

Turning Movement Counts

(1A)

Project No.: 1121A

Date: June 04, 2011

Period: PM

Weather Conditions: fine

Surveyor's Name: Alexander Pushkin

toll: \$8⁰⁰
 mileage: 98ml

Intersection Diagram

Intersection Diagram: Kill Rd (vertical), Arthur Rd (horizontal). Movements: 1 (left turn), 2 (through/right turn), 3 (right turn), 4 (left turn), 5 (through/right turn), 6 (through/right turn), 7 (left turn), 8 (right turn). 4 HV for movement 2, 8 HV for movement 6.

North ↑

KILL RD

ARTHUR RD

SHARROTS RD

HV - Count vehicles w/ more than 4 wheels (Heavy Vehicles)

Time	Movements											
	1	2	3	4	5	6	7	8	9	10	11	12
3:00 to 3:15	5	40	6	7	6	2	8	-				
3:15 to 3:30	6	45	8	1	8	3	8	-				
3:30 to 3:45	2	39	6	-	10	3	16	-				
3:45 to 4:00	3	35	5	4	7	3	14	-				
4:00 to 4:15	6	29	6	4	9	3	8	-				
4:15 to 4:30	3	34	6	1	7	5	6	-				
4:30 to 4:45	6	36	6	1	3	-	6	-				
4:45 to 5:00	3	30	6	2	9	2	17	-				
5:00 to 5:15	2	32	3	2	12	3	15	-				
5:15 to 5:30	2	22	6	1	2	2	9	-				
5:30 to 5:45	3	25	2	1	8	2	7	1				
5:45 to 6:00	2	27	3	1	3	1	4	-				
Peak Hour												

toll: \$8⁰⁰
 mileage: 98ml

Project No.: 1121a

Intersection
Diagram

Date: 06/07/11

Period: MD

Weather Conditions: sunny
Tetiana

Surveyor's Name: Berneoltska

ARTHUR KILL RD



S. BRIDGE ST.

Movements

Time	1	2	3	4	5	6	7	8	9	10	11	12
11:30 to 11:45	71	10	40	5	103	9	26	8				
11:45 to 12:00	53	9	28	—	114	8	30	5				
12:00 to 12:15	70	10	56	3	99	10	45	5				
12:15 to 12:30	69	9	45	1	112	10	33	6				
12:30 to 12:45	69	6	38	2	105	2	23	2				
12:45 to 13:00	77	7	50	4	101	5	30	8				
13:00 to 13:15	64	3	27	2	102	3	29	6				
13:15 to 13:30	66	3	30	—	107	5	43	5				
Peak Hour												

(21A)

Project No.: 1053

Date: 6.8.11

Period: PM

Weather Conditions:

Surveyor's Name: Dmitriy Kobtsev

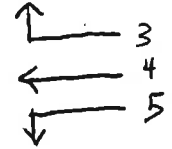
Intersection Diagram

VETERANS RD EAST

BLOOMINGDALE RD

DRUM GOOLE RD WEST

North ↑



Movements

Time	1	2	3	4	5	6	7	8	9	10	11	12
11:30 to 11:45	8	70	24	82	46							
11:45 to 12:00	8	69	23	88	53							
12:00 to 12:15	2	56	29	109	53							
12:15 to 12:30	9	83	13	101	47							
12:30 to 12:45	4	54	26	101	30							
12:45 to 13:00	4	76	30	90	36							
13:00 to 13:15	6	45	35	94	33							
13:15 to 13:30	4	58	34	95	66							

Peak Hour

23A

Project No.: 1053

Date: 06/08/11

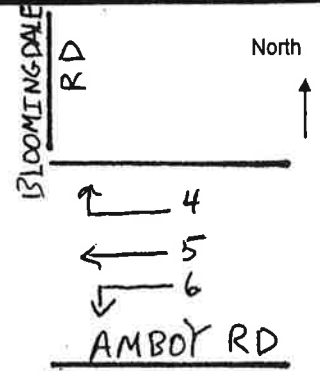
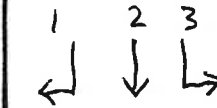
Period: 7³⁰ PM 9³⁰

Weather Conditions:

Surveyor's Name: Inna Sichkan

Intersection Diagram

PLEASANT PLAINS AVE



AMBOY RD

Movements

Time	1	2	3	4	5	6	7	8	9	10	11	12
7:30 to 7:45	4	70	22	11	0	21						
7:45 to 8:00	2	73	38	14	7	35						
8:00 to 8:15	0	77	56	35	10	47						
8:15 to 8:30	1	63	33	30	1	35						
8:30 to 8:45	0	61	22	20	4	35						
8:45 to 9:00	0	57	15	7	0	43						
9:00 to 9:15	1	51	20	15	0	36						
9:15 to 9:30	0	48	11	13	2	28						

Peak Hour

(23B)

Project No.: 1053

Date:

06/08/11

Period: PM

Weather Conditions:

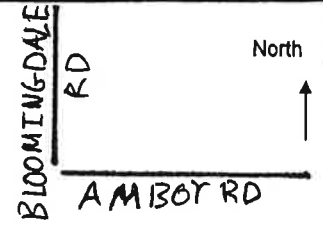
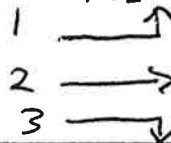
Overcast

Surveyor's Name:

Racher

Intersection Diagram

PLEASANT PLAINS AVE



AMBOY RD

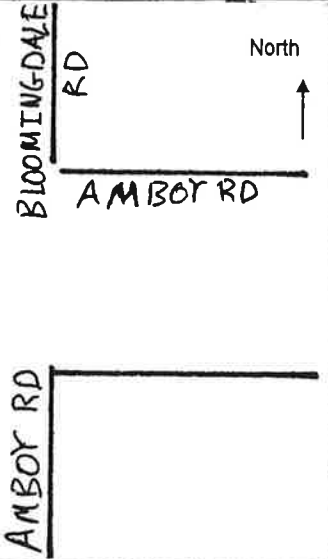
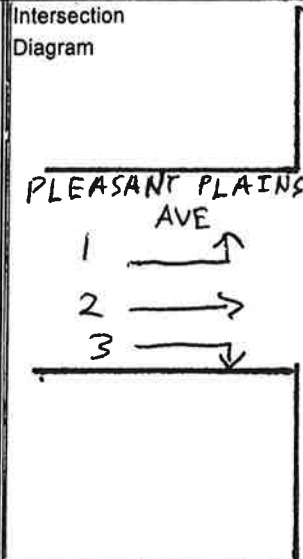
Movements

Time	1	2	3	4	5	6	7	8	9	10	11	12
7:30 to 7:45	3	5	4	1	28	38						
7:45 to 8:00	1	14	5	7	45	25						
8:00 to 8:15	0	8	4	2	48	27						
8:15 to 8:30	3	7	4	2	39	25						
8:30 to 8:45	2	2	0	1	33	40						
8:45 to 9:00	2	2	4	2	30	28						
9:00 to 9:15	1	1	6	1	21	20						
9:15 to 9:30	3	4	3	0	31	34						

Peak Hour

(23B)

Project No.: 1053
 Date: 06/08/11
 Period: PM
 Weather Conditions:
 Surveyor's Name: Lyndora Kacher

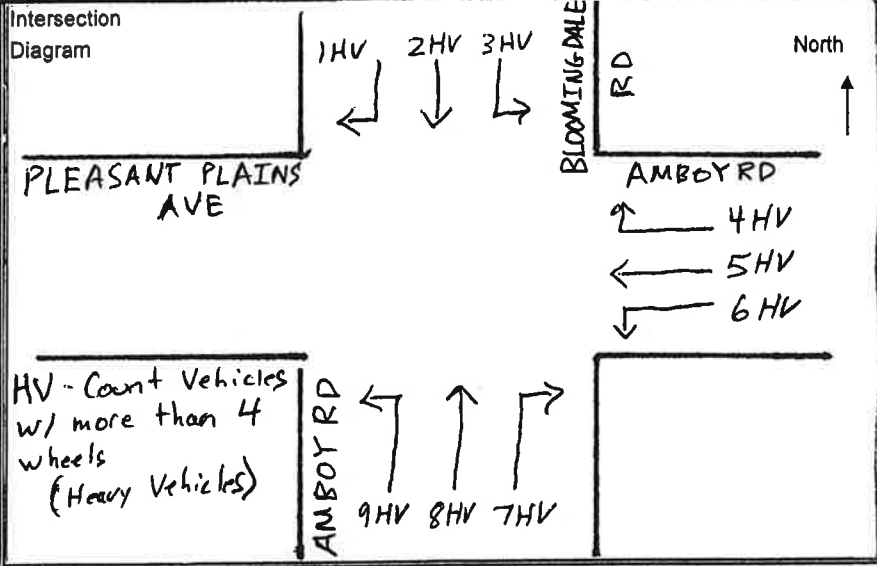


Time	Movements											
	1	2	3	4	5	6	7	8	9	10	11	12
11:30 to 11:45	1	2	4	1	33	29						
11:45 to 12:00	1	1	3	2	25	32						
12:00 to 12:15	1	1	5	4	53	38						
12:15 to 12:30	2	1	4	4	37	28						
12:30 to 12:45	1	1	3	1	53	42						
12:45 to 13:00	1	4	4	3	41	33						
13:00 to 13:15	1	1	2	4	47	45						
13:15 to 13:30	1	3	1	2	33	43						

Peak Hour

23C

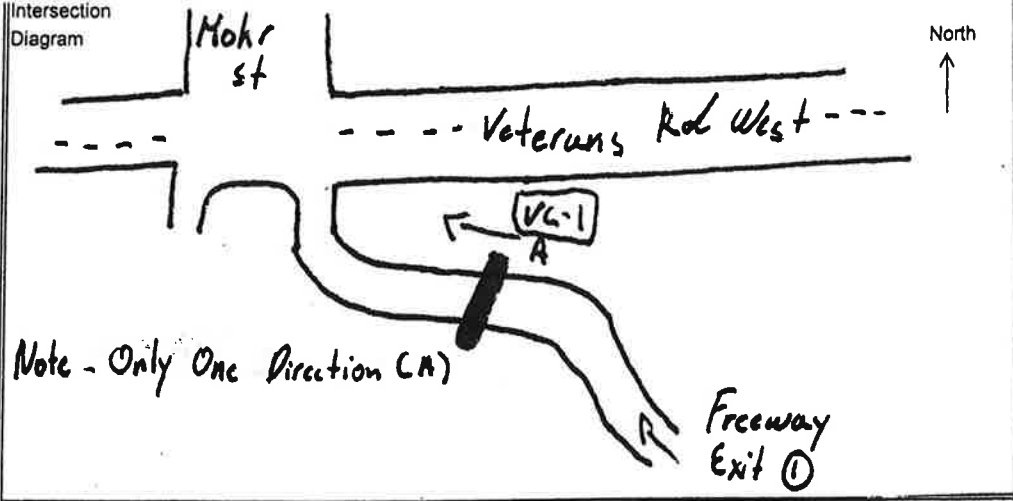
Project No.: 1053
 Date: 06/08/11
 Period: PM
 Weather Conditions:
 Surveyor's Name: Michael Shapiro



Time	Movements											
	1	2	3	4	5	6	7	8	9	10	11	12
11:30 to 11:45	-	4	2	1	-	1	-	-				
11:45 to 12:00	-	4	2	4	3	-	2	2				
12:00 to 12:15	-	-	1	2	2	-	2	4				
12:15 to 12:30	-	-	2	-	4	1	3	2				
12:30 to 12:45	-	-	2	1	3	-	5	2				
12:45 to 13:00	-	-	-	1	4	1	2	-				
13:00 to 13:15	-	-	-	1	1	-	6	-				
13:15 to 13:30	-	1	-	3	-	-	2	1				
Peak Hour										1		

VC1

Project N 1121A
 Location:
 Date: 6/7/2011
 Period: ~~MD~~ AM
 Stanislaw Filer
 Weather Conditions:
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	91	1	2	1	8	42						
7:45 to 8:00	92	4	1	1	2	38						
8:00 to 8:15	85	7	-	-	12	33						
8:15 to 8:30	73	7	1	1	8	32						
8:30 to 8:45	79	6	1	-	10	28						
8:45 to 9:00	83	5	-	-	5	26						
9:00 to 9:15	78	9	2	-	11	27						
9:15 to 9:30	66	2	1	1	13	21						
Peak Hour												

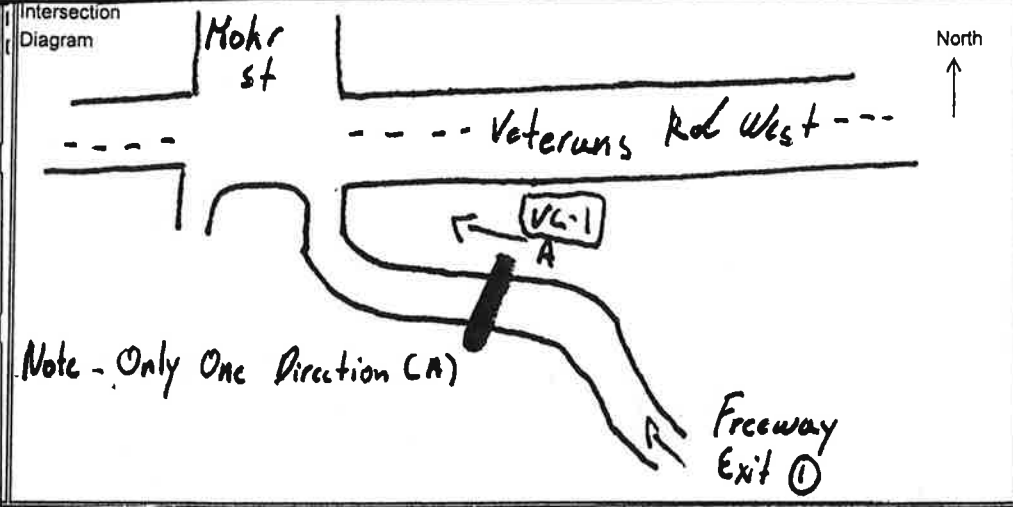
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

PHILIP HABIB & ASSOCIATE
 226, W. 26th St. 9FL
 New York, NY 10001
 (212)929-5656

VCL1

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date: 6/7/2011
 Period: MD
 Weather Conditions:
 Stanislar
 Surveyor's Name: Filer

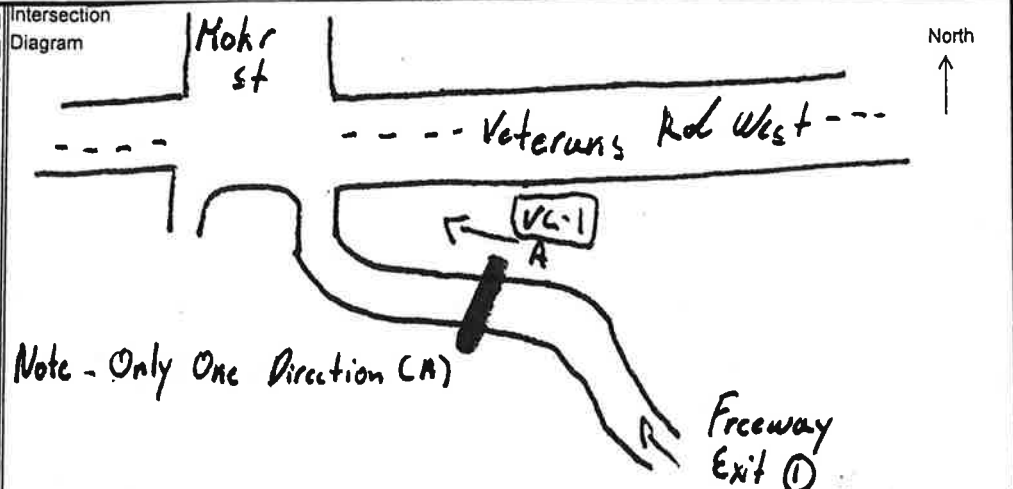


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	92	11	2	-	4	26						
11:45 to 12:00	88	6	1	1	4	39						
12:00 to 12:15	105	3	1	2	5	28						
12:15 to 12:30	99	4	4	-	3	23						
12:30 to 12:45	81	1	1	2	1	34						
12:45 to 13:00	82	5	1	2	1	31						
13:00 to 13:15	75	2	-	-	1	40						
13:15 to 13:30	102	3	3	1	1	36						
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

VC-1

Project N 1121A
 Location:
 Date: 6/7/2011
 Period: PM
 Stanislov
 FILEV
 Weather Conditions:
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	106	3	3	1	2	36						
16:45 to 17:00	98	1	1	1	1	60						
17:00 to 17:15	115	2	3	1	1	67						
17:15 to 17:30	111	1	1	0	2	58						
17:30 to 17:45	114	3	1	-	3	64						
17:45 to 18:00	115	1	-	5	2	55						
18:00 to 18:15	112	2	-	-	3	54						
18:15 to 18:30	111	2	1	-	2	52						
Hour												

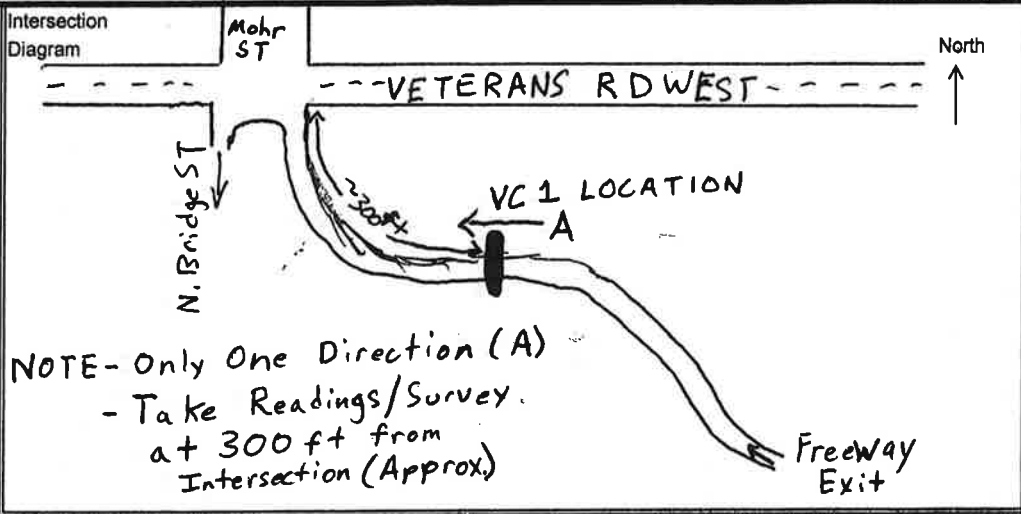
uty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 uty Trucks (6 or more Wheels; Walk-In Trucks)
 rucks (with Vertical Tale Pipe; Tractor-Trailers)

PHILIP HABIB & ASSOCIATES
 226 W. 26th St. 9FL
 New York, NY 10001
 (212)929-5656

VC 1

Traffic / Pedestrian Movement Count Sheet

Project No.: 1121A
 Location: EXIT ONE OFF RICHMOND PKWY
 Date: Jun-4-2011
 Period:
 Weather Conditions:
 Surveyor's Name: STANISLAV FILEV

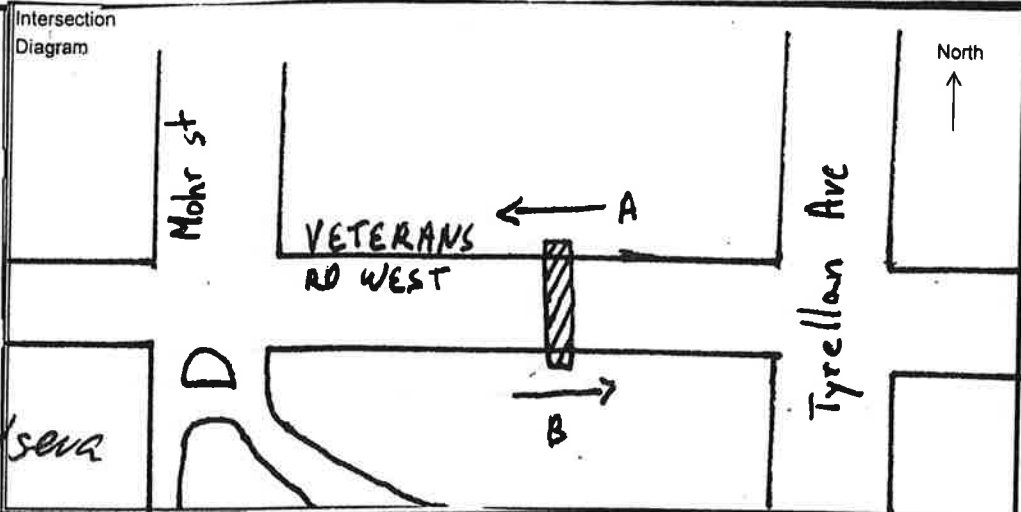


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	121	2	-	-	2	35						
3:15 to 3:30	112	2	-	-	-	44						
3:30 to 3:45	114	2	-	-	1	81						
3:45 to 4:00	151	2	1	-	-	32						
4:00 to 4:15	114	-	-	-	2	44						
4:15 to 4:30	117	-	-	1	-	27						
4:30 to 4:45	134	2	-	-	-	25						
4:45 to 5:00	122	2	-	-	1	33						
5:00 to 5:15	113	9	-	-	-	28						
5:15 to 5:30	124	2	-	-	2	31						
5:30 to 5:45	123	1	-	-	1	43						
5:45 to 6:00	119	1	-	-	1	45						
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC-2

Project N 1121A
 Location:
 Date:
 Period: **mid AM**
 Weather Conditions:
 Surveyor's Name: *Rafiq Budantseva*



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	92	7	1	3	6	4	33	3	1	-	2	1
7:45 to 8:00	106	13	4	2	5	5	48	7	3	2	-	2
8:00 to 8:15	103	7	6	5	3	-	50	9	5	3	1	1
8:15 to 8:30	95	15	2	2	5	-	56	5	1	-	-	-
8:30 to 8:45	107	11	1	3	4	6	59	6	4	1	4	5
8:45 to 9:00	86	10	-	3	2	10	28	7	1	-	-	7
9:00 to 9:15	60	13	7	4	5	15	22	5	2	-	-	13
9:15 to 9:30	74	12	2	3	5	13	29	6	2	-	2	11
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VL-2

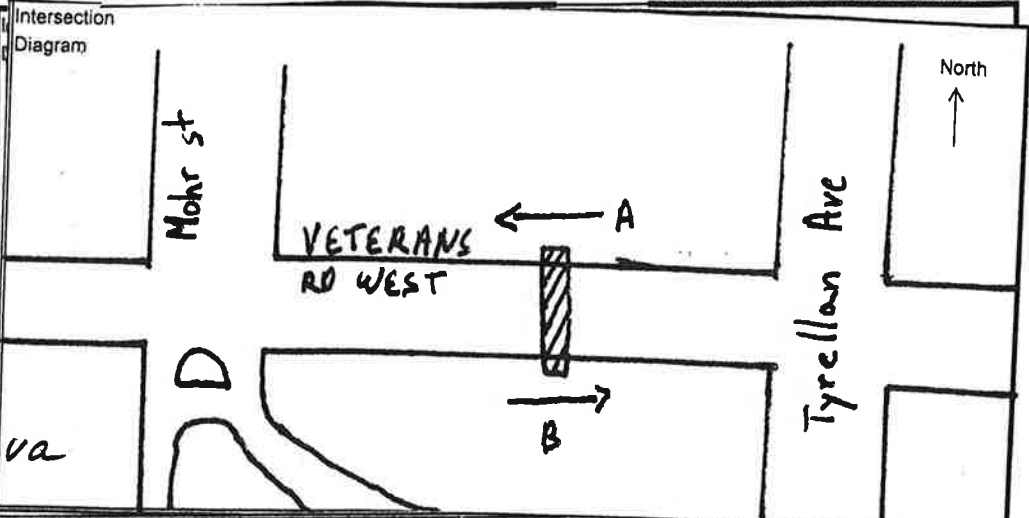
Project N 1121A
 Location:

Date:

Period: MD

Weather Conditions:

Rafiya Budantseva
 Surveyor's Name:

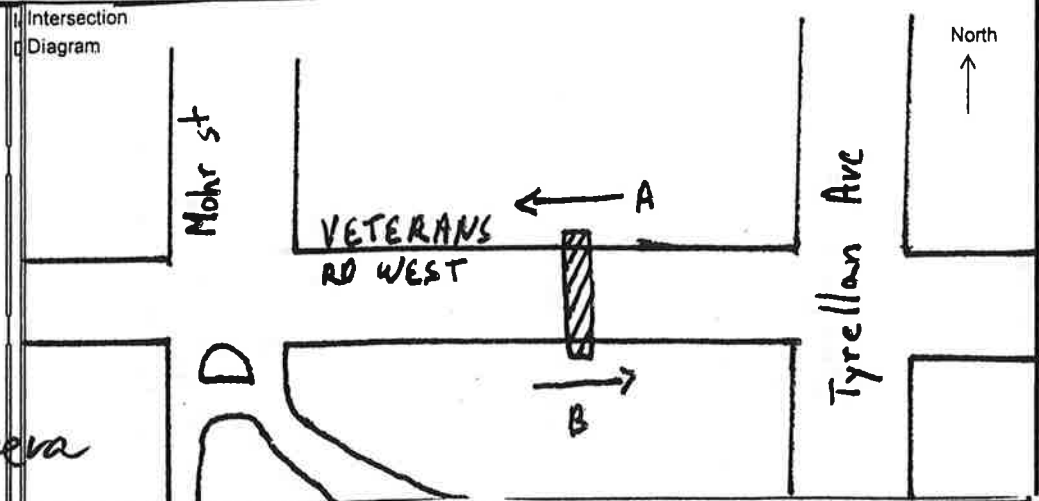


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	87	12	2	4	2	6	59	8	1	-	-	3
11:45 to 12:00	113	12	4	1	2	7	59	8	3	1	-	4
12:00 to 12:15	125	13	5	2	3	8	62	9	1	1	-	3
12:15 to 12:30	103	8	7	2	2	9	66	4	1	-	-	5
12:30 to 12:45	109	11	2	1	3	7	78	12	2	-	-	3
12:45 to 13:00	109	10	5	2	1	1	92	6	1	1	-	2
13:00 to 13:15	102	5	2	2	2	2	73	8	1	1	-	1
13:15 to 13:30	96	5	4	3	2	6	74	5	1	1	1	4
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC-2

Project N 1121A
 Location :
 Date:
 Period: PM
 Weather Conditions:
 Rafiya Budantseva
 Surveyor's Name:

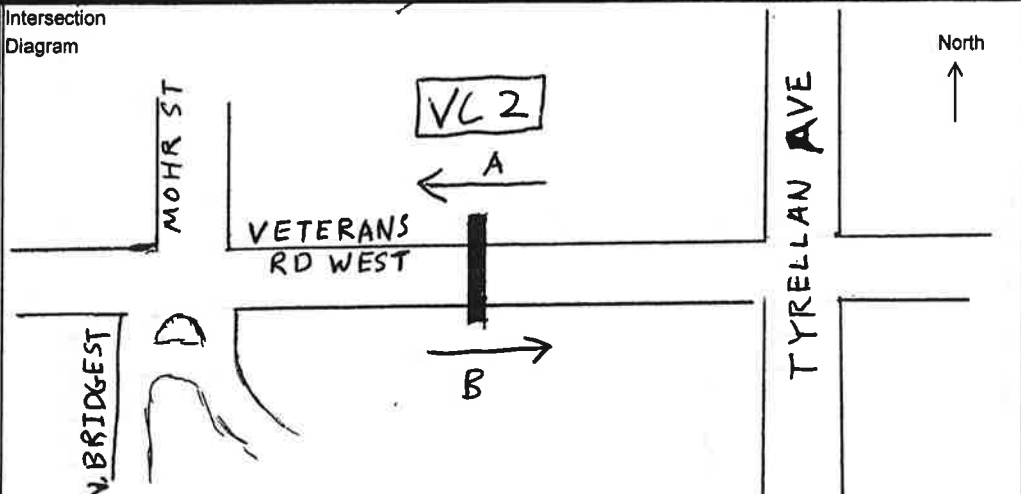


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	126	16	3	1	4	1	83	7	2	1	-	-
16:45 to 17:00	97	14	2	1	4	2	52	3	-	-	-	1
17:00 to 17:15	104	4	3	1	5	2	75	6	-	-	-	1
17:15 to 17:30	100	12	-	-	7	4	77	4	-	-	-	-
17:30 to 17:45	125	5	5	1	4	2	70	4	1	-	1	-
17:45 to 18:00	113	7	4	1	4	-	68	5	1	-	1	-
18:00 to 18:15	82	6	-	2	8	1	75	2	2	-	1	-
18:15 to 18:30	91	11	1	-	8	-	69	7	-	-	1	1
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC 2

Project No.: 1121A
 Location: VETERANS RD W Between MOHR and TYRELLAN
 Date: June-4-2011
 Period:
 Weather Conditions:
 Surveyor's Name:

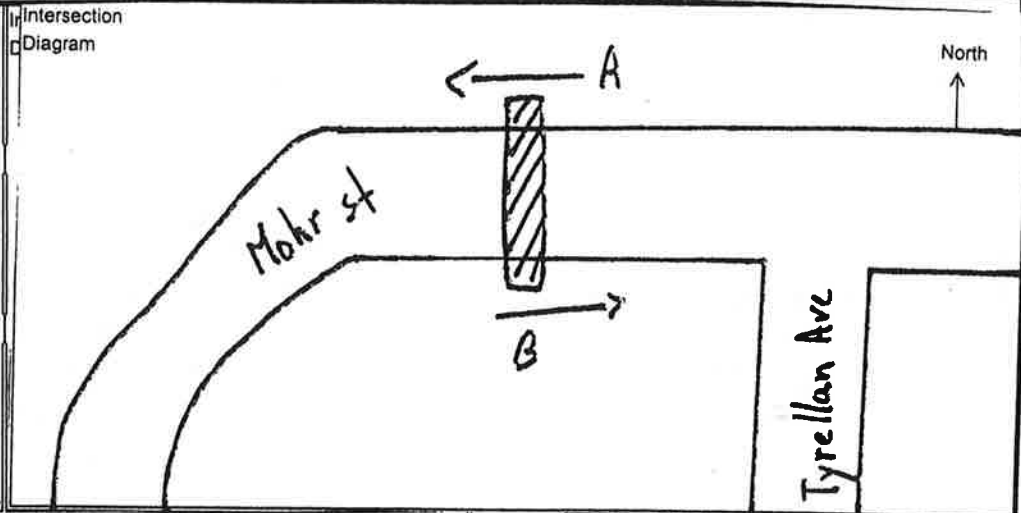


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	84	5	-	-	-	67	64	7	-	-	-	33
3:15 to 3:30	53	7	1	-	3	48	46	3	+	-	-	25
3:30 to 3:45	73	13	1	-	3	38	37	7	1	-	-	26
3:45 to 4:00	57	41	1	-	4	42	47	4	2	-	+	28
4:00 to 4:15	61	9	1	-	2	37	55	3	+	-	+	22
4:15 to 4:30	55	6	1	1	2	40	51	1	+	1	-	30
4:30 to 4:45	68	10	-	1	-	37	59	1	-	+	-	43
4:45 to 5:00	37	4	-	1	3	35	26	2	-	-	-	29
5:00 to 5:15	23	1	-	-	-	12	44	2	1	1	2	26
5:15 to 5:30	38	-	1	-	-	26	32	1	1	-	2	23
5:30 to 5:45	43	2	-	-	1	12	41	6	2	-	-	23
5:45 to 6:00	38	2	1	-	-	24	50	9	-	-	2	34
Peak Hour	3:00	3:30	-	-	4:45	3:00	4:30	5:45	-	-	-	4:30

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC-3

Project N 1121A
 Location:
 Date: Jun 7 2011
 Period: ~~MD~~ AM
 Weather Conditions:
 Surveyor's Name: Valentina Yermakova

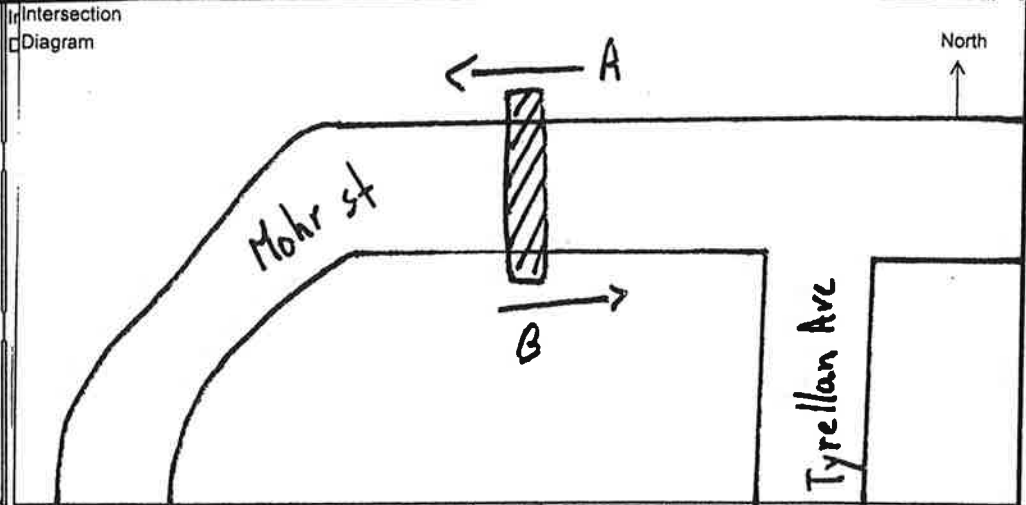


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	2	-	-	-	-	-	8	5	1	-	2	2
7:45 to 8:00	13	-	-	-	-	1	9	2	-	-	1	4
8:00 to 8:15	15	2	-	-	-	3	8	1	-	-	3	-
8:15 to 8:30	6	1	-	1	-	1	11	1	-	-	2	2
8:30 to 8:45	7	1	-	-	-	5	19	1	1	-	2	5
8:45 to 9:00	11	2	1	-	-	3	9	1	-	-	-	6
9:00 to 9:15	6	1	1	1	-	2	10	-	-	-	2	2
9:15 to 9:30	13	-	-	-	-	5	17	2	-	-	5	6
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

VC-3

Project N 1121A
 Location :
 Date: 06/07/2011
 Period: MD
 Weather Conditions:
 Surveyor's Name: Valentina Yermakova

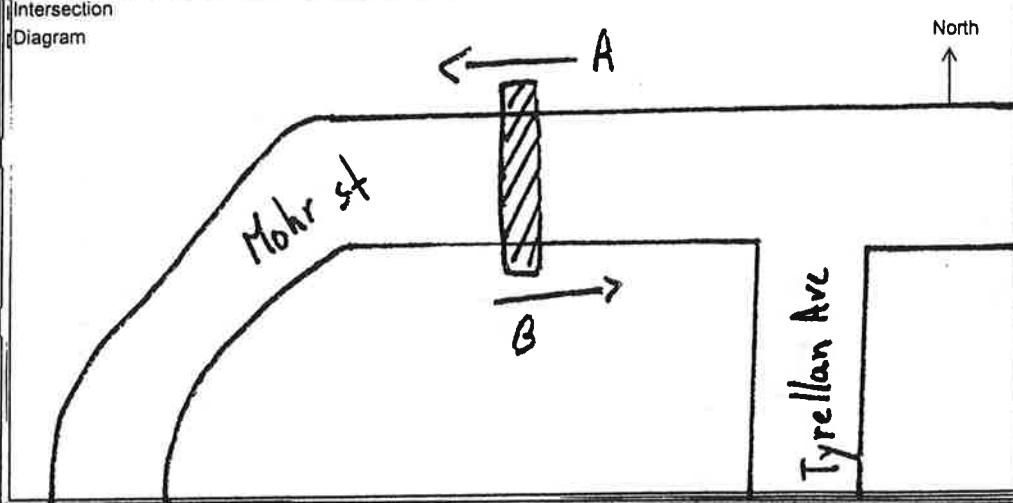


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	15	1	-	-	-	7	19	4	-	-	2	5
11:45 to 12:00	21	1	1	-	1	6	34	2	1	-	1	8
12:00 to 12:15	17	3	-	1	-	4	24	3	-	-	1	11
12:15 to 12:30	38	-	-	-	-	6	18	1	-	-	3	8
12:30 to 12:45	25	2	-	-	-	9	18	-	1	1	2	6
12:45 to 13:00	17	-	-	1	-	5	24	2	-	-	1	12
13:00 to 13:15	22	-	2	-	-	9	31	1	1	-	2	10
13:15 to 13:30	26	-	-	-	-	8	25	1	1	-	-	11
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC-3

Project N 1121A
 Location :
 Date: 06/07/11
 Period: PM
 Weather Conditions:
 Surveyor's Name: Valeriy Yermakova



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	24	-	-	-	-	3	20	2	-	-	4	8
16:45 to 17:00	16	6	-	-	-	6	16	-	-	-	2	6
17:00 to 17:15	31	1	-	-	1	6	24	-	-	-	3	10
17:15 to 17:30	20	1	-	-	-	6	23	2	-	-	1	12
17:30 to 17:45	21	-	-	-	-	12	14	-	-	-	1	7
17:45 to 18:00	16	-	-	-	-	8	18	1	-	-	2	9
18:00 to 18:15	21	-	-	-	-	10	17	-	1	-	2	7
18:15 to 18:30	29	-	2	-	-	5	24	2	-	-	3	10
Peak Hour												

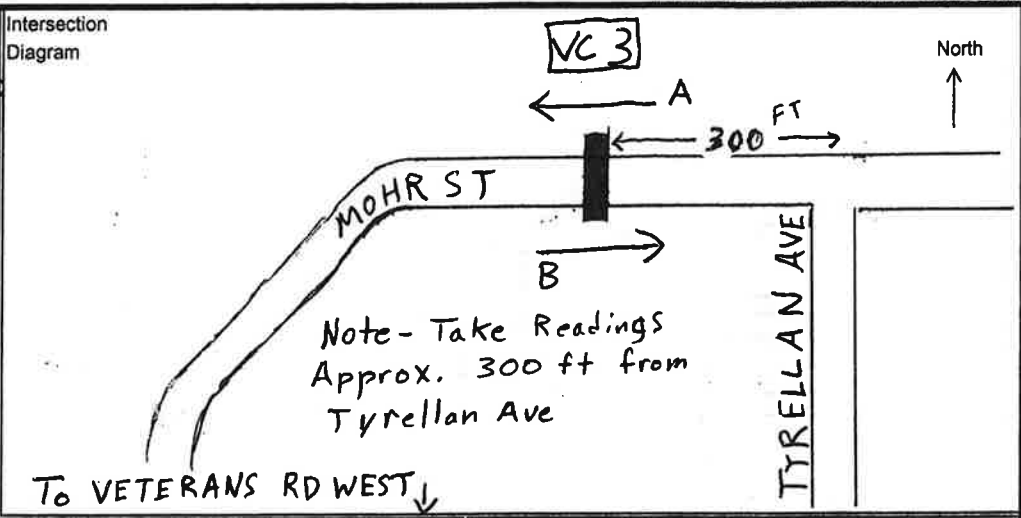
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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 New York, NY 10001
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VC 3

Traffic / Pedestrian Movement Count Sheet

Project No.: 1121A
 Location: MOHR ST between TYRELLAN and VETERANS RD WEST
 Date: Jun 4 2011
 Period:
 Weather Conditions:
 Surveyor's Name: Valentina Yermakova



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	36	4	-	-	-	2	56	2	-	-	1	6
3:15 to 3:30	36	2	1	-	-	2	63	1	-	-	2	4
3:30 to 3:45	33	-	-	-	-	2	47	1	1	-	1	2
3:45 to 4:00	52	3	-	-	-	-	51	1	-	-	3	1
4:00 to 4:15	38	2	-	-	-	-	40	-	-	-	1	-
4:15 to 4:30	36	4	-	-	-	1	36	1	-	-	2	3
4:30 to 4:45	27	6	-	-	-	1	43	2	-	-	-	1
4:45 to 5:00	31	1	1	-	-	1	40	-	-	-	2	2
5:00 to 5:15	30	-	-	-	-	-	34	1	-	-	1	4
5:15 to 5:30	21	1	-	-	-	-	33	-	-	-	2	4
5:30 to 5:45	23	-	-	1	-	2	46	1	-	-	1	2
5:45 to 6:00	36	-	-	-	-	2	37	-	-	-	1	3

Peak Hour

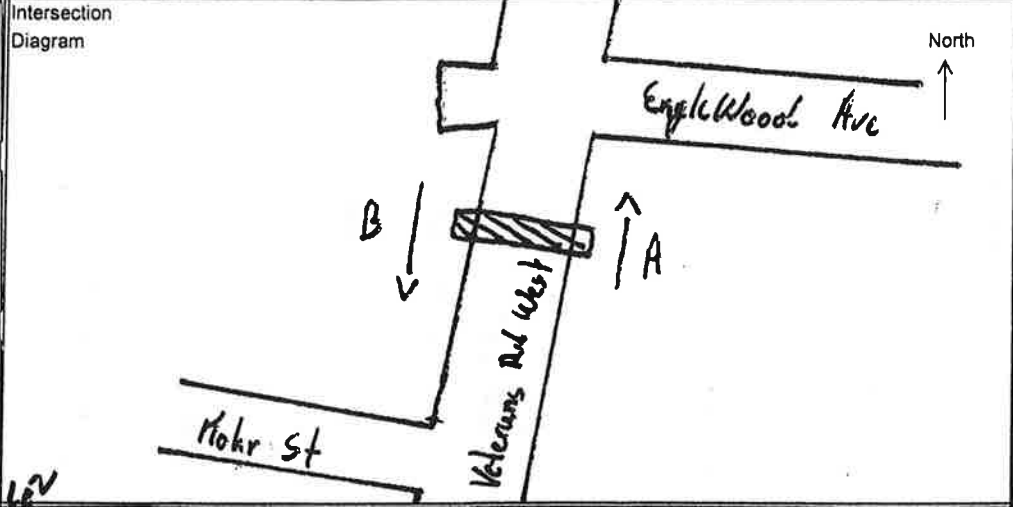
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VC 4

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date: 06/07/2011
 Period: MD AM
 Weather Conditions:
 Surveyor's Name: YURIY MALKOV



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	21	4	-	-	1	1	83	5	3	-	3	14
7:45 to 8:00	25	-	-	-	3	3	96	4	1	-	1	13
8:00 to 8:15	33	1	-	-	-	2	89	2	2	-	3	25
8:15 to 8:30	43	-	-	-	-	4	95	9	2	1	3	16
8:30 to 8:45	36	2	-	1	-	3	118	5	1	-	1	21
8:45 to 9:00	32	4	-	-	-	2	107	2	-	-	1	16
9:00 to 9:15	40	3	-	-	-	4	93	4	3	1	1	16
9:15 to 9:30	31	2	-	1	-	3	121	5	1	-	3	14
Peak Hour												

