street	Phase A: NB/SB Green = 30 s Phase B: WB Green = 20 s	Unmitigated	No change from No-Action
Arthur Kill Road and Richmond Valley Road	Phase A: NB/SB Green = 34 s Phase B: WB Green = 16 s	Unmitigated	No change from No-Action
Page Avenue and Richmond Valley Road ²	Phase A: NB/SB Green = 49 s Phase B: EB/WB Green = 31 s	1) Restripe the WB approach for 100 ft from one 22.5 ft moving lane to one 11 ft left-turn lane and one 11 ft through/right-turn lane; 2) Install "No Standing Anytime" for 110 ft at the SB approach to create an additional right-turn lane; 3) Increase the signal cycle length from 90 seconds to 120 seconds; 4) Create a leading EB phase with SB right-turn overlap.	Phase A: NB/SB Green = 57 s Phase B: EB/SBR Green = 17 s Phase C: EB/WB Green = 31 s
Page / Boscombe Avenue and South Bridge Street	Phase A: NB/SB Green = 50 s Phase B: EB/WB Green = 30 s	1) Restripe the EB approach from one 18 ft moving lane and buffer zone to one 11 ft left-turn lane and one 10 ft left-turn/through lane; 2) Prohibit WB left-turns; 3) Restripe the SB approach from one 23 ft moving lane to one 12 ft left-turn/through lane and one 11' through lane; 4) Create a leading SB phase and a protected EB left-turn phase.	Phase A: SB/WBR Green = 11 s Phase B: NB/SB Green = 36 s Phase C: EB Green = 28 s

Table 20-4, cont'd Recommended Mitigation Measures: Proposed Project Weekday PM Peak Hour

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Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Boscombe Avenue and Route 440 Ramps	Phase A: EB/WB Green = 35 s Phase B ⁴ : NB/SB Green = 16 s Phase C: EB/SBR Green = 54 s	Unmitigated	No change from No-Action
Veterans Road West and Tyrellan Avenue	Phase A: WB Green = 23 s Phase B: EB Green = 23 s Phase C: NB/SB Green = 29 s	1) Restripe the EB approach from one 13 ft travel lane, one 11 ft travel lane and 10 ft median to one 13 ft travel lane, one 11 ft travel lane and one 12 ft left-turn only lane; 2) Restripe the WB approach from one 14 ft travel lane, one 10 ft travel lane and 10 ft median to one 14 ft travel lane, one 10 ft travel lane and 10 ft median to one 14 ft travel lane, one 10 ft travel lane and one 12 ft left-turn only lane; Change EB and WB split phasing to permitted phasing with an EB/WB protected only left-turn phase.	Phase A: NB/SB Green = 32 s Phase B: EB/WB Green = 12 s Phase C: WB Green = 8 sec Phase D: EB/WB Green = 23 s
Veterans Road West and North Bridge Street/ Bricktown Way	Phase A: EB/WB Green = 30 s Phase B: NB Green = 25 s Phase C: SB Green = 20 s	1) Restripe the NB approach from one 23 ft moving lane to one 11 ft left-turn lane and one 12 ft through/right-turn lane; 2) Restripe the WB approach 12 ft right-turn lane to one 12 ft through/right-turn lane; 3) Restripe the WB receiving lanes from one 12.5 ft through lane and one 9.5 ft right-turn only/parking lane to two 11 ft through lanes to Arthur Kill Road; 4) Increase the signal cycle length from 90 seconds to 120 seconds; 5) Change NB and SB split phasing to permitted and create protected NB, SB and EB left-turn phases.	Phase A: NBL/SBL Green = 4 s Phase B: SB Green = 4 s Phase C: NB/SB Green = 42 s Phase D: EB Green = 7s Phase E: EB/WB Green = 43 s
Page Avenue and Amboy Road	Phase A: EB/WB Green = 29 s Phase B: NB/SB Green = 33 s Phase C: EB Green = 13 s	1) Restripe the WB approach from one 10 ft left-turn lane and one 11 ft through/right-turn lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Restripe the NB approach from one 10 ft left-turn lane, one 11 ft through lane and one 11 ft right-turn lane to one 10 ft left-turn lane, one 11 ft through lane and one 11 ft through/right-turn lane; 3) Shift 1 second from Phase A (EB/WB) to Phase C (EB).	Phase A: EB/WB Green = 28 s Phase B: NB/SB Green = 33 s Phase C: EB Green = 14 s
Page Avenue and Hylan Boulevard	Phase A: EB/WB Green = 69 s Phase B: LPI Green = 7 s Phase C: NB/SB Green = 33 s	Unmitigated	No change from No-Action
Bloomingdale Road and Amboy Road	Phase A: EB/WB Green = 40 s Phase B: NB/SB Green = 40 s	Shift the centerline of Amboy Road, south of Pleasant Plains Avenue 1 ft to the west creating a 12 ft NB moving lane; Create NB and WB leading phases.	Phase A: NB Green = 4 s Phase B: NB/SB Green = 34 s Phase C: WB Green = 8 s Phase D: EB/WB Green = 24 s

Table 20-4, cont'd **Recommended Mitigation Measures: Proposed Project** Weekday PM Peak Hour

Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
North Bridge Street and the Route 440 Off-Ramp	Unsignalized	Unmitigated	Unsignalized
Arthur Kill Road and South Bridge Street	Unsignalized	Unmitigated	Unsignalized

Notes: EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound; L = Left; T = Through; R = Right.

(1) Mitigation measures not required for the weekday AM and midday peak hours.
(2) Mitigation measures not required for the weekday AM peak hour.
(3) Any time not used in Phase C will be added to Phase A.
(4) If NB approach doesn't get called, WBR will operate with SB phase during Phase B.

Table 20-5 Recommended Mitigation Measures: Proposed Project Weekday Saturday Peak Hour

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Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Bloomingdale Road and Woodrow Road	Phase A: NB/SB Green = 34 s Phase B: EB/WB Green = 24 s Phase C ³ : EB (Woodrow Rd) Green = 17 s	Shift 3 seconds from Phase C (EB Woodrow Road) to Phase B (EB/WB)	Phase A: NB/SB Green = 34 s Phase B: EB/WB Green = 27 s Phase C ³ : EB (Woodrow Rd) Green = 14 s
Veterans Road East and Englewood Avenue ¹	Phase A: NB Green = 27 s Phase B: EB/WB Green = 9 s	Restripe the EB approach from one 24.5 ft moving lane to one 12 ft left-turn lane and one 12 ft through/right-turn lane.	No change from No-Action
Arthur Kill Road and Veterans Road West/ Allentown Lane	Phase A: NB/SB Green = 25 s Phase B: EB/WB Green = 25 s	1) Shift the centerline of Arthur Kill Road south of Veterans Road West 1 ft to the west. Restripe the NB approach from one 20 ft moving lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Allow right turn on red for the NB right-turn movement; 3) Restripe the WB approach from one 13 ft moving lane and one 9 ft parking lane to one 11 ft travel lane and one 11 ft through/right-turn lane; 4) Increase the signal cycle length from 60 seconds to 90 seconds; 5) Create a protected WB left-turn phase with NB right-turn overlap.	Phase A: NB/SB Green = 34 s Phase B: WB/NBR Green = 23 s Phase C: EB/WB Green = 18 s
Arthur Kill Road and North Bridge Street	Phase A: NB/SB Green = 30 s Phase B: WB Green = 20 s	Unmitigated	No change from No-Action
Arthur Kill Road and Richmond Valley Road	Phase A: NB/SB Green = 34 s Phase B: WB Green = 16 s	Unmitigated	No change from No-Action

Table 20-5, cont'd Recommended Mitigation Measures: Proposed Project Weekday Saturday Peak Hour

	Weekday Saturday Peak Hou				
Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing		
Page Avenue and Richmond Valley Road ²	Phase A: NB/SB Green = 49 s Phase B: EB/WB Green = 31 s	1) Restripe the WB approach for 100 ft from one 22.5 ft moving lane to one 11 ft left-turn lane and one 11 ft through/right-turn lane; 2) Install "No Standing Anytime" for 110 ft at the SB approach to create an additional right-turn lane; 3) Increase the signal cycle length from 90 seconds to 120 seconds; 4) Create a leading EB phase with SB right-turn overlap.	Phase A: NB/SB Green = 63 s Phase B: EB/SBR Green = 16 s Phase C: EB/WB Green = 26 s		
Page / Boscombe Avenue and South Bridge Street	Phase A: NB/SB Green = 50 s Phase B: EB/WB Green = 30 s	1) Restripe the EB approach from one 18 ft moving lane and buffer zone to one 11 ft left-turn lane and one 10 ft left-turn/through lane; 2) Prohibit WB left-turns; 3) Restripe the SB approach from one 23 ft moving lane to one 12 ft left-turn/through lane and one 11' through lane; 4) Create a leading SB phase and a protected EB left-turn phase.	Phase A: SB/WBR Green = 9 s Phase B: NB/SB Green = 35 s Phase C: EB Green = 31 s		
Boscombe Avenue and Route 440 Ramps	Phase A: EB/WB Green = 35 s Phase B ⁴ : NB/SB Green = 16 s Phase C: EB/SBR Green = 54 s	Unmitigated	No change from No-Action		
Veterans Road West and Tyrellan Avenue	Phase A: WB Green = 23 s Phase B: EB Green = 23 s Phase C: NB/SB Green = 29 s	1) Restripe the EB approach from one 13 ft travel lane, one 11 ft travel lane and 10 ft median to one 13 ft travel lane, one 11 ft travel lane and one 12 ft left-turn only lane; 2) Restripe the WB approach from one 14 ft travel lane, one 10 ft travel lane and 10 ft median to one 14 ft travel lane, one 10 ft travel lane, and one 12 ft left-turn only lane; Change EB and WB split phasing to permitted phasing with an EB/WB protected only left-turn phase.	Phase A: NB/SB Green = 35 s Phase B: EB/WB Green = 13 s Phase C: WB Green = 4 sec Phase D: EB/WB Green = 23 s		
Veterans Road West and North Bridge Street/ Bricktown Way	Phase A: EB/WB Green = 30 s Phase B: NB Green = 25 s Phase C: SB Green = 20 s	1) Restripe the NB approach from one 23 ft moving lane to one 11 ft left-turn lane and one 12 ft through/right-turn lane; 2) Restripe the WB approach 12 ft right-turn lane to one 12 ft through/right-turn lane; 3) Restripe the WB receiving lanes from one 12.5 ft through lane and one 9.5 ft right-turn only/parking lane to two 11 ft through lanes to Arthur Kill Road; 4) Increase the signal cycle length from 90 seconds to 120 seconds; 5) Change NB and SB split phasing to permitted and create protected NB, SB and EB left-turn phases.	Phase A: NBL/SBL Green = 4 s Phase B: SB Green = 4 s Phase C: NB/SB Green = 42 s Phase D: EB Green = 7s Phase E: EB/WB Green = 43 s		