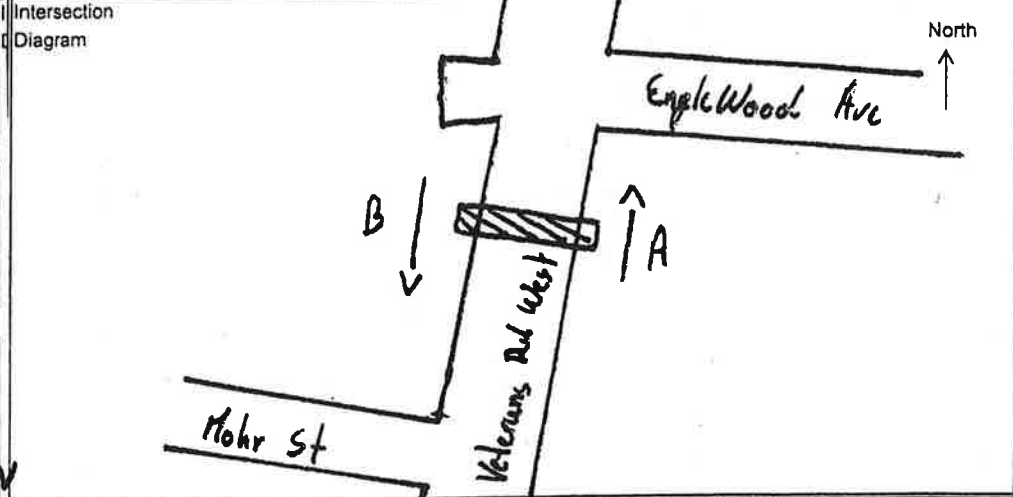
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trallers)

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VC-4

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date: 06/07/2011
 Period: MD
 Weather Conditions:
 Surveyor's Name: YUAIY MALAY



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	62	10	-	-	1	9	120	5	-	-	1	23
11:45 to 12:00	71	2	-	-	1	4	160	5	3	-	2	24
12:00 to 12:15	62	3	-	-	2	4	172	9	2	-	4	17
12:15 to 12:30	77	3	-	-	-	5	146	4	2	-	-	31
12:30 to 12:45	74	3	-	-	-	7	148	9	2	1	1	25
12:45 to 13:00	79	2	-	-	-	3	126	5	-	-	-	15
13:00 to 13:15	86	3	-	-	-	7	122	4	1	-	2	18
13:15 to 13:30	80	4	-	1	1	7	109	6	-	-	1	24
Peak Hour												

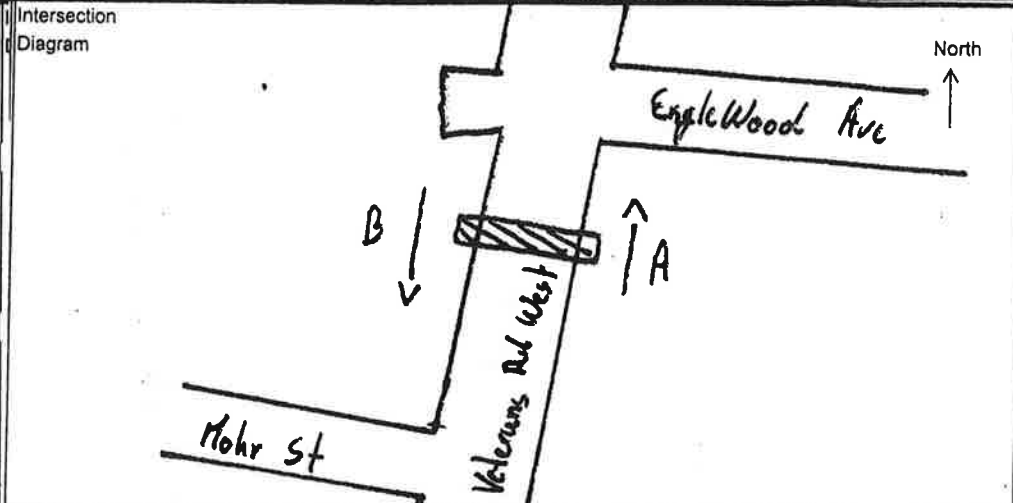
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VL-4

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location :
 Date: 06/07/2011
 Period: PM
 Weather Conditions:
 Surveyor's Name: YURIY MALEV

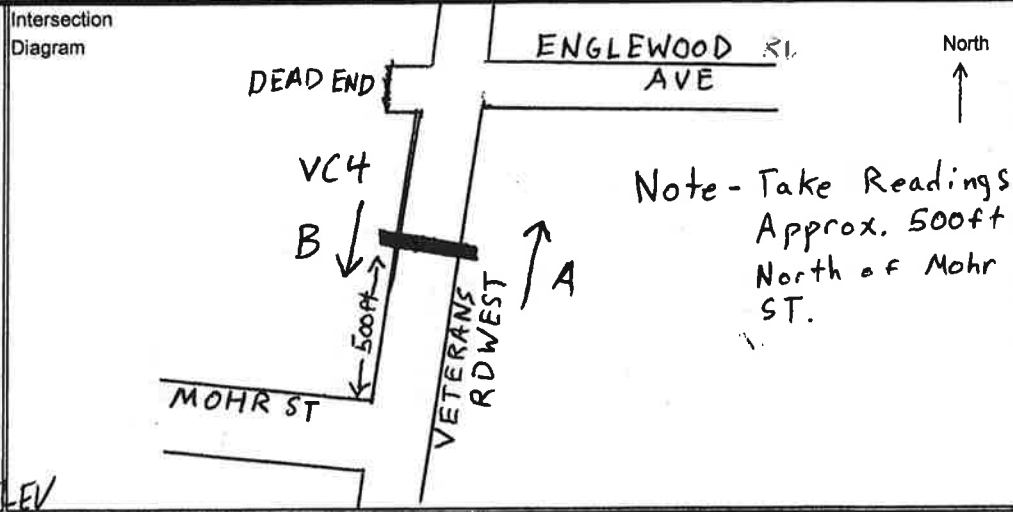


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	69	1	-	-	-	7	127	4	-	-	2	10
16:45 to 17:00	68	2	-	-	-	11	157	14	-	-	2	14
17:00 to 17:15	74	-	-	-	1	19	117	3	-	1	2	18
17:15 to 17:30	63	-	-	-	-	18	126	5	-	-	3	15
17:30 to 17:45	65	1	-	-	-	11	124	7	1	-	4	12
17:45 to 18:00	62	2	-	-	-	14	135	8	-	-	1	12
18:00 to 18:15	74	4	-	-	1	14	130	2	-	-	7	9
18:15 to 18:30	60	-	-	-	-	15	119	2	-	-	4	11
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

VC 4

Project No.: 1121A
 Location: VETERANS RD W between MOHR and ENGLEWOOD
 Date: June, 04, 2011
 Period:
 Weather Conditions:
 Surveyor's Name: YURIY MALEV



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	86	-	-	-	-	8	113	4	-	-	-	16
3:15 to 3:30	105	-	-	-	-	5	138	2	-	-	-	32
3:30 to 3:45	93	2	-	-	-	9	150	5	-	-	3	40
3:45 to 4:00	99	-	-	-	-	14	170	7	-	-	1	35
4:00 to 4:15	109	-	-	-	-	16	161	5	-	-	1	30
4:15 to 4:30	102	1	-	-	-	7	132	5	-	-	-	24
4:30 to 4:45	110	2	-	-	-	10	123	8	-	-	-	17
4:45 to 5:00	93	-	-	-	-	9	147	4	-	-	-	28
5:00 to 5:15	91	-	-	-	-	7	139	2	-	-	-	21
5:15 to 5:30	82	-	-	-	-	6	145	4	-	-	-	14
5:30 to 5:45	78	-	-	-	-	6	130	3	-	-	-	27
5:45 to 6:00	80	-	-	-	-	7	122	2	-	-	-	33
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VC 5

Traffic / Pedestrian Movement Count Sheet

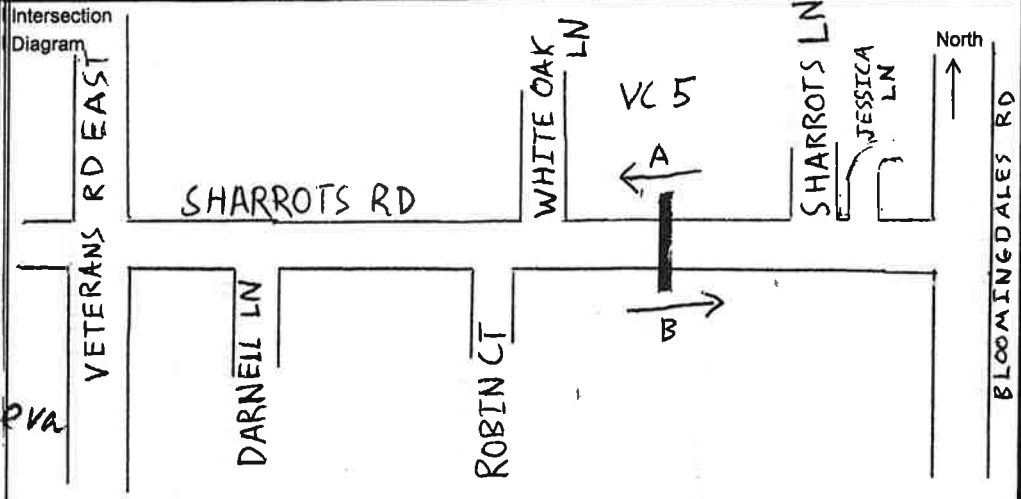
Project N 1121A
 Location:

Date:

Period: AM

Weather Conditions: *sunny*
Rafiya Budantseva

Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	17	2	-	-	2	2	23	4	2	-	1	3
7:45 to 8:00	15	1	1	-	5	1	27	6	2	-	2	4
8:00 to 8:15	20	2	-	-	1	-	20	3	4	-	2	1
8:15 to 8:30	13	3	-	1	2	4	29	3	-	-	1	4
8:30 to 8:45	14	2	2	-	1	-	20	2	1	-	2	1
8:45 to 9:00	24	3	-	-	-	2	15	3	3	-	-	-
9:00 to 9:15	10	3	1	-	-	-	14	2	2	-	-	2
9:15 to 9:30	15	2	3	-	-	1	14	6	1	-	-	2
Peak Hour												

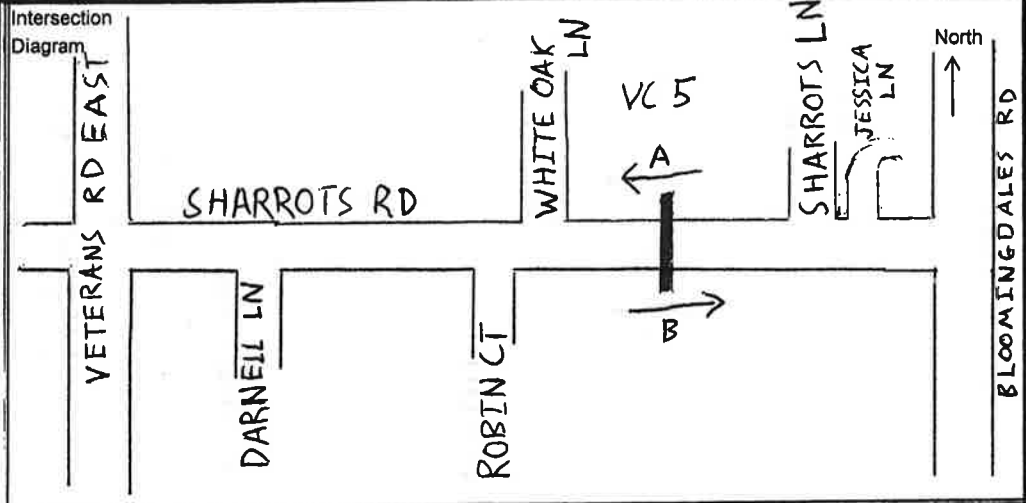
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VC-5

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location :
 Date:
 Period: MD
 Weather Conditions: *SUNNY*
 Surveyor's Name: *Rafiq Budantseva*



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	21	-	-	-	-	3	19	3	3	-	-	1
11:45 to 12:00	24	3	-	-	1	-	16	2	1	-	-	1
12:00 to 12:15	14	5	-	-	-	2	18	3	1	1	1	-
12:15 to 12:30	20	2	-	1	-	-	18	-	-	-	-	2
12:30 to 12:45	17	1	1	-	-	2	21	1	-	-	-	3
12:45 to 13:00	14	1	2	-	-	5	17	1	-	1	-	5
13:00 to 13:15	18	3	2	-	-	-	26	1	-	-	1	-
13:15 to 13:30	9	3	-	-	1	2	16	4	1	-	1	1
Peak Hour												

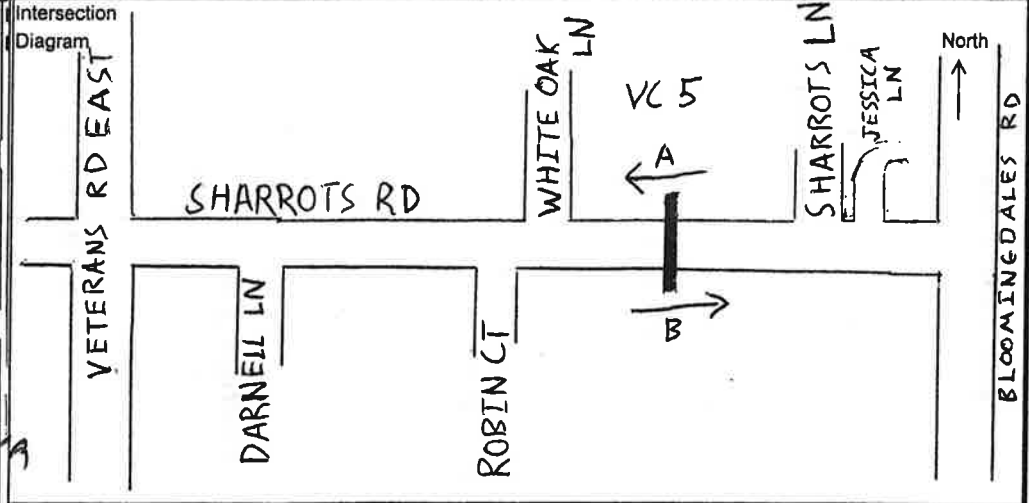
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VC-5

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date:
 Period: PM
 Weather Conditions:
 Surveyor's Name: Rafiya Budantseva



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	16	3	2	-	-	1	24	2	-	-	-	2
16:45 to 17:00	37	1	-	-	-	9	42	3	-	-	-	9
17:00 to 17:15	24	1	1	-	-	2	32	3	2	-	-	4
17:15 to 17:30	42	5	-	-	1	3	36	1	-	-	-	6
17:30 to 17:45	37	1	4	1	1	5	35	3	-	-	-	8
17:45 to 18:00	19	4	-	-	-	6	24	4	-	-	-	2
18:00 to 18:15	37	1	-	1	-	6	33	1	1	-	-	2
18:15 to 18:30	32	4	3	1	1	5	26	5	-	-	-	1
Peak Hour												

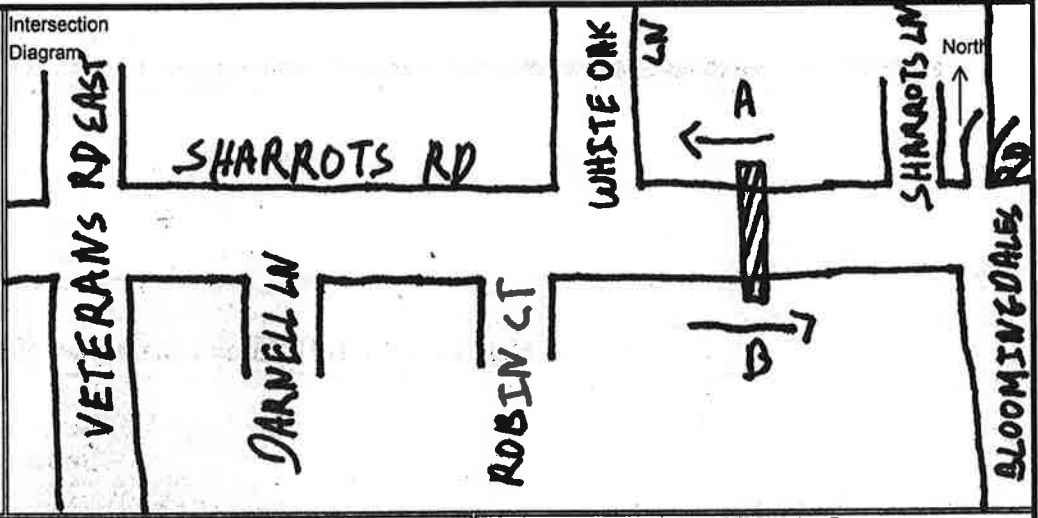
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

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 929-5656

VC-5

Traffic / Pedestrian Movement Count Sheet

Project No.: 1121A
 Location:
 Date:
 Period:
 Weather Conditions:
 Surveyor's Name: Rafiya Budantseva



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	27	1	-	-	-	-	29	1	-	-	-	1
3:15 to 3:30	23	2	1	-	-	6	27	3	-	-	-	6
3:30 to 3:45	24	-	2	-	-	7	24	-	-	-	-	2
3:45 to 4:00	23	3	1	-	-	7	27	2	-	-	-	6
4:00 to 4:15	22	2	-	-	-	4	20	1	-	-	-	2
4:15 to 4:30	21	1	-	-	-	2	29	3	-	-	-	3
4:30 to 4:45	25	-	-	-	-	8	24	2	3	-	-	7
4:45 to 5:00	21	2	-	-	-	7	25	2	1	-	-	7
5:00 to 5:15	24	4	1	-	-	3	27	3	-	-	-	5
5:15 to 5:30	22	1	1	-	-	7	18	3	1	-	-	10
5:30 to 5:45	27	1	-	-	-	6	13	2	-	-	-	5
5:45 to 6:00	26	1	-	-	-	7	24	1	-	-	-	5
Peak Hour												

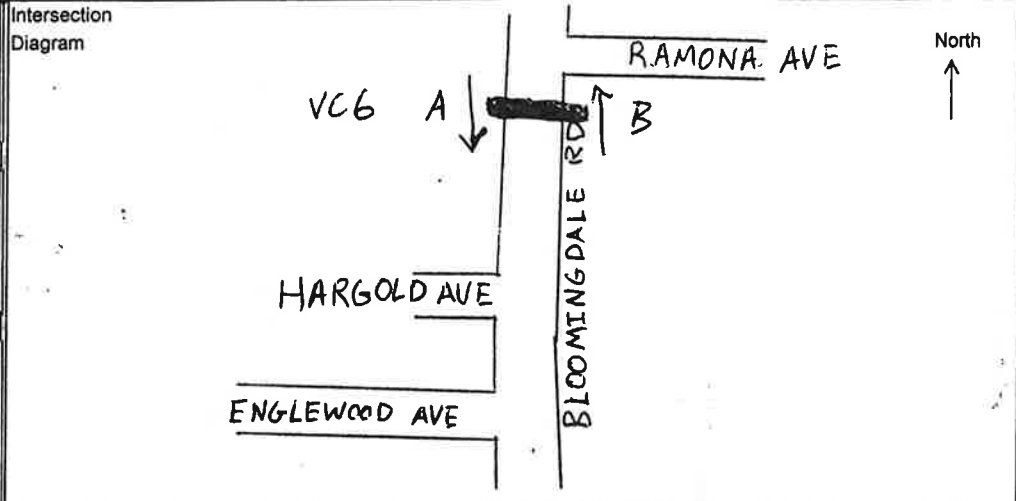
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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 New York, NY 10001
 (212)929-5656

V66

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location :
 Date:
 Period: AM **6/8/11**
 Weather Conditions:
Stanislav Filer
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	60	4	2	-	4	16	49	3	-	-	6	9
7:45 to 8:00	63	5	2	-	4	17	51	4	-	-	5	11
8:00 to 8:15	69	7	2	-	5	19	58	5	-	1	6	17
8:15 to 8:30	81	3	1	-	4	11	77	2	1	-	9	12
8:30 to 8:45	56	4	4	-	6	20	53	-	1	-	3	23
8:45 to 9:00	53	2	2	-	7	7	68	2	1	-	1	5
9:00 to 9:15	51	8	-	-	2	8	57	6	-	-	3	7
9:15 to 9:30	61	5	2	-	-	9	50	3	1	-	4	6
Peak Hour												

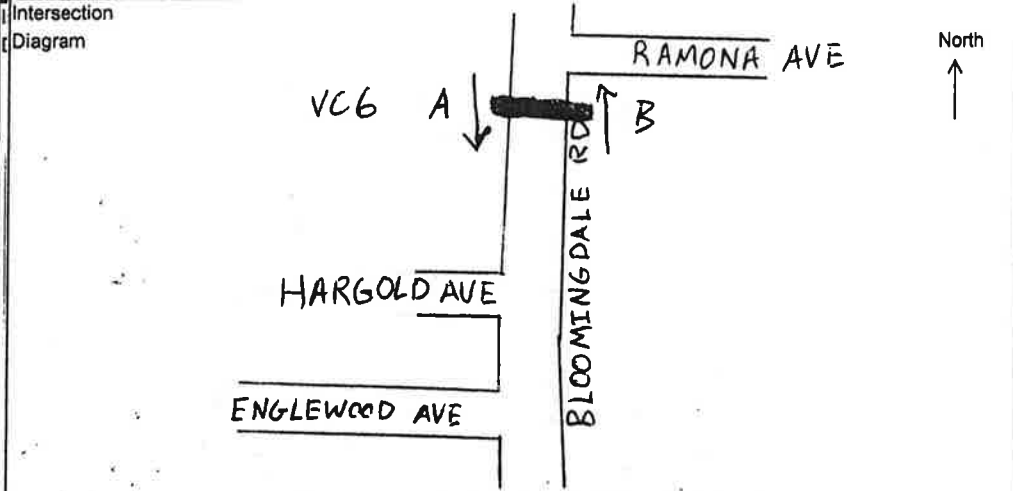
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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VC-6

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date:
 Period: MD 6/8/11
 Weather Conditions:
 Stanislaw
 Filer
 Surveyor's Name:

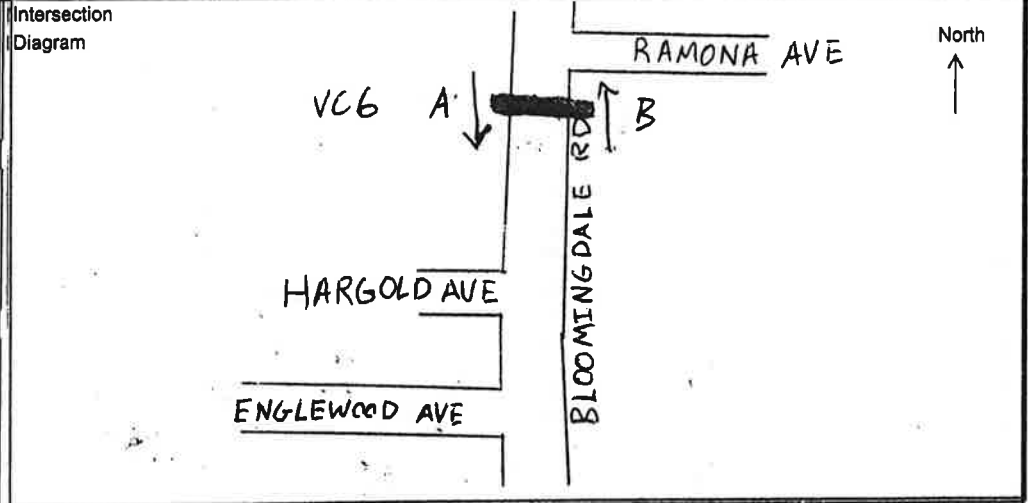


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	56	4	1	-	2	5	53	2	-	-	-	13
11:45 to 12:00	54	4	1	-	2	7	68	2	2	-	3	6
12:00 to 12:15	57	1	1	-	2	9	60	4	1	-	2	10
12:15 to 12:30	56	1	-	-	-	10	51	1	1	-	1	11
12:30 to 12:45	59	1	-	-	1	9	52	1	-	-	-	12
12:45 to 13:00	73	3	-	-	1	10	66	3	-	-	1	9
13:00 to 13:15	57	-	1	-	3	10	68	-	-	-	2	17
13:15 to 13:30	75	3	3	-	1	9	67	2	1	-	2	12
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

VC-6

Project N 1121A
 Location:
 Date: 6/8/11
 Period: PM
 Stanislar
 Filer
 Weather Conditions:
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	66	1	1	1	-	15	79	-	2	-	1	12
16:45 to 17:00	97	2	1	-	1	13	73	1	-	-	1	16
17:00 to 17:15	104	2	2	-	2	11	99	3	-	-	-	15
17:15 to 17:30	84	-	-	-	13	13	96	-	-	-	1	11
17:30 to 17:45	113	2	-	-	5	5	82	-	-	-	-	6
17:45 to 18:00	89	-	1	-	1	3	79	3	1	-	-	2
18:00 to 18:15	103	1	-	-	2	2	91	2	-	-	1	1
18:15 to 18:30	91	1	-	-	1	3	88	1	-	-	2	1
Peak Hour												

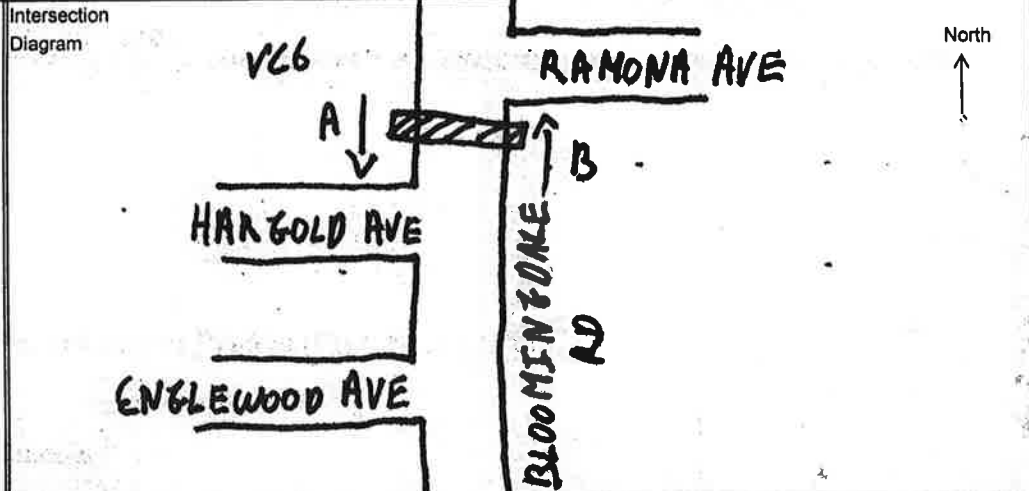
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

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 (212)929-5656

V66

Traffic / Pedestrian Movement Count Sheet

Project No.: 1121A
 Location:
 Date: 6/11/2011
 Period: STANISLAV
 FILEV
 Weather Conditions:
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	55	-	-	-	-	28	74	-	-	-	-	29
3:15 to 3:30	44	-	-	-	-	26	46	-	1	-	-	43
3:30 to 3:45	54	1	-	-	-	20	60	-	1	-	-	17
3:45 to 4:00	52	-	-	-	-	33	53	1	-	-	-	34
4:00 to 4:15	50	1	-	-	-	35	67	-	1	-	-	31
4:15 to 4:30	47	1	-	-	-	26	66	-	-	-	-	27
4:30 to 4:45	48	5	-	-	-	29	47	-	1	-	-	19
4:45 to 5:00	45	-	-	-	-	22	48	2	1	-	-	24
5:00 to 5:15	44	-	1	-	1	31	49	-	-	-	-	18
5:15 to 5:30	50	-	-	-	-	32	51	1	-	-	-	20
5:30 to 5:45	53	1	-	-	-	21	67	1	-	-	-	21
5:45 to 6:00	56	-	-	-	-	24	61	-	-	-	-	23
Peak Hour												

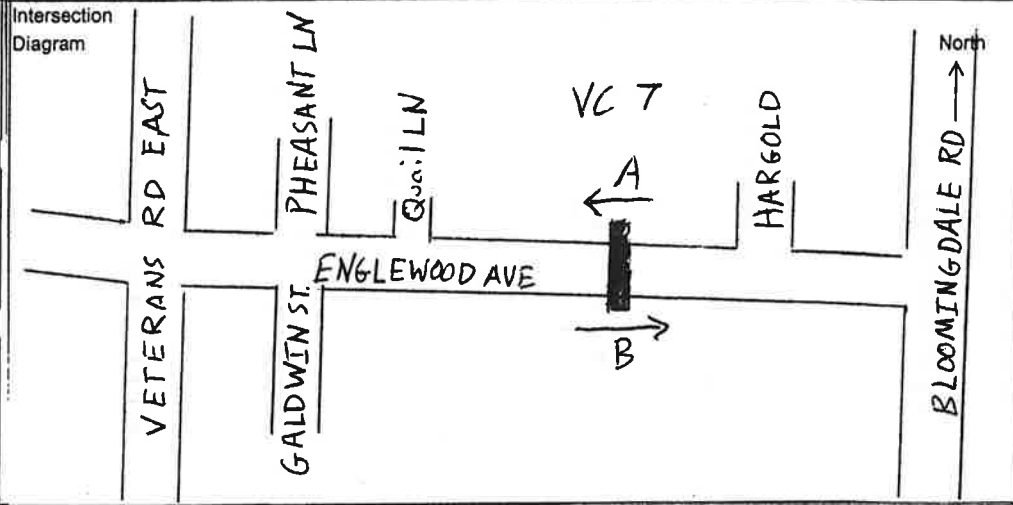
LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

PHILIP HABIB & ASSOCIATE
 226, W. 26th St. 9FL
 New York, NY 10001
 (212)929-5656

VC7

Traffic / Pedestrian Movement Count Sheet

Project N 1121A
 Location:
 Date: 06/08/11
 Period: AM
 Weather Conditions: Sunny
 Surveyor's Name: Valentina Yermakova



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
7:30 to 7:45	7	1	-	-	-	2	12	1	-	-	-	7
7:45 to 8:00	9	2	-	1	1	2	14	5	-	-	2	10
8:00 to 8:15	7	1	-	-	-	6	17	2	1	-	-	12
8:15 to 8:30	13	-	-	-	-	2	7	1	1	1	-	9
8:30 to 8:45	5	2	-	-	-	6	12	4	-	-	-	7
8:45 to 9:00	5	1	-	-	-	5	5	3	-	-	-	6
9:00 to 9:15	6	1	-	-	-	4	14	2	-	-	-	8
9:15 to 9:30	4	-	-	-	-	2	13	0	1	1	-	5
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

PHILIP HABIB & ASSOCIATE
 226, W. 26th St. 9FL
 New York, NY 10001
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VC-7

Traffic / Pedestrian Movement Count Sheet

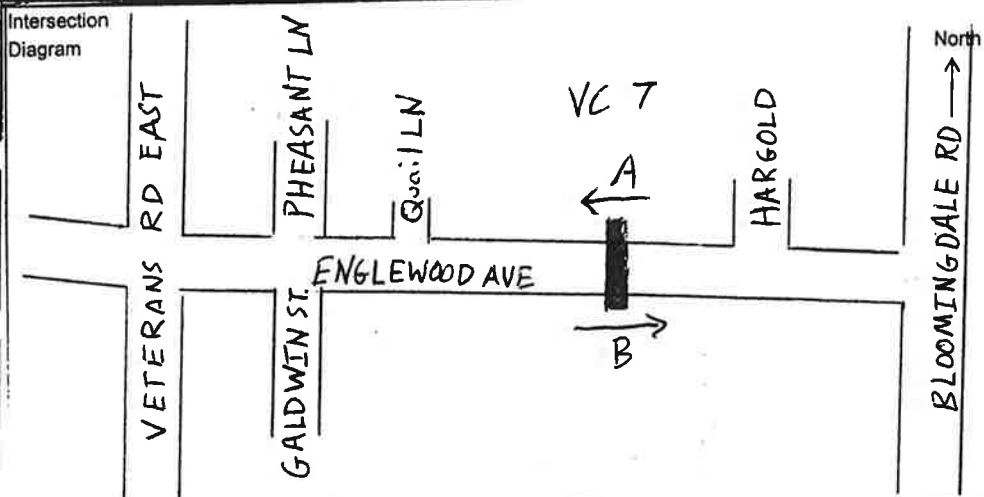
Project N 1121A
 Location:

Date:
 06/08/11

Period: MD

Weather Conditions:

Valentyna Yermakova
 Surveyor's Name:



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
11:30 to 11:45	8	-	-	-	-	4	17	2	-	-	-	6
11:45 to 12:00	4	-	-	-	-	4	16	-	1	-	-	8
12:00 to 12:15	6	-	1	-	-	4	16	1	-	-	-	4
12:15 to 12:30	6	-	-	-	-	1	14	-	-	-	-	7
12:30 to 12:45	8	-	1	-	-	3	9	1	-	-	-	3
12:45 to 13:00	11	-	-	-	-	3	25	-	-	-	-	4
13:00 to 13:15	7	2	-	-	-	4	17	2	-	-	1	5
13:15 to 13:30	8	1	-	1	-	2	10	1	1	-	-	7
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

VC-7

Project N 1121A

Location :

Intersection
Diagram

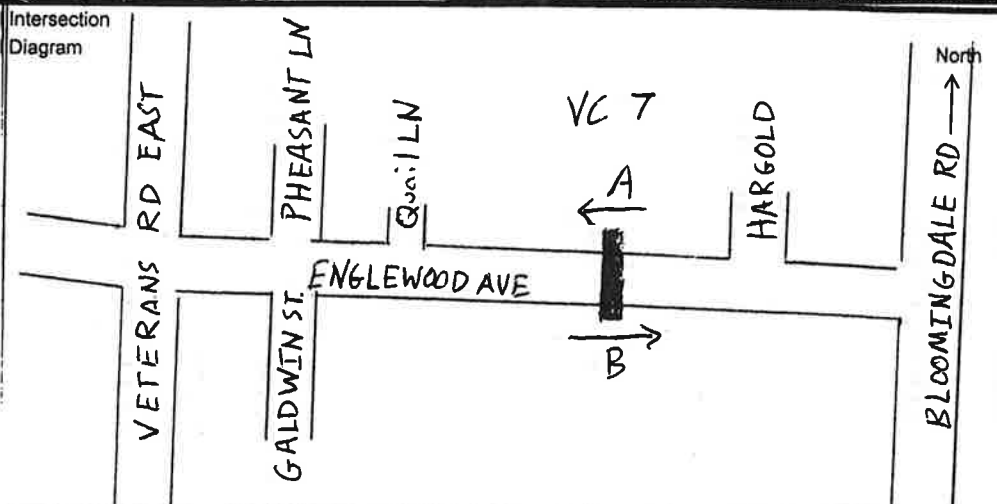
Date:

06/08/11

Period: PM

Weather Conditions:

Valentina Yermakova
 Surveyor's Name:

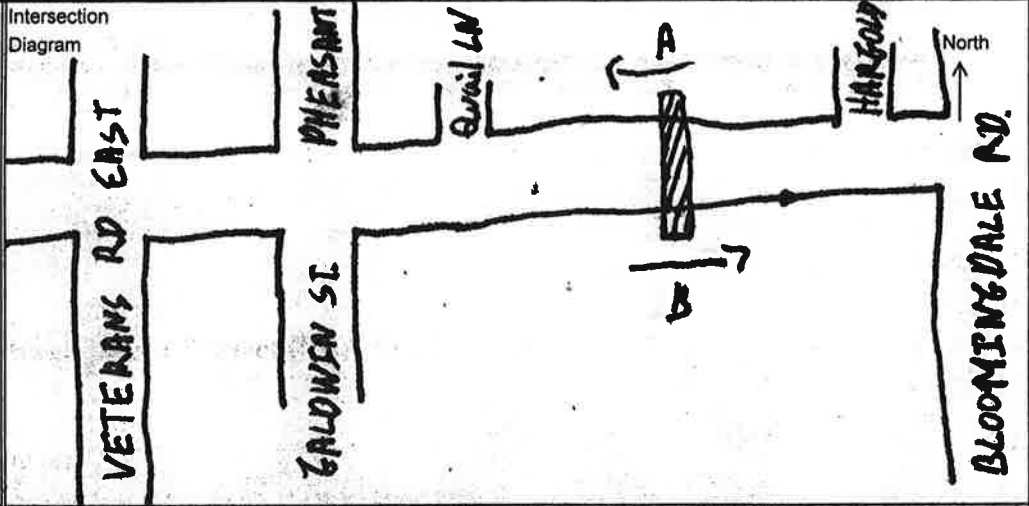


Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
16:30 to 16:45	9	-	-	-	1	5	23	-	2	1	-	8
16:45 to 17:00	10	-	-	-	-	4	19	1	-	-	-	13
17:00 to 17:15	13	3	-	-	-	3	17	3	1	-	-	11
17:15 to 17:30	12	-	-	-	-	4	15	1	-	-	-	7
17:30 to 17:45	10	-	-	-	-	8	17	2	-	-	-	6
17:45 to 18:00	8	-	-	-	-	6	10	2	-	1	-	6
18:00 to 18:15	12	1	-	-	-	8	11	2	1	-	-	9
18:15 to 18:30	11	-	-	-	-	3	16	3	-	-	1	11
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
 HDT= Heavy Duty Trucks (6 or more Wheels; Walk-in Trucks)
 DT= Diesel Trucks (with Vertical Tail Pipe; Tractor-Trailers)

VC7

Project No.: 1121A
Location:
Date: 06/11/11
Period:
Weather Conditions: rain!
Surveyor's Name: Valentyna Yermakova



Time	Location/Direction A						Location/Direction B					
	AUTO 1	LDT 2	HDT 3	DT 4	BUS 5	SUV 6	AUTO 7	LDT 8	HDT 9	DT 10	BUS 11	SUV 12
3:00 to 3:15	4	1	-	-	-	2	11	2	-	-	-	9
3:15 to 3:30	10	1	-	-	-	4	20	1	-	-	-	9
3:30 to 3:45	7	1	-	-	-	2	17	1	-	-	-	8
3:45 to 4:00	13	1	-	-	-	3	14	1	-	-	-	14
4:00 to 4:15	4	1	-	-	-	3	14	1	1	-	-	14
4:15 to 4:30	4	-	-	-	-	3	15	2	-	-	-	13
4:30 to 4:45	7	-	-	-	-	4	16	-	-	-	-	7
4:45 to 5:00	6	-	-	-	-	7	14	-	-	-	-	6
5:00 to 5:15	9	-	-	-	-	6	8	-	-	-	-	10
5:15 to 5:30	12	-	-	-	-	3	8	-	1	-	-	4
5:30 to 5:45	7	-	-	-	-	1	24	-	-	-	-	9
5:45 to 6:00	7	-	1	-	-	1	8	-	-	-	-	3
Peak Hour												

LDT= Light Duty Trucks (4 Wheels; Commercial Mini Vans; Mini School Buses)
HDT= Heavy Duty Trucks (6 or more Wheels; Walk-In Trucks)
DT= Diesel Trucks (with Vertical Tale Pipe; Tractor-Trailers)

Automatic Traffic Recorder Counts

Charleston ATR June 2011	Arthur Kill Rd NB South of Sharrots Rd				Arthur Kill Rd SB North of Sharrots Rd			
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG
AM								
6:30 - 6:45	39	48	51	46	31	30	31	31
6:45 - 7:00	32	39	40	37	45	50	33	43
7:00 - 7:15	40	46	40	42	33	47	42	41
7:15 - 7:30	57	44	50	50	49	53	49	50
7:30 - 7:45	53	45	58	52	61	51	50	54
7:45 - 8:00	61	64	60	62	46	56	44	49
8:00 - 8:15	52	72	48	57	51	49	49	50
8:15 - 8:30	44	47	41	44	50	43	34	42
8:30 - 8:45	55	53	29	46	39	42	39	40
8:45 - 9:00	53	50	33	45	47	64	55	55
9:00 - 9:15	46	59	53	53	57	52	57	55
9:15 - 9:30	36	45	45	42	37	46	43	42
Peak Hour Total	204	222	151	192	187	198	177	187
PHF	0.93	0.77	0.79	0.83	0.92	0.77	0.80	0.83
MD								
11:00 - 11:15	46	53	58	52	58	63	47	56
11:15 - 11:30	47	63	43	51	59	48	64	57
11:30 - 11:45	59	67	36	54	51	46	58	52
11:45 - 12:00	41	51	63	52	76	44	82	67
12:00 - 12:15	68	60	72	67	57	51	61	56
12:15 - 12:30	64	59	48	57	63	56	59	59
12:30 - 12:45	59	58	55	57	56	60	69	62
12:45 - 1:00	82	44	52	59	64	51	52	56
1:00 - 1:15	54	43	47	48	61	64	64	63
1:15 - 1:30	58	50	49	52	62	53	65	60
1:30 - 1:45	58	45	43	49	41	62	72	58
1:45 - 2:00	36	54	54	48	45	52	70	56
2:00 - 2:15	58	56	63	59	64	46	66	59
2:15 - 2:30	58	59	52	56	73	65	66	68
2:30 - 2:45	62	67	69	66	77	50	62	63
2:45 - 3:00	57	52	68	59	79	64	77	73
Peak Hour Total	273	221	227	240	240	218	241	233
PHF	0.83	0.92	0.79	0.85	0.94	0.91	0.87	0.91
PM								
4:00 - 4:15	80	78	66	75	65	105	63	78
4:15 - 4:30	55	62	67	61	68	71	73	71
4:30 - 4:45	59	59	60	59	57	59	68	61
4:45 - 5:00	60	89	74	74	68	58	59	62
5:00 - 5:15	62	98	66	75	78	89	73	80
5:15 - 5:30	45	76	45	55	44	61	66	57
5:30 - 5:45	50	92	54	65	46	58	66	57
5:45 - 6:00	67	85	65	72	66	68	60	65
6:00 - 6:15	73	73	68	71	54	33	41	43
6:15 - 6:30	61	77	61	66	36	40	39	38
6:30 - 6:45	52	56	53	54	56	48	47	50
6:45 - 7:00	39	62	53	51	32	41	41	38
Peak Hour Total	224	351	230	268	234	276	265	258
PHF	0.84	0.90	0.87	0.87	0.75	0.78	0.91	0.81

Charleston ATR June 2011	Arthur Kill Rd NB South of Sharrots Rd			Arthur Kill Rd SB North of Sharrots Rd		
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG
SMD						
12:00 - 12:15	55	48	52	54	45	50
12:15 - 12:30	67	43	55	47	56	52
12:30 - 12:45	58	50	54	57	47	52
12:45 - 1:00	60	45	53	66	59	63
1:00 - 1:15	64	50	57	59	50	55
1:15 - 1:30	65	59	62	65	74	70
1:30 - 1:45	56	59	58	60	60	60
1:45 - 2:00	55	47	51	62	55	59
2:00 - 2:15	54	38	46	56	49	53
2:15 - 2:30	54	52	53	48	40	44
2:30 - 2:45	33	47	40	55	62	59
2:45 - 3:00	46	47	47	44	41	43
3:00 - 3:15	43	37	40	63	39	51
3:15 - 3:30	40	35	38	52	42	47
3:30 - 3:45	48	39	44	44	38	41
3:45 - 4:00	51	51	51	52	48	50
4:00 - 4:15	44	41	43	45	48	47
4:15 - 4:30	44	38	41	41	29	35
4:30 - 4:45	36	35	36	56	44	50
4:45 - 5:00	33	48	41	39	49	44
5:00 - 5:15	43	51	47	39	42	41
5:15 - 5:30	35	33	34	29	29	29
5:30 - 5:45	33	40	37	30	28	29
5:45 - 6:00	24	29	27	33	51	42
Peak Hour Total	245	213	229	250	243	247
PHF	0.94	0.90	0.92	0.95	0.82	0.88

Charleston ATR June 2011	Arthur Kill Rd NB South of S Bridge St				Arthur Kill Rd SB North of N Bridge St					
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG		
AM										
6:30 - 6:45	113	108	91	104	46	51	34	44		
6:45 - 7:00	86	93	112	97	50	43	49	47		
7:00 - 7:15	110	122	118	117	60	61	41	54		
7:15 - 7:30	153	136	140	143	53	64	60	59	204	
7:30 - 7:45	162	147	125	145	501	71	65	57	64	225
7:45 - 8:00	172	172	177	174	578	81	83	48	71	248
8:00 - 8:15	186	186	146	173	634	59	58	58	58	252
8:15 - 8:30	164	167	143	158	649	56	49	51	52	245
8:30 - 8:45	145	146	140	144	648	56	68	60	61	242
8:45 - 9:00	126	132	83	114	588	78	69	61	69	241
9:00 - 9:15	99	142	123	121	537	73	77	76	75	258
9:15 - 9:30	130	124	134	129	508	58	65	68	64	270
Peak Hour Total	621	631	512	588	249	244	230	241		
PHF	0.83	0.85	0.88	0.85	0.80	0.88	0.94	0.87		
MD										
11:00 - 11:15	129	126	132	129	93	74	80	82		
11:15 - 11:30	145	123	102	123	74	81	83	79		
11:30 - 11:45	140	127	84	117	78	93	82	84		
11:45 - 12:00	109	130	146	128	498	95	90	105	97	343
12:00 - 12:15	164	140	130	145	513	87	64	80	//	337
12:15 - 12:30	142	103	130	125	515	83	108	88	93	351
12:30 - 12:45	153	127	138	139	537	76	84	99	86	353
12:45 - 1:00	159	143	118	140	549	79	78	74	77	333
1:00 - 1:15	97	135	125	119	523	87	83	72	81	337
1:15 - 1:30	118	115	129	121	519	92	74	91	86	330
1:30 - 1:45	116	128	118	121	500	78	87	80	82	325
1:45 - 2:00	99	119	124	114	474	86	87	83	85	333
2:00 - 2:15	146	116	116	126	481	77	89	61	76	328
2:15 - 2:30	118	132	139	130	490	84	103	89	92	335
2:30 - 2:45	178	169	164	170	540	97	75	89	87	340
2:45 - 3:00	139	144	143	142	568	79	78	88	82	336
Peak Hour Total	618	513	516	549	325	334	341	333		
PHF	0.94	0.90	0.93	0.92	0.93	0.77	0.86	0.86		
PM										
4:00 - 4:15	187	197	166	183	100	138	114	117		
4:15 - 4:30	148	172	141	154	101	117	114	111		
4:30 - 4:45	140	171	156	156	101	108	102	104		
4:45 - 5:00	142	132	138	137	630	106	106	121	111	443
5:00 - 5:15	148	154	161	154	601	133	114	128	125	450
5:15 - 5:30	157	158	163	159	607	94	104	88	95	435
5:30 - 5:45	141	134	154	143	594	108	113	111	111	442
5:45 - 6:00	174	162	142	159	616	113	92	92	99	430
6:00 - 6:15	179	184	189	184	646	79	85	90	85	390
6:15 - 6:30	159	140	134	144	631	96	94	88	93	387
6:30 - 6:45	148	139	170	152	640	94	94	85	91	367
6:45 - 7:00	172	166	157	165	646	87	93	93	91	359
Peak Hour Total	620	608	620	616	448	423	419	430		
PHF	0.89	0.94	0.95	0.93	0.84	0.93	0.82	0.86		

Charleston ATR June 2011	Arthur Kill Rd NB South of S Bridge St			Arthur Kill Rd SB North of N Bridge St			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	152	160	156	104	100	102	
12:15 - 12:30	169	177	173	96	97	97	
12:30 - 12:45	173	146	160	115	99	107	
12:45 - 1:00	206	172	189	115	98	107	412
1:00 - 1:15	180	163	172	94	113	104	414
1:15 - 1:30	185	148	167	121	115	118	435
1:30 - 1:45	157	167	162	122	89	106	434
1:45 - 2:00	159	172	166	84	112	98	425
2:00 - 2:15	161	157	159	96	94	95	417
2:15 - 2:30	156	147	152	87	80	84	382
2:30 - 2:45	136	137	137	83	76	80	356
2:45 - 3:00	129	149	139	77	89	83	341
3:00 - 3:15	169	121	145	77	53	65	311
3:15 - 3:30	132	117	125	87	36	62	289
3:30 - 3:45	146	143	145	98	43	71	280
3:45 - 4:00	166	163	165	84	40	62	259
4:00 - 4:15	163	161	162	84	47	66	260
4:15 - 4:30	140	159	150	81	60	71	269
4:30 - 4:45	164	141	153	77	61	69	267
4:45 - 5:00	164	115	140	82	56	69	274
5:00 - 5:15	169	113	141	67	58	63	271
5:15 - 5:30	153	186	170	88	62	75	276
5:30 - 5:45	121	133	127	58	60	59	266
5:45 - 6:00	107	129	118	86	75	81	277
Peak Hour Total	728	650	689	452	415	434	
PHF	0.88	0.94	0.91	0.93	0.90	0.91	

Charleston ATR June 2011	Boscombe Ave WB East of Korean War Vet HW Ramp				Boscombe Ave EB West of Korean War Vet HW Ramp			
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG
AM								
6:30 - 6:45	34	33	38	35	166	178	161	168
6:45 - 7:00	40	50	48	46	180	178	171	176
7:00 - 7:15	58	58	65	60	181	194	181	185
7:15 - 7:30	58	73	63	65	212	220	221	218
7:30 - 7:45	75	78	72	75	228	218	214	220
7:45 - 8:00	80	89	83	84	260	275	210	248
8:00 - 8:15	104	75	60	80	278	246	207	244
8:15 - 8:30	110	99	95	101	288	276	221	262
8:30 - 8:45	126	91	108	108	225	228	222	225
8:45 - 9:00	88	93	91	91	232	226	190	216
9:00 - 9:15	91	118	105	105	190	206	207	201
9:15 - 9:30	98	89	78	88	212	189	196	199
Peak Hour Total	428	358	354	380	1023	976	840	946
PHF	0.85	0.90	0.82	0.86	0.89	0.88	0.95	0.91
MD								
11:00 - 11:15	187	138	134	153	220	201	218	213
11:15 - 11:30	158	126	128	137	218	205	52	158
11:30 - 11:45	160	135	146	147	216	202	216	211
11:45 - 12:00	141	129	154	141	219	221	242	227
12:00 - 12:15	159	147	155	154	245	246	239	243
12:15 - 12:30	164	171	187	174	266	186	213	222
12:30 - 12:45	178	188	170	179	228	238	223	230
12:45 - 1:00	190	160	184	178	222	200	215	212
1:00 - 1:15	184	162	169	172	214	218	212	215
1:15 - 1:30	177	165	165	169	245	228	206	226
1:30 - 1:45	172	179	152	168	213	219	209	214
1:45 - 2:00	168	152	156	159	232	203	221	219
2:00 - 2:15	160	193	147	167	206	229	206	214
2:15 - 2:30	172	158	173	168	233	226	240	233
2:30 - 2:45	154	144	146	148	277	263	297	279
2:45 - 3:00	141	145	136	141	213	255	214	227
Peak Hour Total	691	666	696	684	961	870	890	907
PHF	0.91	0.89	0.93	0.91	0.90	0.88	0.93	0.91
PM								
4:00 - 4:15	169	184	165	173	294	270	260	275
4:15 - 4:30	170	152	158	160	238	248	236	241
4:30 - 4:45	180	180	129	163	231	252	216	233
4:45 - 5:00	137	163	157	152	239	241	218	233
5:00 - 5:15	154	165	140	153	281	256	228	255
5:15 - 5:30	150	152	147	150	236	238	253	242
5:30 - 5:45	167	170	154	164	246	213	218	226
5:45 - 6:00	171	162	143	159	223	210	235	223
6:00 - 6:15	154	153	150	152	247	273	245	255
6:15 - 6:30	139	170	145	151	233	249	257	246
6:30 - 6:45	140	160	146	149	208	226	216	217
6:45 - 7:00	156	153	169	159	213	229	220	221
Peak Hour Total	642	649	584	625	986	917	934	946
PHF	0.94	0.95	0.95	0.95	0.88	0.90	0.92	0.90

Charleston ATR June 2011	Boscombe Ave WB East of Korean War Vet HW Ramp				Boscombe Ave EB West of Korean War Vet HW Ramp			
	Sat 1	Sat 2	AVG		Sat 1	Sat 2	AVG	
SMD								
12:00 - 12:15	202	188	195		257	251	254	
12:15 - 12:30	201	202	202		271	241	256	
12:30 - 12:45	198	178	188		269	273	271	
12:45 - 1:00	225	207	216	801	264	272	268	1049
1:00 - 1:15	191	190	191	796	303	267	285	1080
1:15 - 1:30	213	221	217	812	278	276	277	1101
1:30 - 1:45	223	209	216	840	290	260	275	1105
1:45 - 2:00	186	205	196	819	294	313	304	1141
2:00 - 2:15	202	206	204	833	259	302	281	1136
2:15 - 2:30	223	185	204	820	271	241	256	1115
2:30 - 2:45	194	208	201	805	230	244	237	1077
2:45 - 3:00	206	187	197	806	247	247	247	1021
3:00 - 3:15	229	178	204	805	246	199	223	963
3:15 - 3:30	206	181	194	795	222	269	246	952
3:30 - 3:45	191	173	182	776	260	229	245	960
3:45 - 4:00	237	193	215	794	255	263	259	972
4:00 - 4:15	206	188	197	788	249	239	244	993
4:15 - 4:30	218	173	196	790	262	243	253	1000
4:30 - 4:45	207	176	192	799	232	228	230	986
4:45 - 5:00	196	165	181	765	226	231	229	955
5:00 - 5:15	153	171	162	730	261	212	237	948
5:15 - 5:30	185	158	172	706	213	253	233	928
5:30 - 5:45	215	158	187	701	227	237	232	930
5:45 - 6:00	184	151	168	688	223	203	213	915
Peak Hour Total	852	827	840		1135	1075	1105	
PHF	0.95	0.94	0.94		0.94	0.97	0.96	

Charleston ATR June 2011	Veterans Rd West EB *				Veterans Rd West WB *					
	West of N Bridge St				East of N Bridge St					
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG		
AM										
6:30 - 6:45	39	56	39	45	52	52	42	49		
6:45 - 7:00	36	41	44	40	73	49	75	66		
7:00 - 7:15	68	72	57	66	53	79	69	67		
7:15 - 7:30	63	92	71	75	226	78	101	75	85	266
7:30 - 7:45	73	61	57	64	245	143	108	90	114	331
7:45 - 8:00	102	85	76	88	292	134	194	133	154	419
8:00 - 8:15	86	102	72	87	313	114	162	127	134	486
8:15 - 8:30	90	87	75	84	322	108	191	111	137	538
8:30 - 8:45	94	73	83	83	342	91	189	174	151	576
8:45 - 9:00	67	70	77	71	325	113	95	174	127	550
9:00 - 9:15	62	78	71	70	309	80	181	198	153	568
9:15 - 9:30	71	77	75	74	299	115	168	193	159	590
Peak Hour Total	337	332	307	325	426	637	586	550		
PHF	0.90	0.81	0.92	0.88	0.93	0.83	0.84	0.87		
MD										
11:00 - 11:15	70	85	85	80	160	125	199	161		
11:15 - 11:30	90	71	65	75	157	185	232	191		
11:30 - 11:45	85	84	60	76	126	163	169	153		
11:45 - 12:00	90	83	82	85	317	132	185	192	170	675
12:00 - 12:15	68	81	86	78	315	150	145	186	160	674
12:15 - 12:30	80	83	65	76	316	150	183	156	163	646
12:30 - 12:45	104	76	80	87	326	148	200	191	180	673
12:45 - 1:00	92	103	69	88	329	166	157	164	162	665
1:00 - 1:15	84	75	76	78	329	111	170	196	159	664
1:15 - 1:30	74	67	67	69	322	126	142	158	142	643
1:30 - 1:45	97	82	63	81	316	158	159	182	166	630
1:45 - 2:00	73	71	74	73	301	153	145	157	152	619
2:00 - 2:15	93	79	69	80	303	138	193	188	173	633
2:15 - 2:30	82	89	85	85	319	148	160	157	155	646
2:30 - 2:45	118	82	76	92	330	199	162	184	182	661
2:45 - 3:00	90	80	75	82	339	162	224	148	178	688
Peak Hour Total	344	343	300	329	614	685	697	665		
PHF	0.83	0.83	0.87	0.84	0.92	0.86	0.91	0.90		
PM										
4:00 - 4:15	124	100	114	113	118	178	120	139		
4:15 - 4:30	104	119	91	105	135	136	104	125		
4:30 - 4:45	110	132	107	116	147	115	116	126		
4:45 - 5:00	106	111	103	107	440	245	125	131	167	557
5:00 - 5:15	112	121	135	123	450	195	103	161	153	571
5:15 - 5:30	111	116	119	115	461	117	102	239	153	599
5:30 - 5:45	85	110	101	99	443	130	137	256	174	647
5:45 - 6:00	105	95	87	96	432	124	118	250	164	644
6:00 - 6:15	98	118	91	102	412	111	141	231	161	652
6:15 - 6:30	72	89	91	84	381	108	119	196	141	640
6:30 - 6:45	78	81	81	80	362	112	100	198	137	603
6:45 - 7:00	93	84	74	84	350	120	145	191	152	591
Peak Hour Total	413	442	442	432	566	460	906	644		
PHF	0.92	0.91	0.82	0.88	0.73	0.84	0.88	0.82		

*Note - Median ATR value for MD peak (343) was used in balancing network to avoid a dramatic reduction of manual traffic counts

*Note - Median ATR value for PM peak (566) was used in balancing network to avoid a dramatic increase of manual traffic counts

Charleston ATR June 2011	Veterans Rd West EB West of N Bridge St			Veterans Rd West WB East of N Bridge St			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	113	111	112	105	199	152	
12:15 - 12:30	120	120	120	130	220	175	
12:30 - 12:45	123	81	102	113	213	163	
12:45 - 1:00	105	110	108	145	214	180	670
1:00 - 1:15	127	107	117	117	208	163	680
1:15 - 1:30	133	108	121	140	190	165	670
1:30 - 1:45	111	94	103	233	141	187	694
1:45 - 2:00	99	105	102	197	154	176	690
2:00 - 2:15	128	102	115	139	139	139	667
2:15 - 2:30	102	102	102	115	151	133	635
2:30 - 2:45	98	99	99	107	128	118	565
2:45 - 3:00	94	119	107	125	125	125	515
3:00 - 3:15	138	109	124	113	91	102	478
3:15 - 3:30	120	84	102	108	108	108	453
3:30 - 3:45	103	91	97	146	131	139	474
3:45 - 4:00	105	101	103	123	107	115	464
4:00 - 4:15	100	122	111	139	107	123	485
4:15 - 4:30	96	108	102	93	132	113	489
4:30 - 4:45	131	91	111	99	151	125	476
4:45 - 5:00	110	86	98	104	144	124	485
5:00 - 5:15	111	102	107	94	138	116	478
5:15 - 5:30	104	109	107	95	124	110	475
5:30 - 5:45	81	81	81	77	88	83	432
5:45 - 6:00	70	68	69	101	107	104	412
Peak Hour Total	476	419	448	635	753	694	
PHF	0.89	0.95	0.92	0.68	0.88	0.78	

Charleston ATR June 2011	Korean War Vet HW WB Off-Ramp South of Veterans Rd West				Tyrellan Ave NB South of Veterans Rd West				
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG	
AM									
6:30 - 6:45	94	83	91	89	32	45	43	40	
6:45 - 7:00	105	113	112	110	49	42	46	46	
7:00 - 7:15	94	75	87	85	43	46	55	48	
7:15 - 7:30	112	114	113	113	56	64	61	60	194
7:30 - 7:45	162	148	118	143	56	63	53	57	211
7:45 - 8:00	139	156	170	155	89	86	67	81	246
8:00 - 8:15	153	153	128	145	84	86	59	76	275
8:15 - 8:30	134	139	146	140	88	106	68	87	302
8:30 - 8:45	156	130	144	143	81	80	82	81	325
8:45 - 9:00	124	142	138	135	93	74	71	79	324
9:00 - 9:15	142	139	151	144	87	72	86	82	329
9:15 - 9:30	139	152	146	146	78	82	84	81	323
Peak Hour Total	567	564	556	562	346	346	280	324	
PHF	0.91	0.92	0.95	0.93	0.93	0.82	0.85	0.87	
MD									
11:00 - 11:15	138	121	139	133	103	89	97	96	
11:15 - 11:30	148	144	163	152	90	96	98	95	
11:30 - 11:45	158	169	136	154	100	86	113	100	
11:45 - 12:00	179	157	159	165	106	90	114	103	394
12:00 - 12:15	164	151	166	160	109	106	132	116	413
12:15 - 12:30	155	137	142	145	94	105	102	100	419
12:30 - 12:45	139	148	164	150	109	128	112	116	436
12:45 - 1:00	142	143	135	140	90	86	103	93	425
1:00 - 1:15	131	135	142	136	93	96	91	93	403
1:15 - 1:30	156	133	143	144	95	93	85	91	394
1:30 - 1:45	148	152	141	147	104	101	85	97	374
1:45 - 2:00	145	187	146	159	98	82	90	90	371
2:00 - 2:15	166	190	137	164	83	76	93	84	362
2:15 - 2:30	144	155	154	151	110	97	89	99	369
2:30 - 2:45	169	200	159	176	108	99	110	106	378
2:45 - 3:00	166	223	193	194	96	111	92	100	388
Peak Hour Total	600	579	607	595	402	425	449	425	
PHF	0.91	0.96	0.91	0.93	0.92	0.83	0.85	0.87	
PM									
4:00 - 4:15	210	232	208	217	111	113	91	105	
4:15 - 4:30	175	200	177	184	120	101	86	102	
4:30 - 4:45	182	210	184	192	96	88	71	85	
4:45 - 5:00	189	229	127	182	103	115	83	100	393
5:00 - 5:15	204	197	187	196	108	93	104	102	389
5:15 - 5:30	201	222	202	208	90	98	105	98	385
5:30 - 5:45	194	189	210	198	99	96	83	93	392
5:45 - 6:00	209	246	213	223	103	78	95	92	384
6:00 - 6:15	181	202	206	196	98	103	95	99	381
6:15 - 6:30	181	238	199	206	93	89	105	96	379
6:30 - 6:45	215	265	209	230	74	75	89	79	366
6:45 - 7:00	194	208	209	204	83	88	75	82	356
Peak Hour Total	808	854	812	825	400	365	387	384	
PHF	0.97	0.87	0.95	0.93	0.93	0.93	0.92	0.93	

Charleston ATR June 2011	Korean War Vet HW WB Off-Ramp South of Veterans Rd West			Tyrellan Ave NB South of Veterans Rd West			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	177	159	168	119	106	113	
12:15 - 12:30	174	176	175	131	98	115	
12:30 - 12:45	190	155	173	121	127	124	
12:45 - 1:00	176	176	176	119	122	121	472
1:00 - 1:15	144	197	171	128	130	129	488
1:15 - 1:30	193	164	179	117	143	130	504
1:30 - 1:45	192	167	180	119	105	112	492
1:45 - 2:00	189	152	171	115	122	119	490
2:00 - 2:15	151	151	151	129	158	144	504
2:15 - 2:30	184	149	167	115	118	117	491
2:30 - 2:45	166	164	165	108	118	113	492
2:45 - 3:00	164	167	166	122	97	110	483
3:00 - 3:15	176	156	166	100	79	90	429
3:15 - 3:30	187	160	174	103	117	110	422
3:30 - 3:45	221	155	188	126	102	114	423
3:45 - 4:00	213	167	190	100	109	105	418
4:00 - 4:15	165	173	169	91	101	96	425
4:15 - 4:30	166	189	178	104	108	106	421
4:30 - 4:45	178	171	175	103	96	100	406
4:45 - 5:00	173	154	164	82	104	93	395
5:00 - 5:15	146	136	141	97	94	96	394
5:15 - 5:30	165	134	150	74	98	86	374
5:30 - 5:45	174	134	154	71	83	77	352
5:45 - 6:00	194	150	172	102	87	95	353
Peak Hour Total	705	704	705	483	500	492	
PHF	0.91	0.89	0.90	0.94	0.87	0.91	

Charleston ATR June 2011	Tyrellan Ave SB North of Veterans Rd West				Mohr Rd WB (In) East of Tyrellan Ave				
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG	
AM									
6:30 - 6:45	16	22	19	19	8	14	16	13	
6:45 - 7:00	14	21	24	20	14	21	22	19	
7:00 - 7:15	33	36	30	33	17	19	19	18	
7:15 - 7:30	19	24	22	22	21	23	13	19	69
7:30 - 7:45	25	27	33	28	15	21	22	19	76
7:45 - 8:00	19	22	31	24	29	36	33	33	89
8:00 - 8:15	26	32	40	33	42	41	36	40	111
8:15 - 8:30	49	45	49	48	33	45	32	37	128
8:30 - 8:45	42	52	60	51	48	50	37	45	154
8:45 - 9:00	60	65	69	65	42	68	56	55	177
9:00 - 9:15	63	70	48	60	54	55	55	55	192
9:15 - 9:30	66	81	74	74	75	52	54	60	215
Peak Hour Total	177	194	218	196	165	204	161	177	
PHF	0.74	0.75	0.79	0.76	0.86	0.75	0.72	0.78	
MD									
11:00 - 11:15	96	87	93	92	69	83	66	73	
11:15 - 11:30	105	88	116	103	88	63	70	74	
11:30 - 11:45	104	112	131	116	78	67	76	74	
11:45 - 12:00	105	119	157	127	81	72	81	78	298
12:00 - 12:15	125	139	124	129	110	86	82	93	318
12:15 - 12:30	145	138	150	144	97	66	68	77	321
12:30 - 12:45	112	100	132	115	108	88	68	88	336
12:45 - 1:00	126	130	141	132	73	92	80	82	339
1:00 - 1:15	139	138	136	138	75	78	52	68	315
1:15 - 1:30	126	150	122	133	74	79	66	73	311
1:30 - 1:45	151	129	107	129	71	67	55	64	287
1:45 - 2:00	137	115	111	121	67	56	57	60	266
2:00 - 2:15	130	142	121	131	71	69	80	73	271
2:15 - 2:30	123	102	102	109	59	45	67	57	255
2:30 - 2:45	112	120	105	112	61	67	67	65	255
2:45 - 3:00	114	126	103	114	76	55	54	62	257
Peak Hour Total	508	507	547	521	388	332	298	339	
PHF	0.88	0.91	0.91	0.90	0.88	0.90	0.91	0.90	
PM									
4:00 - 4:15	124	141	108	124	76	72	66	71	
4:15 - 4:30	112	137	113	121	53	74	58	62	
4:30 - 4:45	113	133	111	119	58	67	67	64	
4:45 - 5:00	113	134	132	126	80	79	71	77	274
5:00 - 5:15	97	99	122	106	67	83	62	71	273
5:15 - 5:30	93	106	117	105	63	72	65	67	278
5:30 - 5:45	107	108	109	108	64	64	55	61	275
5:45 - 6:00	91	113	120	108	62	61	66	63	261
6:00 - 6:15	87	110	124	107	70	60	66	65	256
6:15 - 6:30	94	93	101	96	69	57	56	61	250
6:30 - 6:45	91	709	77	292	69	70	60	66	255
6:45 - 7:00	92	79	86	86	74	74	56	68	260
Peak Hour Total	388	426	468	427	256	280	248	261	
PHF	0.91	0.94	0.96	0.94	0.96	0.84	0.94	0.91	

*Note - ATR Data lost from 6/7 -6/10, Weekday data based on 6/14 - 6/16

Charleston ATR June 2011	Tyrellan Ave SB North of Veterans Rd West			Mohr Rd WB (In) East of Tyrellan Ave			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	175	145	160	106	109	108	
12:15 - 12:30	166	159	163	121	108	115	
12:30 - 12:45	174	184	179	97	121	109	
12:45 - 1:00	179	159	169	119	128	124	455
1:00 - 1:15	157	153	155	98	110	104	451
1:15 - 1:30	179	187	183	105	104	105	441
1:30 - 1:45	177	171	174	116	116	116	448
1:45 - 2:00	172	155	164	125	94	110	434
2:00 - 2:15	171	162	167	131	97	114	444
2:15 - 2:30	187	195	191	99	96	98	437
2:30 - 2:45	154	162	158	109	96	103	424
2:45 - 3:00	172	141	157	97	98	98	412
3:00 - 3:15	183	145	164	109	74	92	389
3:15 - 3:30	183	142	163	87	74	81	372
3:30 - 3:45	191	131	161	111	78	95	364
3:45 - 4:00	184	149	167	121	83	102	369
4:00 - 4:15	179	155	167	113	80	97	374
4:15 - 4:30	163	126	145	96	77	87	380
4:30 - 4:45	178	134	156	87	71	79	364
4:45 - 5:00	164	151	158	88	66	77	339
5:00 - 5:15	123	122	123	89	84	87	329
5:15 - 5:30	160	124	142	75	67	71	314
5:30 - 5:45	159	122	141	76	73	75	309
5:45 - 6:00	134	112	123	88	78	83	315
Peak Hour Total	692	670	681	438	458	448	
PHF	0.97	0.90	0.93	0.92	0.89	0.91	

Charleston ATR June 2011	Mohr Rd EB (Out) East of Tyrellan Ave				Veterans Rd NB South of Englewood Ave				
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG	
AM									
6:30 - 6:45	5	2	0	2	9	5	7	7	
6:45 - 7:00	3	6	4	4	5	10	9	8	
7:00 - 7:15	5	4	7	5	4	13	20	12	
7:15 - 7:30	9	16	13	13	3	34	16	18	45
7:30 - 7:45	10	11	14	12	12	20	27	20	58
7:45 - 8:00	6	15	11	11	9	42	25	25	75
8:00 - 8:15	11	11	11	11	8	44	23	25	88
8:15 - 8:30	15	17	17	16	23	53	33	36	106
8:30 - 8:45	14	29	16	20	26	55	35	39	125
8:45 - 9:00	16	19	15	17	32	38	27	32	132
9:00 - 9:15	23	31	17	24	35	51	36	41	148
9:15 - 9:30	26	22	22	23	47	42	48	46	157
Peak Hour Total	56	76	59	64	89	190	118	132	
PHF	0.88	0.66	0.87	0.80	0.70	0.86	0.84	0.80	
MD									
11:00 - 11:15	71	37	44	51	108	64	77	83	
11:15 - 11:30	33	44	31	36	72	66	58	65	
11:30 - 11:45	41	48	37	42	85	73	68	75	
11:45 - 12:00	37	39	29	35	78	75	62	72	295
12:00 - 12:15	44	28	40	37	72	67	79	73	285
12:15 - 12:30	38	32	39	36	85	55	79	73	293
12:30 - 12:45	44	42	40	42	78	69	74	74	291
12:45 - 1:00	49	48	34	44	87	85	68	80	299
1:00 - 1:15	60	53	55	56	94	102	78	91	318
1:15 - 1:30	50	29	47	42	101	64	88	84	329
1:30 - 1:45	50	59	33	47	89	99	57	82	337
1:45 - 2:00	47	50	43	47	81	75	80	79	336
2:00 - 2:15	48	54	32	45	74	82	64	73	318
2:15 - 2:30	39	43	44	42	76	78	69	74	308
2:30 - 2:45	48	36	49	44	75	83	90	83	309
2:45 - 3:00	45	40	33	39	72	92	56	73	304
Peak Hour Total	175	150	153	159	322	276	300	299	
PHF	0.89	0.78	0.96	0.88	0.93	0.81	0.95	0.90	
PM									
4:00 - 4:15	34	40	43	39	84	86	87	86	
4:15 - 4:30	52	36	46	45	107	82	81	90	
4:30 - 4:45	39	55	39	44	77	96	84	86	
4:45 - 5:00	33	39	40	37	78	95	81	85	346
5:00 - 5:15	40	52	44	45	96	106	88	97	357
5:15 - 5:30	39	46	29	38	84	99	76	86	353
5:30 - 5:45	36	51	32	40	78	107	74	86	354
5:45 - 6:00	35	44	27	35	78	79	65	74	343
6:00 - 6:15	41	41	44	42	92	99	87	93	339
6:15 - 6:30	40	33	29	34	82	80	69	77	330
6:30 - 6:45	32	39	32	34	64	72	80	72	316
6:45 - 7:00	47	37	27	37	99	83	101	94	336
Peak Hour Total	150	193	132	158	336	391	303	343	
PHF	0.94	0.93	0.75	0.87	0.88	0.91	0.86	0.88	

Charleston ATR June 2011	Mohr Rd EB (Out) East of Tyrellan Ave			Veterans Rd NB South of Englewood Ave			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	74	61	68	110	84	97	
12:15 - 12:30	70	52	61	106	105	106	
12:30 - 12:45	59	57	58	98	101	100	
12:45 - 1:00	75	63	69	114	107	111	413
1:00 - 1:15	58	63	61	110	101	106	421
1:15 - 1:30	74	86	80	110	136	123	439
1:30 - 1:45	68	58	63	93	103	98	437
1:45 - 2:00	61	65	63	99	114	107	433
2:00 - 2:15	76	67	72	128	132	130	458
2:15 - 2:30	83	69	76	118	112	115	450
2:30 - 2:45	62	50	56	93	100	97	448
2:45 - 3:00	64	55	60	102	92	97	439
3:00 - 3:15	65	62	64	97	103	100	409
3:15 - 3:30	63	58	61	109	107	108	402
3:30 - 3:45	66	55	61	91	82	87	392
3:45 - 4:00	66	49	58	111	93	102	397
4:00 - 4:15	76	62	69	117	107	112	409
4:15 - 4:30	63	44	54	110	74	92	393
4:30 - 4:45	78	39	59	119	86	103	409
4:45 - 5:00	66	43	55	102	75	89	395
5:00 - 5:15	54	40	47	89	79	84	367
5:15 - 5:30	62	54	58	83	96	90	365
5:30 - 5:45	51	56	54	80	97	89	351
5:45 - 6:00	57	40	49	84	69	77	339
Peak Hour Total	275	270	273	427	447	437	
PHF	0.92	0.78	0.85	0.94	0.82	0.88	

Charleston ATR June 2011	Veterans Rd SB North of Englewood Ave				Sharrots Rd EB West of NB West Shore Parkway Service Rd				
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG	
AM									
6:30 - 6:45	16	15	8	13	13	12	9	11	
6:45 - 7:00	15	8	17	13	12	10	12	11	
7:00 - 7:15	26	26	22	25	21	20	16	19	
7:15 - 7:30	27	38	21	29	12	11	6	10	51
7:30 - 7:45	41	37	17	32	18	8	24	17	57
7:45 - 8:00	45	50	22	39	22	16	11	16	62
8:00 - 8:15	51	39	28	39	15	15	20	17	59
8:15 - 8:30	41	48	31	40	13	14	9	12	62
8:30 - 8:45	38	32	37	36	20	12	15	16	61
8:45 - 9:00	36	33	28	32	19	18	15	17	62
9:00 - 9:15	30	37	29	32	15	11	12	13	58
9:15 - 9:30	37	24	37	33	2	17	8	9	55
Peak Hour Total	166	152	124	147	67	59	59	62	
PHF	0.81	0.79	0.84	0.81	0.84	0.82	0.74	0.80	
MD									
11:00 - 11:15	44	34	41	40	14	27	12	18	
11:15 - 11:30	42	40	38	40	21	13	12	15	
11:30 - 11:45	51	39	37	42	20	17	14	17	
11:45 - 12:00	46	49	48	48	27	13	15	18	68
12:00 - 12:15	63	33	40	45	28	18	18	21	72
12:15 - 12:30	46	33	42	40	23	20	15	19	76
12:30 - 12:45	37	44	34	38	23	18	15	19	78
12:45 - 1:00	34	53	38	42	22	14	13	16	76
1:00 - 1:15	45	45	40	43	26	11	28	22	76
1:15 - 1:30	57	39	42	46	27	20	20	22	79
1:30 - 1:45	38	44	34	39	22	17	17	19	79
1:45 - 2:00	40	41	38	40	13	13	19	15	78
2:00 - 2:15	51	42	39	44	21	27	15	21	77
2:15 - 2:30	42	33	39	38	28	17	16	20	75
2:30 - 2:45	35	45	22	34	17	14	18	16	73
2:45 - 3:00	42	43	43	43	32	18	22	24	82
Peak Hour Total	180	163	154	166	96	70	61	76	
PHF	0.71	0.77	0.92	0.80	0.86	0.88	0.85	0.86	
PM									
4:00 - 4:15	48	65	54	56	42	60	39	47	
4:15 - 4:30	35	50	49	45	29	31	22	27	
4:30 - 4:45	52	49	45	49	42	27	33	34	
4:45 - 5:00	55	54	46	52	27	23	26	25	134
5:00 - 5:15	43	54	46	48	37	38	33	36	123
5:15 - 5:30	46	51	48	48	24	23	25	24	119
5:30 - 5:45	50	53	54	52	26	36	35	32	118
5:45 - 6:00	46	39	45	43	22	27	25	25	117
6:00 - 6:15	46	62	62	57	31	38	37	35	116
6:15 - 6:30	47	45	58	50	30	26	30	29	121
6:30 - 6:45	46	45	42	44	14	21	17	17	106
6:45 - 7:00	47	41	33	40	23	17	25	22	103
Peak Hour Total	185	197	193	192	109	124	118	117	
PHF	0.93	0.91	0.89	0.91	0.74	0.82	0.84	0.80	

Charleston ATR June 2011	Veterans Rd SB North of Englewood Ave				Sharrots Rd EB West of NB West Shore Parkway Service Rd			
	Sat 1	Sat 2	AVG		Sat 1	Sat 2	AVG	
SMD								
12:00 - 12:15	60	68	64		17	14	16	
12:15 - 12:30	67	52	60		14	24	19	
12:30 - 12:45	69	64	67		26	30	28	
12:45 - 1:00	50	69	60	250	19	24	22	84
1:00 - 1:15	61	69	65	251	16	19	18	86
1:15 - 1:30	65	56	61	252	27	8	18	85
1:30 - 1:45	53	56	55	240	31	21	26	83
1:45 - 2:00	61	62	62	242	17	20	19	80
2:00 - 2:15	55	50	53	229	30	24	27	89
2:15 - 2:30	47	71	59	228	21	13	17	89
2:30 - 2:45	52	33	43	216	45	44	45	107
2:45 - 3:00	59	58	59	213	23	15	19	108
3:00 - 3:15	48	39	44	204	12	20	16	97
3:15 - 3:30	61	54	58	202	8	14	11	91
3:30 - 3:45	52	45	49	208	17	8	13	59
3:45 - 4:00	67	38	53	202	21	17	19	59
4:00 - 4:15	50	49	50	208	21	15	18	61
4:15 - 4:30	52	47	50	200	19	10	15	64
4:30 - 4:45	51	50	51	202	24	15	20	71
4:45 - 5:00	42	37	40	189	18	19	19	71
5:00 - 5:15	36	57	47	186	18	19	19	71
5:15 - 5:30	37	41	39	176	21	13	17	74
5:30 - 5:45	35	44	40	165	13	9	11	65
5:45 - 6:00	41	37	39	164	15	12	14	60
Peak Hour Total	229	250	240		93	72	83	
PHF	0.88	0.91	0.89		0.75	0.75	0.75	

Charleston ATR June 2011	Sharrots Rd WB East of NB West Shore Parkway Service Rd				Bloomingdale Rd NB South of Sharrots Rd				
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG	
AM									
6:30 - 6:45	13	11	4	9	60	60	37	52	
6:45 - 7:00	11	11	15	12	65	69	64	66	
7:00 - 7:15	21	18	14	18	72	62	48	61	
7:15 - 7:30	22	19	15	19	71	67	66	68	247
7:30 - 7:45	35	38	18	30	83	83	77	81	276
7:45 - 8:00	32	26	17	25	102	106	68	92	302
8:00 - 8:15	30	30	17	26	107	99	70	92	333
8:15 - 8:30	28	17	18	21	92	107	87	95	360
8:30 - 8:45	14	21	24	20	81	95	81	86	365
8:45 - 9:00	24	34	21	26	76	89	82	82	355
9:00 - 9:15	24	23	21	23	92	80	85	86	349
9:15 - 9:30	17	18	19	18	64	62	72	66	320
Peak Hour Total	96	102	80	93	356	390	320	355	
PHF	0.80	0.75	0.83	0.79	0.83	0.91	0.92	0.89	
MD									
11:00 - 11:15	15	13	23	17	78	75	82	78	
11:15 - 11:30	20	23	16	20	82	75	62	73	
11:30 - 11:45	52	19	24	32	87	69	73	76	
11:45 - 12:00	31	35	31	32	70	88	94	84	312
12:00 - 12:15	36	19	26	27	94	77	89	87	320
12:15 - 12:30	28	21	14	21	71	72	71	71	318
12:30 - 12:45	25	26	20	24	104	49	77	77	319
12:45 - 1:00	17	32	23	24	89	83	84	85	320
1:00 - 1:15	17	22	23	21	78	82	80	80	313
1:15 - 1:30	22	18	23	21	70	83	76	76	318
1:30 - 1:45	18	20	9	16	86	74	83	81	323
1:45 - 2:00	22	19	27	23	83	103	71	86	323
2:00 - 2:15	27	19	20	22	72	77	66	72	315
2:15 - 2:30	25	19	23	22	116	90	94	100	338
2:30 - 2:45	21	26	29	25	104	113	67	95	352
2:45 - 3:00	16	20	22	19	86	104	79	90	356
Peak Hour Total	106	98	83	96	358	281	321	320	
PHF	0.74	0.77	0.80	0.77	0.86	0.85	0.90	0.87	
PM									
4:00 - 4:15	40	27	19	29	112	91	83	95	
4:15 - 4:30	28	39	33	33	110	93	94	99	
4:30 - 4:45	29	26	20	25	115	91	101	102	
4:45 - 5:00	28	34	20	27	99	125	79	101	398
5:00 - 5:15	32	35	27	31	113	99	114	109	411
5:15 - 5:30	26	42	26	31	94	115	97	102	414
5:30 - 5:45	24	30	22	25	105	92	92	96	408
5:45 - 6:00	0	21	27	16	96	82	127	102	409
6:00 - 6:15	0	36	28	21	95	107	91	98	398
6:15 - 6:30	0	27	21	16	88	110	98	99	394
6:30 - 6:45	0	20	23	14	83	93	94	90	388
6:45 - 7:00	0	27	41	23	103	80	86	90	376
Peak Hour Total	82	128	102	104	408	388	430	409	
PHF	0.64	0.76	0.94	0.78	0.90	0.84	0.85	0.86	

Charleston ATR June 2011	Sharrots Rd WB East of NB West Shore Parkway Service Rd				Bloomingdale Rd NB South of Sharrots Rd			
	Sat 1	Sat 2	AVG		Sat 1	Sat 2	AVG	
SMD								
12:00 - 12:15	35	34	35		115	106	111	
12:15 - 12:30	38	19	29		105	97	101	
12:30 - 12:45	24	34	29		94	85	90	
12:45 - 1:00	25	26	26	118	113	92	103	404
1:00 - 1:15	34	33	34	117	103	89	96	389
1:15 - 1:30	29	23	26	114	104	108	106	394
1:30 - 1:45	30	35	33	118	106	103	105	409
1:45 - 2:00	36	31	34	126	112	75	94	400
2:00 - 2:15	28	26	27	119	93	97	95	399
2:15 - 2:30	22	43	33	126	102	114	108	401
2:30 - 2:45	34	32	33	126	102	64	83	380
2:45 - 3:00	30	21	26	118	80	72	76	362
3:00 - 3:15	32	24	28	119	97	87	92	359
3:15 - 3:30	32	29	31	117	89	97	93	344
3:30 - 3:45	41	30	36	120	87	92	90	351
3:45 - 4:00	35	24	30	124	102	99	101	375
4:00 - 4:15	38	32	35	131	91	97	94	377
4:15 - 4:30	26	26	26	126	104	69	87	371
4:30 - 4:45	33	29	31	122	96	73	85	366
4:45 - 5:00	27	28	28	120	96	91	94	359
5:00 - 5:15	25	29	27	112	82	62	72	337
5:15 - 5:30	17	20	19	104	89	74	82	332
5:30 - 5:45	22	26	24	97	93	82	88	335
5:45 - 6:00	25	26	26	95	65	58	62	303
Peak Hour Total	118	117	118		426	392	409	
PHF	0.87	0.84	0.85		0.94	0.91	0.92	