Table 20-5, cont'd Recommended Mitigation Measures: Proposed Project Weekday Saturday Peak Hour

		77 CCIXUU	y Saturday I Cak Hour
Intersection	No Action Signal Timing	Recommended Mitigation Measures	Recommended Signal Timing
Page Avenue and Amboy Road	Phase A: EB/WB Green = 29 s Phase B: NB/SB Green = 35 s Phase C: EB Green = 11 s	1) Restripe the WB approach from one 10 ft left-turn lane and one 11 ft through/right-turn lane to one 11 ft left-turn/through lane and one 10 ft right-turn lane; 2) Restripe the NB approach from one 10 ft left-turn lane, one 11 ft through lane and one 11 ft right-turn lane to one 10 ft left-turn lane, one 11 ft through lane and one 11 ft through lane and one 11 ft through/right-turn lane.	No change from No-Action
Page Avenue and Hylan Boulevard	Phase A: EB/WB Green = 69 s Phase B: LPI Green = 7 s Phase C: NB/SB Green = 33 s	Unmitigated	No change from No-Action
Bloomingdale Road and Amboy Road	Phase A: EB/WB Green = 40 s Phase B: NB/SB Green = 40 s	Shift the centerline of Amboy Road, south of Pleasant Plains Avenue 1 ft to the west creating a 12 ft NB moving lane; Create NB and WB leading phases.	Phase A: NB Green = 4 s Phase B: NB/SB Green = 33 s Phase C: WB Green = 18 s Phase D: EB/WB Green = 15 s
North Bridge Street and the Route 440 Off-Ramp	Unsignalized	Unmitigated	Unsignalized
Arthur Kill Road and South Bridge Street	Unsignalized	Unmitigated	Unsignalized

Notes: EB = Eastbound; WB = Westbound; NB = Northbound; SB = Southbound; L = Left; T = Through; R = Right.

With the implementation of these standard traffic mitigation measures (including primarily signal timing changes, approach daylighting, and lane restriping), which are subject to review and approval by NYCDOT, the significant adverse traffic impacts identified in Chapter 12, "Transportation," could be fully mitigated except for the intersections of Arthur Kill Road at North Bridge Street (all peak hours), Arthur Kill Road at Richmond Valley Road (all peak hours), Page Avenue at Richmond Valley Road (weekday midday, PM, and Saturday peak hours), Boscombe Avenue at the Route 440 Ramps (all peak hours), Page Avenue at Hylan Boulevard (all peak hours), Page Avenue and Amboy Road (weekday PM and Saturday peak hours), North Bridge Street at the 440 westbound off-ramp (all peak hours), and Arthur Kill Road at South Bridge Street (weekday midday, PM, and Saturday peak hours). At two of these intersections (i.e., Page Avenue at Richmond Valley Road and Page Avenue at Amboy Road), improvement measures were recommended to partially mitigate the projected impacts at one or more of the impacted movements.

A discussion of the recommended mitigation measures is provided below. **Tables 20-6** through **20-9** compare the levels of service (LOS) and lane group delays for the impacted intersections under the 2019 No Action, With Action (the Proposed Project), and Mitigation conditions for the four analysis peak hours.

⁽¹⁾ Mitigation measures not required for the weekday AM and midday peak hours.

⁽²⁾ Mitigation measures not required for the weekday AM peak hour.

⁽³⁾ Any time not used in Phase C will be added to Phase A.

⁽⁴⁾ If NB approach doesn't get called, WBR will operate with SB phase during Phase B.

Table 20-6 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday AM Peak Hour

								N	<u>/eek</u> da	ıy AM	Peak	<u>Hou</u> r	
		2019 N	lo Build			2019 E	Build			Mitig	ation		
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	
				Signa	alized In	tersec	tions						
Veterans Ro	ad East	t & Eng	glewood	Avenu	ıe ¹								
Eastbound	LTR	0.46	18.8	В	LTR	0.48	19.1	В	L	0.18	14.9	В	
									TR	0.32	17.3	В	
Westbound	LTR	0.24	6.8	Α	LTR	0.28	7.2	Α	LTR	0.28	7.2	Α	
Northbound	LTR	0.43	12.3	В	LTR	0.43	12.3	В	LTR	0.43	12.3	В	
	Interse		13.6	В	Interse		13.7	В	Inters	ection	12.9	В	
Arthur Kill R	oad & A	Allento	wn Lane	/ Vete	rans Ro	ad We	st						
Eastbound	LTR	0.03	10.4	В	LTR	0.03	10.5	В	LTR	0.10	34.2	С	
Westbound	LTR	0.91	39.1	D	LTR	1.09	82.4	F +	L	0.59	21.2	С	
									TR	0.35	16.9	В	
Northbound	LTR	0.87	17.7	В	LTR	0.93	22.2	С	LT	0.25	17.6	В	
									R	0.34	6.6	Α	
Southbound	LTR	0.72	25.7	С	LTR	0.79	30.5	С	LTR	0.51	22.1	С	
	Interse		26.3	С	Interse	ection	45.1	D	Inters	ection	15.8	В	
Arthur Kill R	oad & I	North E	3ridge St	reet									
Westbound	LR	0.75	25.9	С	LR	1.05	70.8	E+					
Northbound	T	0.59	9.0	Α	Т	0.63	20.7	С		Unmit	igated		
Southbound	T	0.49	11.9	В	T	0.61	12.9	В		Omm	igateu		
	Interse		15.2	В	Interse	ection	36.6	D					
Arthur Kill R	oad & I	Richm	ond Valle	y Roa	d				1				
Eastbound					L	0.18	36.3	D					
					TR	0.32	40.2	D					
Westbound	LR	0.65	26.4	С	L	0.27	36.2	D					
					TR	0.90	71.5	E+					
Northbound					L	0.06	13.2	В		Unmit	igated		
	TR	0.74	14.3	В	TR	0.88	36.4	D					
Southbound	LT	1.28	155.0	F	L	0.85	51.7	D					
					TR	0.52	15.5	В					
	Interse	ection	68.2	E	Interse	ection	37.8	D					