Charleston ATR June 2011	Bloomingdale Rd SB North of Sharrots Rd				Bloomingdale Rd SB North of Amboy Ave			
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG
AM								
6:30 - 6:45	48	44	32	41	42	32	38	37
6:45 - 7:00	54	49	42	48	58	44	40	47
7:00 - 7:15	62	59	43	55	64	70	47	60
7:15 - 7:30	86	73	54	71	86	70	55	70
7:30 - 7:45	100	84	71	85	122	102	91	105
7:45 - 8:00	98	80	78	85	145	119	89	118
8:00 - 8:15	100	102	75	92	152	146	104	134
8:15 - 8:30	104	92	67	88	103	113	80	99
8:30 - 8:45	73	84	62	73	90	92	67	83
8:45 - 9:00	75	76	84	78	81	91	90	87
9:00 - 9:15	68	76	74	73	82	83	76	80
9:15 - 9:30	64	68	59	64	60	62	76	66
Peak Hour Total	352	354	288	331	426	442	341	403
PHF	0.85	0.87	0.86	0.86	0.70	0.76	0.82	0.76
MD								
11:00 - 11:15	97	69	73	80	96	87	79	87
11:15 - 11:30	86	84	79	83	104	82	82	89
11:30 - 11:45	82	69	78	76	90	83	97	90
11:45 - 12:00	85	74	83	81	104	101	110	105
12:00 - 12:15	84	73	87	81	111	76	93	93
12:15 - 12:30	83	73	72	76	106	101	87	98
12:30 - 12:45	77	69	80	75	95	73	82	83
12:45 - 1:00	82	85	71	79	77	99	94	90
1:00 - 1:15	84	68	88	80	90	90	73	84
1:15 - 1:30	83	74	84	80	81	91	73	82
1:30 - 1:45	77	105	78	87	72	126	814	337
1:45 - 2:00	82	87	84	84	79	102	90	90
2:00 - 2:15	91	97	67	85	75	125	94	98
2:15 - 2:30	97	84	80	87	88	109	87	95
2:30 - 2:45	110	114	106	110	100	101	87	96
2:45 - 3:00	107	119	92	106	87	120	74	94
Peak Hour Total	326	300	310	312	389	349	356	365
PHF	0.97	0.88	0.89	0.91	0.88	0.86	0.95	0.90
PM								
4:00 - 4:15	106	136	101	114	96	108	102	102
4:15 - 4:30	110	116	115	114	86	101	101	96
4:30 - 4:45	115	86	80	94	112	107	80	100
4:45 - 5:00	118	133	96	116	106	120	110	112
5:00 - 5:15	111	118	99	109	114	135	89	113
5:15 - 5:30	117	117	87	107	101	100	85	95
5:30 - 5:45	99	113	130	114	83	144	95	107
5:45 - 6:00	113	111	131	118	103	134	115	117
6:00 - 6:15	119	126	123	123	85	99	102	95
6:15 - 6:30	108	116	97	107	128	125	101	118
6:30 - 6:45	103	107	101	104	99	101	101	100
6:45 - 7:00	100	102	100	101	91	137	110	113
Peak Hour Total	440	459	447	449	401	513	384	433
PHF	0.94	0.97	0.85	0.92	0.88	0.89	0.83	0.87

Charleston ATR June 2011	Bloomingdale Rd SB North of Sharrots Rd			Bloomingdale Rd SB North of Amboy Ave			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	112		112	128	84	106	
12:15 - 12:30	124		124	141	77	109	
12:30 - 12:45	123		123	121	92	107	
12:45 - 1:00	114		114	105	97	101	423
1:00 - 1:15	117		117	103	113	108	425
1:15 - 1:30	89		89	108	89	99	414
1:30 - 1:45	106		106	118	95	107	414
1:45 - 2:00	116		116	115	108	112	425
2:00 - 2:15	114		114	131	96	114	430
2:15 - 2:30	111		111	113	85	99	431
2:30 - 2:45	121		121	126	94	110	434
2:45 - 3:00	92		92	101	114	108	430
3:00 - 3:15	123		123	99	83	91	408
3:15 - 3:30	90	67	79	105	78	92	400
3:30 - 3:45	110	88	99	85	84	85	375
3:45 - 4:00	114	91	103	98	92	95	362
4:00 - 4:15	122	95	109	106	85	96	367
4:15 - 4:30	82	75	79	109	91	100	375
4:30 - 4:45	104	99	102	112	80	96	387
4:45 - 5:00	98	83	91	112	80	96	388
5:00 - 5:15	98	93	96	117	73	95	387
5:15 - 5:30	84	93	89	84	73	79	366
5:30 - 5:45	81	93	87	73	78	76	345
5:45 - 6:00	94	91	92.5	102	71	87	336
Peak Hour Total	426	0	426	434	394	414	
PHF	0.91	-	0.91	0.92	0.87	0.90	

Charleston ATR June 2011	Bloomingdale Rd NB South of Amboy Ave				Amboy Rd WB East of Bloomingdale Rd			
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG
AM								
6:30 - 6:45	44	24	27	32	13	19	13	15
6:45 - 7:00	38	47	30	38	18	22	15	18
7:00 - 7:15	56	52	41	50	39	36	27	34
7:15 - 7:30	61	59	44	55	36	38	33	36
7:30 - 7:45	100	86	61	82	48	35	46	43
7:45 - 8:00	72	94	79	82	55	68	51	58
8:00 - 8:15	85	78	54	72	88	94	51	78
8:15 - 8:30	98	78	60	79	80	80	54	71
8:30 - 8:45	68	84	59	70	65	59	55	60
8:45 - 9:00	69	65	58	64	72	63	55	63
9:00 - 9:15	71	49	64	61	61	53	64	59
9:15 - 9:30	63	76	58	66	49	56	55	53
Peak Hour Total	320	305	231	285	305	296	215	272
PHF	0.82	0.91	0.96	0.90	0.87	0.79	0.98	0.88
MD								
11:00 - 11:15	77	93	85	85	85	85	90	87
11:15 - 11:30	89	61	75	75	82	71	83	79
11:30 - 11:45	107	64	105	92	121	94	95	103
11:45 - 12:00	104	87	86	92	102	78	95	92
12:00 - 12:15	91	92	89	91	117	82	86	95
12:15 - 12:30	118	69	90	92	110	88	109	102
12:30 - 12:45	116	111	98	108	95	57	95	82
12:45 - 1:00	106	83	99	96	87	102	78	89
1:00 - 1:15	101	103	77	94	78	87	88	84
1:15 - 1:30	93	86	86	88	62	110	87	86
1:30 - 1:45	102	108	84	98	62	85	84	77
1:45 - 2:00	104	99	76	93	86	108	70	88
2:00 - 2:15	80	77	88	82	99	108	95	101
2:15 - 2:30	102	88	97	96	104	139	109	117
2:30 - 2:45	111	137	98	115	88	130	110	109
2:45 - 3:00	96	109	81	95	125	159	100	128
Peak Hour Total	431	355	376	387	409	329	368	369
PHF	0.91	0.80	0.95	0.89	0.87	0.81	0.84	0.84
PM								
4:00 - 4:15	109	114	101	108	119	143	123	128
4:15 - 4:30	89	98	104	97	96	106	79	94
4:30 - 4:45	86	105	102	98	88	124	94	102
4:45 - 5:00	106	102	92	100	102	123	93	106
5:00 - 5:15	101	99	100	100	97	119	72	96
5:15 - 5:30	120	105	99	108	106	119	88	104
5:30 - 5:45	115	94	113	107	91	92	71	85
5:45 - 6:00	107	97	95	100	78	99	100	92
6:00 - 6:15	94	90	129	104	90	97	86	91
6:15 - 6:30	99	86	91	92	84	96	76	85
6:30 - 6:45	95	71	87	84	68	96	101	88
6:45 - 7:00	102	90	116	103	97	82	90	90
Peak Hour Total	443	395	407	415	372	429	331	377
PHF	0.92	0.94	0.90	0.92	0.88	0.90	0.83	0.87

Charleston ATR June 2011	Bloomingdale Rd NB South of Amboy Ave			Amboy Rd WB East of Bloomingdale Rd		
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG
SMD						
12:00 - 12:15	117	117	117	103	109	106
12:15 - 12:30	135	103	119	73	109	91
12:30 - 12:45	129	103	116	104	116	110
12:45 - 1:00	136	110	123	90	92	91
1:00 - 1:15	127	126	127	106	103	105
1:15 - 1:30	120	100	110	96	84	90
1:30 - 1:45	120	124	122	113	92	103
1:45 - 2:00	125	107	116	102	95	99
2:00 - 2:15	117	113	115	77	81	79
2:15 - 2:30	130	120	125	87	84	86
2:30 - 2:45	127	110	119	85	84	85
2:45 - 3:00	114	110	112	83	58	71
3:00 - 3:15	108	107	108	79	49	64
3:15 - 3:30	109	120	115	80	43	62
3:30 - 3:45	107	106	107	89	42	66
3:45 - 4:00	114	105	110	91	71	81
4:00 - 4:15	109	105	107	77	83	80
4:15 - 4:30	120	92	106	89	61	75
4:30 - 4:45	113	89	101	82	65	74
4:45 - 5:00	97	93	95	86	53	70
5:00 - 5:15	87	79	83	80	75	78
5:15 - 5:30	94	89	92	68	60	64
5:30 - 5:45	90	90	90	72	60	66
5:45 - 6:00	100	73	87	65	55	60
Peak Hour Total	503	460	482	405	371	388
PHF	0.92	0.91	0.92	0.90	0.90	0.90

Charleston ATR June 2011	Page Ave NB * South of Richmond Valley Rd				Page Ave SB North of Richmond Valley Rd					
	Tues	Wed	Thur	AVG	Tues	Wed	Thur	AVG		
AM										
6:30 - 6:45	98	111	108	106	45	56	58	53		
6:45 - 7:00	105	120	112	112	75	74	72	74		
7:00 - 7:15	127	113	116	119	86	69	79	78		
7:15 - 7:30	134	144	143	140	78	87	103	89	294	
7:30 - 7:45	142	165	149	152	523	109	97	120	109	350
7:45 - 8:00	189	200	157	182	593	142	151	114	136	412
8:00 - 8:15	225	202	146	191	665	117	106	96	106	440
8:15 - 8:30	226	210	156	197	722	92	124	83	100	450
8:30 - 8:45	182	183	159	175	745	100	94	124	106	448
8:45 - 9:00	185	167	142	165	728	105	90	87	94	406
9:00 - 9:15	154	160	169	161	698	98	125	102	108	408
9:15 - 9:30	166	143	144	151	651	106	113	96	105	413
Peak Hour Total	818	762	603	728	414	414	390	406		
PHF	0.90	0.91	0.95	0.92	0.88	0.83	0.79	0.84		
MD										
11:00 - 11:15	150	138	150	146	138	124	108	123		
11:15 - 11:30	172	140	171	161	122	88	99	103		
11:30 - 11:45	185	150	176	170	128	120	145	131		
11:45 - 12:00	207	158	202	189	666	131	135	144	137	494
12:00 - 12:15	204	176	201	194	714	120	109	126	118	489
12:15 - 12:30	194	159	160	171	724	128	140	139	136	522
12:30 - 12:45	199	191	167	186	739	136	134	162	144	535
12:45 - 1:00	176	160	178	171	722	162	143	159	155	553
1:00 - 1:15	164	181	150	165	693	128	150	152	143	578
1:15 - 1:30	181	162	158	167	689	118	138	142	133	575
1:30 - 1:45	163	169	167	166	670	133	150	110	131	562
1:45 - 2:00	158	153	179	163	662	116	137	126	126	533
2:00 - 2:15	165	165	165	165	662	138	157	130	142	532
2:15 - 2:30	180	154	176	170	665	148	144	140	144	543
2:30 - 2:45	179	193	237	203	701	112	125	126	121	533
2:45 - 3:00	183	201	140	175	713	122	144	127	131	538
Peak Hour Total	773	686	706	722	546	526	586	553		
PHF	0.95	0.90	0.88	0.91	0.84	0.92	0.90	0.89		
PM										
4:00 - 4:15	196	180	165	180	126	175	145	149		
4:15 - 4:30	159	178	151	163	166	170	138	158		
4:30 - 4:45	165	178	150	164	185	172	135	164		
4:45 - 5:00	197	181	161	180	687	184	197	150	177	648
5:00 - 5:15	177	179	170	175	682	141	153	159	151	650
5:15 - 5:30	148	169	175	164	683	142	151	156	150	642
5:30 - 5:45	190	146	153	163	682	141	128	154	141	619
5:45 - 6:00	155	147	177	160	662	165	184	190	180	621
6:00 - 6:15	176	181	194	184	670	179	168	165	171	641
6:15 - 6:30	166	164	178	169	676	135	180	166	160	652
6:30 - 6:45	139	148	145	144	657	152	171	182	168	679
6:45 - 7:00	160	153	166	160	657	148	165	188	167	666
Peak Hour Total	670	641	675	662	589	616	659	621		
PHF	0.88	0.90	0.95	0.91	0.89	0.84	0.87	0.87		

*Note - Median ATR value for AM peak (762) was used in balancing network to avoid a dramatic reduction of manual traffic counts

Charleston ATR June 2011	Page Ave NB South of Richmond Valley Rd			Page Ave SB North of Richmond Valley Rd			
	Sat 1	Sat 2	AVG	Sat 1	Sat 2	AVG	
SMD							
12:00 - 12:15	190	209	200	150	157	154	
12:15 - 12:30	220	188	204	173	167	170	
12:30 - 12:45	239	215	227	147	175	161	
12:45 - 1:00	284	219	252	177	160	169	653
1:00 - 1:15	286	222	254	134	139	137	636
1:15 - 1:30	194	221	208	157	160	159	625
1:30 - 1:45	238	186	212	159	162	161	624
1:45 - 2:00	221	249	235	152	150	151	607
2:00 - 2:15	202	209	206	148	165	157	627
2:15 - 2:30	208	190	199	153	128	141	609
2:30 - 2:45	175	180	178	164	153	159	607
2:45 - 3:00	197	197	197	138	150	144	600
3:00 - 3:15	181	162	172	137	135	136	579
3:15 - 3:30	164	190	177	150	145	148	586
3:30 - 3:45	168	180	174	149	160	155	582
3:45 - 4:00	157	160	159	167	155	161	599
4:00 - 4:15	171	162	167	145	153	149	612
4:15 - 4:30	190	162	176	152	140	146	611
4:30 - 4:45	165	153	159	148	145	147	603
4:45 - 5:00	166	171	169	147	139	143	585
5:00 - 5:15	157	163	160	148	129	139	574
5:15 - 5:30	176	176	176	160	136	148	576
5:30 - 5:45	164	138	151	123	135	129	559
5:45 - 6:00	170	125	148	146	123	135	550
Peak Hour Total	1002	848	925	627	621	624	
PHF	0.88	0.95	0.92	0.89	0.96	0.92	

Charleston ATR June 2011	Peak Hour Totals for All Intersections	
	AVG	
AM		
6:30 - 6:45	635	
6:45 - 7:00	777	
7:00 - 7:15	872	
7:15 - 7:30	2511	4795
7:30 - 7:45	2861	7021
7:45 - 8:00	3244	9488
8:00 - 8:15	3519	12134
8:15 - 8:30	3639	13262
8:30 - 8:45	3622	14023
8:45 - 9:00	3495	14274
9:00 - 9:15	3469	14225
9:15 - 9:30	3388	13974
Peak Hour Total		14274
PHF		0.98
MD		
11:00 - 11:15	1505	
11:15 - 11:30	1496	
11:30 - 11:45	1561	
11:45 - 12:00	4107	8669
12:00 - 12:15	4189	11353
12:15 - 12:30	4202	14059
12:30 - 12:45	4288	16787
12:45 - 1:00	4209	16888
1:00 - 1:15	4235	16935
1:15 - 1:30	4159	16891
1:30 - 1:45	4229	16832
1:45 - 2:00	4144	16767
2:00 - 2:15	4158	16689
2:15 - 2:30	4172	16703
2:30 - 2:45	4320	16794
2:45 - 3:00	4371	17022
Peak Hour Total		16888
PHF		0.98
PM		
4:00 - 4:15	1903	
4:15 - 4:30	1663	
4:30 - 4:45	1659	
4:45 - 5:00	4712	9936
5:00 - 5:15	4746	12779
5:15 - 5:30	4749	15865
5:30 - 5:45	4715	18922
5:45 - 6:00	4678	18888
6:00 - 6:15	4652	18795
6:15 - 6:30	4622	18668
6:30 - 6:45	4498	18451
6:45 - 7:00	4545	18318
Peak Hour Total		18795
PHF		0.99

Charleston ATR June 2011	Peak Hour Totals for All Intersections	
	AVG	
SMD		
12:00 - 12:15	1515	
12:15 - 12:30	1562	
12:30 - 12:45	1522	
12:45 - 1:00	4814	9412
1:00 - 1:15	4836	12733
1:15 - 1:30	4900	16071
1:30 - 1:45	4861	19410
1:45 - 2:00	4784	19380
2:00 - 2:15	4744	19289
2:15 - 2:30	4672	19061
2:30 - 2:45	4476	18676
2:45 - 3:00	4344	18236
3:00 - 3:15	4165	17657
3:15 - 3:30	4095	17079
3:30 - 3:45	4144	16747
3:45 - 4:00	4158	16561
4:00 - 4:15	4150	16545
4:15 - 4:30	4116	16566
4:30 - 4:45	4138	16560
4:45 - 5:00	4044	16446
5:00 - 5:15	3924	16221
5:15 - 5:30	3895	16000
5:30 - 5:45	3765	15627
5:45 - 6:00	3746.5	15330
Peak Hour Total		19410
PHF		0.99

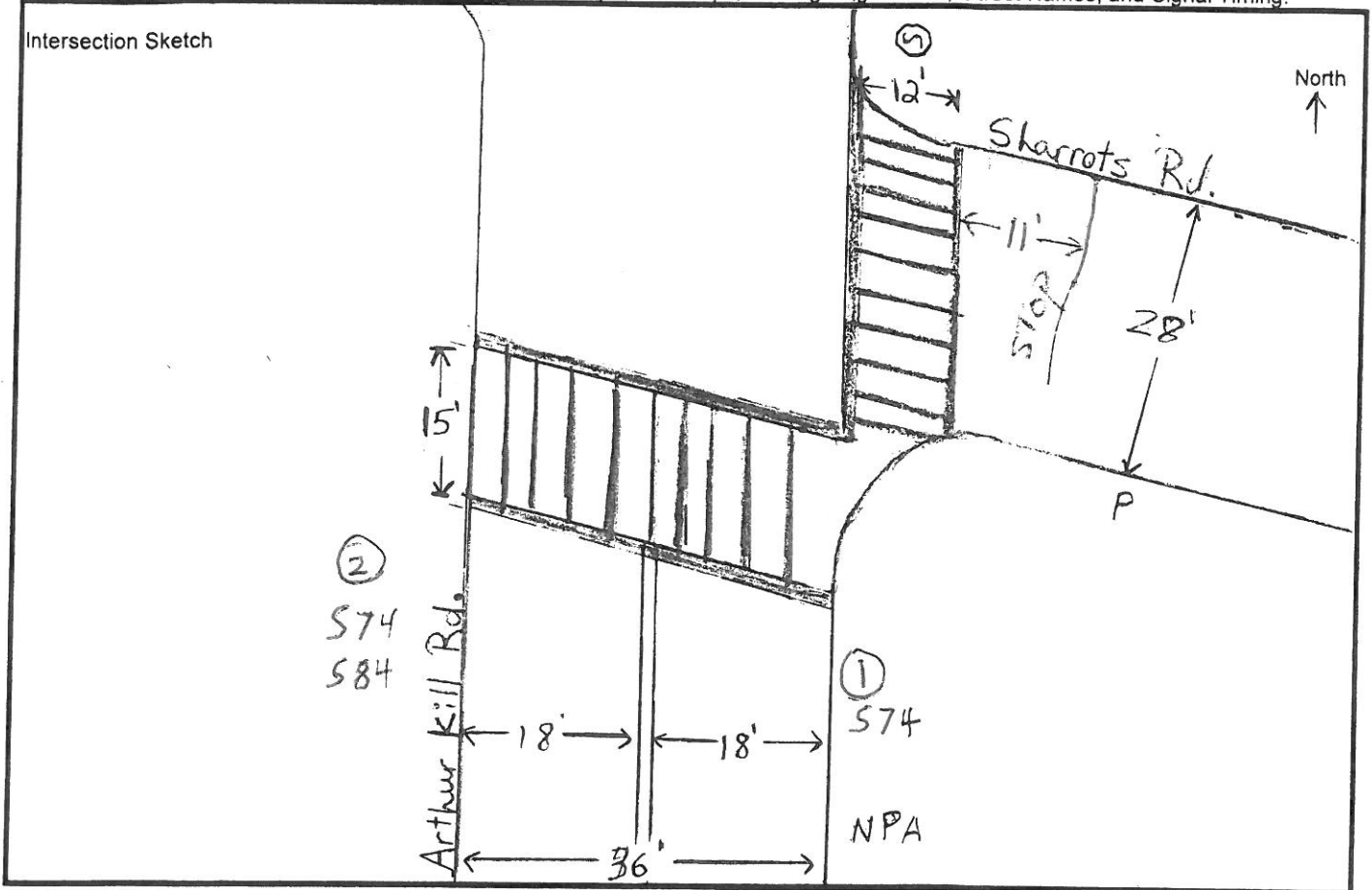
Geometry

Project: Charleston Project #: 11/21A Physical Inventory Sheet

Location: Sharrots Road and Arthur Kill Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

- (1) NB Appr. S74 stop (bus) No standing ()
- (2) SB Appr. S74, S84 bus stop ()
- (P) No regs ()
- () ()
- () ()
- () ()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

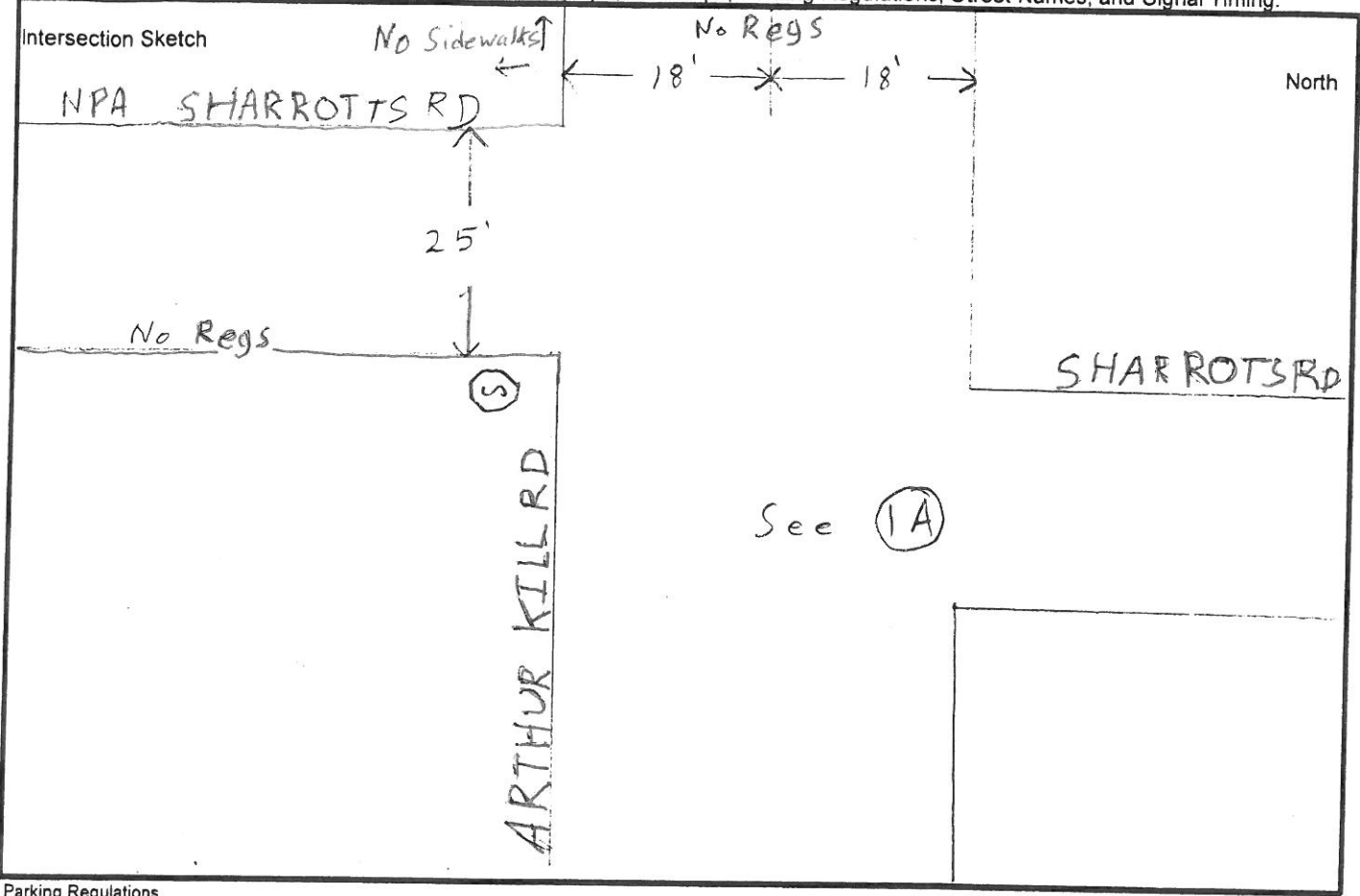
Cycle Length	
AM	MD
PM	Sat. MD
Pedestrian Button	
Y	N
Signal Type	
P	A

Project: Charles-on Project #: 1121A

Location: Sharrots Rd + Arthur Kill Rd

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	()
()	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

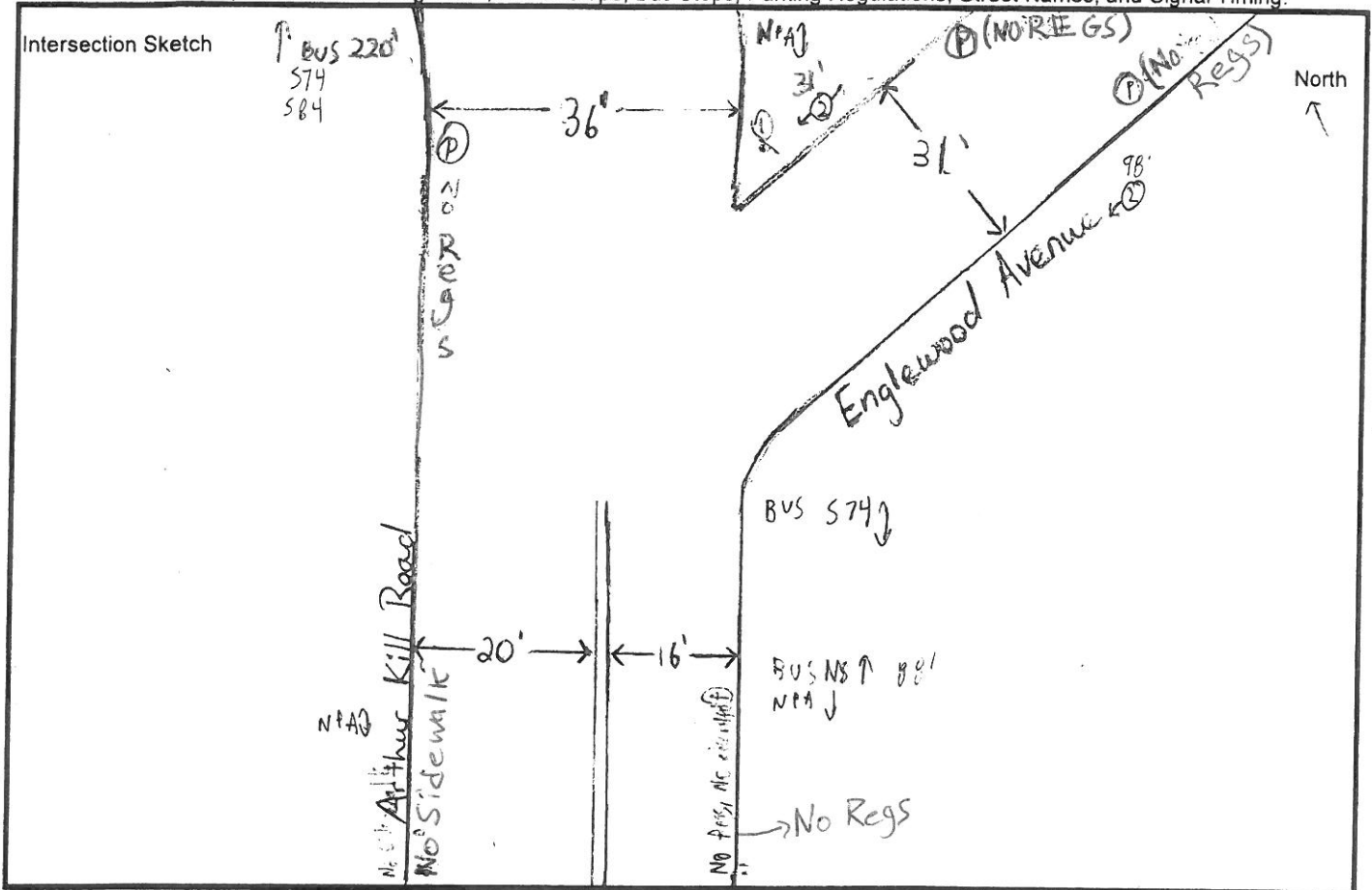
P	A
---	---

Project: Charleston Project #: 1121A

Location: Arthur Kill Road and Englewood Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) STOP	()
(2) NPA-SI M-F	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

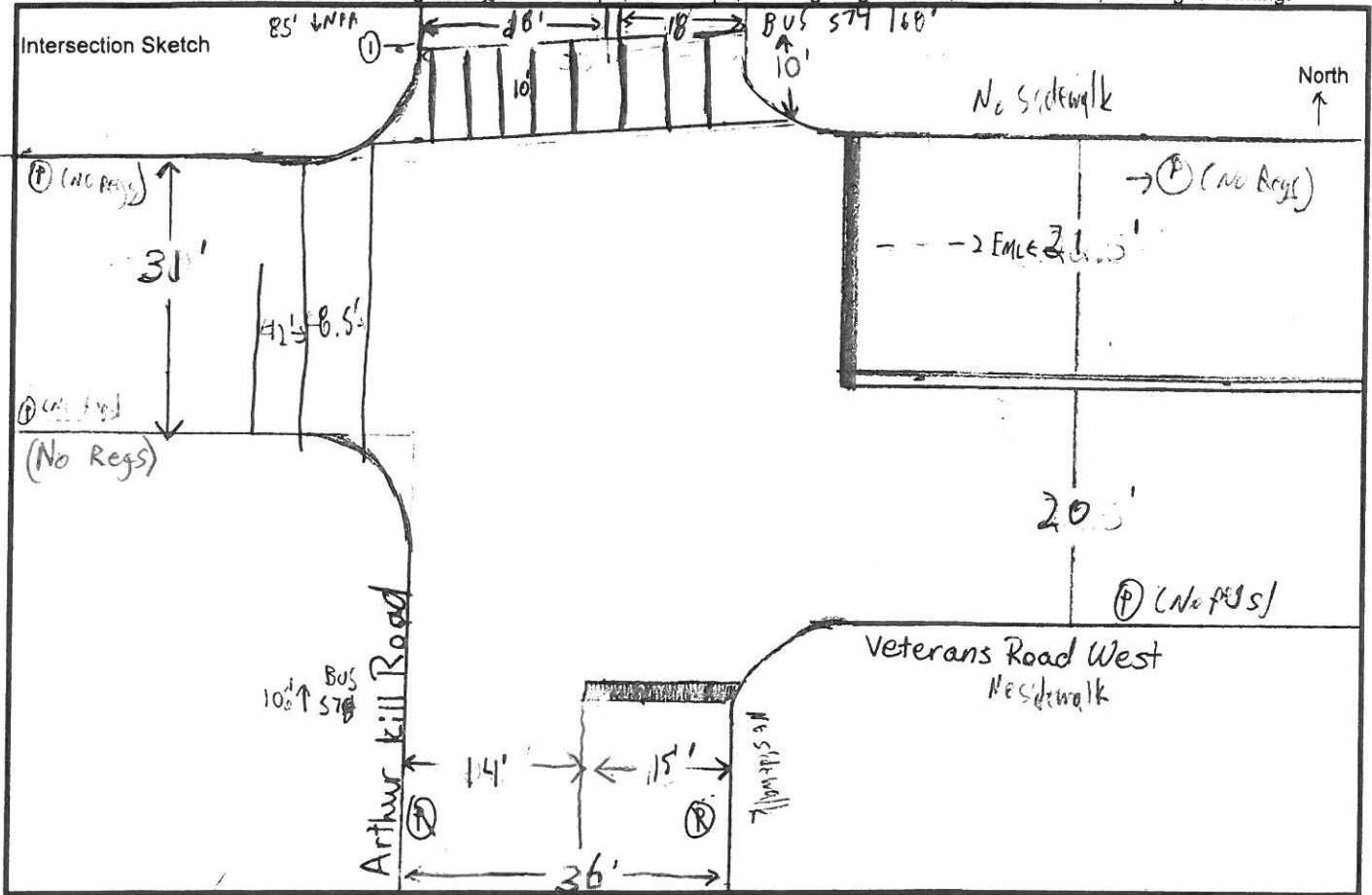
P	A
---	---

Project: Charleston Project #: 11.21.A

Location: Arthur Kill Road and Veterans Road West

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) AFTER STOP RIGHT TURN PERMITTED ON RED	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

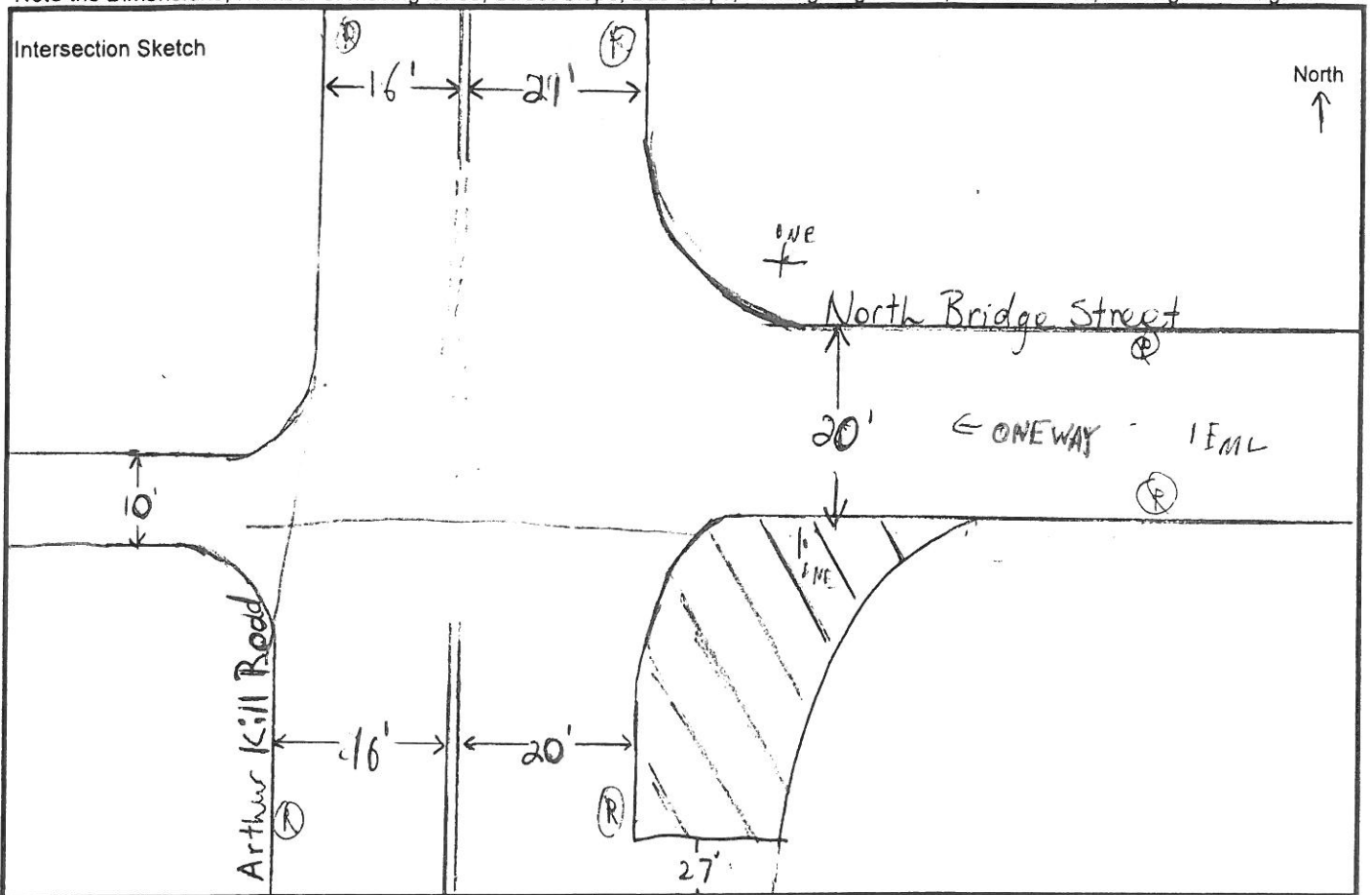
P	A
---	---

Project: Charleston Project #: 1121A

Location: Arthur Kill Road and North Bridge Street

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

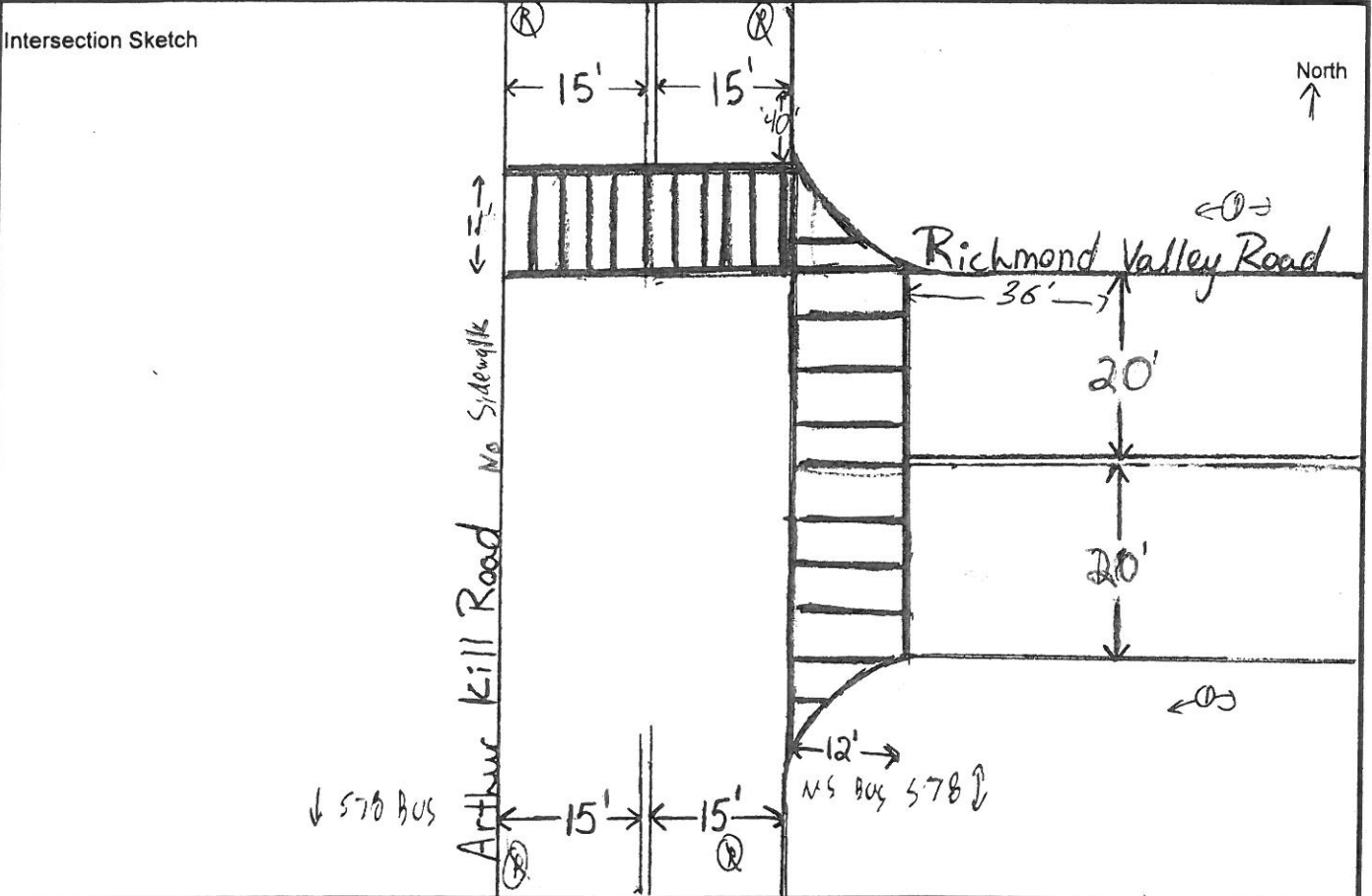
P	A
---	---

Project: Charleston Project #: 1121A

Location: Arthur Kill Road and Richmond Valley Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) NIGHT REG NS 10P-6A INCL SUN	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length	
AM	MD
PM	Sat. MD