Table 20-6, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis

Weekday AM Peak Hour

•								•	eekuz	ty 11111	1 Cuit	Hour
		2019 N	lo Build			2019 E	Build			Mitig	ation	
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
					alized In	tersec	tions					
Page Avenue	e & Ric	hmond	d Valley F	Road¹								
Eastbound	L	0.21	22.1	С	L	0.47	27.6	С	L	0.49	30.4	С
	TR	0.30	22.9	С	TR	0.41	24.7	С	TR	0.40	25.8	С
Westbound	LTR	0.30	22.8	С	LTR	0.34	23.5	С	L	0.13	32.8	С
									TR	0.37	36.7	D
Northbound	L	0.50	19.3	В	L	0.73	25.6	С	L	0.60	23.9	С
	TR	0.75	22.5	С	TR	0.75	22.8	С	TR	0.73	23.8	С
Southbound	LTR	0.41	11.5	В	LTR	0.43	11.8	В	LT	0.30	14.6	В
									R	0.18	11.7	В
	Interse	ection	19.0	В	Interse	ction	20.7	С	Inters	ection	22.9	С
Page Avenue												
Eastbound	LT	0.77	38.9	D	LT	0.95	61.6	E+	L	0.45	27.8	С
									LT	0.44	27.6	C
	R	0.07	20.7	С	R	0.09	20.9	С	R	0.06	21.9	C
Westbound	LTR	0.09	21.0	C	LTR	0.09	21.0	C	R	0.26	39.1	D
Northbound	TR	0.39	13.9	В	TR	0.43	13.4	В	TR	0.59	23.0	C
Southbound	LT	0.48	13.7	В	LT	0.50	14.0	В	LT	0.29	10.1	В
Coduibodila	Interse		19.2	В	Interse		24.5	С		ection	20.8	С
Boscombe A						CHOIT	24.0		IIICIS	COLIOIT	20.0	
Eastbound	L	1.11	101.1	F	L	1.32	183.6	F+				
Lastbourid	TR	0.29	4.2	Α	TR	0.29	4.2	A				
Westbound	LT	0.29	61.0	E	LT	0.89	61.0	E				
Westbound	R	0.42	30.0	С	R	0.03	30.0	С				
Northbound	LTR	0.42	79.1	E	LTR	0.42	79.1	E		Unmit	tigated	
Southbound	LT	0.80	79.1	E	LT	0.80	79.1	E				
Southbound	R	0.23	12.5	В	R	0.80	12.8	В				
	Interse		61.0	E	Interse		97.8	F				
Votorono Do					merse	CHOIT	91.0	Г				
Veterans Ro Eastbound	LTR	0.39	29.8	C	LTR	0.41	29.8	D		0.17	38.2	D
Easibouriu	LIK	0.39	29.0	C	LIK	0.41	29.0	U	TR	0.17	28.9	С
Moothound	LTR	0.02	40.0	D	LTR	0.02	49.5	D +			41.6	D
Westbound	LIK	0.83	40.9	ען	LIK	0.92	49.5	דע	L	0.70		С
Northbound	LTD	0.60	20.0	_	LTD	0.60	20.0	_	TR	0.36	20.4	С
Northbound Southbound	LTR LTR	0.60	28.8 24.4	C	LTR LTR	0.60	28.8 24.4	C C	LTR LTR	0.59	27.6 23.6	С
Southbound												
Votens D	Interse		33.1	C	Interse		37.0	D	inters	ection	27.8	С
Veterans Ro						1		- ·		0.04	24.4	
Eastbound	L	0.79	46.4	D	L	1.00	88.7	F +	L	0.61	31.4	С
10/4/	TR	0.55	28.7	С	TR	0.61	30.6	C .	TR	0.47	24.6	С
Westbound	L	1.18	119.1	F	L	1.31	168.2		L	0.79	52.8	D
	T	0.42	9.4	Α	T	0.57	9.5	Α	TR	0.32	28.6	С
N 1 (17 :	R	0.03	28.9	С	R	0.03	19.2	В		0.10	05.4	
Northbound	LT	0.45	29.9	С	LT	0.45	29.9	С	L	0.18	25.1	С
	R	0.30	27.5	С	R	0.30	27.5	C	TR	0.40	31.5	С
Southbound	L	0.25	30.4	С	L	0.25	30.4	С	L	0.21	20.7	С
	TR	0.60	38.9	D	TR	0.60	38.9	D	TR	0.35	27.9	С
	Interse	ection	42.8	D	Interse	ection	53.0	D	Intersection 31.1 C			

Table 20-6, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service **Analysis**

Weekday AM Peak Hour

								•	cckuz	•		HOU	
	2	019 N	o Build			2019 E	Build			Mitig			
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	
				Sigr	nalized l	nterse	ctions						
Page Avenu	e & Am	boy R	oad										
Eastbound	L	0.99	63.1	Е	L	1.03	74.2	E+	L	0.95	49.9	D	
	TR	0.32	14.4	В	TR	0.32	14.4	В	TR	0.31	13.1	В	
Westbound	L	0.29	26.7	С	L	0.29	26.7	C	L	0.26	25.2	С	
	TR	0.75	38.8	D	TR	0.75	38.8	D	TR	0.73	36.5	D	
Northbound	L	0.27	21.2	С	L	0.30	22.0	С	L	0.33	24.4	С	
	Т	0.78	32.8	С	Т	0.87	39.4	D	TR	0.54	24.3	С	
	R	0.10	17.9	В	R	0.10	17.9	В					
Southbound	L	0.27	26.9	С	L	0.36	31.6	С	L	0.22	22.3	С	
	T	0.51	28.7	С	TR	0.56	29.4	С	Т	0.59	27.2	С	
	R	0.44	27.5	С	R	0.46	27.3	С	R	0.49	25.2	С	
	Interse	ection	34.5	С	Interse	ection	38.0	D	Inters	ection	29.5	С	
Page Avenu	e & Hyla	an Boı	ulevard										
Eastbound	L	1.00	74.8	Ε	L	1.12	110.6	F+					
	TR	0.32	13.7	В	TR	0.32	13.7	В					
Westbound	L	0.07	11.7	В	L	0.07	11.7	В					
	TR	0.39	14.7	В	TR	0.42	15.0	В		Unmit	iaatad		
Northbound	LTR	0.21	34.6	С	LTR	0.21	34.6	С		Unimil	igated		
Southbound	L	0.75	55.0	D	L	0.82	62.0	E+					
	TR	0.38	37.6	D	TR	0.41	38.2	D					
	Interse	ection	30.7	С	Interse	ection	37.7	D					
Bloomingda	le Road	I & Am	boy Ro	oad &	Pleasan	t Plair	ıs Aver	nue¹					
Eastbound	LTR	0.11	15.0	В	LTR	0.11	15.0	В	LTR	0.15	21.7	С	
Westbound	L	0.47	20.5	С	L	0.52	21.7	С	L	0.54	24.4	С	
	T	0.04	14.2	В	T	0.04	14.2	В	Т	0.04	14.8	В	
	R	0.22	16.2	В	R	0.22	16.2	В	R	0.23	16.9	В	
Northbound	LTR	0.58	22.2	С	LTR	0.61	23.0	С	LTR	0.57	21.3	С	
Southbound	L	0.77	38.6	D	L	0.80	41.5	D	L	0.77	43.0	D	
	TR	0.48	19.8	В	TR	0.48	19.8	В	TR	0.60	28.1	С	
	Interse	ection	22.8	С	Interse	ection	23.6	C	Inters	ection	26.2	С	
				Unsig	nalized	Inters	ections	3					
North Bridge	Street	& The	Route										
Westbound	L	0.74		D	L	1.13	104.9	F+					
Southbound	Т	0.27	0.0	Α	Т	0.28	0.0	Α		Unmit	igated		
Notos: L - La	oft Turn	T _ T	brough	D - D	iaht Tur	- 100		l of C	onioo I	-D - Ca	athaund	\A/D	

Notes: L = Left Turn, T = Through, R = Right Turn, LOS = Level of Service, EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound

+ Denotes a significant adverse traffic impact.

(1) Intersection not impacted during the weekday AM peak hour; analysis presented to

Intersection not impacted during the weekday AM peak hour; analysis presented to demonstrate the proposed mitigation measures would not result in additional significant adverse traffic impacts.

Table 20-7 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday Midday Peak Hour

								wee	ekday N			Hour
		2019 No				2019 E				Mitiga		
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)		Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
					nalized I	ntersect	ions					
Veterans Roa					1			1		1	1	
Eastbound	LTR	0.87	31.6	С	LTR	0.95	41.3	D	L	0.30	15.1	В
			_					-	TR	0.73	22.8	С
Westbound	LTR	0.19	7.6	Α	LTR	0.28	7.3	Α	LTR	0.28	7.3	Α
Northbound	LTR	0.89	13.0	В	LTR	0.92	13.1	В	LTR	0.92	13.1	В
	Interse		21.0	С		ection	25.2	С	Interse	ection	16.1	В
Arthur Kill Ro								-				_
Eastbound	LTR	0.04	10.5	В	LTR	0.04	10.5	В	LTR	0.12	33.6	С
Westbound	LTR	1.02	59.6	E	LTR	1.35	185.0	F+		0.87	40.2	D
								_	TR	0.46	21.9	С
Northbound	LTR	0.97	30.4	С	LTR	1.18	96.4	F+	LT	0.34	15.7	В
				_				_	R	0.41	7.6	Α
Southbound	LTR	1.90	435.4	F	LTR	2.45	684.4	F+	LTR	1.00	58.9	Е
	Interse		156.1	F	Inters	section	277.8	F	Interse	ection	30.9	С
Arthur Kill Ro												
Westbound	LR	0.73	24.8	С	LR	1.52	262.3	F+				
Northbound	Т	0.64	13.8	В	Т	0.79	11.8	В		Unmitig	ated	
Southbound	Т	0.58	10.6	В	Т	0.80	13.7	В		Ommag	jatou	
	Interse		15.9	В	Inters	section	109.5	F				
Arthur Kill Ro	ad & Rich	mond \	/alley R	oad								
Eastbound					L	1.03	127.9	F+				
					TR	0.88	53.4	D +				
Westbound	LR	0.93	47.4	D	L	0.64	47.7	D				
					TR	1.08	100.6	F+				
Northbound					L	0.24	24.2	С		Unmitig	jated	
	TR	0.56	10.3	В	TR	0.84	41.9	D				
Southbound	LT	1.30	157.0	F	L	1.66	367.9	F				
					TR	0.72	43.2	D				
	Interse		79.4	Е	Inters	section	95.5	F				
Page Avenue	& Richme	ond Vall										
Eastbound	L	0.27	23.1	С	L	1.37	215.8	F+	L	0.96	37.7	D
	TR	0.45	25.5	С	TR	0.90	47.3	D +	TR	0.70	27.5	С
Westbound	LTR	0.30	22.8	С	LTR	0.54	28.0	С	L	0.12	36.0	D
									TR	0.56	44.4	D
Northbound	L	0.87	43.0	D	L	1.52	270.9		L	1.39	225.1	F+
	TR	0.62	19.3		TR	0.62	20.2		TR	0.71	29.1	С
Southbound	LTR	0.49	13.5	В	LTR	0.53	14.9	В	LT	0.45	22.2	С
									R	0.26	10.8	В
	Interse		21.4	С		section	85.5	F	Interse	ection	55.6	Е
Page Avenue			nue & Sc		ridge St							
Eastbound	LT	0.70	34.1	С	LT	1.37	208.5	F +	L	0.64	32.7	С
									LT	0.64	32.6	С
	R	0.08	20.8	С	R	0.13	21.4	С	R	0.08	22.2	С
Westbound	LTR	0.09	20.9	С	LTR	0.17	22.5	С	R	0.23	38.4	D
Northbound	TR	0.39	10.9	В	TR	0.53	8.6	Α	TR	0.74	26.4	С
Southbound	LT	0.57	15.1	В	LT	0.62	16.2	В	LT	0.36	10.7	В
	Interse	ection	17.1	В	Inters	section	61.7	Е	Interse	ection	23.9	С

Table 20-7, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday Midday Peak Hour

I	2	019 No	Build			2019 E	Suild	,,,,,,	ekuay IV	Mitigat		iioui
		v/c		i	Long	2019 E		i	Lone	v/c	Delay	1
Intersection	Lane Group	Ratio	Delay (sec)	100	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	(sec)	LOS
intersection	Огоир	itatio	(360)			ntersect		LOG	Group	itatio	(360)	LOO
Boscombe Av	renije & T	he Rout	e 440 R		ianzea n	iilei 3ect	10113					
Eastbound		1.00	64.9	E	L	1.83	413.7	F+				
Laotodila	TR	0.31	4.4	A	TR	0.31	4.4	Α				
Westbound	LT	0.90	61.7	E	LT	0.90	61.7	E				
770012001110	R	1.23	156.3	F	R	1.23	156.3	F				
Northbound	LTR	0.44	70.0	E	LTR	0.44	70.0	E		Unmitig	ated	
Southbound	LT	0.57	60.2	Е	LT	0.57	60.2	Е				
	R	0.17	11.9	В	R	0.22	12.4	В				
	Interse		76.0	Е		ection	221.1	F				
Veterans Roa				е								
Eastbound	LTR	0.97	43.2	D	LTR	0.97	47.2	D	L	0.36	41.0	D
									TR	0.99	36.6	D
Westbound	LTR	0.71	35.3	D	LTR	0.90	46.3	D +	L	0.64	42.7	D
									TR	0.52	26.5	С
Northbound	LTR	2.07	81.8	F	LTR	2.07	81.8	F	LTR	1.25	39.8	D
Southbound	LTR	1.21	135.8	F	LTR	1.21	135.8	F	LTR	0.93	43.2	D
	Interse		80.5	F		ection	80.9	F	Interse	ection	37.9	D
Veterans Roa	d West &	North B			Bricktov	vn Way						
Eastbound	L	1.02	91.9	F	L	1.90	463.0	F+	L	0.81	47.4	D
	TR	0.76	36.5	D	TR	0.99	66.7	E+	TR	0.75	33.8	С
Westbound	L	1.70	341.5	F	L	2.96	900.7	F+	L	1.25	189.2	F
	T	0.51	9.8	Α	T	0.80	10.9	В	TR	0.43	30.4	С
	R	0.03	38.0	D	R	0.03	16.3	В				
Northbound	LT	0.53	31.4	С	LT	0.53	31.4	С	L	0.12	24.3	С
	R	0.30	27.4	С	R	0.30	27.4	С	TR	0.52	34.1	С
Southbound	L	0.32	31.6	С	L	0.32	31.6	С	L	0.33	23.0	С
	TR	0.97	76.6	E	TR	0.97	76.6	Е	TR	0.56	32.9	С
	Interse		75.6	E	Inters	ection	166.3	F	Interse	ection	47.6	D
Page Avenue				-				1_				-
Eastbound	<u>L</u>	0.72	27.5	С	L	0.87	42.2	D	L_	0.85	36.9	D
) A () .	TR	0.17	12.6	В	TR	0.20	13.0	В	TR	0.20	13.6	В
Westbound	<u>L</u>	0.08	21.7	С	L	0.09	21.8	С	L	0.10	25.5	С
N. (1)	TR	0.57	29.2	С	TR	0.65	32.2	С	TR	0.79	43.9	D
Northbound	<u> </u>	0.14	18.8	В	L	0.23	21.6	С	L	0.21	20.4	С
	<u>T</u>	0.69	28.5	С	T	0.87	39.0	В	TR	0.47	21.1	С
0	R	0.06	17.5	В	R	0.06	17.4	В		0.40	07.0	
Southbound	<u> </u>	0.58	34.5	С	L	1.05	112.1	F+	L	0.48	27.3	С
	<u>T</u>	0.50	24.3	С	T	0.73	25.0	С	T	0.71	28.5	С
	R	0.45	23.7	С	R	0.54	21.0	С	R	0.53	23.9	С
	Intersection		25.8	С	Inters	ection	34.1	С	Interse	ection	28.2	С

Table 20-7, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday Midday Peak Hour

									Kuay IV.			
	2	019 No	Build			2019 E	Build			Mitigat	tion	
	Lane	v/c	Delay		Lane	v/c	Delay	'	Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
				Sign	nalized l	ntersect	ions					
Page Avenue	& Hylan E	Bouleva	rd									
Eastbound	L	0.45	17.9	В	L	0.62	23.7	С				
	TR	0.10	11.6	В	TR	0.10	11.6	В				
Westbound	L	0.02	11.0	В	L	0.02	11.0	В				
	TR	0.29	13.5	В	TR	0.35	14.1	В		Unmitio	uatad	
Northbound	LTR	0.08	32.6	С	LTR	0.08	32.6	С	Unmitigated			
Southbound	L	0.68	49.0	D	L	0.97	84.2	F+				
	TR	0.37	37.3	D	TR	0.48		D				
	Interse		23.5	С		section		С				
Bloomingdale Road & Amboy Road & Pleasant Plains Avenue ¹												
Eastbound	LTR	0.04	14.3	В	LTR	0.04	14.3	В	LTR	0.05	20.6	С
Westbound	L	0.52	21.5	С	L	0.63	24.6	С	L	0.66	28.0	С
	T	0.02	14.1	В	Т	0.02	14.1	В	T	0.02	14.6	В
	R	0.16	15.4	В	R	0.16	15.4	В	R	0.16	16.1	В
Northbound	LTR	0.61	22.7	С	LTR	0.72	26.4	С	LTR	0.67	24.0	С
Southbound	L	0.28	18.3	В	L	0.32		В	L	0.31	24.0	С
	TR	0.41	18.6	В	TR	0.41		В	TR	0.52	25.9	С
	Interse	ection	20.3	С	Inters	section	22.6	С	Interse	ection	24.7	С
				Unsig	ınalized	Intersec	ctions					
North Bridge	Street 7 T	he Rout	te 440 O	ff-Ran	np		-					
Westbound	L	0.62	22.4	С	L	1.72	354.0	F+	Lipmitigated			
Southbound	Т	0.29	0.0	Α	Т	0.33	0.0	Α	Unmitigated			
Arthur Kill Ro	ad & Sou	th Bridg	je Street	l								
Northbound	TR	0.56	0.0	Α	TR	0.85	0.0	Α	Unmitigated			
Southbound	LT	0.31	8.0	Α	LT	0.95	91.2	F+	Unmitigated			

Notes: L = Left Turn, T = Through, R = Right Turn, LOS = Level of Service, EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound

⁺ Denotes a significant adverse traffic impact.

Intersection not impacted during the weekday midday peak hour; analysis presented to demonstrate the proposed mitigation measures would not result in additional significant adverse traffic impacts.

Table 20-8 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday PM Peak Hour

								<u> </u>	Veekda	y PM	<u>Peak</u> f	<u>tour</u>
		2019 N	o Build			2019	Build			Mitiga	tion	
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
				Signa	alized Int	tersectio	ns					
Bloomingdale	Road & V	<u>Voodr</u> o	w Road	School	Drivewa	у						
Eastbound												
(School												
Driveway)	L	0.08	25.1	С	L	0.08	25.1	С	L	0.07	20.7	С
	TR	0.29	27.9	С	TR	0.29	27.9	С	TR	0.23	22.7	С
Eastbound												
(Woodrow		0.50	50.0	_		0.50	50.0	_		0.50	50.0	_
Road)	LTR	0.50	59.2	E	LTR	0.50	59.2	E.	LTR	0.50	59.2	E
Westbound	L	0.90	65.3	E	L	0.97	80.3	F+	L	0.77	42.4	D
Namthalaarus -1	R	0.27	27.9	С	R	0.27	27.9	С	R	0.22	22.6	С
Northbound	LT	0.62	17.2	В	LT	0.65	18.0	В	LTR	0.74	25.2	C
Southhound	R	0.38	13.1	B B	R	0.41	13.5	B B	R	0.44	17.5	В
Southbound	L	0.23	12.1	В	L TD	0.24	12.4	В	L	0.32	17.7	
	TR	0.25	11.3 23.7	С	TR	0.27	11.5 26.2	C	TR	0.31	15.1 23.9	B C
Votorana Baad	Interse				inters	ection	20.2	<u> </u>	Interse	ะบแบท	∠3.9	U
Veterans Road			174.9	nue F	LTD	1 / 2	220.3	F+	l 1	0.56	10 F	Ь
Eastbound	LTR	1.32	174.9	F	LTR	1.43	220.3	FF	TR	0.56 1.00	19.5 51.6	B D
Westbound	LTR	0.25	6.1	Α	LTR	0.34	7.2	Α	LTR	0.50	9.6	A
Northbound	LTR	0.25	13.4	В	LTR	0.88	13.6	В	LTR	0.88	13.6	В
Northbourid	Interse		99.2	F		ection	122.6	F	Interse		28.8	С
Arthur Kill Roa							122.0	11	1 1116156	JUIUII	20.0	ı U
Eastbound	LTR	0.04	10.5	В	LTR	0.04	10.5	В	LTR	0.09	31.5	С
Westbound	LTR	1.47	240.4	F	LTR	1.84	402.4	F+	I	1.17	113.2	F
VVCStDOUTIU	LIIX	1.77	270.7	<u> </u>	LIIX	1.04	702.7	i i	TR	0.32	15.1	В
Northbound	LTR	1.11	69.2	E	LTR	1.31	152.0	F+	LT	0.40	21.1	С
TTOTTIBOUTIO	LIIX	1.11	00.2		LIIX	1.01	102.0	<u> </u>	R	0.47	9.2	A
Southbound	LTR	1.90	436.4	F	LTR	2.44	676.1	F+	LTR	1.14	110.5	F
Codinbound	Interse		220.0	F		ection	363.4	F	Interse		64.8	E
Arthur Kill Roa					1 111013	500011	1 000.7	1.	1 1110130	550511	U 7.0	<u> </u>
Westbound	LR	0.93	41.1	D	LR	1.62	303.7	F+				
Northbound	T	0.70	12.1	В	T	0.84	21.3	C	1			
Southbound	Ť	0.79	14.1	В	Ť	1.00	25.1	C	1	Unmitio	gated	
- Countrio	Interse		21.3	С	•	ection	125.6	F	1			
Arthur Kill Roa						200011	120.0	1.	1			
Eastbound			J 5 10			1.02	130.0	F+				
					TR	1.08	108.3	F+	1			
Westbound	LR	1.08	84.9	F	L	1.29	209.8	F+	1			
					TR	1.28	183.6	F+	1			
Northbound					L	0.43	38.0	D	1	Unmitig	gated	
	TR	0.75	14.7	В	TR	1.15	118.3	F+				
Southbound	LT	2.60	735.1	F	L	1.39	237.3	F				
					TR	0.83	34.2	С	1			
	Interse	ection	341.1	F		ection	124.1	F	1			
				<u> </u>				ı :	1			