Pedestrian Button	
Y	N

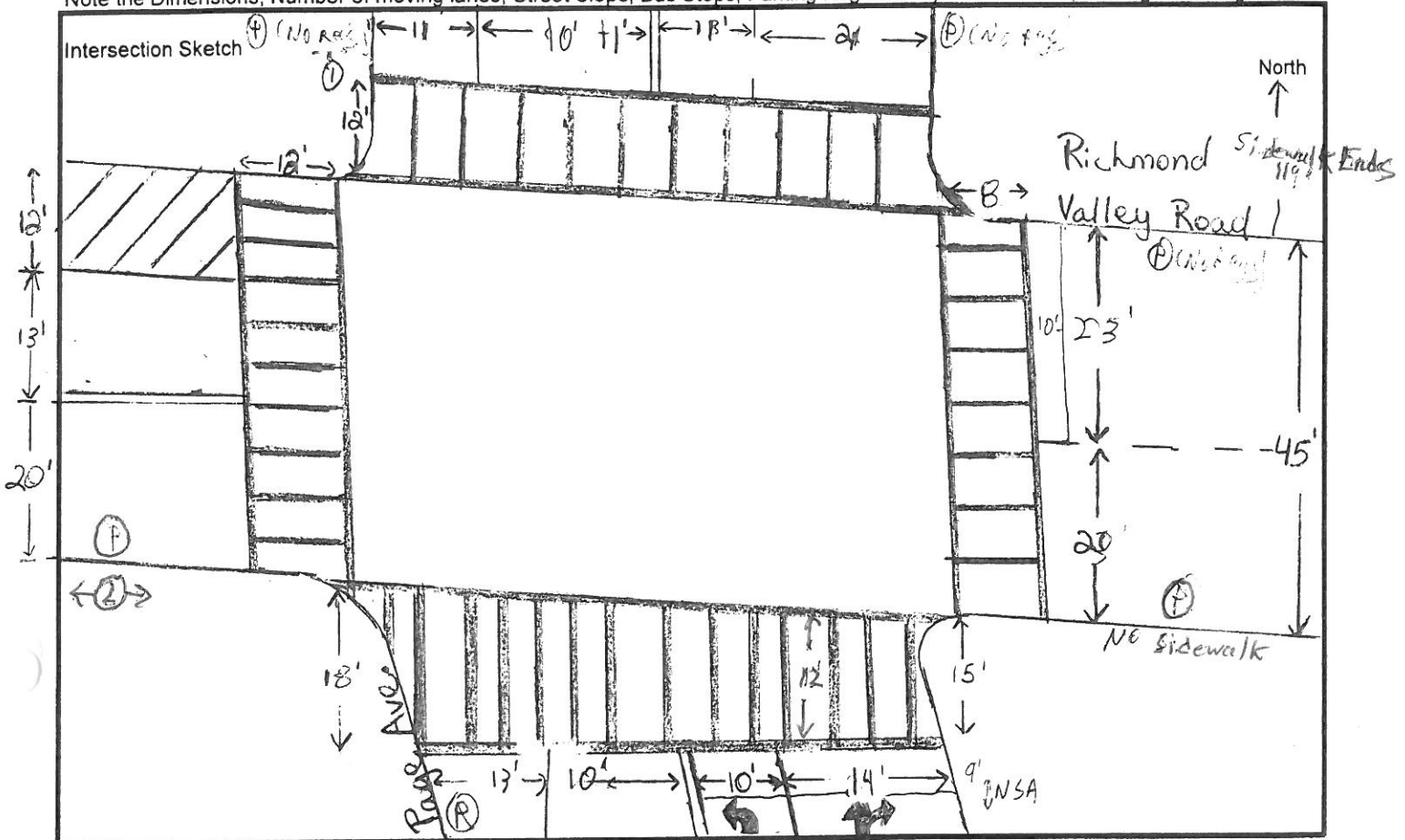
Signal Type	
P	A

Project: Charleston Project #: USA

Location: Richmond Valley Road and Page Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) AFTER STOP RT PERM ON RED	()
(2) NS 10P-6A	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length	
AM	MD
PM	Sat. MD

Pedestrian Button	
Y	N

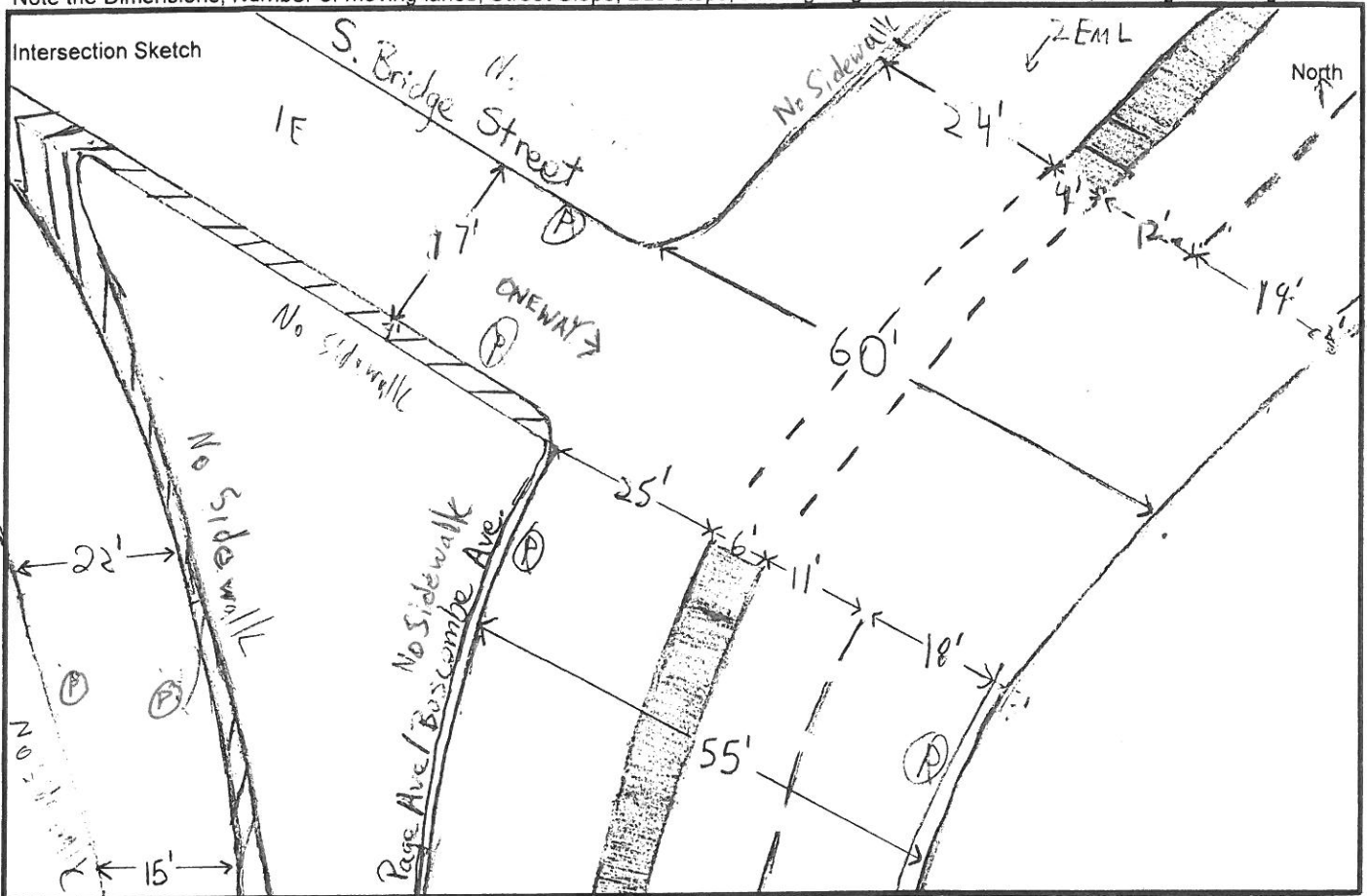
Signal Type	
P	A

Project: Charleston Project #: 1191A

Location: Page Avenue/Boscombe Avenue and S. Bridge Street

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

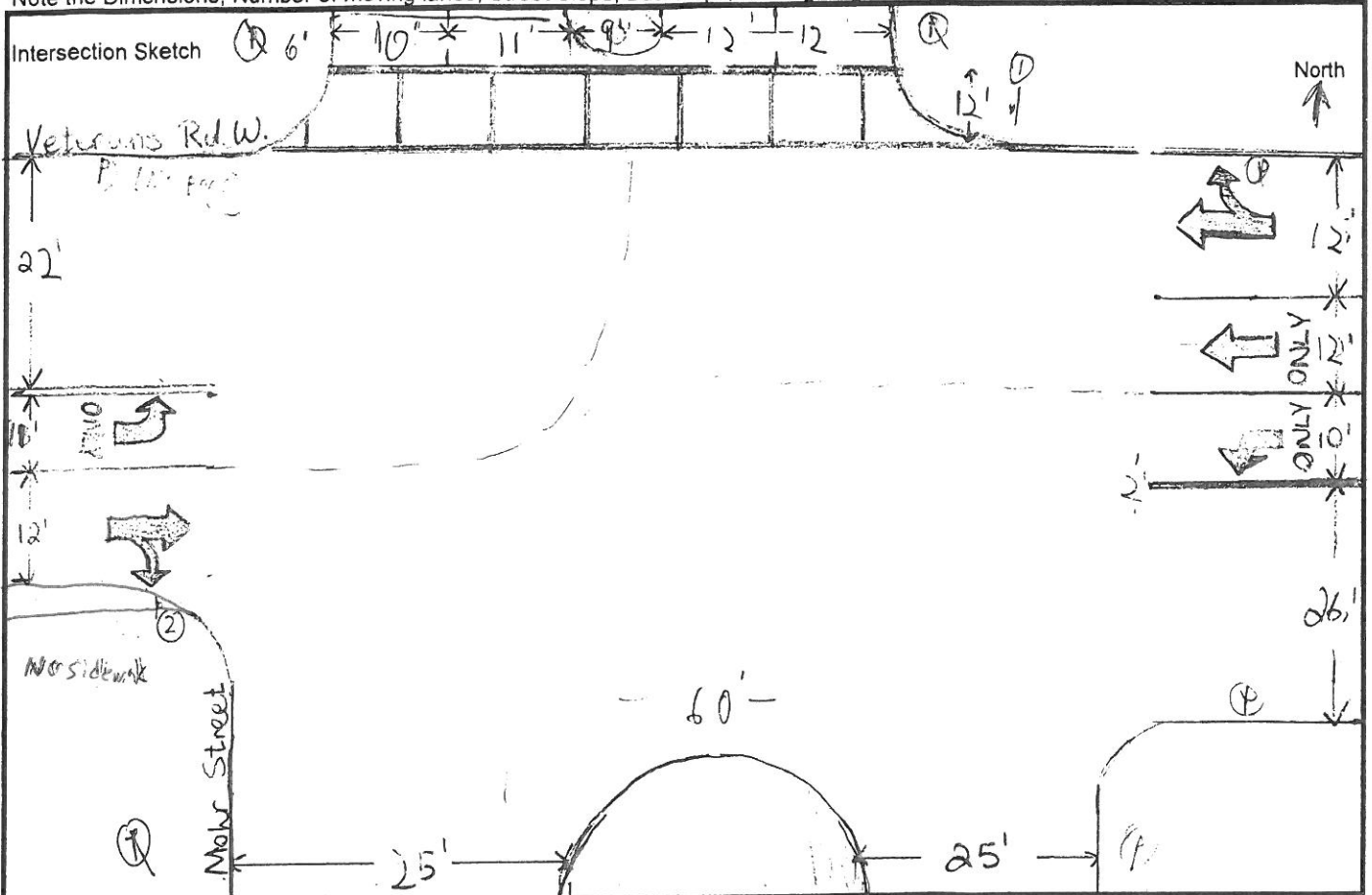
P	A
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Project: Charleston Project #: 1121A

Location: Veterans Road W. and Mohr Street

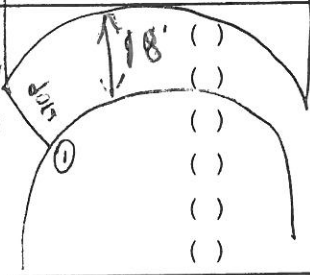
Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

- (1) STOP
- (2) AFTER STOP RT PERMITTED ON RED
- ()
- ()
- ()
- ()
- ()



Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

P	A
---	---

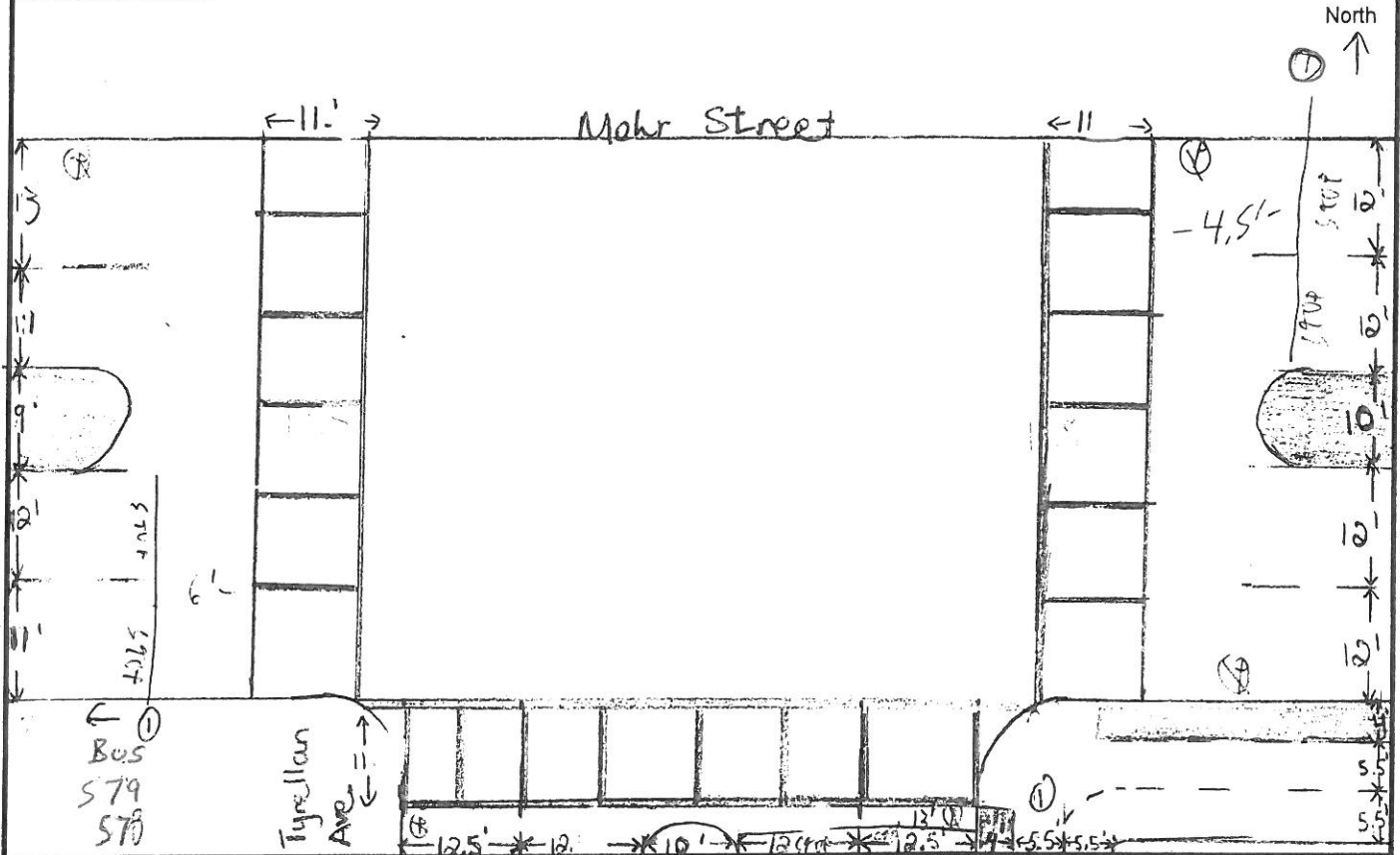
Project: Charleston Project #: 1181.A

Location: Mahr Street and Tyrellan Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.

Intersection Sketch



Parking Regulations

(1) Stop 147'	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

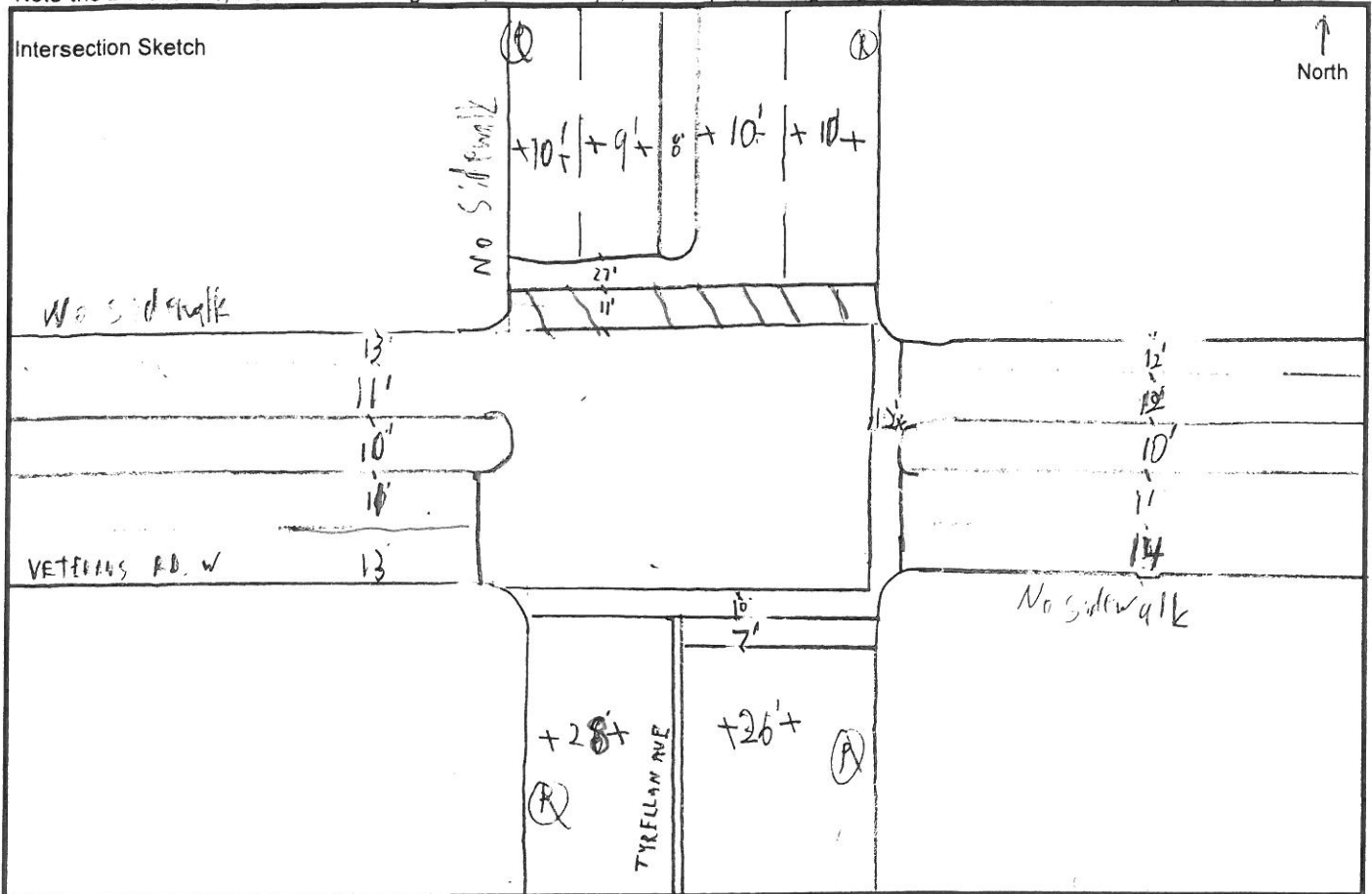
P	A
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Project: Charleston Project #: 11.31.A

Location: Veterans Rd. W @ Tyrellan Ave

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	no observed parking (No Regs)	()
()		()
()		()
()		()
()		()
()		()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

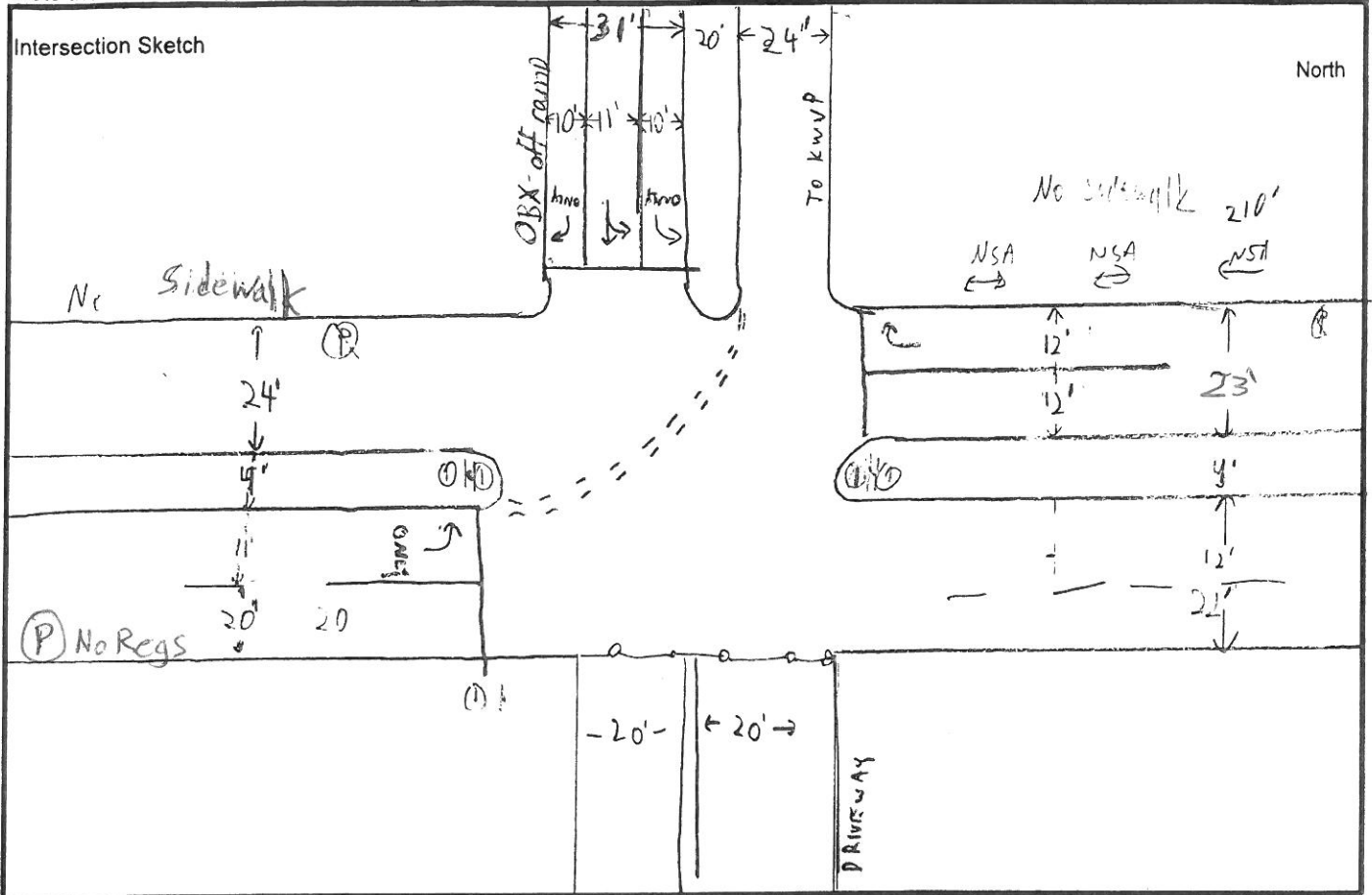
P	A
---	---

Project: Charleston Project #: 1131A

Location: Boscombe Ave @ Overbridge Crossing off-Ramp

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

() No Regs, no obstruct parking	()
() NO U TURN	()
() AFTER STOP RT PERMITTING ON RED	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

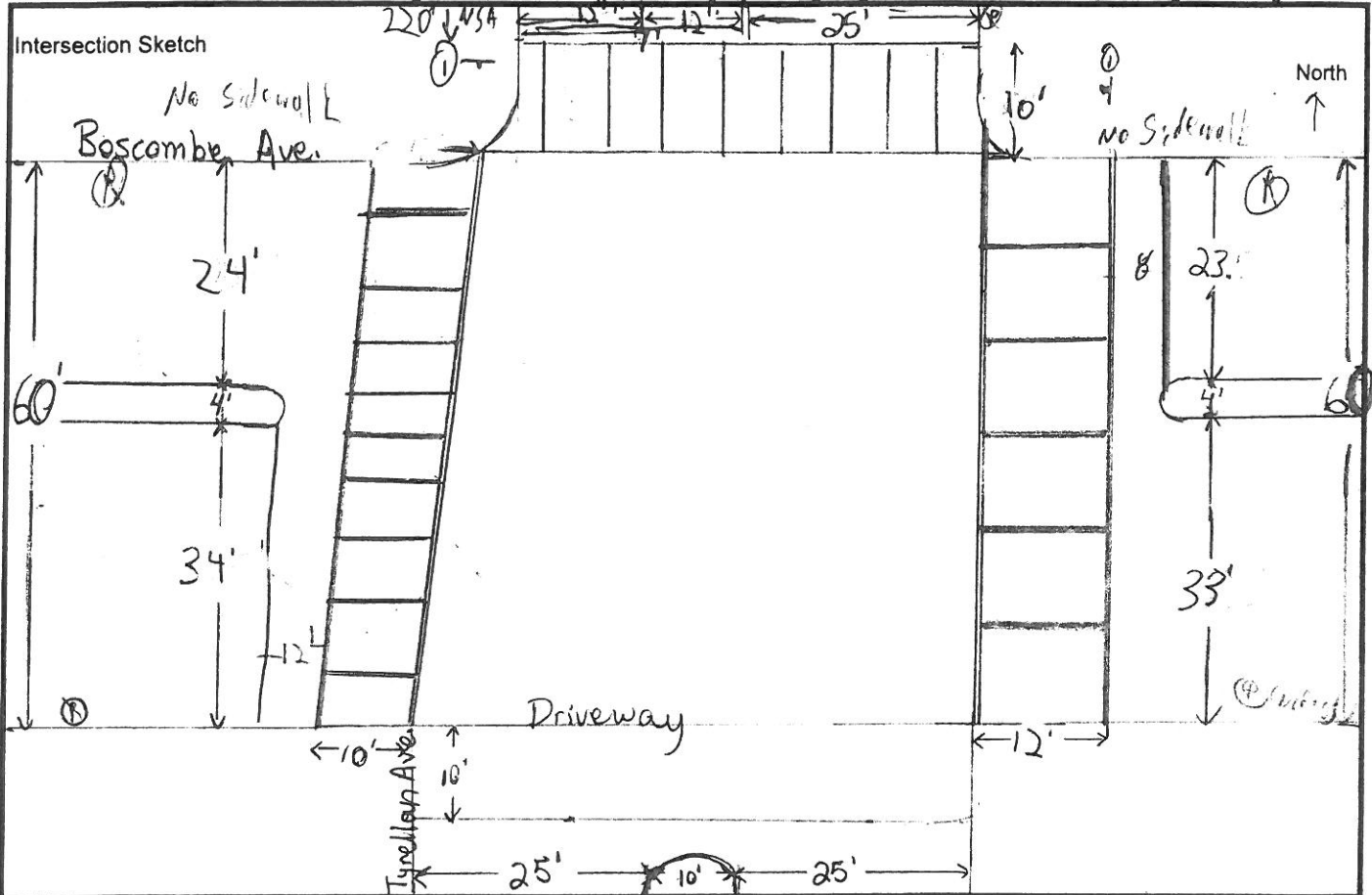
P	A
---	---

Project: Charleston Project #: 1121A

Location: Boscombe Ave. and Tyrellan Ave.

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

- (P) - no regs, observed parking ()
- (I) AFTER STOP RIGHT TURN PERMITTED ON RED ()
- () ()
- () ()
- () ()
- () ()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

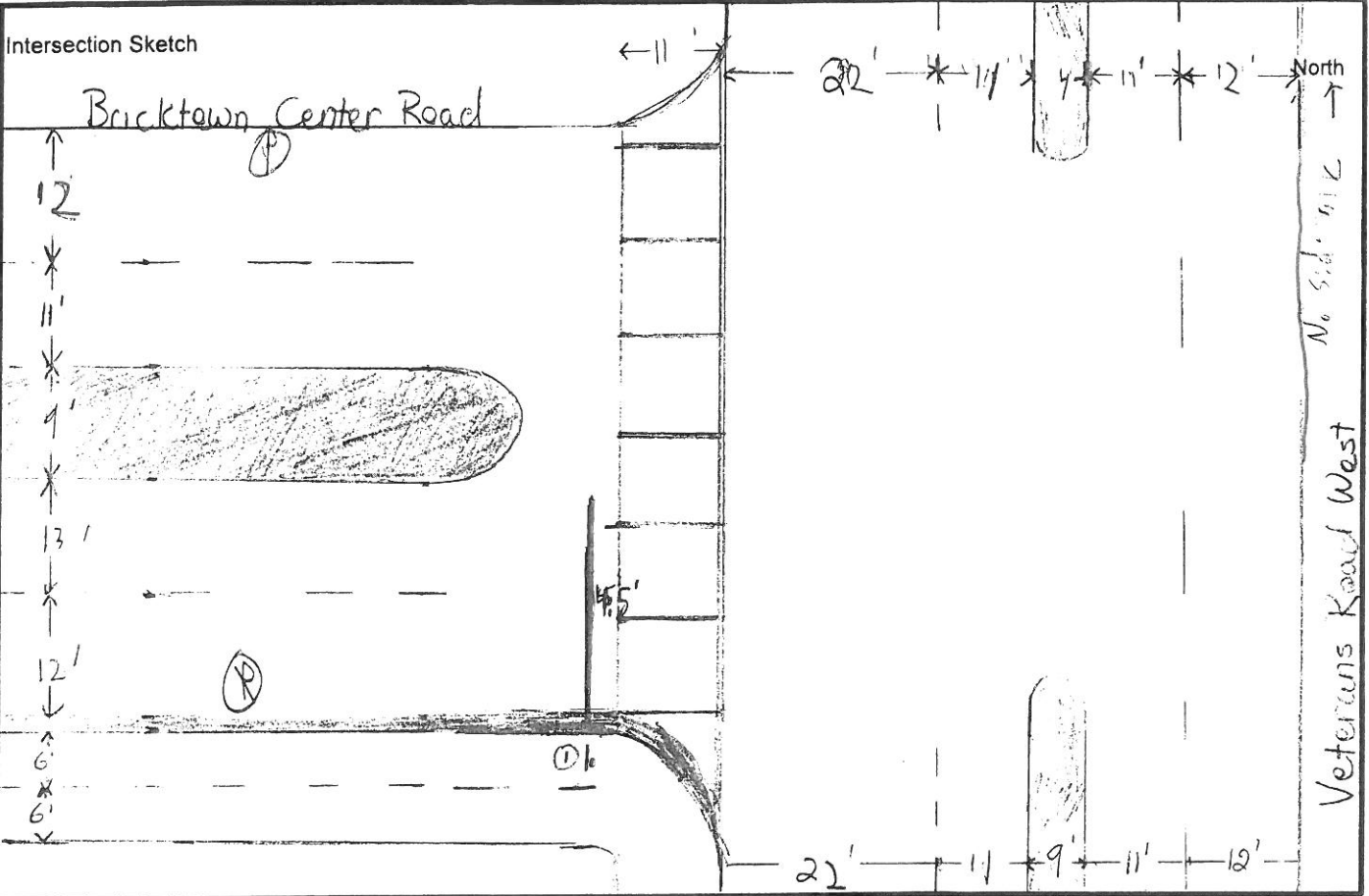
P	A
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Project: Charleston Project #: 1181A

Location: Veterans Road West and Bricktown Center Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

() AFTER STOP RIGHT TURN PERMITTED ONLY ()
 () NO REGS, NO OBSERVED PARKING ()
 () ()
 () ()
 () ()
 () ()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

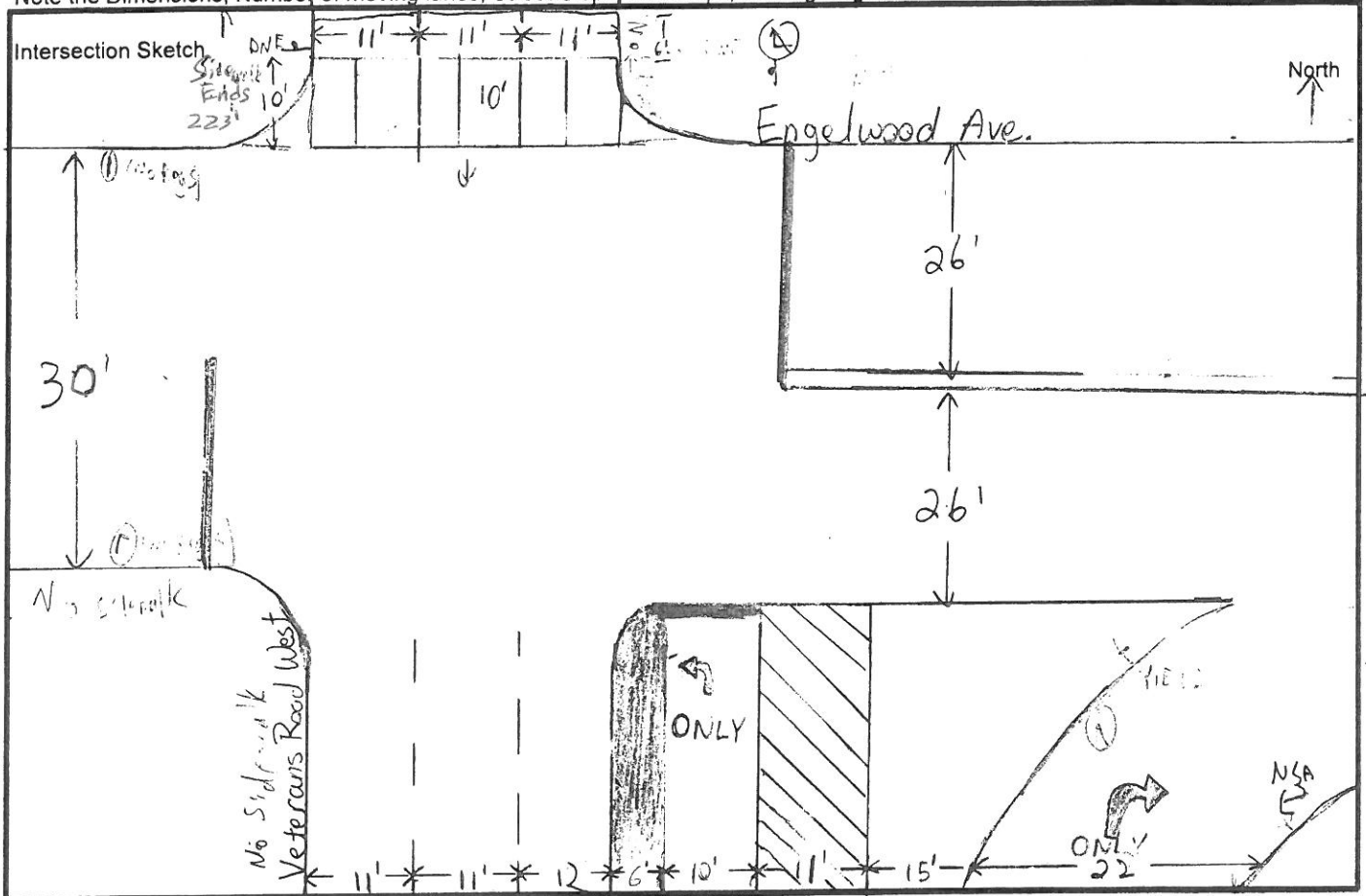
P	A
---	---

Project: Charleston Project #: 1181A

Location: Veterans Road West and Engelwood Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

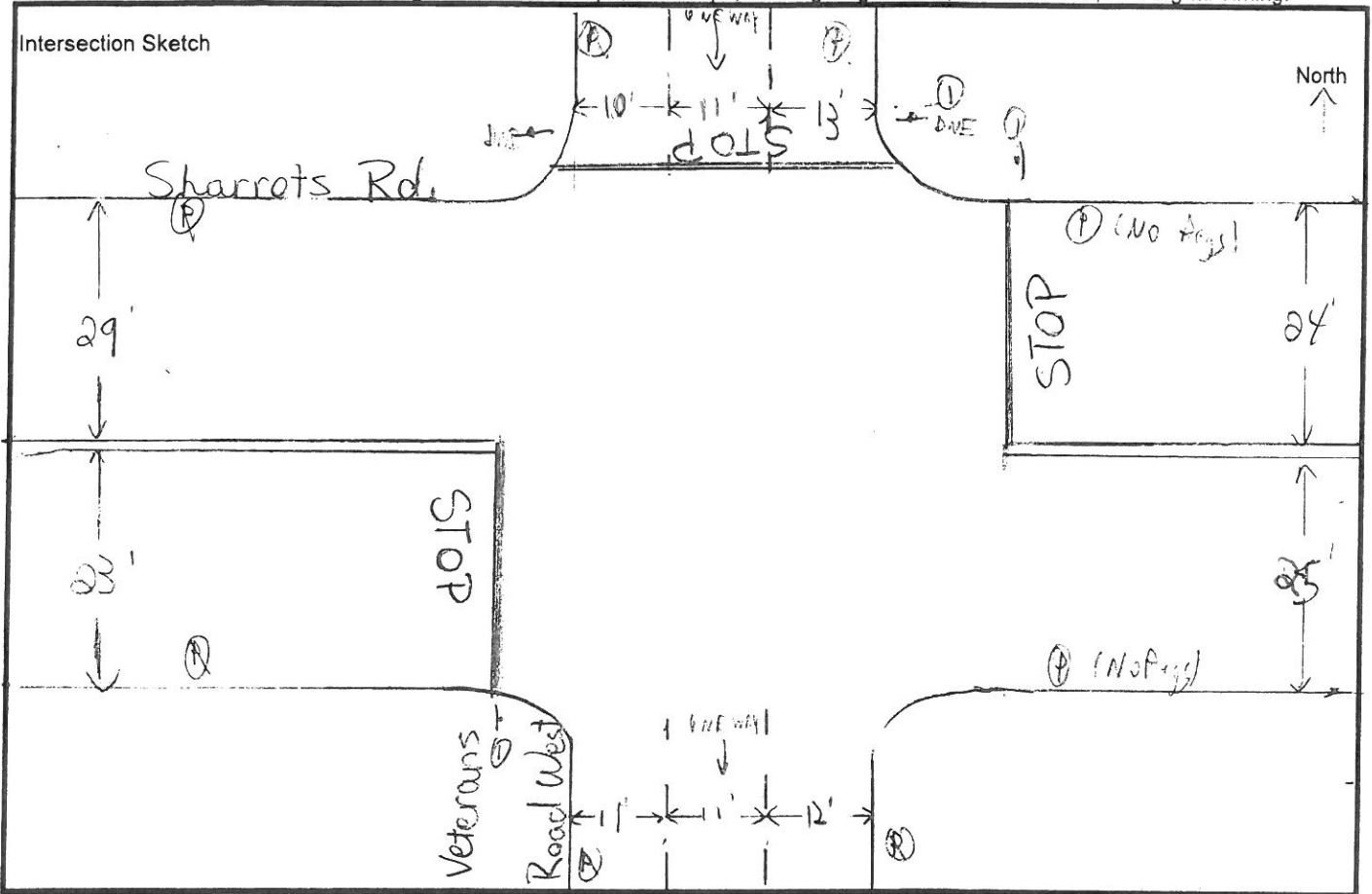
P	A
---	---

Project: Charleston Project #: 1121A

Location: Veterans Road West and Sharrots Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) STOP ALL WAY	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.)		Y+AR
		AM/MD/PM/Sat. MD		

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

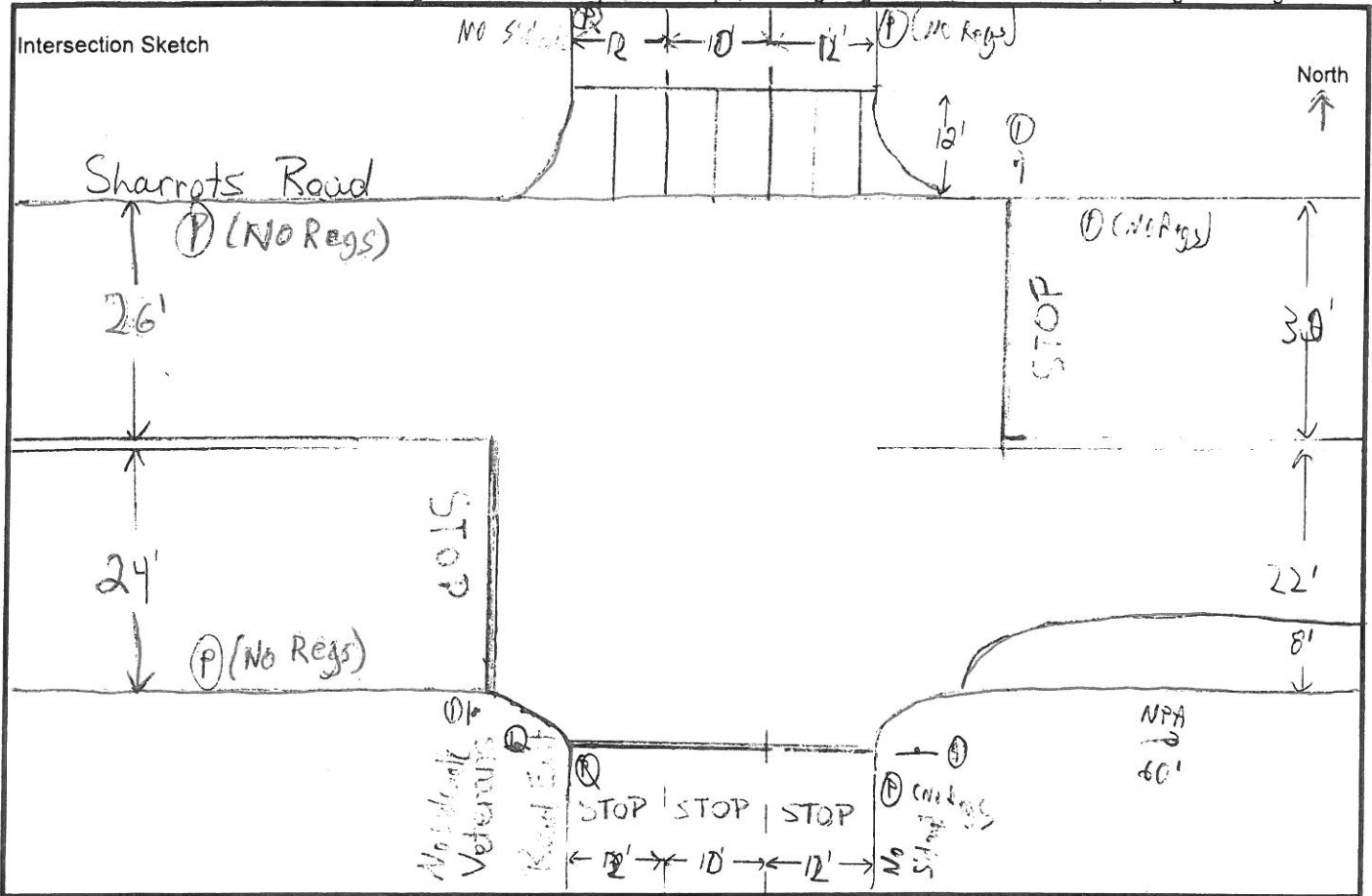
P	A
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Project: Charleston Project #: 1191A

Location: Veterans Road from 119th Street to Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

() STOP ALL-WAY	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time(Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