Table 20-8, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday PM Peak Hour

		2019 N	lo Build			2019	Build			Mitiga	tion	
	Lane	v/c	Delay	1	Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
				Sigr	nalized Ir	ntersectio		1				•
Page Avenue 8	& Richmo	nd Valley	Road									
Eastbound	L	0.31	24.0	С	L	1.36	211.7	F +	L	0.93	42.4	D
	TR	0.49	26.2	С	TR	0.91	48.9	D +	TR	0.71	31.7	С
Westbound	LTR	0.40	24.4	С	LTR	0.94	63.4	E+	L	0.26	38.4	D
									TR	0.60	44.8	D
Northbound	L	1.16	110.4	F	L	1.99	468.7	F +	L	1.80	408.6	F +
	TR	0.72	18.8	В	TR	0.72	17.9	В	TR	0.83	35.2	D
Southbound	LTR	0.66	18.5	В	LTR	0.71	19.8	В	LT	0.69	28.2	С
									R	0.27	11.4	В
	Inters	ection	29.6	С	Inters	section	105.5	F	Inters	ection	79.1	E
Page Avenue/	Boscomi	be Avenu	e & South	Bridge S	treet / Ga	as Statior	1					
Eastbound	LT	0.94	58.4	E	LT	1.65	330.2	F +	L	0.77	38.7	D
									LT	0.76	38.3	D
	R	0.17	21.9	С	R	0.23	22.6	С	R	0.16	23.2	С
Westbound	LTR	0.09	20.9	С	LTR	0.13	21.7	С	R	0.25	38.8	D
Northbound	TR	0.48	14.1	В	TR	0.61	11.9	В	TR	0.85	30.9	С
Southbound	LT	0.72	18.6	В	LT	0.77	20.4	С	LT	0.46	11.7	В
	Inters	ection	24.3	С	Inters	section	93.8	F	Inters	section	27.2	С
Boscombe Ave	enue & Th	ne Route	440 Ramps	;								
Eastbound	L	1.14	112.1	F	L	1.86	424.5	F+				
	TR	0.37	4.7	Α	TR	0.37	4.7	Α				
Westbound	LT	1.13	124.0	F	LT	1.13	124.0	F				
	R	1.17	130.6	F	R	1.17	130.6	F		Unmiti	actod	
Northbound	LTR	0.69	116.6	F	LTR	0.69	116.6	F		Offiffility	gat e u	
Southbound	LT	0.72	70.2	E	LT	0.72	70.2	E				
	R	0.26	12.9	В	R	0.31	13.5	В				
	Inters	ection	91.3	F	Inters	section	221.7	F				
Veterans Road	West &	Tyrellan A	venue									
Eastbound	LTR	1.13	47.8	D	LTR	1.13	55.3	E+	L	0.30	38.5	D
									TR	1.15	43.4	D
Westbound	LTR	0.95	52.9	D	LTR	1.13	105.8	F +	L	0.76	45.7	D
									TR	0.58	26.3	С
Northbound	LTR	1.43	131.6	F	LTR	1.43	131.6	F	LTR	1.14	78.0	E
Southbound	LTR	1.21	138.1	F	LTR	1.21	138.1	F	LTR	1.05	74.4	E
		ection	93.3	F		section	107.9	F	Inters	section	55.4	E
Veterans Road	West &	North Brid	dge Street/	Bricktov	vn Way							
Eastbound	L	1.40	234.1	F	L	2.70	816.4	F +	L	1.12	119.1	F
	TR	0.84	42.5	D	TR	1.05	82.6	F +	TR	0.79	36.0	D
Westbound	L	2.21	555.0	F	L	2.74	794.6	F +	L	1.40	252.5	F
	Т	0.53	6.4	Α	Т	0.83	6.9	Α	TR	0.46	30.9	С
	R	0.04	8.4	Α	R	0.04	3.8	Α				
Northbound	LT	0.57	32.6	С	LT	0.57	32.6	С	L	0.17	25.0	С
	R	0.35	28.3	С	R	0.35	28.3	С	TR	0.54	34.7	С
Southbound	L	0.45	34.0	С	L	0.45	34.0	С	L	0.47	25.6	С
	TR	1.19	145.7	F	TR	1.19	145.7	F	TR	0.69	37.3	D
	14	ection	123.5	F	Intore	section	214.9	F	Intere	section	61.3	Е

Table 20-8, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Weekday PM Peak Hour

	1	0040 11	- D:11.1		1	0040	D!!!		w eeku			Hour	
		2019 N		i		2019		i		Mitiga			
1.4	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay		
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	
	A L	D		Sign	alized In	tersecti	ons						
Page Avenue 8			70.0	I =		4.45	444=	l = .		4.45	445.4	T = .	
Eastbound	L	1.04	76.6	E	L	1.15	114.7	F+	L	1.15	115.4	F+	
	TR	0.30	13.0	В	TR	0.30	13.0	В	TR	0.30	13.0	В	
Westbound	L	0.21	23.7	С	L	0.21	23.7	С	<u> </u>	0.22	23.9	C	
	TR	0.85	43.9	D	TR	0.85	43.9	D	TR	0.83	41.4	D	
Northbound	L	0.29	24.2	С	L	0.56	43.8	D	L	0.56	43.8	D	
	Т	0.90	44.8	D	Т	1.07	85.1	F+	TR	0.62	25.7	С	
	R	0.11	19.2	В	R	0.11	19.2	В					
Southbound	L	1.33	226.8	F	L	1.52	295.7	F+	L	0.77	55.7	Е	
	Т	0.68	29.8	С	T	0.87	34.2	С	T	0.87	40.9	D	
	R	0.62	29.1	С	R	0.70	27.5	С	R	0.70	31.5	С	
	Interse		47.7	D	Inters	ection	64.7	E	Inters	ection	43.8	D	
Page Avenue &	& Hylan I	Bouleva											
Eastbound	L	1.12	115.6	F	L	1.43	241.0	F +					
	TR	0.15	12.0	В	TR	0.15	12.0	В					
Westbound	L	0.02	11.0	В	L	0.02	11.0	В					
	TR	0.38	14.4	В	TR	0.41	14.9	В		Unmitigated			
Northbound	LTR	0.17	33.9	С	LTR	0.17	33.9	С		Offiffili	galeu		
Southbound	L	0.80	57.6	E	L	1.06	107.5	F+					
	TR	0.48	39.5	D	TR	0.57	41.9	D					
	Interse	ection	39.9	D	Inters	ection	70.0	E					
Bloomingdale	Road & A	Amboy	Road &	Pleasan	t Plains	Avenue							
Eastbound	LTR	0.07	14.6	В	LTR	0.07	14.6	В	LTR	0.12	25.7	С	
Westbound	L	0.67	26.2	С	L	0.78	31.6	С	L	0.83	38.7	D	
	Т	0.03	14.2	В	Т	0.03	14.2	В	T	0.04	15.9	В	
	R	0.21	16.0	В	R	0.21	16.0	В	R	0.23	18.1	В	
Northbound	LTR	0.94	46.1	D	LTR	1.05	72.4	E+	LTR	0.94	42.6	D	
Southbound	L	0.55	28.8	С	L	0.62	34.4	С	L	0.54	31.7	С	
	TR	0.53	20.7	С	TR	0.53	20.7	С	TR	0.62	27.1	С	
	Interse	ection	31.3	С	Inters	ection	43.6	D	Inters	ection	35.2	D	
	ı			Unsia	nalized l								
North Bridge S	treet & T	he Rou	te 440 C				-						
Westbound	L	0.92	50.6	F	L	1.93	446.4	F+					
Southbound	Т	0.00	0.0	Α	Т	0.33	0.0	Α	Unmitigated				
Arthur Kill Roa	d & Sout		e Street										
Northbound	TR	0.65	0.0	Α	TR	0.94	0.0	Α					
Southbound	LT	0.51	16.6	С	LT	2.54	770.9	F+	1	Unmiti	gated		
Notes: L = Left	Turn, T =				n, LOS =		Service,	EB = E	astbound	I, WB = '	Westbou	nd,	

Notes: L = Left Turn, T = Through, R = Right Turn NB = Northbound, SB = Southbound + Denotes a significant adverse traffic impact.

Table 20-9 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Saturday Peak Hour

									Sat	turday	<u>Peak</u>	<u>Hou</u> r
		2019 No	Build			2019	Build			Mitiga	tion	
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
					alized In		ions					
Bloomingdale	Road &	Woodro	w Road	School	l Drivew	<i>y</i> ay						
Eastbound												
(School								_				
Driveway)		0.00	0.0	Α	L	0.00	0.0	Α	L	0.00	0.0	Α
F " 1	TR	0.00	0.0	Α	TR	0.00	0.0	Α	TR	0.00	0.0	Α
Eastbound												
(Woodrow Road)	LTR	0.37	46.9	D	LTR	0.37	46.9	D	LTR	0.37	46.9	D
Westbound	L	0.90	62.7	E	L	1.02	87.9	F+	LIN	0.90	58.5	E
VVESIDOUTIU	R	0.90	27.1	C	R	0.22	27.1	C	R	0.20	24.5	С
Northbound	LT	0.51	15.9	В	LT	0.54	16.5	В	LTR	0.58	19.2	В
Northboaria	R	0.32	13.2	В	R	0.36	13.8	В	R	0.37	15.6	В
Southbound	L	0.21	12.3	В	L	0.22	12.6	В	L	0.24	14.7	В
Countracting	TR	0.33	13.0	В	TR	0.37	13.4	В	TR	0.39	15.4	В
	Interse		23.9	C	Inters		29.D	C		ection	25.3	C
Veterans Roa												
Eastbound	LTR	1.31	166.6	F	LTR	1.62	304.5	F+	L	0.62	21.5	С
									TR	1.01		D
Westbound	LTR	0.35	7.8	Α	LTR	0.50	9.3	Α	LTR	0.50		Α
Northbound	LTR	1.15	15.2	В	LTR	1.19	15.7	В	LTR	1.19		В
	Interse	ection	82.2	F	Inters	ection	142.5	F	Inters	ection		С
Arthur Kill Ro	ad & Alle	entown	Lane/ Ve	terans	Road W	/est		•	•			
Eastbound	LTR	0.04	10.5	В	LTR	0.04	10.5	В	LTR	0.09	29.8	С
Westbound	LTR	1.65	318.2	F	LTR	2.23	577.6	F+	L	1.32	174.5	F
									TR	0.42		В
Northbound	LTR	1.37	180.5	F	LTR	1.66	307.5	F+	LT	0.40		С
									R	0.64		В
Southbound	LTR	2.50	706.7	F	LTR	3.51	1161.5		LTR	1.17		F
	Interse		333.8	F	Inters	ection	566.8	F	Inters	ection	83.3	F
Arthur Kill Ro					,		,					
Westbound	LR	0.97	48.1	D	LR	2.01	481.4	F+				
Northbound	T	0.80	15.0	В	T	1.00	41.8	D		Unmitig	nated	
Southbound	T	0.79	14.4	В	T	1.13	76.5	E+		Orminas	gatoa	
	Interse		24.1	С	Inters	ection	219.3	F				
Arthur Kill Ro		hmond	Valley R	oad				1	1			
Eastbound					L		248.9					
100			10- :		TR	1.63	337.8	F+				
Westbound	LR	1.19	125.4	F	L	0.45	30.3	С				
					TR	1.46	257.7	F+				
Northbound		0 = 0	45.0		L	0.66	55.0	D .	Unmitigated			
0 "	TR	0.78	15.8	В	TR	1.28	172.3	F+				
Southbound	LT	3.06	940.8	F	L	2.02	509.4	F				
	1 (- 4 : -	404.7	_	TR	0.96	55.6					
	Interse	ection	404.7	F	Inters	ection	222.1	F				

Table 20-9, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Saturday Peak Hour

					1				Sat	turday		Hour
		2019 No	Build			2019 I	Build		ļ	Mitiga	tion	
Intersection	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS	Lane Group	v/c Ratio	Delay (sec)	LOS
	<u> </u>		. ,		alized In							
Page Avenue	& Richm	ond Val	ley Road									
Eastbound	L	0.44	27.1	С	L	2.36	652.8	F+	L	1.96	490.0	F+
	TR	0.50	26.3	С	TR	1.01	69.8	E+	TR	0.89		D +
Westbound	LTR	0.42	24.7	С	LTR	1.15	125.2	F+	L	0.34		D +
									TR	0.94		E+
Northbound	L	1.65	319.2	F	L	3.19	1015.7	F+	L	2.30	625.0	F+
	TR	0.83	22.6	С	TR	0.83	23.7	С	TR	0.86	33.4	С
Southbound	LTR	0.77	20.4	С	LTR	0.83	23.6	С	LT	0.65		С
									R	0.30	9.6	Α
	Interse		57.8	E	Interse		273.1	F	Inters	ection	186.6	F
Page Avenue												
Eastbound	LT	0.92	53.6	D	LT	1.91	448.4	F+	L	0.89		D
									LT	0.85		D
	R	0.14	21.5	С	R	0.20	22.2	С	R	0.14		С
Westbound	LTR	0.09	21.0	С	LTR	0.17	22.3	С	R ¹	0.36		D
Northbound	TR	0.52	15.8	В	TR	0.71	13.2	В	TR	0.96		D
Southbound	LT	0.70	18.2	В	LT	0.78	20.8	O I	LT	0.53		В
D	Interse		23.6	С	Interse	ection	130.5	F	Inters	ection	34.4	С
Boscombe Av						0.11	F	_				
Eastbound	L	1.11	97.8	F	L	2.13	543.8	F +				
10/	TR	0.50	5.9	А	TR	0.50	5.9	A				ļ
Westbound	LT	1.15	129.0	F	LT	1.15	129.0	F				ļ
Nicosti-i-	R	1.32	189.6	F	R	1.32	189.6	F		Unmitig	gated	ļ
Northbound	LTR	0.00	0.0 54.4	A D	LTR	0.00	0.0	A D		`		
Southbound	LT R	0.48	54.4 12.4	В	LT R	0.48	54.4	В				
	Interse		104.6	F	Interse		13.3 274.7	F				
Veterans Roa					iiiieise	JULIUII	£14.1	1	<u> </u>			
Eastbound	LTR	1.34	79.6	e E	LTR	1.34	105.8	F +	L	0.45	41.0	D
Lasibouriu	LIIN	1.04	7 3.0	+	-11X	1.04	100.0	• •	TR	1.36		E
Westbound	LTR	1.00	62.8	E	LTR	1.28	170.0	F+	1	0.80		D
	-111	1.00	JZ.U		-111	1.20	., 5.0		TR	0.83		D
Northbound	LTR	2.08	212.5	F	LTR	2.08	212.5	F	LTR	2.08		F
Southbound	LTR	1.64	323.7	F	LTR	1.64	323.7	F	LTR	1.22		F
	Interse		178.7	F	Interse		205.9	F		ection		F
Veterans Roa												
Eastbound	L	2.58		F	L	5.84	2237.2	F +	L	1.89	447.0	F
	TR	0.96	58.0	F	TR	1.23	150.1	F +	TR	0.93		D
Westbound	L	2.79	818.8	F	L	2.79	815.5	F	L	2.35	678.0	
	 T	0.70	7.6	Α	T	1.16	81.3	F+	TR	0.62		C
	R	0.04	9.0	Α	Ŕ	0.04	3.3	Α				
Northbound	LT	0.81	43.2	D	LT	0.81	43.2	D	L	0.16	24.8	С
	R	0.38	28.7	С	R	0.38	28.7	С	TR	0.76		D
Southbound	L	0.51	35.4	D	L	0.51	35.4	D	L	0.76		D
								. — —	,			
Journalia	TR	1.43	244.4	F	TR	1.43	244.4	F	TR	0.83	45.8 136.5	D

Table 20-9, cont'd 2019 No Action, With Action, and Mitigation Conditions Level of Service Analysis Saturday Peak Hour