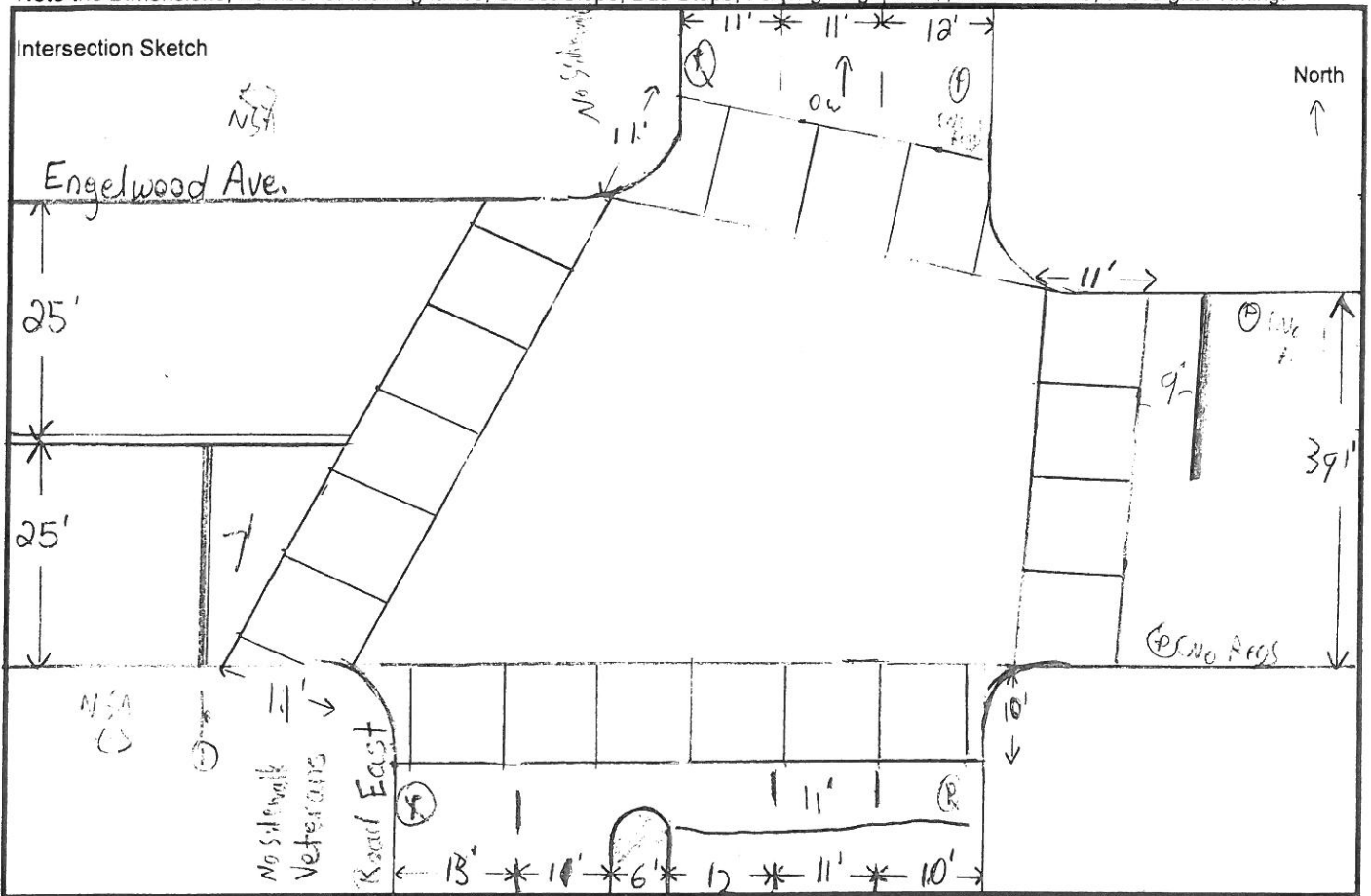
P	A
---	---

Project: Charleston Project #: 1121A

Location: Veterans Road East and Engelwood Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

() AFTER STOP RT PERMITTED ON RED	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

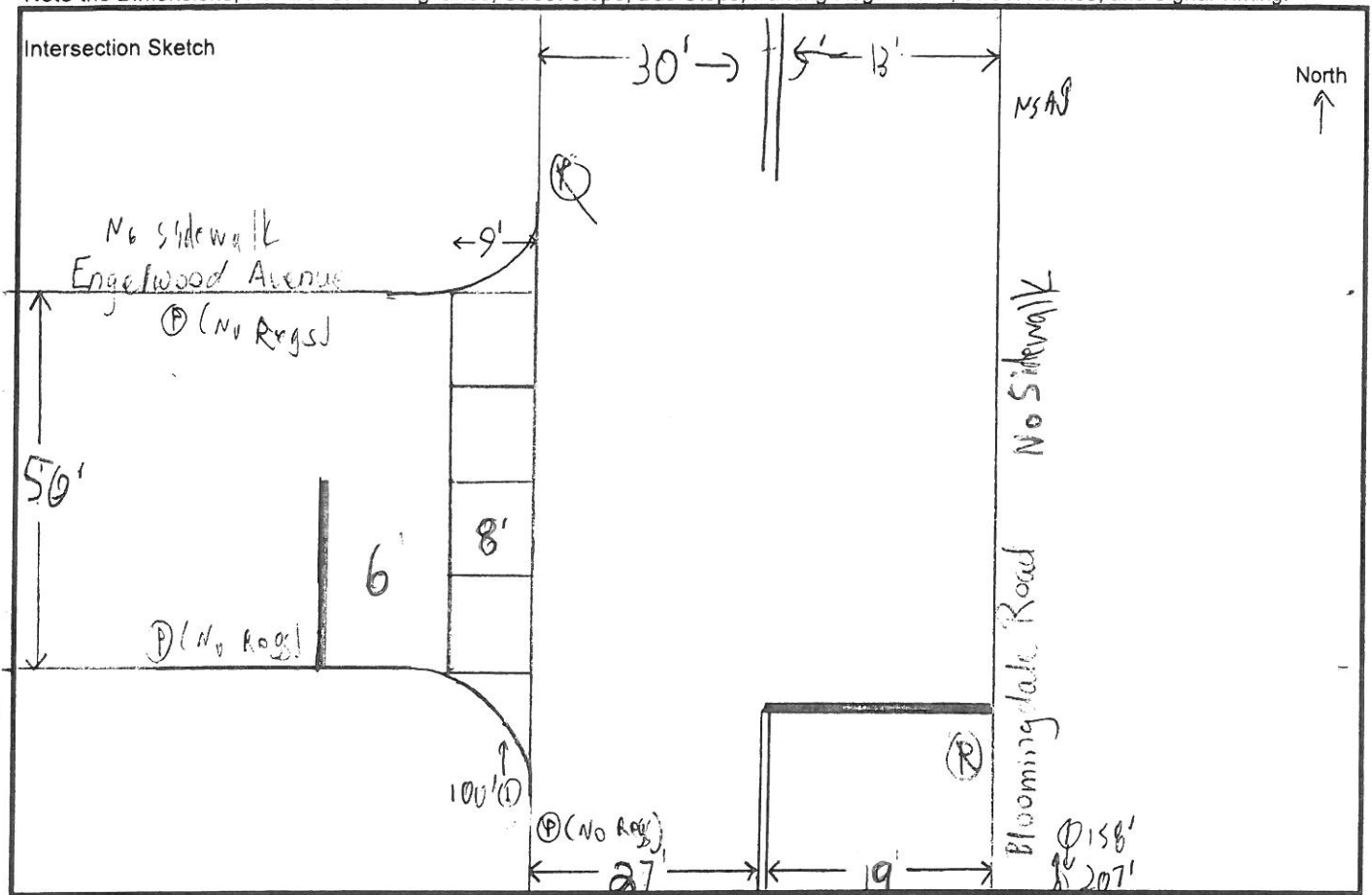
P	A
---	---

Project: Charleston Project #: 1121A

Location: Bloomington Road and Engelwood Avenue

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) NS M-F Bus 555 X 2	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

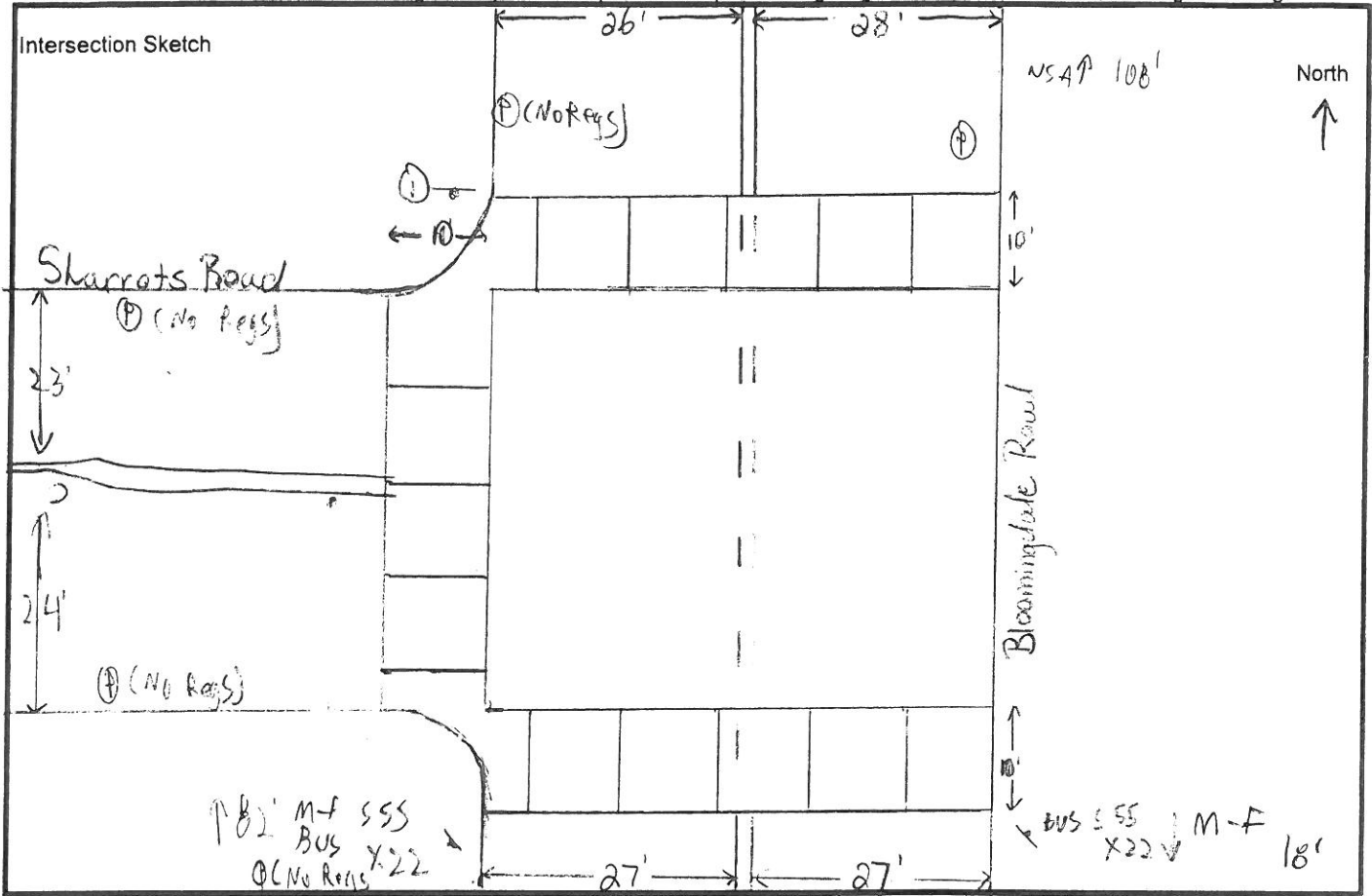
P	A
---	---

Project: Charleston Project #: 11.21.A

Location: Bloomington Road and Sharrots Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) AFTER STOP AT TRAFFIC LIGHTS ON RED	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length	
AM	MD
PM	Sat. MD

Pedestrian Button	
Y	N

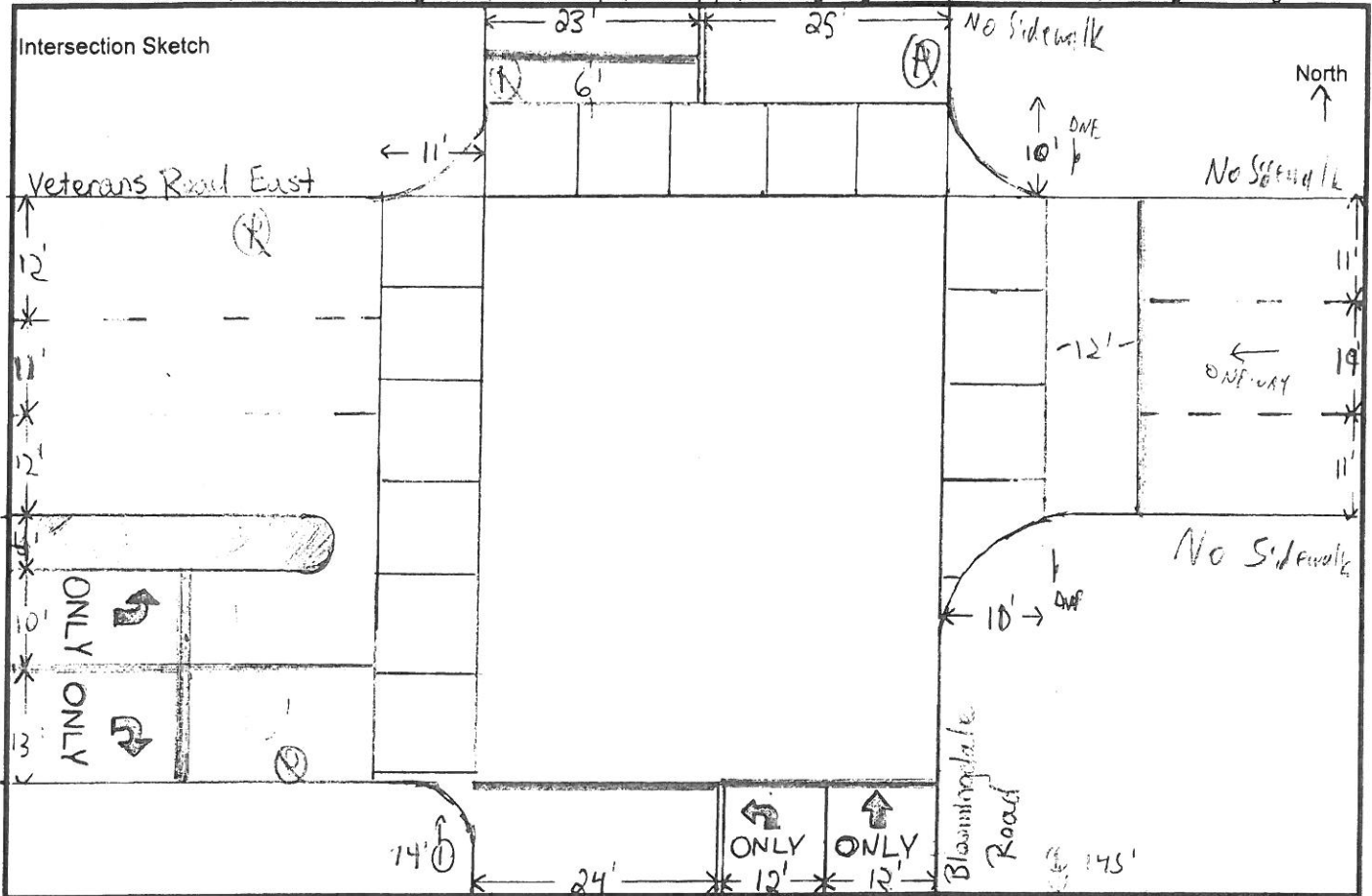
Signal Type	
P	A

Project: Charleston Project #: 1112

Location: Veterans Road East and Bloomingdale Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

()	()
()	()
()	()
()	()
()	()
()	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

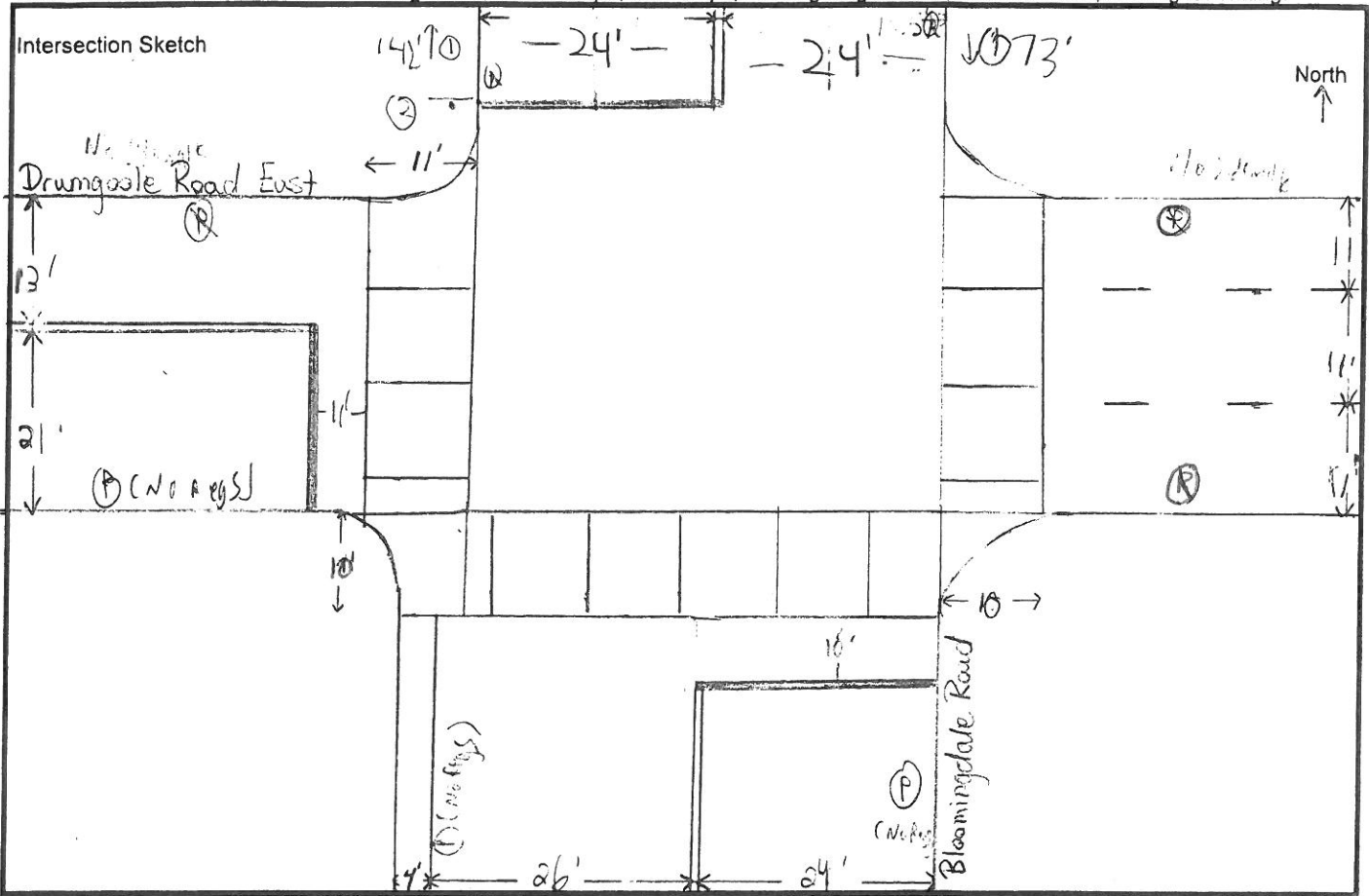
P	A
---	---

Project: Charleston Project #: 11217 Physical Inventory Sheet

Location: Bloomfield Road and Drumgale Road East

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



- Parking Regulations**
- (1) No BUS M-F 555 x 22 ()
 - (2) AFTER STOP RIGHT TURN PERMITTED ON RED XEPT 8 A-4P ()
 - () ()
 - () ()
 - () ()
 - () ()
 - () ()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length

AM	MD
PM	Sat. MD

Pedestrian Button

Y	N
---	---

Signal Type

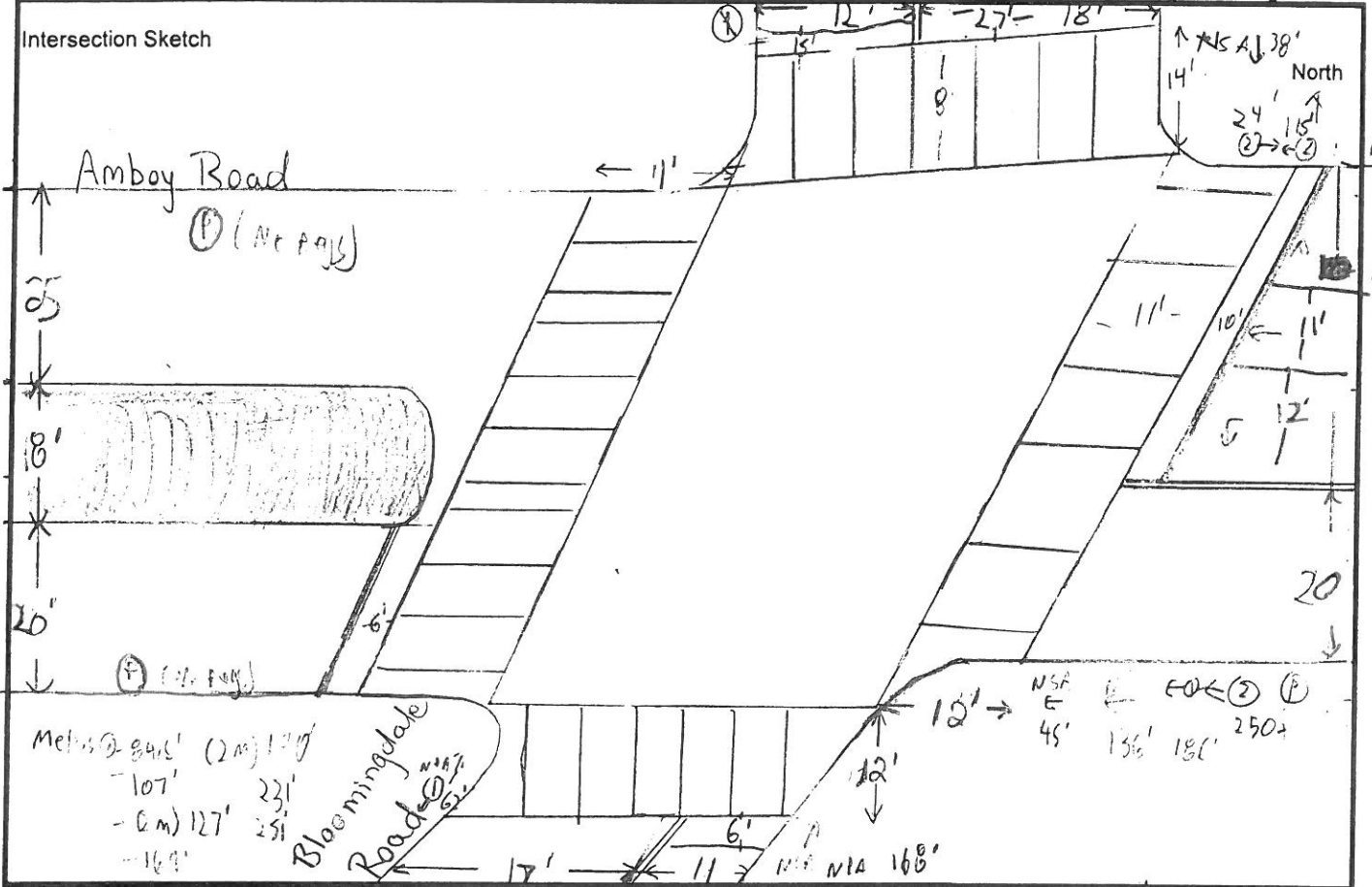
P	A
---	---

Project: Charleston Project #: 1121A Physical Inventory Sheet

Location: Bloomingdale Road and Amboy Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

(1) 2 NR (P) 8A-7P Fy SUN	()
(-) NS 6A-7P M-F BUS 555	()
()	()
()	()
()	()
()	()
() NSA (P) 255	()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

Cycle Length	
AM	MD
PM	Sat. MD

Pedestrian Button	
Y	N

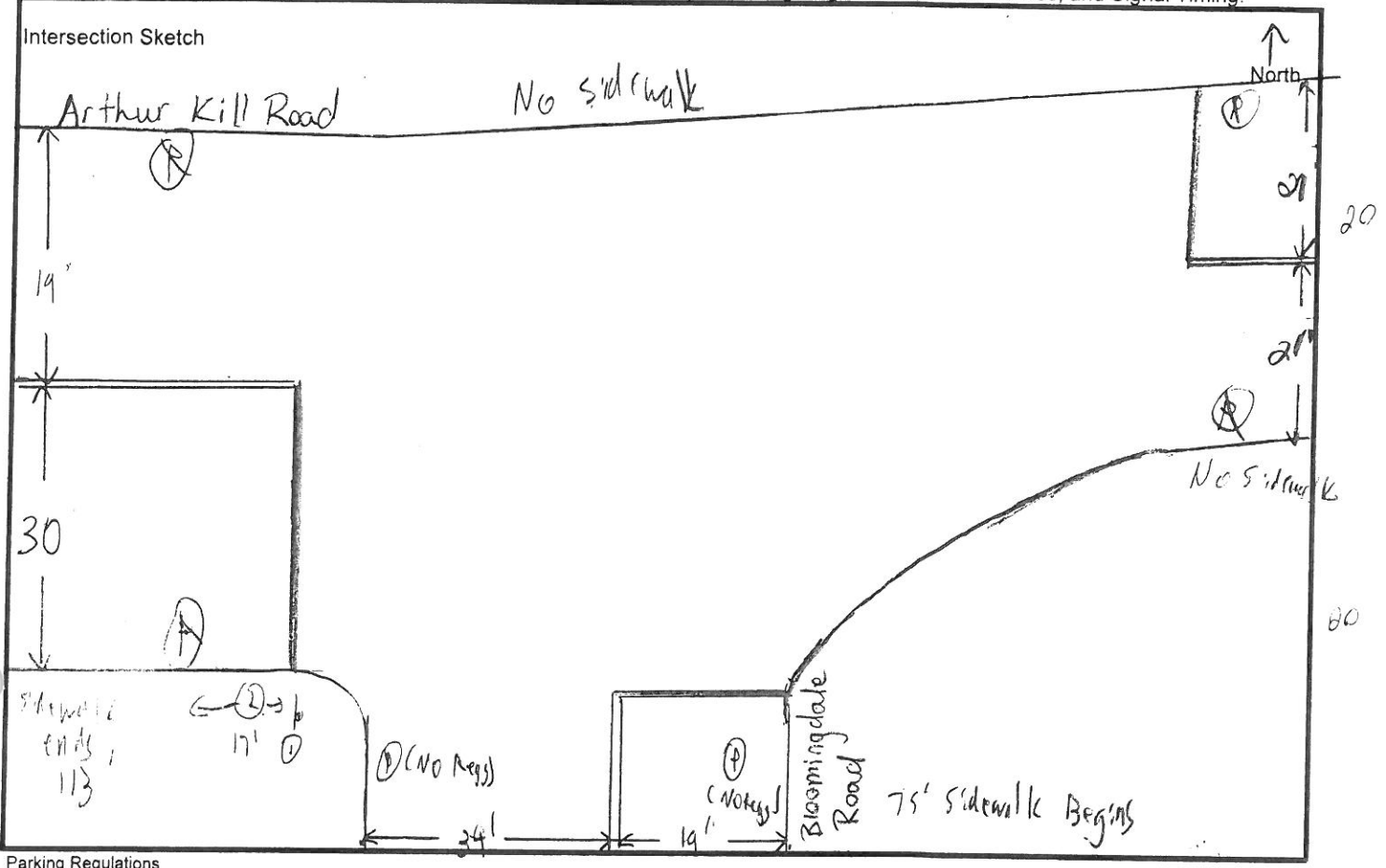
Signal Type	
P	A

Project: Charleston Project #: 1181A

Location: Bloomingdale Road and Arthur Kill Road

Date: _____ Weather: _____ Surveyor: _____

Note the Dimensions, Number of moving lanes, Street Slope, Bus Stops, Parking Regulations, Street Names, and Signal Timing.



Parking Regulations

- (1) AFTER STOP RT PERMITTED AND RED ()
- (2) NO BUS 555 574 ()
- () ()
- () ()
- () ()
- () ()

Signal Timing

Approach	Phasing Diagram	Green Time (Sec.) AM/MD/PM/Sat. MD	Y+AR

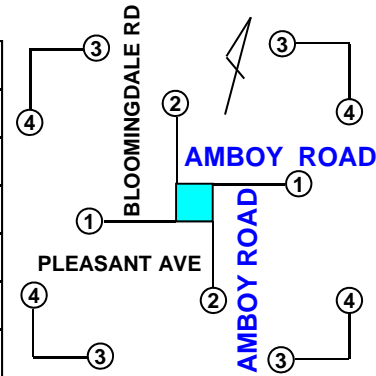
Cycle Length	
AM	MD
PM	Sat. MD

Pedestrian Button	
Y	N

Signal Type	
P	A

Signal Timing

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										
Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



6:00 – 9:45 MON-FRI	9:45 – 15:45 19:30 – 6:00 MON-FRI Weekend -AAT	15:45 – 19:30 MON-FRI	
90 SEC CYCLE	90 SEC CYCLE	90 SEC CYCLE	
28	28	28	
2	2	2	
10	10	10	
3	3	3	
2	2	2	
21	21	21	
2	2	2	
2	2	2	
13	13	13	
2	2	2	
3	3	3	
2	2	2	
OFFSET			
82	0	73	

01/07	YL	INSTALL ADDITIONAL PEDESTRIAN SIGNALS
08/06	YL	REVISE "FLDW"
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

ASTC – 6 Type Controller

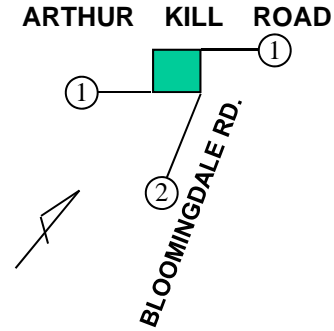
CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
34-02 Queens Blvd, Long Island City, NY 11101

AMBOY ROAD @ BLOOMINGDALE ROAD

Prep. Y. Lak Date 02-19-04
Appr. _____ Date _____

Dwg No.
LB 1057a R

1	2	3	4	5	6	7	8	9	10	11	12
R	R										
A	A										
G	G										



Phase A	G	R									
SPARE	G	R									
SPARE	G	R									
VEH CL	A	R									
VEH CL	R	R									
Phase B	R	G									
SPARE	R	G									
SPARE	R	G									
SPARE	R	G									
SPARE	R	G									
VEH CL	R	A									
VEH CL	R	R									

AT ALL TIMES			
90 SEC CYCLE			
23			
2			
20			
3			
2			
22			
2			
2			
7			
2			
3			
2			
OFFSET			
00			

DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

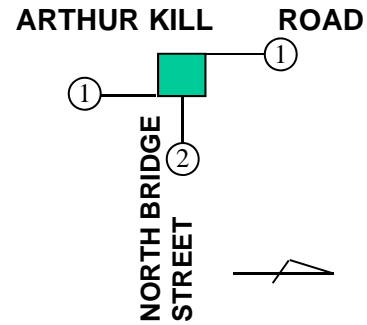
ASTC – 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

ARTHUR KILL ROAD @ BLOOMINGDALE ROAD

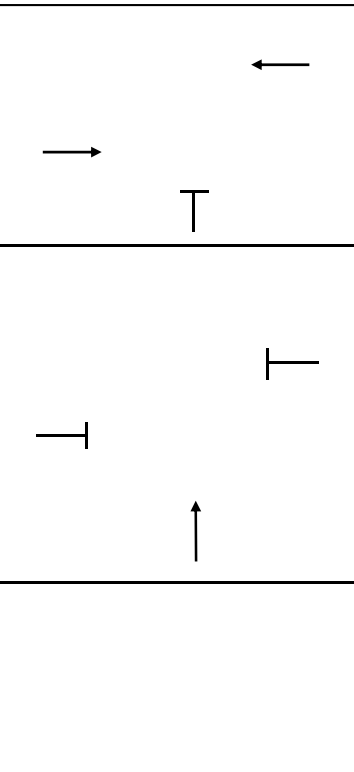
Prep. <u>Y. Lak</u>	Date <u>08-28-04</u>	Dwg No. LB 13117 R
Appr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R										
A	A										
G	G										



AT ALL TIMES			
60 SEC CYCLE			
18			
2			
10			
3			
2			
9			
2			
2			
5			
2			
3			
2			
OFFSET			
35			

Phase A	G	R									
SPARE	G	R									
SPARE	G	R									
VEH CL	A	R									
VEH CL	R	R									
Phase B	R	G									
SPARE	R	G									
SPARE	R	G									
SPARE	R	G									
SPARE	R	G									
VEH CL	R	A									
VEH CL	R	R									



09/06	YL	CHANGE OFFSET
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

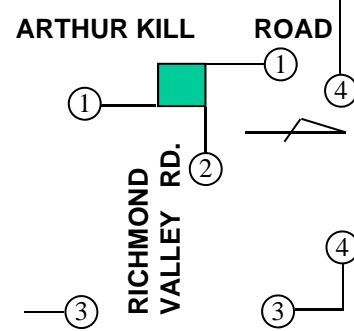
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

ARTHUR KILL ROAD @ NORTH BRIDGE STREET

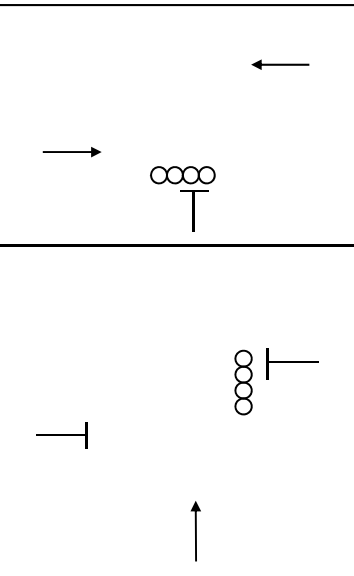
Prep. <u>Y. Lak</u>	Date <u>02-13-06</u>	Dwg No. LB 14215 R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



AT ALL TIMES			
60 SEC CYCLE			
22			
2			
10			
3			
2			
4			
2			
2			
6			
2			
3			
2			
OFFSET			
00			

Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



05/05	YL	CYCLE LENGTH CHANGED
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

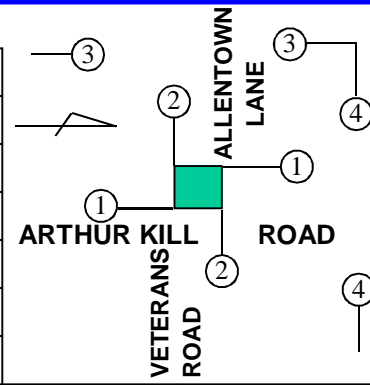
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

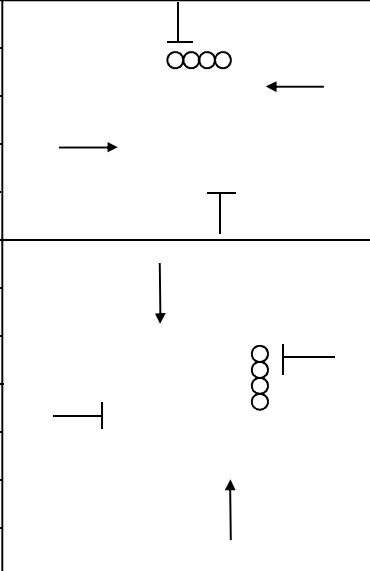
ARTHUR KILL ROAD @ RICHMOND VALLEY ROAD

Prep. <u>Y. Lak</u>	Date <u>05-09-05</u>	Dwg No. LB 140832 R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



AT ALL TIMES			
60 SEC CYCLE			
13			
2			
10			
3			
2			
11			
2			
2			
8			
2			
3			
2			
OFFSET			
00			

DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

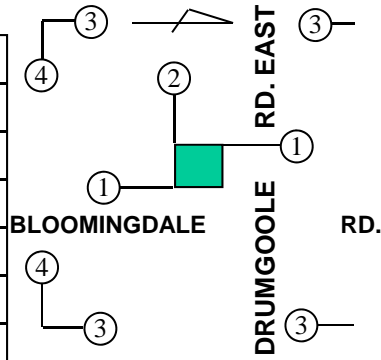
ASTC – 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

ARTHUR KILL ROAD @
VETERANS ROAD WEST & ALLENTOWN LANE

Prep. <u>Y. Lak</u>	Date <u>05-20-05</u>	Dwg No. LB 14093 R
Appr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



AT ALL TIMES			
60 SEC CYCLE			
22			
2			
9			
3			
2			
4			
2			
2			
7			
2			
3			
2			
OFFSET			
00			

Phase A	G	R	WK	DW											
SPARE	G	R	WK	DW											
PED CL	G	R	FLDW	DW											
VEH CL	A	R	DW	DW											
VEH CL	R	R	DW	DW											
Phase B	R	G	DW	WK											
SPARE	R	G	DW	WK											
SPARE	R	G	DW	WK											
PED CL	R	G	DW	FLDW											
PED CL	R	G	DW	DW											
VEH CL	R	A	DW	DW											
VEH CL	R	R	DW	DW											

DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

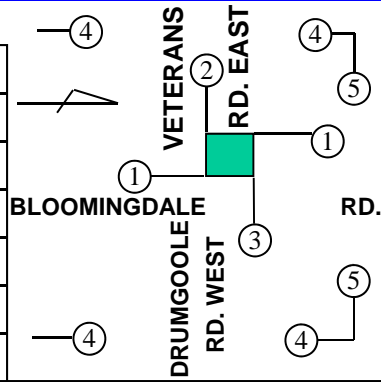
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

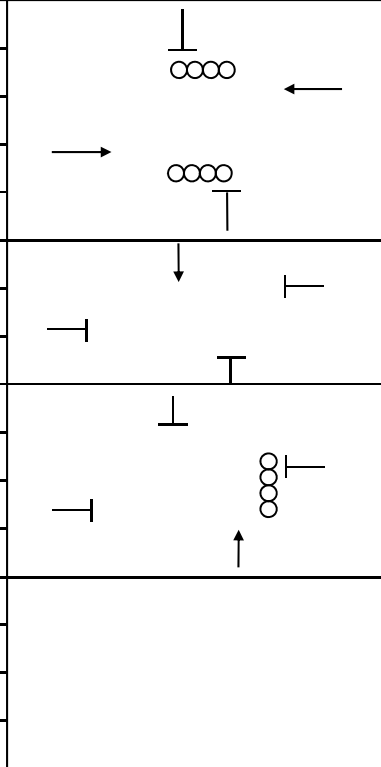
BLOOMINGDALE ROAD @ DRUMGOOLE ROAD EAST

Prep. <u>Y. Lak</u>	Date <u>01-19-05</u>	Dwg No. LB 14027 R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	R	DW	DW							
A	A	A	WK	WK							
G	G	G									



Phase A	G	R	R	WK	DW										
SPARE	G	R	R	WK	DW										
PED CL	G	R	R	FLDW	DW										
VEH CL	A	R	R	DW	DW										
VEH CL	R	R	R	DW	DW										
Phase B	R	G	R	DW	DW										
VEH CL	R	A	R	DW	DW										
VEH CL	R	R	R	DW	DW										
Phase C	R	R	G	DW	WK										
PED CL	R	R	G	DW	FLDW										
VEH CL	R	R	A	DW	DW										
VEH CL	R	R	R	DW	DW										



AT ALL TIMES			
60 SEC CYCLE			
5			
2			
13			
3			
2			
8			
3			
2			
6			
11			
3			
2			
OFFSET			
00			

DATE	BY	REVISIONS

NOTES: SEMI - ACTUATED (PHASE B ONLY)
 ASTC - 6 Type Controller

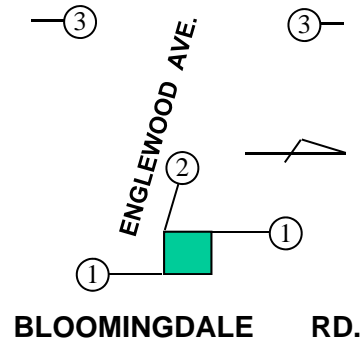
CITY OF NEW YORK
 BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

**BLOOMINGDALE ROAD @
 DRUMGOOLE ROAD WEST & VETERANS RD. EAST**

Prep. Y. Lak Date 01-24-05
 Apr. _____ Date _____

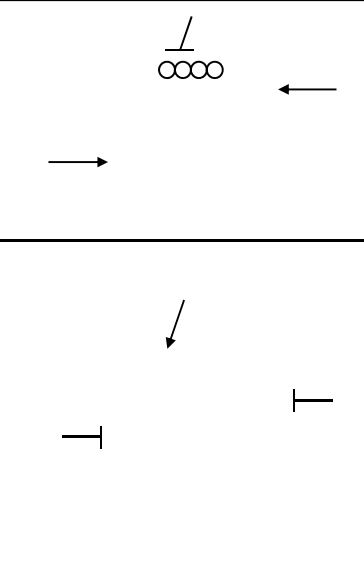
Dwg No. LB 12830 R

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW									
A	A	WK									
G	G										



AT ALL TIMES			
60 SEC CYCLE			
22			
2			
10			
3			
2			
4			
2			
2			
6			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	WK								
SPARE	G	R	WK								
PED CL	G	R	FLDW								
VEH CL	A	R	DW								
VEH CL	R	R	DW								
Phase B	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
VEH CL	R	A	DW								
VEH CL	R	R	DW								



DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

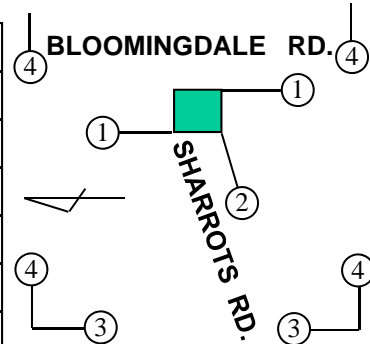
ASTC – 6 Type Controller

**CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
34-02 Queens Blvd, Long Island City, NY 11101**

BLOOMINGDALE ROAD @ ENGLEWOOD AVENUE

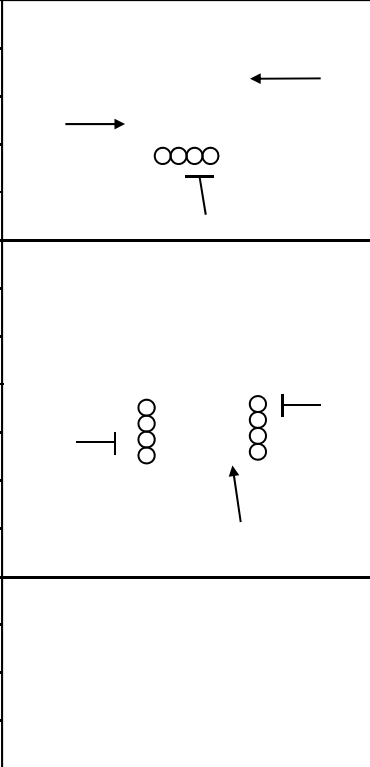
Prep. <u>Y. Lak</u>	Date <u>01-22-05</u>	Dwg No. LB 14139 R
Appr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



AT ALL TIMES			
60 SEC CYCLE			
16			
2			
12			
3			
2			
4			
2			
2			
10			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	WK	DW										
SPARE	G	R	WK	DW										
PED CL	G	R	FLDW	DW										
VEH CL	A	R	DW	DW										
VEH CL	R	R	DW	DW										
Phase B	R	G	DW	WK										
SPARE	R	G	DW	WK										
SPARE	R	G	DW	WK										
PED CL	R	G	DW	FLDW										
PED CL	R	G	DW	DW										
VEH CL	R	A	DW	DW										
VEH CL	R	R	DW	DW										



DATE	BY	REVISIONS

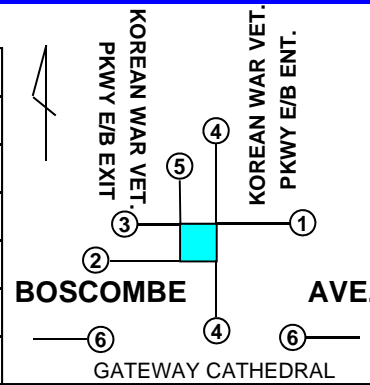
NOTES: SEMI - ACTUATED
 ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

BLOOMINGDALE ROAD @ SHARROTT'S ROAD

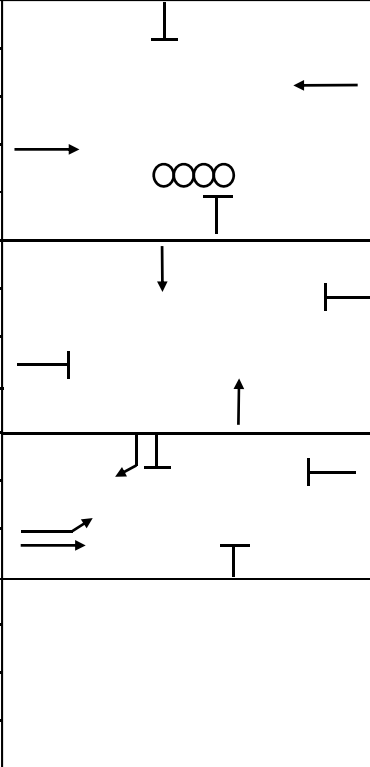
Prep. <u>Y. Lak</u>	Date <u>09-04-04</u>	Dwg No. LB 12820 R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	R	R	R	DW						
A	A	A	A	A	WK						
G	G	G	G	G							
		A		A							
		G		G							



ALL OTHER TIMES	16:00 - 19:30	15:00 - 19:30	
	MON - FRI	SATURDAY	
90 SEC CYCLE	90 SEC CYCLE	90 SEC CYCLE	
10	16	16	
2	2	2	
10	10	10	
3	3	3	
2	2	2	
14	12	12	
2	2	2	
3	3	3	
2	2	2	
37	33	33	
3	3	3	
2	2	2	
OFFSET			
58	58	58	

Phase A	G	G	G	R	R	WK								
SPARE	G	G	G	R	R	WK								
PED CL	G	G	G	R	R	FLDW								
VEH CL	A	A	A	R	R	DW								
VEH CL	R	R	R	R	R	DW								
Phase B	R	R	R	G	G	DW								
SPARE	R	R	R	G	G	DW								
VEH CL	R	R	R	A	A	DW								
VEH CL	R	R	R	R	R	DW								
Phase C	R	G	G/A	R	R/G	DW								
VEH CL	R	G	G/A	R	R/A	DW								
VEH CL	R	G	G	R	R	DW								



09/10	YL	MODIFY TIMING FOR AM AND PM
DATE	BY	REVISIONS

NOTES: NON - ACTUATED

ASTC - 6 Type Controller

PC = 3.0 ft/sec

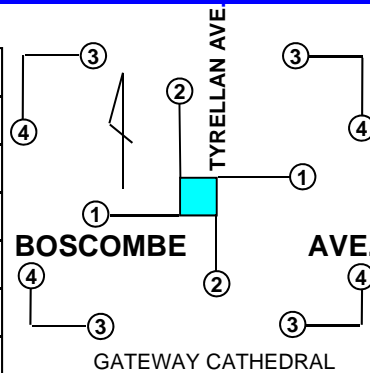
CABINET ADDRESS: 7421

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

BOSCOMBE AVENUE @
KOREAN WAR VETERANS PKWY E/B ENT/EXIT

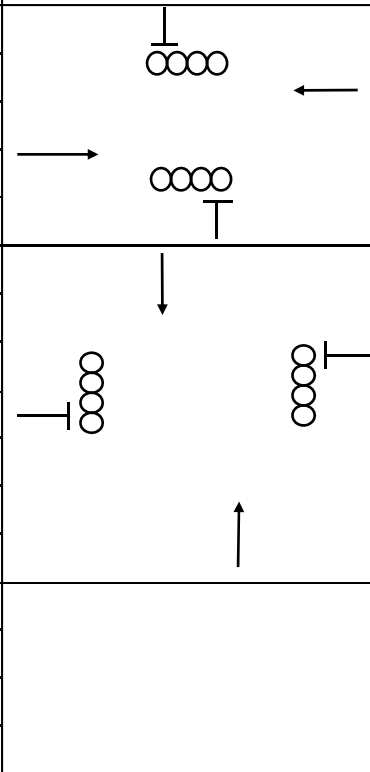
Prep. <u>Y. Lak</u>	Date <u>09-21-10</u>	Dwg No. LB 13223 R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



AAT			
90 SEC CYCLE			
31			
2			
12			
3			
2			
17			
2			
2			
12			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	WK	DW										
SPARE	G	R	WK	DW										
PED CL	G	R	FLDW	DW										
VEH CL	A	R	DW	DW										
VEH CL	R	R	DW	DW										
Phase B	R	G	DW	WK										
SPARE	R	G	DW	WK										
SPARE	R	G	DW	WK										
PED CL	R	G	DW	FLDW										
PED CL	R	G	DW	DW										
VEH CL	R	A	DW	DW										
VEH CL	R	R	DW	DW										



12/05	YL	CHANGED CYCLE LENGTH FROM 60 SEC TO 90 SEC
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

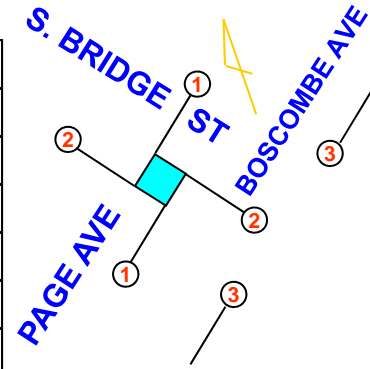
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

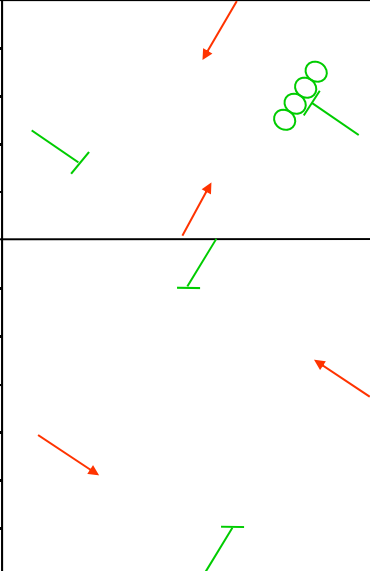
BOSCOMBE AVENUE @ TYRELLAN AVENUE

Prep. <u>Y. Lak</u>	Date <u>09-23-05</u>	Dwg No. LB 14140R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW									
A	A	WK									
G	G										



Phase A	G	R	WK								
SPARE	G	R	WK								
PED CL	G	R	FLDW								
VEH CL	A	R	DW								
VEH CL	R	R	DW								
Phase B	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
PED CL	R	G	DW								
PED CL	R	G	DW								
VEH CL	R	A	DW								
VEH CL	R	R	DW								



AAT			
90 SEC CYCLE			
38			
2			
10			
3			
2			
17			
2			
2			
7			
2			
3			
2			
OFFSET			
0			

12/05	YL	CHANGED CYCLE LENGTH FROM 60 SEC TO 90 SEC
DATE	BY	REVISIONS

NOTES: NON - ACTUATED

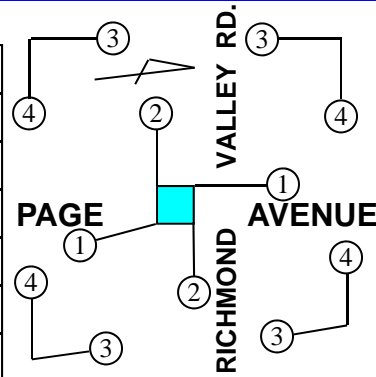
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

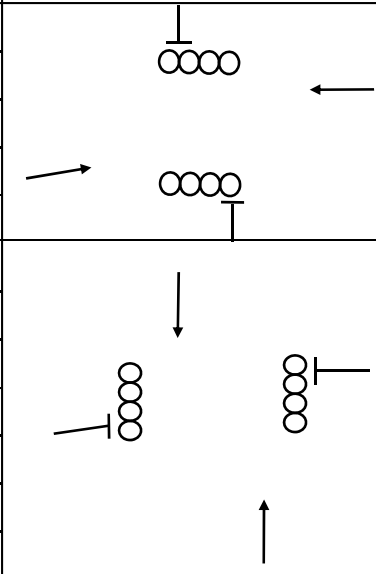
PAGE AVENUE @ BOSCOMBE AVE & SOUTH BRIDGE STREET

Prep. <u>N.Ibrahim</u>	Date <u>10-09-04</u>	Dwg No. LB
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



AAT			
90 SEC CYCLE			
35			
2			
12			
3			
2			
10			
2			
2			
15			
2			
3			
2			
OFFSET			
0			

04/07	YL	MODIFY PHASING
04/07	YL	CHANGE TO FIXED TIME OPERATION
DATE	BY	REVISIONS

NOTES: NON - ACTUATED

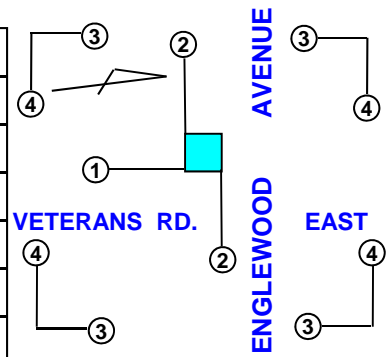
ASTC - 6 Type Controller

CITY OF NEW YORK
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PAGE AVENUE @ RICHMOND VALLEY ROAD

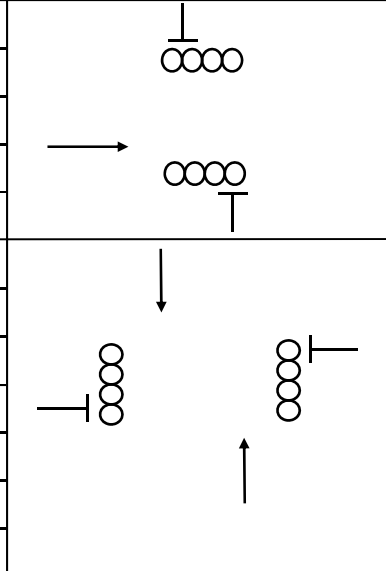
Prep. <u>Y. Lak</u>	Date <u>04-12-07</u>	Dwg No. LB 12008R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



AAT			
60 SEC CYCLE			
15			
2			
12			
3			
2			
3			
2			
2			
12			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

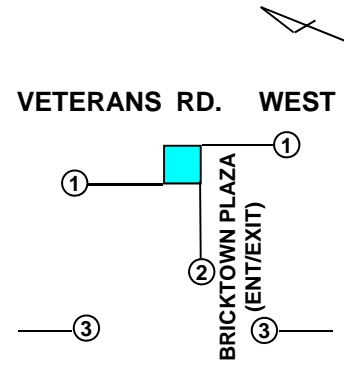
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

VETERANS ROAD EAST @ ENGLEWOOD AVENUE

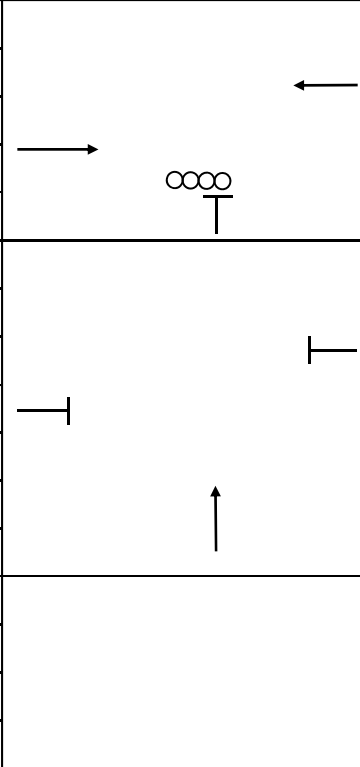
Prep. <u>Y. Lak</u>	Date <u>11-02-06</u>	Dwg No. LB 14141R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW									
A	A	WK									
G	G										



AAT			
60 SEC CYCLE			
5			
2			
24			
3			
2			
11			
2			
2			
2			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	WK								
SPARE	G	R	WK								
PED CL	G	R	FLDW								
VEH CL	A	R	DW								
VEH CL	R	R	DW								
Phase B	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
SPARE	R	G	DW								
VEH CL	R	A	DW								
VEH CL	R	R	DW								



DATE	BY	REVISIONS

NOTES: NON - ACTUATED

ASTC - 6 Type Controller

(PC: 3.0 Ft/Sec)

CABINET ADDRESS 744E

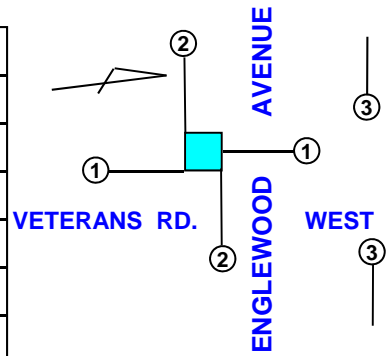
CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
34-02 Queens Blvd, Long Island City, NY 11101

VETERANS ROAD WEST @
BRICKTOWN PLAZA (ENT/EXIT)
(1600' EAST OF TYRELLAN AVENUE)

Prep. T. Manina Date 05-19-10
Appr. _____ Date _____

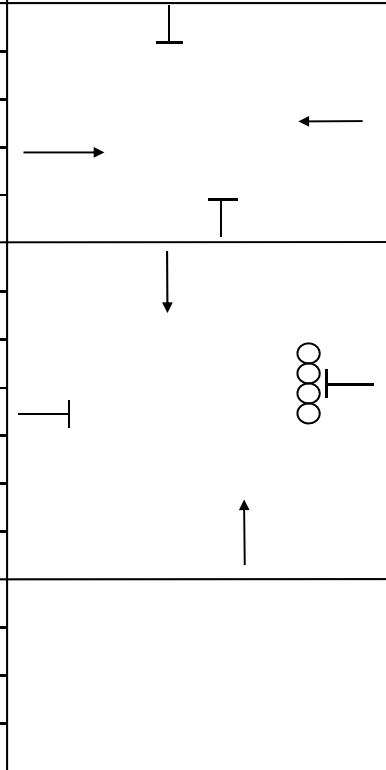
Dwg No. LB R

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW									
A	A	WK									
G	G										



AAT			
60 SEC CYCLE			
11			
2			
12			
3			
2			
7			
2			
2			
12			
2			
3			
2			
OFFSET			
0			

Phase A	G	R	DW								
SPARE	G	R	DW								
PED CL	G	R	DW								
VEH CL	A	R	DW								
VEH CL	R	R	DW								
Phase B	R	G	WK								
SPARE	R	G	WK								
SPARE	R	G	WK								
PED CL	R	G	FLDW								
PED CL	R	G	DW								
VEH CL	R	A	DW								
VEH CL	R	R	DW								



DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

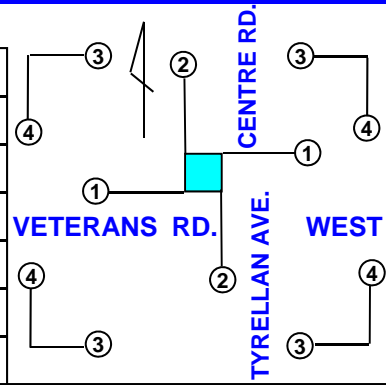
ASTC - 6 Type Controller

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
 34-02 Queens Blvd, Long Island City, NY 11101

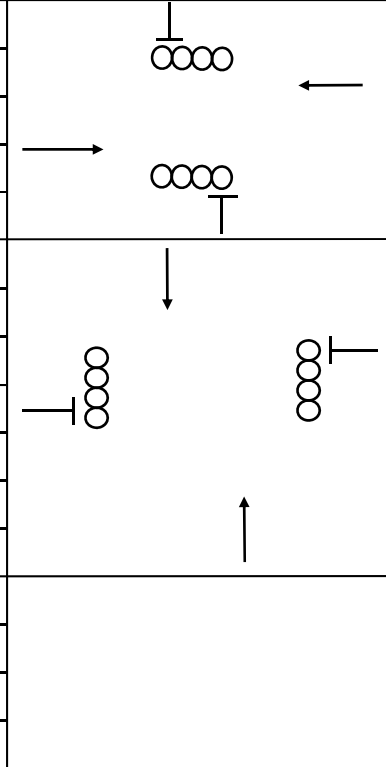
VETERANS ROAD WEST @ ENGLEWOOD AVENUE

Prep. <u>Y. Lak</u>	Date <u>09-16-05</u>	Dwg No. LB 14138R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	DW	DW								
A	A	WK	WK								
G	G										



Phase A	G	R	WK	DW							
SPARE	G	R	WK	DW							
PED CL	G	R	FLDW	DW							
VEH CL	A	R	DW	DW							
VEH CL	R	R	DW	DW							
Phase B	R	G	DW	WK							
SPARE	R	G	DW	WK							
SPARE	R	G	DW	WK							
PED CL	R	G	DW	FLDW							
PED CL	R	G	DW	DW							
VEH CL	R	A	DW	DW							
VEH CL	R	R	DW	DW							



01/06	YL	CHANGED CYCLE LENGTH FROM 60 SEC TO 90 SEC.
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

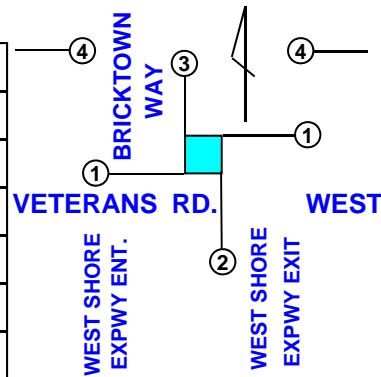
ASTC - 6 Type Controller

**CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
34-02 Queens Blvd, Long Island City, NY 11101**

**VETERANS ROAD WEST @
TYRELLAN AVENUE & CENTRE ROAD**

Prep. <u>Y. Lak</u>	Date <u>09-19-05</u>	Dwg No. LB 14137R
Apr. _____	Date _____	

1	2	3	4	5	6	7	8	9	10	11	12
R	R	R	DW								
A	A	A	WK								
G	G	G									



AOT			
90 SEC CYCLE			
14			
2			
14			
3			
2			
23			
2			
3			
2			
20			
3			
2			
OFFSET			
0			

Phase A	G	R	R	WK										
SPARE	G	R	R	WK										
PED CL	G	R	R	FLDW										
VEH CL	A	R	R	DW										
VEH CL	R	R	R	DW										
Phase B	R	G	R	DW										
SPARE	R	G	R	DW										
VEH CL	R	A	R	DW										
VEH CL	R	R	R	DW										
Phase C	R	R	G	DW										
VEH CL	R	R	A	DW										
VEH CL	R	R	R	DW										

01/09	YL	CHANGE TIMING
DATE	BY	REVISIONS

NOTES: **NON - ACTUATED**

ASTC - 6 Type Controller

CABINET ADDRESS 742D

CITY OF NEW YORK
BUREAU OF TRAFFIC OPERATIONS
34-02 Queens Blvd, Long Island City, NY 11101

VETERANS ROAD WEST @
WEST SHORE EXPWY ENT/EXIT & BRICKTOWN WAY

Prep. <u>Y. Lak</u>	Date <u>10-06-05</u>	Dwg No. LB 14136R
Appr. _____	Date _____	

Existing 2011

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: Sharrotts Road				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	25	129	38	27	124	36	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	
Hourly Flow Rate, HFR (veh/h)	30	155	45	32	149	43	
Percent Heavy Vehicles	20	--	--	13	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	8	7	7	56	9	26	
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.95	0.95	0.95	
Hourly Flow Rate, HFR (veh/h)	10	8	8	58	9	27	
Percent Heavy Vehicles	32	32	32	5	5	5	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	30	32		94			26
C (m) (veh/h)	1281	1309		528			481
v/c	0.02	0.02		0.18			0.05
95% queue length	0.07	0.08		0.64			0.17
Control Delay (s/veh)	7.9	7.8		13.3			12.9
LOS	A	A		B			B
Approach Delay (s/veh)	--	--		13.3			12.9
Approach LOS	--	--		B			B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		177	30	19	168		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	213	36	22	202	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				7		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.69	1.00	0.69	
Hourly Flow Rate, HFR (veh/h)	0	0	0	10	0	21	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		22		31			
C (m) (veh/h)		1266		700			
v/c		0.02		0.04			
95% queue length		0.05		0.14			
Control Delay (s/veh)		7.9		10.4			
LOS		A		B			
Approach Delay (s/veh)	--	--	10.4				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group	LTR			LT R			LTR			LTR		
Volume, V (vph)	3	6	1	136	32	244	0	132	256	63	104	8
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			193	280		456			201	
Lane Group Capacity, c		721			545	514		703			519	
v/c Ratio, X		0.02			0.35	0.54		0.65			0.39	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.0	13.2		14.0			12.2	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.0			1.7	3.9		4.1			2.2	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0		0.0
Control Delay		10.3		13.7	17.1		18.1		14.3
Lane Group LOS		B		B	B		B		B
Approach Delay		10.3		15.7			18.1		14.3
Approach LOS		B		B			B		B
Intersection Delay		16.4		$X_c = 0.60$			Intersection LOS		B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				246		5		383			241	
% Heavy Vehicles, %HV				8		8		11			15	
Peak-Hour Factor, PHF				0.88		0.88		0.85			0.87	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.893			0.886			0.754	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			3	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					286			451			277	
Lane Group Capacity, c					632			970			925	
v/c Ratio, X					0.45			0.46			0.30	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					15.7			9.8			8.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					2.1			1.4			0.6	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				17.8			11.2			9.4	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>A</i>	
Approach Delay				17.8	11.2			9.4			
Approach LOS				<i>B</i>	<i>B</i>			<i>A</i>			
Intersection Delay	12.6			$X_c = 0.46$	Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	7/28/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		383	205	111	376		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.87	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	450	241	127	432	0	
Percent Heavy Vehicles	0	--	--	20	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		127					
C (m) (veh/h)		825					
v/c		0.15					
95% queue length		0.54					
Control Delay (s/veh)		10.2					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				54		125		463	65	77	299	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.980			0.769			0.842	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					192			621			433	
Lane Group Capacity, c					384			1032			839	
v/c Ratio, X					0.50			0.60			0.52	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					18.6			8.5			8.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					4.5			2.0			1.9	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				23.1			10.6			9.9	
Lane Group LOS				C			B			A	
Approach Delay				23.1				10.6			
Approach LOS				C				B			
Intersection Delay	12.3		$X_c = 0.57$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	60	21	61	20	107	15	48	610	104	49	333	24
% Heavy Vehicles, %HV	1	1	1	1	1	1	3	3	3	9	9	9
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.94	0.94	0.94	0.92	0.92	0.92	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.810			1.000		0.649	0.649			0.986	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	6
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		158			151		52	776			475	
Lane Group Capacity, c		569			585		436	1048			1108	
v/c Ratio, X		0.28			0.26		0.12	0.74			0.43	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		21.4			21.2		10.0	15.6			12.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.0			1.1		0.4	3.1			1.2	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	22.4	22.3	10.3	18.7
Lane Group LOS	C	C	B	B
Approach Delay	22.4	22.3	18.2	13.4
Approach LOS	C	C	B	B
Intersection Delay	17.6	$X_c = 0.56$	Intersection LOS	B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					351		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	417	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			55				
Peak-Hour Factor, PHF	1.00	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	78	0	0	0	
Percent Heavy Vehicles	0	0	60	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							78
C (m) (veh/h)							706
v/c							0.11
95% queue length							0.37
Control Delay (s/veh)							10.7
LOS							B
Approach Delay (s/veh)	--	--				10.7	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	261							685			351	
% Heavy Vehicles, %HV	2							1			3	
Peak-Hour Factor, PHF	0.90							0.91			0.84	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.665			0.986	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	290							753			418	
Lane Group Capacity, c	669							2156			1951	
v/c Ratio, X	0.43							0.35			0.21	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.4							11.0			10.1	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.0							0.3			0.2	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	25.4						11.3			10.3
Lane Group LOS	C						B			B
Approach Delay	25.4						11.3		10.3	
Approach LOS	C						B		B	
Intersection Delay	13.8		$X_C = 0.38$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: KWVP WB Off-Ramp U-Turn				North/South Street: North Bridge Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					357		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	375	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				264			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	283	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11 12
Lane Configuration			L				
v (veh/h)			283				
C (m) (veh/h)			615				
v/c			0.46				
95% queue length			2.42				
Control Delay (s/veh)			15.7				
LOS			C				
Approach Delay (s/veh)	--	--	15.7				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Mohr St/KWVP					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	46	128	151	186	316	48	71	114	113	4	20	25
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	52	273		214	414			321		6	67	
Lane Group Capacity, c	273	577		262	1096			882		376	351	
v/c Ratio, X	0.19	0.47		0.82	0.38			0.36		0.02	0.19	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	21.4	23.7		27.5	22.9			26.1		27.3	28.4	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	1.2	2.2		23.4	1.0			1.2		0.1	1.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	22.6	25.9		50.8	23.9			27.3		27.4	29.6	
Lane Group LOS	C	C		D	C			C		C	C	
Approach Delay	25.4			33.1				27.3		29.4		
Approach LOS	C			C				C		C		
Intersection Delay	29.6			$X_c = 0.50$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	AS			Intersection	Mohr Street & Tyrellan Avenue			
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY			
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID 1121A - Charleston								
East/West Street: Mohr Street				North/South Street: Tyrellan Avenue				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	44	17	31	39	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	10	0	20	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.80	0.80	0.78	0.78	0.86	0.86		
Flow Rate (veh/h)	27	48	63	25	11	23		
% Heavy Vehicles	21	21	1	1	5	5		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.6	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.2	0.2	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.4	0.1	0.3	0.0	0.3	-0.5		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.02	0.04	0.06	0.02	0.01	0.02		
hd, final value (s)	4.99	4.69	4.95	4.64	4.56	3.77		
x, final value	0.04	0.06	0.09	0.03	0.01	0.02		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	2.7	2.4	2.7	2.3	2.6	1.8		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	277	298	313	275	261	273		
Delay (s/veh)	7.89	7.70	8.12	7.50	7.63	6.86		
LOS	A	A	A	A	A	A		
Approach: Delay (s/veh)	7.77		7.95		7.11			
LOS	A		A		A			
Intersection Delay (s/veh)	7.73							
Intersection LOS	A							

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Veterans Rd W & Tyrellan Ave						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0	
Lane Group		LTR			LTR		DefL	TR			LTR		
Volume, V (vph)	50	23	172	97	243	6	212	69	43	10	91	95	
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8	
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.916			0.953		1.000	1.000			1.000		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0			12.0		13.0	13.0			9.5		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		278			397		244	128			258		
Lane Group Capacity, c		1079			1202		497	791			1192		
v/c Ratio, X		0.26			0.33		0.49	0.16			0.22		
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44		
Uniform Delay, d ₁		15.7			16.3		17.8	15.0			15.4		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		0.5			0.7		3.4	0.4			0.4		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	16.2	17.0	21.2	15.4
Lane Group LOS	B	B	C	B
Approach Delay	16.2	17.0	19.2	15.8
Approach LOS	B	B	B	B
Intersection Delay	17.2	$X_c = 0.41$	Intersection LOS	B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2010					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	729	199	18	4	214	162	4	15	6	88	5	133
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.86	0.86	0.86	0.37	0.37	0.37	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.603	0.603			0.932	0.932		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	801	239			254	188		68		101	6	153
Lane Group Capacity, c	903	1156			448	383		354		226	320	952
v/c Ratio, X	0.89	0.21			0.57	0.49		0.19		0.45	0.02	0.16
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	12.2	4.4			29.8	29.2		31.5		33.0	30.5	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	8.1	0.2			4.8	4.1		1.2		6.3	0.1	0.4

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	20.3	4.6			34.6	33.3		32.7		39.3	30.6	6.7
Lane Group LOS	C	A			C	C		C		D	C	A
Approach Delay	16.7			34.1			32.7			19.9		
Approach LOS	B			C			C			B		
Intersection Delay	22.0			$X_c = 0.71$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	269	24	0	13	63	35	6	20	10	14	35	311
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	296	26			141			79			64	205
Lane Group Capacity, c	707	1066			1545			1232			676	616
v/c Ratio, X	0.42	0.02			0.09			0.06			0.09	0.33
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	14.2	11.4			11.8			17.2			17.4	19.3
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	1.8	0.0			0.1			0.1			0.3	1.4

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	16.0	11.4			11.9			17.3			17.7	20.7
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	15.6				11.9				17.3		20.0	
Approach LOS	<i>B</i>				<i>B</i>				<i>B</i>		<i>C</i>	
Intersection Delay	16.6				$X_c = 0.38$				Intersection LOS		<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	Philip Habib & Associates						W					
Date Performed	8/03/2011					Area Type	All other areas					
Time Period	Weekday 8-9 AM Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	60		4				4	72			173	342
% Heavy Vehicles, %HV	11		11				2	2			1	1
Peak-Hour Factor, PHF	0.80		0.80				0.80	0.80			0.81	0.81
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.995		0.995					1.000			0.752	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	4	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	75		0					95			636	
Lane Group Capacity, c	532		461					1679			1888	
v/c Ratio, X	0.14		0.00					0.06			0.34	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	14.7		14.0					7.2			8.5	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	0.6		0.0					0.1			0.4	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	15.2		14.0					7.3			8.8	
Lane Group LOS	B		B					A			A	
Approach Delay	15.2						7.3			8.8		
Approach LOS	B						A			A		
Intersection Delay	9.3			$X_c = 0.26$			Intersection LOS			A		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Veterans Rd W Right Turn Bay			
Intersection Orientation: East-West				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		23					
Peak-Hour Factor, PHF	1.00	0.82	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	28	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	0	0	
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			129				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	165	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					165		
C (m) (veh/h)					1050		
v/c					0.16		
95% queue length					0.56		
Control Delay (s/veh)					9.1		
LOS					A		
Approach Delay (s/veh)	--	--	9.1				
Approach LOS	--	--	A				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	390	1		3			22	125	0	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		247	248		4				181		
Lane Group Capacity, c		776		600	576		463				2049		
v/c Ratio, X		0.01		0.41	0.43		0.01				0.09		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.3	12.4		10.2				10.6		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.0	2.3		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		14.4	14.7		10.3				10.7	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		14.6			10.3			10.7		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		13.5		$X_c = 0.26$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	AS				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY			
Date Performed	7/28/2011				Analysis Year	2011 Existing Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID 1121A - Charleston									
East/West Street: Sharrotts Road					North/South Street: Veterans Road West				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	52	20	65	65	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	10	62	26			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT		TR
PHF	0.80		0.79				0.81		0.81
Flow Rate (veh/h)	88		164				50		70
% Heavy Vehicles	24		5				3		3
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.5				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.5	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.2		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.08		0.15				0.04	0.06	
hd, final value (s)	4.60		4.46				4.59	4.27	
x, final value	0.11		0.20				0.06	0.08	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.6		2.5				2.6	2.3	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	338		414				300		320
Delay (s/veh)	8.18		8.59				7.90		7.65
LOS	A		A				A		A
Approach: Delay (s/veh)	8.18		8.59				7.76		
LOS	A		A				A		
Intersection Delay (s/veh)	8.22								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	AS	Intersection	Sharrotts Rd @ Veterans Rd E
Agency/Co.	Philip Habib & Associates	Jurisdiction	Staten Island, NY
Date Performed	7/28/2011	Analysis Year	2011 Existing Conditions
Analysis Time Period	Weekday 8-9 AM Peak Period		

Project ID 1121A - Charleston

East/West Street: Sharrotts Road

North/South Street: Veterans Road East

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	6	56	0	0	76	54
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	54	49	38	0	0	0
%Thrus Left Lane	50					

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.79		0.87	0.87		
Flow Rate (veh/h)	76		164		89	71		
% Heavy Vehicles	17		7		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.0		0.7	0.0		
Prop. Right-Turns	0.0		0.4		0.0	0.6		
Prop. Heavy Vehicle	0.2		0.1		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.3		-0.1		0.2	-0.3		

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.07		0.15		0.08	0.06		
hd, final value (s)	4.74		4.22		4.61	4.10		
x, final value	0.10		0.19		0.11	0.08		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.7		2.2		2.6	2.1		

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	326		414		339	321		
Delay (s/veh)	8.26		8.21		8.20	7.46		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.26		8.21		7.87			
LOS	A		A		A			
Intersection Delay (s/veh)	8.09							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	69	41	42	6	37	1	354	71	1			
% Heavy Vehicles, %HV	3	3	3	1	1	1	3	3	3			
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.76	0.76	0.76			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.998			0.978				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	21	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		139	27		56			560				
Lane Group Capacity, c		533	549		546			2253				
v/c Ratio, X		0.26	0.05		0.10			0.25				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		13.9	12.9		13.1			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		1.2	0.2		0.4			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		15.1	13.1		13.5			9.4			
Lane Group LOS		B	B		B			A			
Approach Delay		14.8			13.5			9.4			
Approach LOS		B			B			A			
Intersection Delay		10.8			$X_c = 0.25$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	24		17				15	319			29	439
% Heavy Vehicles, %HV	1		1				7	7			9	9
Peak-Hour Factor, PHF	0.74		0.74				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.997						0.936			0.808	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		55						375			544	
Lane Group Capacity, c		456						1014			970	
v/c Ratio, X		0.12						0.37			0.56	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		16.7						7.1			8.3	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.5						1.0			1.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		17.2					8.1			10.2	
Lane Group LOS		B					A			B	
Approach Delay	17.2					8.1		10.2			
Approach LOS	B					A		B			
Intersection Delay	9.8		$X_C = 0.42$			Intersection LOS		A			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	45		49				28	327			266	65
% Heavy Vehicles, %HV	14		14				8	8			11	11
Peak-Hour Factor, PHF	0.80		0.80				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.982						0.866			0.927	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	17
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		117						398			365	
Lane Group Capacity, c		500						811			943	
v/c Ratio, X		0.23						0.49			0.39	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.5						9.9			9.3	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.1						1.8			1.1	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		15.5					11.8			10.4	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		15.5					11.8			10.4	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		11.7			$X_C = 0.39$		Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		54	218	367	152	39	179			436	20
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		68		838		43	199			600	
Lane Group Capacity, c	218		216		1314		120	575			662	
v/c Ratio, X	0.02		0.31		0.64		0.36	0.35			0.91	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		23.5		18.8		15.1	15.1			19.1	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		3.7		1.7		7.7	1.6			6.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		27.2		20.5		22.9	16.6			25.8	
Lane Group LOS	C		C		C		C	B			C	
Approach Delay	27.0		20.5		17.7		25.8					
Approach LOS	C		C		B		C					
Intersection Delay	22.2		$X_c = 0.70$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	11	36	10				5	207	81	241	437	30
% Heavy Vehicles, %HV	3	3	3				7	7	7	7	7	7
Peak-Hour Factor, PHF	0.85	0.85	0.85				0.90	0.90	0.90	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.941		0.518	0.518	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7		0	2	
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		67						326		301	583	
Lane Group Capacity, c		457						899		565	960	
v/c Ratio, X		0.15						0.36		0.53	0.61	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.1						7.6		8.6	9.1	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.7						1.1		1.9	1.5	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.8					8.7		10.5	10.6	
Lane Group LOS	B					A		B	B	
Approach Delay	16.8				8.7			10.6		
Approach LOS	B				A			B		
Intersection Delay	10.4		$X_c = 0.45$		Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	7	19	12	160	15	97	9	154	122	129	266	8
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		56		182	17	110		317			531	
Lane Group Capacity, c		651		549	777	574		689			566	
v/c Ratio, X		0.09		0.33	0.02	0.19		0.46			0.94	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.4		16.3	14.0	15.2		17.5			23.8	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		1.6	0.1	0.7		1.9			3.8	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.7	17.9	14.1	15.9	19.3	27.6
Lane Group LOS	B	B	B	B	B	C
Approach Delay	14.7	16.9	19.3	27.6		
Approach LOS	B	B	B	C		
Intersection Delay	22.1	$X_c = 0.63$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		118	175	28	140		209		44				
% Heavy Vehicles, %HV		10	10	13	13		9		9				
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.964			0.986			0.858					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		270			188			269					
Lane Group Capacity, c		891			881			538					
v/c Ratio, X		0.30			0.21			0.50					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.3			12.6			20.9					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		0.8			0.5			2.8					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay		14.1			13.1			23.7			
Lane Group LOS		B			B			C			
Approach Delay	14.1		13.1			23.7					
Approach LOS	B		B			C					
Intersection Delay	17.4		$X_c = 0.39$			Intersection LOS			B		

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: Sharrotts Road				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	34	176	30	30	182	21	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	39	207	35	32	199	23	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	22	16	36	28	18	23	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	28	20	46	31	20	26	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11 12
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	39	32	77			94	
C (m) (veh/h)	1312	1236	439			514	
v/c	0.03	0.03	0.18			0.18	
95% queue length	0.09	0.08	0.63			0.66	
Control Delay (s/veh)	7.8	8.0	14.9			13.6	
LOS	A	A	B			B	
Approach Delay (s/veh)	--	--	14.9			13.6	
Approach LOS	--	--	B			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	7/28/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		226	17	18	228		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	265	19	19	250	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				41		14	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	43	0	14	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		19		57			
C (m) (veh/h)		1207		533			
v/c		0.02		0.11			
95% queue length		0.05		0.36			
Control Delay (s/veh)		8.0		12.6			
LOS		A		B			
Approach Delay (s/veh)	--	--	12.6				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	16	6	172	22	338	1	124	214	113	155	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		27			215	376		369			312	
Lane Group Capacity, c		713			527	518		709			575	
v/c Ratio, X		0.04			0.41	0.73		0.52			0.54	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			12.3	14.6		13.0			13.2	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			2.1	7.7		2.5			3.6	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.5			14.4	22.3		15.6			16.8	
Lane Group LOS		B			B	C		B			B	
Approach Delay		10.5			19.4			15.6			16.8	
Approach LOS		B			B			B			B	
Intersection Delay		17.5			$X_c = 0.63$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				325		3		336			333	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					372			365			387	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.58			0.38			0.41	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.5			9.2			9.4	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					3.2			1.0			0.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				19.7			10.2			10.2	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>B</i>	
Approach Delay				19.7	10.2			10.2			
Approach LOS				<i>B</i>	<i>B</i>			<i>B</i>			
Intersection Delay	13.3		$X_c = 0.48$		Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		336	213	131	527		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	365	231	152	612	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11 12
Lane Configuration		LT					
v (veh/h)		152					
C (m) (veh/h)		915					
v/c		0.17					
95% queue length		0.59					
Control Delay (s/veh)		9.7					
LOS		A					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	7/28/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				84		157		392	62	134	393	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					277			493			613	
Lane Group Capacity, c					386			1059			771	
v/c Ratio, X					0.72			0.47			0.80	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.0			7.7			10.3	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					10.3			1.3			4.3	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				30.3			8.9			14.6	
Lane Group LOS				C			A			B	
Approach Delay				30.3	8.9			14.6			
Approach LOS				C	A			B			
Intersection Delay	15.7		$X_c = 0.77$			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	91	41	64	35	115	33	73	523	126	85	415	53
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		228			199		80	713			606	
Lane Group Capacity, c		521			548		370	1050			1037	
v/c Ratio, X		0.44			0.36		0.22	0.68			0.58	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.8			22.1		10.6	14.8			13.7	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.7			1.9		1.0	2.6			2.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	24.5	24.0	11.6	17.4
Lane Group LOS	C	C	B	B
Approach Delay	24.5	24.0	16.8	16.0
Approach LOS	C	C	B	B
Intersection Delay	18.3	$X_c = 0.59$	Intersection LOS	B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					469		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	526	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			84				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	100	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							100
C (m) (veh/h)							715
v/c							0.14
95% queue length							0.48
Control Delay (s/veh)							10.9
LOS							B
Approach Delay (s/veh)	--	--				10.9	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	260							647			469	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	295							711			527	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.45							0.33			0.27	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.5							10.9			10.5	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.2							0.3			0.3	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	25.7						11.2			10.8
Lane Group LOS	C						B			B
Approach Delay	25.7						11.2		10.8	
Approach LOS	C						B		B	
Intersection Delay	13.8		$X_C = 0.38$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: KWVP WB Off-Ramp U-Turn				North/South Street: North Bridge Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					338		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	388	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				166			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	178	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			178				
C (m) (veh/h)			612				
v/c			0.29				
95% queue length			1.20				
Control Delay (s/veh)			13.3				
LOS			B				
Approach Delay (s/veh)	--	--	13.3				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Mohr St/KWVP					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	87	131	125	175	438	52	57	112	260	45	38	37
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	104	267		194	530			461		56	93	
Lane Group Capacity, c	221	588		271	1124			887		380	358	
v/c Ratio, X	0.47	0.45		0.72	0.47			0.52		0.15	0.26	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	23.7	23.6		26.3	23.7			27.4		28.1	28.9	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	6.0	2.2		14.0	1.3			2.2		0.8	1.7	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	29.8	25.7		40.3	25.0			29.6		28.9	30.6	
Lane Group LOS	C	C		D	C			C		C	C	
Approach Delay	26.9			29.1				29.6		30.0		
Approach LOS	C			C				C		C		
Intersection Delay	28.8			$X_c = 0.53$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	AS				Intersection	Mohr Street & Tyrellan Avenue		
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011				Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID 1121A - Charleston								
East/West Street: Mohr Street					North/South Street: Tyrellan Avenue			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	82	30	121	86	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	34	0	38	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.88	0.88	0.90	0.90	0.78	0.78		
Flow Rate (veh/h)	46	80	181	47	43	48		
% Heavy Vehicles	42	42	4	4	3	3		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.7	0.4	0.4	0.1	0.3	-0.5		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.04	0.07	0.16	0.04	0.04	0.04		
hd, final value (s)	5.64	5.34	5.28	4.90	5.02	4.22		
x, final value	0.07	0.12	0.27	0.06	0.06	0.06		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	3.3	3.0	3.0	2.6	3.0	2.2		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	296	330	431	297	293	298		
Delay (s/veh)	8.77	8.76	9.87	7.94	8.33	7.47		
LOS	A	A	A	A	A	A		
Approach: Delay (s/veh)	8.76		9.47		7.88			
LOS	A		A		A			
Intersection Delay (s/veh)	8.94							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	58	83	295	101	238	17	217	145	63	21	290	210
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889			0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		519			395		249	239			578	
Lane Group Capacity, c		1188			1052		321	809			1159	
v/c Ratio, X		0.44			0.38		0.78	0.30			0.50	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.2			16.7		21.2	16.0			17.8	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.0			1.0		16.6	0.9			1.5	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	18.3	17.6	37.8	16.9
Lane Group LOS	B	B	D	B
Approach Delay	18.3	17.6	27.6	19.4
Approach LOS	B	B	C	B
Intersection Delay	20.8	$X_c = 0.61$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	08/03/2010					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	583	321	3	1	350	333	0	0	1	69	0	119
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	641	356			386	273		4		71	0	123
Lane Group Capacity, c	793	1158			451	383		328		233	320	952
v/c Ratio, X	0.81	0.31			0.86	0.71		0.01		0.30	0.00	0.13
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	16.8	4.8			32.5	31.1		30.5		32.2	30.4	6.2
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	6.5	0.5			11.1	6.1		0.1		3.3	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	23.3	5.3			43.5	37.2		30.6		35.5	30.4	6.5
Lane Group LOS	C	A			D	D		C		D	C	A
Approach Delay	16.9		40.9		30.6		17.1					
Approach LOS	B		D		C		B					
Intersection Delay	25.5		$X_c = 0.77$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	356	35	0	2	60	66	4	3	0	60	6	620
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	391	38			122		7	5			74	344
Lane Group Capacity, c	713	1056			1538		518	732			542	593
v/c Ratio, X	0.55	0.04			0.08		0.01	0.01			0.14	0.58
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	15.5	11.5			11.7		16.9	16.9			17.7	21.7
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	2.9	0.1			0.1		0.0	0.0			0.5	3.7