					1				Sa	turuay		Hour
		2019 No	Build			2019 I	Build			Mitiga	tion	
	Lane	v/c	Delay		Lane	v/c	Delay		Lane	v/c	Delay	
Intersection	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS	Group	Ratio	(sec)	LOS
				Signa	alized In	tersecti	ons					
Page Avenue	& Ambo	y Road										
Eastbound	L	1.29	171.5	F	L	1.48	253.0	F+	L	1.51	263.7	F+
	TR	0.23	13.3	В	TR	0.23	13.3	В	TR	0.23	13.3	В
Westbound	L	0.13	22.3	С	L	0.13	22.3	С	L	0.13	21.6	С
	TR	0.87	45.4	D	TR	0.87	45.4	D	TR	0.80	38.5	D
Northbound	L	0.26	22.3	С	L	0.56	47.6	D +	L	0.56	47.6	D +
	Т	0.95	50.2	D	T	1.20	132.7	F+	TR	0.67	25.5	С
	R	0.08	17.6	В	R	0.08	17.6	В				
Southbound	L	1.57	328.2	F	L	1.57	306.4	F	L	0.95	94.7	F
	Т	0.73	31.4	С	Т	1.01	48.7	D +	Т	1.01	63.3	E +
	R	0.56	27.1	С	R	0.66	23.1	С	R	0.66	28.3	С
	Interse		68.9	Е	Inters	ection	102.2	F	Inters	ection	71.7	E
Page Avenue	& Hylan											
Eastbound	L	0.81	38.6	D	L	1.16	128.9	F+				
	TR	0.15	12.0	В	TR	0.15	12.0	В				
Westbound	L	0.06	11.4	В	L	0.06	11.4	В	Unmitigated			
	TR	0.29	13.5	В	TR	0.34	14.0	В				
Northbound	LTR	0.14	33.4	С	LTR	0.14	33.4	С		Ommu	galeu	
Southbound	L	0.92	74.2	E	L	1.29	193.7	F+				
	TR	0.56	41.6	D	TR	0.69	46.4	D				
	Interse		31.4	С	Inters		70.4	Е				
Bloomingdale												
Eastbound	LTR	0.06	14.4	В	LTR	0.06	14.4	В	LTR	0.15	33.5	С
Westbound	L	0.77	30.8	С	L	0.96	51.9	D +	L	0.92	46.3	D
	Т	0.02	14.0	В	Т	0.02	14.0	В	TR	0.02	15.2	В
	R	0.15	15.3	В	R	0.15	15.3	В	R	0.16	16.6	В
Northbound	LTR	0.86	35.5	D	LTR	0.99	56.7	E+	LTR	0.91	39.2	D
Southbound	L	0.42	22.9	С	L	0.48	26.3	С	L	0.44	28.1	С
	TR	0.52	20.4	С	TR	0.52	20.4	С	TR	0.63	28.0	С
	Interse	ection	28.3	С	Inters		42.9	D	Inters	ection	36.8	D
					nalized I	ntersec	tions					
North Bridge	Street &				np							
Westbound	L	0.94		F	L	2.58	739.2	F+		Unmitig	nated	
Southbound	Т	0.33	0.0	Α	Т	0.37	0.0	Α		Ommu	Jaica	
Arthur Kill Ro												
Northbound	TR	0.72	0.0	Α	TR	1.12	0.0	Α	Unmitigated			
Southbound	LT	0.55	18.7	С	LT	5.60	302.7	F+	Offinitigated			

Notes: L = Left Turn, T = Through, R = Right Turn, LOS = Level of Service, EB = Eastbound, WB = Westbound, NB = Northbound, SB = Southbound

⁺ Denotes a significant adverse traffic impact.

⁽¹⁾ Though the approach delay is greater than mid-LOS D, the approach is not public right-of-way and is therefore not considered an impact.

Woodrow Road and Bloomingdale Road

The significant adverse impact at the westbound approach of this intersection during the weekday PM and Saturday peak hours could be fully mitigated by shifting 6 seconds and 3 seconds of green time from the eastbound (Woodrow Road) phase to the westbound phase, respectively. As shown in Tables 20-8 and 20-9, the significant adverse impacts could be fully mitigated.

Veterans Road East and Englewood Avenue

The significant adverse impact at the eastbound approach of this intersection during the weekday PM and Saturday peak hours could be fully mitigated by restriping the eastbound approach from one 24.5-foot moving lane, to one 12-foot left-turn lane and one 12.5-foot through/right-turn lane As shown in Tables 20-8 and 20-9, the significant adverse impact could be fully mitigated.

Arthur Kill Road and Veterans Road West/Allentown Lane

The significant adverse impact at the westbound approach of this intersection during the weekday AM, midday, PM, and Saturday peak hours, and the northbound and southbound approaches during the weekday midday, PM, and Saturday peak hours could be fully mitigated by (1) shifting the centerline of Arthur Kill Road (south of Veterans Road West) one foot to the west and restriping the northbound approach from one 20-foot moving lane to one 11-foot left-turn/through lane and one 10-foot right-turn lane, and (2) allowing northbound right-turns on red. Also, restriping the westbound approach from one 13-foot moving lane and one 9-foot parking lane to one 11-foot travel lane and one 11-foot through/right-turn lane (a loss of approximately 10 parking spaces) would be necessary to fully mitigate the impacts at this intersection. In addition, increasing the cycle length from 60 seconds to 90 seconds and creating a protected westbound left-turn phase with northbound right-turn overlap would be needed to fully mitigate the impacts at this intersection (see Tables 20-2 through 20-5 for proposed signal timing). As shown in Tables 20-6 through 20-9, the significant adverse impacts could be fully mitigated with these proposed mitigation measures.

Arthur Kill Road and North Bridge Street

The significant adverse impact at the westbound approach of this intersection during the weekday AM, midday, PM, and Saturday peak hours could not be mitigated (see Tables 20-6 through 20-9). In addition, the southbound approach of this intersection during the Saturday peak hour could not be mitigated (see Table 20-9).

Arthur Kill Road and Richmond Valley Road

The significant adverse impacts at the eastbound left-turn and through/right-turn lane groups of this intersection during the weekday midday, PM, and Saturday peak hours could not be mitigated (see Tables 20-3 through 20-5).

The significant adverse impact at the westbound through/right-turn lane group of this intersection during the weekday AM, midday, PM, and Saturday peak hours could not be mitigated (see Tables 20-2 through 20-5). In addition, the westbound left-turn lane group of this intersection during the weekday PM peak hour could not be mitigated (see Table 20-4).

The significant adverse impact at the northbound through/right-turn lane group of this intersection during the weekday PM and Saturday peak hours could not be mitigated (see Tables 20-4 and 20-5).

Page Avenue and Richmond Valley Road

The significant adverse impact at the eastbound left-turn and through/right-turn lane groups of this intersection during the weekday midday and PM peak hours, as well as the significant adverse impacts at the westbound approach during the weekday PM peak hour could be fully mitigated by (1) restriping the westbound approach from one 22-foot moving lane to one 11-foot left-turn lane and one 11-foot through/right-turn lane (a loss of approximately five parking spaces), and (2) prohibiting parking (installing "No Standing Anytime" sign) on the west curb of the southbound approach for approximately 100 feet (a loss of approximately six parking spaces) from the intersection to create a 10-foot right-turn lane. In addition, increasing the cycle length from 90 seconds to 120 seconds and creating a leading eastbound phase with southbound right-turn overlap would be necessary to fully mitigate these impacts at this intersection (see Tables 20-2 through 20-5 for proposed signal timing). As shown in Tables 20-7 and 20-8, these significant adverse impacts could be fully mitigated with the above proposed mitigation measures.

The significant adverse impacts at the northbound left-turn lane group of this intersection during the weekday midday and PM peak hours could not be fully mitigated (see Tables 20-3 and 20-4).

The significant adverse impacts at the eastbound left-turn and through/right-turn lane groups of this intersection as well as the westbound approach during the Saturday peak hour could not be fully mitigated (see Table 20-5).

The significant adverse impacts at the northbound left-turn lane group of this intersection during the Saturday peak hour could not be fully mitigated (see Table 20-5).

Though this intersection could not be fully mitigated, the New York City Department of Design and Construction (DDC) Capital Project HWR00509/SE-812 is currently in the preliminary design phase of evaluating the reconstruction and widening of Richmond Valley Road from Arthur Kill Road to Page Avenue to create new through and/or turning lanes in order to relieve congestion. As this project would not be constructed by 2019, this project was not included in the No Action; however, if constructed, these roadway improvements have the potential to further improve traffic operating conditions at this intersection.

Page Avenue/Boscombe Avenue and South Bridge Street

The significant adverse impacts at the eastbound left-turn lane group of this intersection during the weekday AM, midday, PM and Saturday peak hours and the southbound approach during the weekday midday, PM and Saturday peak hours could be fully mitigated by (1) restriping the eastbound approach from one 18-foot left-turn/through lane with 6-foot buffer to one 11-foot left-turn lane and one 10-foot left-turn/through lane with 4-foot buffer, (2) installing signage to prohibit the westbound left-turn movements, and (3) creating a leading southbound phase and protected eastbound left-turn phase (see Tables 20-2 through 20-5 for proposed signal timing).

Under existing conditions, there are minimal westbound left-turn movements, approximately 10 to 15 vehicles during the peak periods, which would be effected by the restriction. Vehicles needing to make a left-turn would instead make a right-turn onto Boscombe Avenue and be diverted to southbound Madsen Avenue.

As shown in Tables 20-6 through 20-9, the significant adverse impact could be fully mitigated with these proposed mitigation measures.

Boscombe Avenue and the Route 440 Ramps

The significant adverse impacts at the eastbound left-turn lane group of this intersection during the weekday AM, midday, PM, and Saturday peak hours could not be mitigated (see Tables 20-6 through 20-9).

Veterans Road West and Tyrellan Avenue

The significant adverse impacts at the eastbound approach of this intersection during the weekday PM and Saturday peak hours as well as the significant adverse impacts at the westbound approach of this intersection during the weekday AM, midday, PM, and Saturday peak hours could be fully mitigated by (1) restriping the eastbound approach from one 13-foot travel lane, one 11-foot travel lane and 10-foot median to one 13-foot travel lane, one 11-foot travel lane, one 12-foot left-turn lane, (2) restriping the westbound approach from one 14-foot travel lane, one 10-foot travel lane and 10-foot median to one 14-foot travel lane, one 10-foot travel lane and one 12-foot left-turn lane, (3) removing the eastbound/westbound split phase and (4) creating a protected only eastbound/westbound left-turn phase (see Tables 20-2 through 20-5 for proposed signal timing). As shown in Tables 20-6 through 20-9, the significant adverse impacts could be fully mitigated with these proposed mitigation measures.

Veterans Road West and North Bridge Street/ Bricktown Way

The significant adverse impact at the eastbound left-turn lane group during the weekday AM, midday, PM and Saturday peak hours, the eastbound through/right-turn lane group during the weekday midday, PM and Saturday peak hours, as well as the westbound left-turn lane group during the weekday AM, midday, and PM peak hours and westbound through lane group during the Saturday peak hour could be fully mitigated by (1) restriping the northbound approach from one 23-foot moving lane to one 11-foot left-turn lane and one 12-foot through/right-turn lane, (2) restriping the 12-foot westbound right-turn lane to a 12-foot westbound through/right-turn lane and (3) Restripe the WB receiving lanes from one through lane and one right-turn only/parking lane to two through lanes to Arthur Kill Road (a loss of approximately 31 parking spaces). In addition, (1) increasing the cycle length from 90 seconds to 120 seconds, (2) removing the northbound/southbound split phase and (3) creating permitted/protected eastbound, northbound, and southbound left-turn phases (see Tables 20-2 through 20-5 for proposed signal timings) would be needed to fully mitigate the projected impacts, as shown in Tables 20-6 through 20-9.

The significant adverse impacts are considered fully mitigated with these proposed mitigation measures pending public review and additional input from the community.

Page Avenue and Amboy Road

The significant adverse impact at the eastbound left-turn lane group of this intersection during the weekday AM and Saturday peak hours, the southbound left-turn lane group during the weekday midday and PM peak hours, and the northbound through lane group during the weekday PM and Saturday peak hours could be fully mitigated by restriping the northbound approach from one 10-foot left-turn lane, one 11-foot through lane, and one 11-foot right-turn lane to one 10-foot left-turn lane, one 11-foot through lane, and one 11-foot through/right-turn lane. In addition, a leading eastbound phase (see Tables 20-2 through 20-5 for proposed signal timing) would need to be created in combination with the above restriping to fully mitigate the impacts identified for these lane groups, as shown in Tables 20-6 through 20-9.

The significant adverse impacts at the eastbound left-turn during the weekday PM and Saturday peak hour could not be fully mitigated (see Tables 20-4 and 20-5).

The significant adverse impacts at the northbound left-turn and southbound through lane groups of this intersection during the Saturday peak hour could not be fully mitigated (see Table 20-5).

Page Avenue and Hylan Boulevard

The significant adverse impacts at the eastbound left-turn lane group of this intersection during the weekday AM, PM, and Saturday peak hours as well as the significant impacts at the southbound left-turn lane group of this intersection during the weekday AM, midday, PM, and Saturday peak hours could not be mitigated (see Tables 20-6 through 20-9).

Bloomingdale Avenue and Amboy Road/Pleasant Plains Road

The significant adverse impacts at the westbound left-turn lane group of this intersection during the Saturday peak hour as well as the significant impacts at the northbound approach of this intersection during the weekday PM and Saturday peak hours could be fully mitigated by (1) shifting the centerline of Amboy Road (south of Pleasant Plains Road) 1 foot to the west and (2) creating northbound and westbound leading phases during all peak hours (see Tables 20-2 through 20-5 for proposed signal timing). As shown in Tables 20-6 through 20-9, the significant adverse impacts could be fully mitigated with these proposed mitigation measures. However, the conditions at this intersection do not meet the requirements of the NYCDOT left-turn warrant analysis. Alternative mitigation measures will be explored further for this location between Draft and Final EIS in consultation with DCP and NYCDOT. If no feasible mitigation measures can be identified, the significant adverse impacts would be unmitigated.

North Bridge Street and the Route 440 Off-Ramp

The significant adverse impact at the westbound approach of this intersection during the weekday AM, midday, PM, and Saturday peak hours could not be mitigated (see Table 20-6 through 20-9).

Arthur Kill Road and South Bridge Street

The significant adverse impact at the southbound approach of this intersection during the weekday midday, PM, and Saturday peak hours could not be mitigated (see Table 20-7 through 20-9).

MITIGATION IMPLEMENTATION

Each of the proposed mitigation measures described above would require approval from and coordination with the respective divisions of NYCDOT, such as Borough Commissioner Office, Geometric Design, Signals, Borough Engineering, and possibly other groups within NYCDOT. The proposed mitigation measures identified in the EIS will be coordinated with NYCDOT for timely implementation to ensure projected significant adverse impacts are mitigated to the extent practicable. Preliminary and final Design drawings for the proposed mitigation measures in accordance with NYCDOT specifications and standards will be submitted by DCP/Applicant for NYCDOT review and approval.

Additionally, to verify the need and effectiveness of the proposed mitigation measures identified in this EIS, the Applicant would develop and conduct a detailed traffic monitoring plan at full buildout of the Proposed Project. The Applicant would submit for NYCDOT's review and approval a scope of work that would include all locations where significant traffic impacts have been identified in the EIS. Data collection conducted for the monitoring plan would include 24-hour Automatic Traffic Recorder (ATR) machine counts, manual turning movement counts, vehicle classification counts, intersection geometry and field observations, signal timing and any

relevant information necessary for conducting intersection capacity and level of service analyses to determine whether the volumes and delays projected for the Proposed Project were found to have occurred and whether the proposed mitigation measures identified in the EIS or similar measures would reasonably mitigate the potential project-related impacts.

The Applicant would be responsible for all costs associated with the traffic monitoring plan. In addition, the Applicant would be responsible for the cost of the design and construction of any or all improvement measures identified in the EIS or through the traffic monitoring plan as warranted due to project-generated traffic.

D. AIR QUALITY

EFFECTS OF PROPOSED TRAFFIC MITIGATION MEASURES

Chapter 13, "Air Quality," concludes that the Proposed Project would not result in significant adverse impacts on air quality. Therefore, no air quality mitigation is required. Since the proposed traffic mitigation measures described above would alter traffic conditions when compared with the Proposed Project, the localized air quality impacts with mitigation were modeled. Of the intersections selected for mobile source analysis in the Draft EIS, the intersection of Page Avenue and Richmond Valley Road was analyzed for traffic mitigation.

Table 20-10 shows the future maximum predicted 8-hour average CO concentration without the proposed project (No Action condition), as well as with the Proposed Project with and without the implementation of the traffic mitigation measures (With Action condition and With Traffic Mitigation).

Table 20-10 8-Hour Average CO Concentrations with Traffic Mitigation

			8-	Hour Average Co	ncentration (ppm)							
Analysis Site	Location	Time Period	No Action	With Action	With Traffic Mitigation	De Minimis						
3	Page Avenue and Richmond Valley Road	Saturday midday	2.1	2.4	2.3	5.5						
Notes:												

The results indicate that the Proposed Project with the traffic mitigation measures would not result in any violations of the 8-hour CO standard (9 ppm) as the 8-hour average concentration with traffic mitigation (2.3 ppm) for the Saturday midday peak period would be less than the standard.

Table 20-11 shows the maximum predicted 24-hour average PM₁₀ concentrations without the Proposed Project, with the Proposed Project, and with the Proposed Project and implementation of the traffic mitigation measures (No Action, With Action, With Traffic Mitigation).

Table 20-11 24-Hour Average PM₁₀ Concentrations with Traffic Mitigation

		24-Hour Concentration (µg/m³) ¹				
Analysis Site	Location	No Action	With Action	With Traffic Mitigation		
3	Page Avenue and Richmond Valley Road	54.7	57.4	59.3		
Note: ¹ NAAQS—24-hour average 150 μg/m ³ .						

The results indicate that the implementation of the traffic mitigation measures for the proposed project would not result in any violations of the PM₁₀ standard at any of the receptor locations analyzed.

Future maximum predicted 24-hour and annual average $PM_{2.5}$ concentrations were determined so that they could be compared with the *de minimis* criteria for $PM_{2.5}$. Consistent with current CEQR guidance, $PM_{2.5}$ concentrations are presented as an incremental change in concentrations for both the Proposed Project without traffic mitigation measures (as compared with the No Action condition) and for the Proposed Project with traffic mitigation measures (as compared with the No Action condition). The maximum predicted localized 24-hour average and neighborhood-scale annual average $PM_{2.5}$ concentration increments are presented in **Tables 20-12** and **20-13**, respectively. The results show that the maximum daily (24-hour) $PM_{2.5}$ increments with traffic mitigation measures are predicted to be below the *de minimis* criterion of 7.4 μ g/m³, and the maximum annual average $PM_{2.5}$ increments are not predicted to exceed the applicable *de minimis* criterion of 0.1 μ g/m³.

Table 20-12 24-Hour Average PM_{2.5} Concentration Increments with Traffic Mitigation

Analysis Site	Location	Increment (µg/m³)	Increment with Traffic Mitigation (µg/m³)	De Minimis
3	Page Avenue and Richmond Valley Road	1.4	1.7	7.4

Notes: $PM_{2.5}$ de minimis criteria—24-hour average, not to exceed more than half the difference between the background concentration and the 24-hour standard of 35 μ g/m³.

Table 20-13 Neighborhood Scale PM_{2.5} Concentration Increments with Traffic Mitigation

Analysis Site	Location	Increment (µg/m³)	Increment with Traffic Mitigation (µg/m³)				
3	Page Avenue and Richmond Valley Road	0.095	0.092				
Notes: PM _{2.5} de minimis criteria—annual (neighborhood scale), 0.1 μg/m ³ .							

Therefore, no significant adverse air quality impacts would occur as a result of the proposed traffic mitigation measures.

*