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	18.4	11.5			11.8		16.9	16.9			18.2	25.4
Lane Group LOS	B	B			B		B	B			B	C
Approach Delay	17.8				11.8		16.9				24.1	
Approach LOS	B				B		B				C	
Intersection Delay	19.7				$X_c = 0.56$		Intersection LOS				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	Philip Habib & Associates						W					
Date Performed	8/03/2011					Area Type	All other areas					
Time Period	Weekday 12-1 PM MD Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	136		23				4	163			333	335
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	155		19					185			835	
Lane Group Capacity, c	537		465					1665			1920	
v/c Ratio, X	0.29		0.04					0.11			0.43	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.4		14.2					7.4			9.0	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.3		0.2					0.1			0.5	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.7		14.4					7.6			9.5	
Lane Group LOS	<i>B</i>		<i>B</i>					<i>A</i>			<i>A</i>	
Approach Delay	16.5						7.6			9.5		
Approach LOS	<i>B</i>						<i>A</i>			<i>A</i>		
Intersection Delay	10.2			$X_c = 0.38$			Intersection LOS			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Veterans Rd W Right Turn Bay			
Intersection Orientation: East-West				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	22						
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	39	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	0	0	
Configuration	T						
Upstream Signal	0			0			
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	298						
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	331	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage	0			0			
RT Channelized			0			0	
Lanes	0	0	1	0	0	0	
Configuration	R						
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					331		
C (m) (veh/h)					1035		
v/c					0.32		
95% queue length					1.39		
Control Delay (s/veh)					10.1		
LOS					B		
Approach Delay (s/veh)	--	--	10.1				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	523	1		1			21	145	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 25.0	G =	G =	G =	G =	G = 25.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		276	276		1				207		
Lane Group Capacity, c		784		612	587		451				2031		
v/c Ratio, X		0.01		0.45	0.47		0.00				0.10		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.6	12.7		10.2				10.7		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.4	2.6		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		14.9	15.3		10.2				10.8	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.1			10.2			10.8		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		13.9		$X_c = 0.29$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	AS				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY				
Date Performed	8/03/2011				Analysis Year	2011 Existing Conditions				
Analysis Time Period	Weekday 12-1 PM MD Peak Period									
Project ID 1121A - Charleston										
East/West Street: Sharrotts Road					North/South Street: Veterans Road West					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	47	29	64	50	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	29	73	19				
%Thrus Left Lane				50						
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT	TR		
PHF	0.86		0.77				0.80	0.80		
Flow Rate (veh/h)	87		147				80	69		
% Heavy Vehicles	17		4				1	1		
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.6				0.4	0.0		
Prop. Right-Turns	0.4		0.0				0.0	0.3		
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	0.1		0.2				0.1	-0.2		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20	3.20		
x, initial	0.08		0.13				0.07	0.06		
hd, final value (s)	4.46		4.51				4.56	4.26		
x, final value	0.11		0.18				0.10	0.08		
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.5		2.5				2.6	2.3		
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	337		397				330	319		
Delay (s/veh)	8.00		8.53				8.07	7.64		
LOS	A		A				A	A		
Approach: Delay (s/veh)	8.00		8.53				7.87			
LOS	A		A				A			
Intersection Delay (s/veh)	8.15									
Intersection LOS	A									

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	AS	Intersection	Sharrotts Rd @ Veterans Rd E
Agency/Co.	Philip Habib & Associates	Jurisdiction	Staten Island, NY
Date Performed	8/03/2011	Analysis Year	2011 Existing Conditions
Analysis Time Period	Weekday 12-1 PM MD Peak Period		

Project ID 1121A - Charleston

East/West Street: Sharrotts Road

North/South Street: Veterans Road East

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	37	39	0	0	75	21
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	39	59	63	0	0	0
%Thrus Left Lane	50					

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.86		0.77		0.88	0.88		
Flow Rate (veh/h)	88		124		76	105		
% Heavy Vehicles	11		5		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.5		0.0		0.6	0.0		
Prop. Right-Turns	0.0		0.2		0.0	0.7		
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.3		-0.0		0.1	-0.4		

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.08		0.11		0.07	0.09		
hd, final value (s)	4.71		4.34		4.53	4.01		
x, final value	0.12		0.15		0.10	0.12		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.7		2.3		2.5	2.0		

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	338		374		326	355		
Delay (s/veh)	8.32		8.11		8.01	7.54		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.32		8.11		7.74			
LOS	A		A		A			
Intersection Delay (s/veh)	7.98							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	120	95	105	2	36	5	488	36	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		236	58		45			542				
Lane Group Capacity, c		546	560		531			2287				
v/c Ratio, X		0.43	0.10		0.08			0.24				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		14.9	13.2		13.1			9.0				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		2.5	0.4		0.3			0.2				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		17.4	13.5		13.4			9.3			
Lane Group LOS		B	B		B			A			
Approach Delay		16.6			13.4			9.3			
Approach LOS		B			B			A			
Intersection Delay		12.0			$X_c = 0.32$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	60		37				13	260			306	30
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		120						314			369	
Lane Group Capacity, c		458						1094			1172	
v/c Ratio, X		0.26						0.29			0.31	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.3						6.7			6.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.4						0.6			0.7	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		18.7					7.4			7.5	
Lane Group LOS		B					A			A	
Approach Delay		18.7					7.4			7.5	
Approach LOS		B					A			A	
Intersection Delay		9.1			$X_c = 0.30$		Intersection LOS			A	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	52		50				41	279			257	55
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		118						368			327	
Lane Group Capacity, c		550						849			1038	
v/c Ratio, X		0.21						0.43			0.32	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.4						9.6			8.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.9						1.5			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		15.2					11.0			9.7	
Lane Group LOS		B					B			A	
Approach Delay	15.2						11.0			9.7	
Approach LOS	B						B			A	
Intersection Delay	11.1			$X_c = 0.35$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	9		98	186	437	118	59	146			313	30
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	12		127		852		66	164			381	
Lane Group Capacity, c	222		220		1340		190	593			690	
v/c Ratio, X	0.05		0.58		0.64		0.35	0.28			0.55	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		24.4		18.8		15.1	14.7			16.3	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		8.9		1.7		4.7	1.1			2.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		33.3		20.5		19.7	15.8			18.9	
Lane Group LOS	C		C		C		B	B			B	
Approach Delay	32.4		20.5		16.9		18.9					
Approach LOS	C		C		B		B					
Intersection Delay	20.6		$X_c = 0.59$		Intersection LOS		C					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	5	14	7				6	200	105	213	358	26	
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1	
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.932		0.785	0.785		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						2		0	2		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		41						350		237	427		
Lane Group Capacity, c		456						919		579	1016		
v/c Ratio, X		0.09						0.38		0.41	0.42		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		15.8						7.7		7.8	7.9		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.4						1.1		1.7	1.0		

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.2						8.8		9.5	8.9	
Lane Group LOS		B						A		A	A	
Approach Delay		16.2						8.8		9.1		
Approach LOS		B						A		A		
Intersection Delay		9.3		$X_c = 0.31$			Intersection LOS		A			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	7	16	247	24	98	20	208	159	84	263	18
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		36		294	29	117		435			405	
Lane Group Capacity, c		627		548	763	563		689			644	
v/c Ratio, X		0.06		0.54	0.04	0.21		0.63			0.63	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.2	14.1	15.3		19.3			19.3	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.1	0.1	0.7		2.9			3.0	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.4	21.3	14.2	16.0	22.2	22.3
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.4	19.5	22.2	22.3		
Approach LOS	B	B	C	C		
Intersection Delay	21.1	$X_c = 0.58$	Intersection LOS	C		

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	AS					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	Philip Habib & Associates					Area Type	All other areas								
Date Performed	8/03/2011					Jurisdiction	Staten Island, NY								
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2011 Existing Conditions								
						Project ID	1121A - Charleston								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		125	175	16	141		178		26						
% Heavy Vehicles, %HV		8	8	16	16		2		2						
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.961			0.989			0.922							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		5			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		285			173			232							
Lane Group Capacity, c		910			889			577							
v/c Ratio, X		0.31			0.19			0.40							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		13.3			12.5			19.9							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		0.9			0.5			1.9							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay		14.2			12.9			21.8			
Lane Group LOS		B			B			C			
Approach Delay	14.2		12.9			21.8					
Approach LOS	B		B			C					
Intersection Delay	16.5		$X_c = 0.35$			Intersection LOS			B		

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description 1121A - Charleston							
East/West Street: Sharrotts Road				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	27	181	60	54	178	26	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	31	208	68	66	219	32	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	32	17	27	49	16	32	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	37	19	31	60	19	39	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	31	66		118			87
C (m) (veh/h)	1291	1248		392			397
v/c	0.02	0.05		0.30			0.22
95% queue length	0.07	0.17		1.25			0.82
Control Delay (s/veh)	7.9	8.0		18.1			16.6
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		18.1			16.6
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		243	17	13	241		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	279	19	16	297	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				27		25	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	39	0	36	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		16		75			
C (m) (veh/h)		1235		545			
v/c		0.01		0.14			
95% queue length		0.04		0.47			
Control Delay (s/veh)		8.0		12.7			
LOS		A		B			
Approach Delay (s/veh)	--	--	12.7				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	10	2	211	19	218	4	122	262	160	217	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		14			280	266		417			439	
Lane Group Capacity, c		726			535	523		742			516	
v/c Ratio, X		0.02			0.52	0.51		0.56			0.85	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			13.1	13.0		13.3			15.8	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.0			3.4	3.3		2.8			16.1	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			16.4	16.2		16.1			31.9	
Lane Group LOS		B			B	B		B			C	
Approach Delay		10.3			16.3			16.1			31.9	
Approach LOS		B			B			B			C	
Intersection Delay		21.0			$X_c = 0.69$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				483		4		384			430	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					580			413			500	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.87			0.40			0.52	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					18.7			9.4			10.2	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					5.9			1.1			0.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				24.6			10.5			10.9		
Lane Group LOS				C			B			B		
Approach Delay				24.6				10.5				10.9
Approach LOS				C				B				B
Intersection Delay	16.1			$X_c = 0.66$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		384	232	175	738		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	412	249	203	858	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		203					
C (m) (veh/h)		856					
v/c		0.24					
95% queue length		0.92					
Control Delay (s/veh)		10.5					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				116		126		490	88	97	641	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					288			622			858	
Lane Group Capacity, c					398			1098			870	
v/c Ratio, X					0.72			0.57			0.99	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.0			8.3			12.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					9.9			1.7			7.6	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				29.9			10.0			20.4	
Lane Group LOS				C			A			C	
Approach Delay				29.9				10.0			
Approach LOS				C				A			
Intersection Delay	18.3		$X_c = 0.90$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	61	30	94	32	155	32	50	512	100	96	488	37
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		210			271		55	673			702	
Lane Group Capacity, c		555			568		327	1065			1085	
v/c Ratio, X		0.38			0.48		0.17	0.63			0.65	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.2			23.1		10.3	14.2			14.4	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.0			2.9		0.9	2.2			2.9	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	23.2	26.0	11.1	16.5
Lane Group LOS	C	C	B	B
Approach Delay	23.2	26.0	16.0	17.3
Approach LOS	C	C	B	B
Intersection Delay	18.7	$X_c = 0.58$	Intersection LOS	B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					555		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	637	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			66				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	84	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							84
C (m) (veh/h)							615
v/c							0.14
95% queue length							0.47
Control Delay (s/veh)							11.8
LOS							B
Approach Delay (s/veh)	--	--				11.8	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	341							605			555	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	375							672			638	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.56							0.31			0.32	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.5							10.8			10.8	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	3.3							0.3			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	27.8						11.0			11.2	
Lane Group LOS	C						B			B	
Approach Delay	27.8						11.0		11.2		
Approach LOS	C						B		B		
Intersection Delay	14.8		$X_C = 0.41$		Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description 1121A - Charleston							
East/West Street: KWVP WB Off-Ramp U-Turn				North/South Street: North Bridge Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					445		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	500	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				432			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	464	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			464				
C (m) (veh/h)			529				
v/c			0.88				
95% queue length			9.74				
Control Delay (s/veh)			42.9				
LOS			E				
Approach Delay (s/veh)	--	--	42.9				
Approach LOS	--	--	E				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Mohr St/KWVP					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Sat Flow Rate WB-L = 2000					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	101	152	179	225	300	41	96	86	211	43	41	52
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	115	325		250	403			395		57	90	
Lane Group Capacity, c	283	583		246	1148			905		384	367	
v/c Ratio, X	0.41	0.56		1.02	0.35			0.44		0.15	0.25	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	23.1	24.6		29.5	22.0			26.7		28.2	28.8	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	3.0	2.7		60.1	0.8			1.5		0.8	1.5	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	26.2	27.3		89.6	22.8			28.2		28.9	30.3	
Lane Group LOS	C	C		F	C			C		C	C	
Approach Delay	27.0			48.4			28.2			29.8		
Approach LOS	C			D			C			C		
Intersection Delay	36.1			$X_c = 0.62$			Intersection LOS			D		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	AS				Intersection	Mohr Street & Tyrellan Avenue			
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY			
Date Performed	8/03/2011				Analysis Year	2011 Existing Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID 1121A - Charleston									
East/West Street: Mohr Street					North/South Street: Tyrellan Avenue				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	0	94	27	137	124	0			
%Thrus Left Lane	50			50					
Approach	Northbound				Southbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	12	0	64	0	0	0			
%Thrus Left Lane									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	T	TR	LT	T	L	R			
PHF	0.87	0.87	0.91	0.91	0.76	0.76			
Flow Rate (veh/h)	54	85	218	68	15	84			
% Heavy Vehicles	7	7	1	1	1	1			
No. Lanes	2		2		2		0		
Geometry Group	5		5		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0			
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0			
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0			
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2			
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.1	-0.1	0.4	0.0	0.2	-0.6			
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20			
x, initial	0.05	0.08	0.19	0.06	0.01	0.07			
hd, final value (s)	5.10	4.85	5.21	4.87	5.10	4.30			
x, final value	0.08	0.11	0.32	0.09	0.02	0.10			
Move-up time, m (s)	2.3		2.3		2.0				
Service Time, t _s (s)	2.8	2.5	2.9	2.6	3.1	2.3			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	304	335	468	318	265	334			
Delay (s/veh)	8.23	8.17	10.30	8.06	8.21	7.78			
LOS	A	A	B	A	A	A			
Approach: Delay (s/veh)	8.20		9.76		7.84				
LOS	A		A		A				
Intersection Delay (s/veh)	8.98								
Intersection LOS	A								

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Veterans Rd W & Tyrellan Ave						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0	
Lane Group		LTR			LTR		DefL	TR			LTR		
Volume, V (vph)	49	94	263	92	257	9	194	95	95	20	292	115	
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2	
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.884			0.927		1.000	1.000			1.000		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0			12.0		13.0	13.0			9.5		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		462			436		209	204			454		
Lane Group Capacity, c		1207			1121		396	799			1300		
v/c Ratio, X		0.38			0.39		0.53	0.26			0.35		
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44		
Uniform Delay, d ₁		16.7			16.8		18.1	15.7			16.4		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		0.8			0.9		5.0	0.8			0.7		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	17.6	17.7	23.1	16.4	17.2
Lane Group LOS	B	B	C	B	B
Approach Delay	17.6	17.7	19.8	17.2	
Approach LOS	B	B	B	B	
Intersection Delay	18.0	$X_c = 0.46$	Intersection LOS	B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	08/04/2010					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	659	283	4	3	293	329	2	3	2	90	0	260
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	732	318			311	346		17		112	0	325
Lane Group Capacity, c	865	1205			584	497		148		204	283	862
v/c Ratio, X	0.85	0.26			0.53	0.70		0.11		0.55	0.00	0.38
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	10.6	4.0			25.6	27.3		32.7		35.1	32.1	10.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	7.1	0.4			2.3	5.2		1.6		10.2	0.0	1.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	17.8	4.3			27.9	32.5		34.2		45.3	32.1	11.5
Lane Group LOS	B	A			C	C		C		D	C	B
Approach Delay	13.7			30.3			34.2			20.2		
Approach LOS	B			C			C			C		
Intersection Delay	20.2			$X_c = 0.80$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	341	34	0	1	34	40	3	3	2	58	1	588
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	379	38			69			16			63	313
Lane Group Capacity, c	757	1066			1563			1220			552	616
v/c Ratio, X	0.50	0.04			0.04			0.01			0.11	0.51
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.0	11.5			11.5			16.9			17.6	20.9
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	2.2	0.1			0.1			0.0			0.4	2.8

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	17.3	11.5			11.6			16.9			18.0	23.7
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	16.7			11.6			16.9			22.8		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	18.9			$X_c = 0.50$			Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	136		22				2	207			336	259
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	156		18					237			654	
Lane Group Capacity, c	547		473					1732			1941	
v/c Ratio, X	0.29		0.04					0.14			0.34	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.4		14.2					7.5			8.5	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.3		0.1					0.2			0.4	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.7		14.3					7.7			8.9	
Lane Group LOS	<i>B</i>		<i>B</i>					<i>A</i>			<i>A</i>	
Approach Delay	16.4						7.7			8.9		
Approach LOS	<i>B</i>						<i>A</i>			<i>A</i>		
Intersection Delay	9.8			$X_c = 0.32$			Intersection LOS			<i>A</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/03/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period							
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Veterans Rd W Right Turn Bay			
Intersection Orientation: East-West				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		22					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	30	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			329				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	373	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					373		
C (m) (veh/h)					1047		
v/c					0.36		
95% queue length					1.63		
Control Delay (s/veh)					10.3		
LOS					B		
Approach Delay (s/veh)	--	--	10.3				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	428	2		4			21	167	4	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 25.0	G =	G =	G =	G =	G = 25.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		241	242		5				211		
Lane Group Capacity, c		784		600	576		449				2047		
v/c Ratio, X		0.01		0.40	0.42		0.01				0.10		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.3	12.4		10.3				10.7		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.0	2.2		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		14.2	14.6		10.3				10.8	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		14.4			10.3			10.8		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		13.3		$X_c = 0.26$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	AS				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY				
Date Performed	8/03/2011				Analysis Year	2011 Existing Conditions				
Analysis Time Period	Weekday 5-6 PM Peak Period									
Project ID 1121A - Charleston										
East/West Street: Sharrotts Road					North/South Street: Veterans Road West					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	95	36	73	74	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	22	83	23				
%Thrus Left Lane				50						
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT		TR	
PHF	0.80		0.78				0.91		0.91	
Flow Rate (veh/h)	162		187				69		71	
% Heavy Vehicles	2		3				1		1	
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.5				0.3	0.0		
Prop. Right-Turns	0.3		0.0				0.0	0.4		
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	-0.1		0.2				0.1	-0.2		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20	3.20		
x, initial	0.14		0.17				0.06	0.06		
hd, final value (s)	4.31		4.56				4.79	4.50		
x, final value	0.19		0.24				0.09	0.09		
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.3		2.6				2.8	2.5		
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	412		437				319		321	
Delay (s/veh)	8.35		8.96				8.27		7.94	
LOS	A		A				A		A	
Approach: Delay (s/veh)	8.35		8.96				8.10			
LOS	A		A				A			
Intersection Delay (s/veh)	8.51									
Intersection LOS	A									

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	AS	Intersection	Sharrotts Rd @ Veterans Rd E
Agency/Co.	Philip Habib & Associates	Jurisdiction	Staten Island, NY
Date Performed	8/03/2011	Analysis Year	2011 Existing Conditions
Analysis Time Period	Weekday 5-6 PM Peak Period		

Project ID 1121A - Charleston

East/West Street: Sharrotts Road

North/South Street: Veterans Road East

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	36	81	0	0	86	18
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	61	93	111	0	0	0
%Thrus Left Lane	50					

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.78		0.99	0.99		
Flow Rate (veh/h)	145		133		107	159		
% Heavy Vehicles	2		5		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.3		0.0		0.6	0.0		
Prop. Right-Turns	0.0		0.2		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		-0.0		0.1	-0.4		

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.13		0.12		0.10	0.14		
hd, final value (s)	4.73		4.63		4.71	4.17		
x, final value	0.19		0.17		0.14	0.18		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.7		2.6		2.7	2.2		

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	395		383		357	409		
Delay (s/veh)	8.84		8.59		8.47	8.10		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.84		8.59		8.25			
LOS	A		A		A			
Intersection Delay (s/veh)	8.49							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	159	95	107	7	44	8	386	98	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		299	64		68			546				
Lane Group Capacity, c		524	560		534			2302				
v/c Ratio, X		0.57	0.11		0.13			0.24				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		15.8	13.2		13.3			9.0				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		4.5	0.4		0.5			0.2				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		20.3	13.6		13.8			9.3			
Lane Group LOS		C	B		B			A			
Approach Delay		19.1			13.8			9.3			
Approach LOS		B			B			A			
Intersection Delay		13.2			$X_c = 0.38$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	65		32				17	426			427	42
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		101						515			510	
Lane Group Capacity, c		451						1100			1127	
v/c Ratio, X		0.22						0.47			0.45	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.2						7.7			7.6	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.1						1.3			1.2	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	18.3				8.9		8.7
Lane Group LOS	B				A		A
Approach Delay	18.3				8.9		8.7
Approach LOS	B				A		A
Intersection Delay	9.7		$X_c = 0.39$		Intersection LOS		A

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	112		80				21	388			366	83
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		240						475			465	
Lane Group Capacity, c		570						896			997	
v/c Ratio, X		0.42						0.53			0.47	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.5						10.2			9.8	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.1						1.9			1.4	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		17.6					12.1			11.2	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		17.6					12.1			11.2	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		12.8		$X_C = 0.49$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		110	242	432	256	49	183			454	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		116		1057		53	199			528	
Lane Group Capacity, c	222		220		1328		124	616			670	
v/c Ratio, X	0.02		0.53		0.80		0.43	0.32			0.79	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		24.2		19.9		15.5	14.9			18.1	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		7.5		2.6		9.7	1.3			8.0	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		31.7		22.5		25.3	16.2			26.1	
Lane Group LOS	C		C		C		C	B			C	
Approach Delay	31.4		22.5		18.1		26.1					
Approach LOS	C		C		B		C					
Intersection Delay	23.4		$X_c = 0.74$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	9	19	6				8	223	113	296	462	48
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.922		0.564	0.564	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	7	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		54						374		318	536	
Lane Group Capacity, c		459						952		556	985	
v/c Ratio, X		0.12						0.39		0.57	0.54	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		15.9						7.7		8.9	8.7	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.5						1.1		2.4	1.2	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0
Control Delay	16.5					8.9		11.3	9.9
Lane Group LOS	B					A		B	A
Approach Delay	16.5						8.9		10.4
Approach LOS	B						A		B
Intersection Delay	10.2		$X_c = 0.42$		Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	12	19	248	27	102	17	236	162	87	347	48
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		53		285	31	117		451			554	
Lane Group Capacity, c		633		561	792	585		728			656	
v/c Ratio, X		0.08		0.51	0.04	0.20		0.62			0.84	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.4		17.9	14.1	15.2		19.2			22.2	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.0	0.1	0.7		2.6			3.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.7	20.9	14.2	15.9	21.8	25.5	
Lane Group LOS	B	C	B	B	C	C	
Approach Delay	14.7	19.1	21.8	25.5			
Approach LOS	B	B	C	C			
Intersection Delay	22.1	$X_c = 0.68$	Intersection LOS	C			

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	AS					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	Philip Habib & Associates					Area Type	All other areas								
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY								
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2011 Existing Conditions								
						Project ID	1121A - Charleston								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		119	235	19	131		134		27						
% Heavy Vehicles, %HV		5	5	8	8		1		1						
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.927			0.989			0.917							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		5			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		360			181			240							
Lane Group Capacity, c		924			934			581							
v/c Ratio, X		0.39			0.19			0.41							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		14.0			12.5			20.0							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		1.1			0.5			2.0							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay	15.1	12.9	22.0	
Lane Group LOS	B	B	C	
Approach Delay	15.1	12.9	22.0	
Approach LOS	B	B	C	
Intersection Delay	16.7	$X_c = 0.40$	Intersection LOS	B

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description 1121A - Charleston							
East/West Street: Sharrotts Road				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	11	176	42	27	194	26	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	11	191	45	30	220	29	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	25	31	22	51	16	38	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	62	77	54	65	20	48	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	11	30		133			193
C (m) (veh/h)	1311	1314		439			475
v/c	0.01	0.02		0.30			0.41
95% queue length	0.03	0.07		1.26			1.95
Control Delay (s/veh)	7.8	7.8		16.7			17.7
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		16.7			17.7
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		211	12	15	252		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	229	13	17	286	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				9		18	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	12	0	24	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		17		36			
C (m) (veh/h)		1307		657			
v/c		0.01		0.05			
95% queue length		0.04		0.17			
Control Delay (s/veh)		7.8		10.8			
LOS		A		B			
Approach Delay (s/veh)	--	--	10.8				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	9	5	246	25	320	5	145	313	126	183	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		15			294	348		508			339	
Lane Group Capacity, c		724			551	539		745			512	
v/c Ratio, X		0.02			0.53	0.65		0.68			0.66	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			13.1	14.0		14.3			14.1	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			3.1	5.0		4.3			6.6	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			16.3	19.0		18.6			20.7	
Lane Group LOS		B			B	B		B			C	
Approach Delay		10.3			17.7			18.6			20.7	
Approach LOS		B			B			B			C	
Intersection Delay		18.6			$X_c = 0.66$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				461		8		455			434	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					545			500			477	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.81			0.49			0.46	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					18.2			9.9			9.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					5.1			1.4			0.6	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				23.3			11.3			10.3	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				23.3	11.3			10.3			
Approach LOS				<i>C</i>	<i>B</i>			<i>B</i>			
Intersection Delay	15.3			$X_c = 0.62$	Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description 1121A - Charleston							
East/West Street: South Bridge Street				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		455	234	168	727		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	499	257	184	798	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		184					
C (m) (veh/h)		837					
v/c		0.22					
95% queue length		0.84					
Control Delay (s/veh)		10.5					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				89		166		523	74	105	622	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					287			656			799	
Lane Group Capacity, c					394			1133			866	
v/c Ratio, X					0.73			0.58			0.92	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.0			8.4			11.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					10.6			1.7			5.8	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				30.6			10.1			17.6
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>
Approach Delay				30.6	10.1			17.6		
Approach LOS				<i>C</i>	<i>B</i>			<i>B</i>		
Intersection Delay	16.9		$X_c = 0.86$		Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	65	31	83	28	53	21	122	670	133	14	530	80
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		229			140		133	873			656	
Lane Group Capacity, c		585			532		348	1065			1483	
v/c Ratio, X		0.39			0.26		0.38	0.82			0.44	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.4			21.3		11.8	16.9			12.3	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.1			1.2		1.8	4.1			0.9	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	23.4	22.5	13.6	21.0	13.2
Lane Group LOS	C	C	B	C	B
Approach Delay	23.4	22.5	20.0		13.2
Approach LOS	C	C	B		B
Intersection Delay	18.4	$X_c = 0.65$	Intersection LOS		B

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	AS			Intersection	S Bridge St RT & Page Ave			
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY			
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project Description 1121A - Charleston								
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)					571			
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	620	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0				0	
Lanes	0	0	0	0	2	0		
Configuration					T			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			53					
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	60	0	0	0		
Percent Heavy Vehicles	0	0	5	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (veh/h)								60
C (m) (veh/h)								723
v/c								0.08
95% queue length								0.27
Control Delay (s/veh)								10.4
LOS								B
Approach Delay (s/veh)	--	--				10.4		
Approach LOS	--	--				B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	349							756			571	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	406							788			621	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.61							0.37			0.32	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.1							11.2			10.8	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	4.1							0.3			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	29.1						11.5			11.2	
Lane Group LOS	C						B			B	
Approach Delay	29.1						11.5			11.2	
Approach LOS	C						B			B	
Intersection Delay	15.3			$X_C = 0.46$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description 1121A - Charleston							
East/West Street: KWVP WB Off-Ramp U-Turn				North/South Street: North Bridge Street			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					528		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	567	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				207			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	230	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			230				
C (m) (veh/h)			487				
v/c			0.47				
95% queue length			2.49				
Control Delay (s/veh)			18.8				
LOS			C				
Approach Delay (s/veh)	--	--	18.8				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Mohr St/KWVP					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Sat Flow Rate WB-L =2050					
Time Period	Saturday 12:45-1:45 PM MD Peak					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	92	166	190	246	404	44	101	176	221	41	92	86
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.94	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	99	331		262	560			554		49	212	
Lane Group Capacity, c	210	584		249	1196			935		384	362	
v/c Ratio, X	0.47	0.57		1.05	0.47			0.59		0.13	0.59	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	23.7	24.7		29.5	23.1			28.1		28.0	31.3	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	5.2	2.8		68.6	1.2			2.8		0.6	5.8	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	29.0	27.4		98.1	24.2			30.9		28.6	37.1	
Lane Group LOS	C	C		F	C			C		C	D	
Approach Delay	27.8			47.8				30.9		35.5		
Approach LOS	C			D				C		D		
Intersection Delay	37.5			$X_c = 0.78$				Intersection LOS		D		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	AS			Intersection	Mohr Street & Tyrellan Avenue			
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY			
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID 1121A - Charleston								
East/West Street: Mohr Street				North/South Street: Tyrellan Avenue				
Volume Adjustments and Site Characteristics								
Approach	Eastbound			Westbound				
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	185	31	103	166	0		
%Thrus Left Lane	50			50				
Approach	Northbound			Southbound				
Movement	L	T	R	L	T	R		
Volume (veh/h)	53	0	88	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.85	0.85	0.91	0.91	0.89	0.89		
Flow Rate (veh/h)	108	145	204	91	59	98		
% Heavy Vehicles	0	0	0	1	1	1		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.6	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.2	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.0	-0.2	0.3	0.0	0.2	-0.6		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.10	0.13	0.18	0.08	0.05	0.09		
hd, final value (s)	5.19	5.02	5.42	5.16	5.36	4.56		
x, final value	0.16	0.20	0.31	0.13	0.09	0.12		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	2.9	2.7	3.1	2.9	3.4	2.6		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	358	395	454	341	309	348		
Delay (s/veh)	8.85	8.99	10.50	8.63	8.88	8.21		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	8.93		9.92		8.46			
LOS	A		A		A			
Intersection Delay (s/veh)	9.24							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	76	98	254	120	318	23	220	192	80	24	501	156
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.830			0.865		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		466			591		234	299			733	
Lane Group Capacity, c		1069			1090		273	826			1332	
v/c Ratio, X		0.44			0.54		0.86	0.36			0.55	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.2			18.3		22.4	16.6			18.4	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.1			1.7		27.7	1.2			1.6	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	18.3	20.0	50.1	17.8	20.0
Lane Group LOS	B	B	D	B	C
Approach Delay	18.3	20.0	32.0	20.0	
Approach LOS	B	B	C	C	
Intersection Delay	22.4	$X_c = 0.70$	Intersection LOS	C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	08/04/2010					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	776	329			378	462				152		193
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	800	343			402	354				165		210
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	0.89	0.29			0.87	0.91				0.29		0.22
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	18.9	4.7			32.7	33.0				32.1		6.6
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	7.4	0.3			4.4	6.7				1.2		0.5
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

Control Delay	26.3	5.1			37.1	39.7				33.3		7.1
Lane Group LOS	C	A			D	D				C		A
Approach Delay	19.9			38.3						18.7		
Approach LOS	B			D						B		
Intersection Delay	25.8			$X_c = 0.82$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	438	43	0	0	43	54	0	0	1	78	0	797
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	456	45			91			1			84	429
Lane Group Capacity, c	741	1066			1634			1184			548	622
v/c Ratio, X	0.62	0.04			0.06			0.00			0.15	0.69
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	16.3	11.5			11.6			16.8			17.9	23.0
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	3.7	0.1			0.1			0.0			0.5	5.3

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	19.9	11.6			11.6			16.8			18.4	28.3
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	19.2			11.6			16.8			26.6		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	22.0			$X_c = 0.65$			Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	Philip Habib & Associates						W					
Date Performed	8/04/2011					Area Type	All other areas					
Time Period	Saturday 12:45-1:45 PM					Jurisdiction	Staten Island, NY					
	Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	240		33				5	197			428	443
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	282		28					230			979	
Lane Group Capacity, c	573		497					1699			1937	
v/c Ratio, X	0.49		0.06					0.14			0.51	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	16.6		14.3					7.5			9.5	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	2.7		0.2					0.2			0.5	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	19.3		14.5					7.7			9.9	
Lane Group LOS	B		B					A			A	
Approach Delay	18.8						7.7			9.9		
Approach LOS	B						A			A		
Intersection Delay	11.4			$X_c = 0.50$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	AS			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	Philip Habib & Associates			Jurisdiction	Staten Island, NY		
Date Performed	8/04/2011			Analysis Year	2011 Existing Conditions		
Analysis Time Period	Saturday 12-1						
Project Description 1121A - Charleston							
East/West Street: Englewood Avenue				North/South Street: Veterans Rd W Right Turn Bay			
Intersection Orientation: East-West				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		27					
Peak-Hour Factor, PHF	1.00	0.69	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	39	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			432				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	490	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					490		
C (m) (veh/h)					1038		
v/c					0.47		
95% queue length					2.58		
Control Delay (s/veh)					11.5		
LOS					B		
Approach Delay (s/veh)	--	--	11.5				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	658	2		5			26	213	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		542	183		6				269		
Lane Group Capacity, c		784		606	583		428				2032		
v/c Ratio, X		0.01		0.89	0.31		0.01				0.13		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		16.3	11.7		10.3				10.8		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		17.8	1.4		0.1				0.1		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	34.0	13.1	10.3	10.9
Lane Group LOS	B	C	B	B	B
Approach Delay	10.2	28.8	10.3	10.9	
Approach LOS	B	C	B	B	
Intersection Delay	23.8	$X_c = 0.51$	Intersection LOS	C	

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	AS				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	Philip Habib & Associates				Jurisdiction	Staten Island, NY				
Date Performed	8/04/2011				Analysis Year	2011 Existing Conditions				
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd									
Project ID 1121A - Charleston										
East/West Street: Sharrotts Road					North/South Street: Veterans Road West					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	67	33	96	49	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	16	111	21				
%Thrus Left Lane				50						
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT		TR	
PHF	0.75		0.85				0.89		0.89	
Flow Rate (veh/h)	133		169				78		85	
% Heavy Vehicles	2		4				0		0	
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.7				0.2	0.0		
Prop. Right-Turns	0.3		0.0				0.0	0.3		
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	-0.2		0.2				0.0	-0.2		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20	3.20		
x, initial	0.12		0.15				0.07	0.08		
hd, final value (s)	4.31		4.62				4.64	4.44		
x, final value	0.16		0.22				0.10	0.10		
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.3		2.6				2.6	2.4		
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	383		419				328		335	
Delay (s/veh)	8.12		8.89				8.16		7.96	
LOS	A		A				A		A	
Approach: Delay (s/veh)	8.12		8.89				8.05			
LOS	A		A				A			
Intersection Delay (s/veh)	8.38									
Intersection LOS	A									

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	AS	Intersection	Sharrotts Rd @ Veterans Rd E
Agency/Co.	Philip Habib & Associates	Jurisdiction	Staten Island, NY
Date Performed	8/04/2011	Analysis Year	2011 Existing Conditions
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd		

Project ID 1121A - Charleston

East/West Street: Sharrotts Road

North/South Street: Veterans Road East

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	24	59	0	0	96	22
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	49	99	102	0	0	0
%Thrus Left Lane	50					

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.75		0.85		0.95	0.95		
Flow Rate (veh/h)	110		137		102	159		
% Heavy Vehicles	2		2		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.3		0.0		0.5	0.0		
Prop. Right-Turns	0.0		0.2		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		-0.1		0.1	-0.4		

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.10		0.12		0.09	0.14		
hd, final value (s)	4.71		4.51		4.61	4.10		
x, final value	0.14		0.17		0.13	0.18		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.7		2.5		2.6	2.1		

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	360		387		352	409		
Delay (s/veh)	8.50		8.45		8.30	8.01		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.50		8.45		8.12			
LOS	A		A		A			
Intersection Delay (s/veh)	8.29							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	175	152	132	0	59	14	601	61	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		436	88		88			715				
Lane Group Capacity, c		534	560		557			2267				
v/c Ratio, X		0.82	0.16		0.16			0.32				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		17.7	13.4		13.4			9.4				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		12.9	0.6		0.6			0.4				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		30.7	14.0		14.0			9.8			
Lane Group LOS		C	B		B			A			
Approach Delay		27.9			14.0			9.8			
Approach LOS		C			B			A			
Intersection Delay		17.2			$X_c = 0.53$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	103		52				16	366			344	57
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		169						415			441	
Lane Group Capacity, c		459						1113			1185	
v/c Ratio, X		0.37						0.37			0.37	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.9						7.1			7.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.1						0.9			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		20.0					8.0			8.0	
Lane Group LOS		C					A			A	
Approach Delay		20.0					8.0			8.0	
Approach LOS		C					A			A	
Intersection Delay		10.0			$X_c = 0.37$		Intersection LOS			A	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	99		62				40	369			348	78
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		215						444			446	
Lane Group Capacity, c		572						869			1046	
v/c Ratio, X		0.38						0.51			0.43	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.2						10.1			9.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.8						1.8			1.2	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		17.0					11.9			10.7	
Lane Group LOS		B					B			B	
Approach Delay	17.0					11.9		10.7			
Approach LOS	B					B		B			
Intersection Delay	12.4		$X_C = 0.46$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	18		114	258	573	161	76	203			380	16
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	25		161		1141		83	221			440	
Lane Group Capacity, c	222		220		1338		163	621			707	
v/c Ratio, X	0.11		0.73		0.85		0.51	0.36			0.62	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		25.0		20.3		16.1	15.1			16.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.7		14.3		3.6		10.0	1.5			3.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.6		39.3		23.9		26.1	16.6			19.9	
Lane Group LOS	<i>C</i>		<i>D</i>		<i>C</i>		<i>C</i>	<i>B</i>			<i>B</i>	
Approach Delay	37.2		23.9		19.2		19.9					
Approach LOS	<i>D</i>		<i>C</i>		<i>B</i>		<i>B</i>					
Intersection Delay	23.5		$X_c = 0.73$		Intersection LOS		<i>C</i>					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	AS					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	Philip Habib & Associates					Area Type	All other areas						
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions						
						Project ID	1121A - Charleston						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	29	31	5				3	250	137	293	415	44	
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1	
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.899		0.644	0.644		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						0		0	0		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		86						424		326	498		
Lane Group Capacity, c		462						966		525	1023		
v/c Ratio, X		0.19						0.44		0.62	0.49		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		16.3						8.0		9.2	8.3		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.9						1.3		3.5	1.1		

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.2					9.3		12.8	9.4	
Lane Group LOS		B					A		B	A	
Approach Delay		17.2					9.3		10.7		
Approach LOS		B					A		B		
Intersection Delay		10.7				$X_c = 0.47$	Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	AS					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions					
						Project ID	1121A - Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	7	7	17	273	17	98	12	245	225	80	327	7
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		36		303	19	109		524			460	
Lane Group Capacity, c		630		581	808	601		735			642	
v/c Ratio, X		0.06		0.52	0.02	0.18		0.71			0.72	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.1	14.0	15.1		20.3			20.4	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.0	0.0	0.6		3.1			3.4	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.4	21.0	14.1	15.7	23.4	23.8	
Lane Group LOS	B	C	B	B	C	C	
Approach Delay	14.4	19.4	23.4	23.8			
Approach LOS	B	B	C	C			
Intersection Delay	22.1	$X_c = 0.62$	Intersection LOS	C			

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	AS					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	Philip Habib & Associates					Area Type	All other areas								
Date Performed	8/04/2011					Jurisdiction	Staten Island, NY								
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2011 Existing Conditions								
						Project ID	1121A - Charleston								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		127	223	28	115		141		20						
% Heavy Vehicles, %HV		2	2	4	4		1		1						
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.927			0.992			0.953							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		2			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		377			160			190							
Lane Group Capacity, c		967			927			583							
v/c Ratio, X		0.39			0.17			0.33							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		14.0			12.3			19.2							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		1.1			0.4			1.4							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay		15.1			12.7			20.7			
Lane Group LOS		B			B			C			
Approach Delay	15.1		12.7			20.7					
Approach LOS	B		B			C					
Intersection Delay	16.0		$X_c = 0.36$			Intersection LOS			B		

No-Action

2015

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	35	196	35	31	205	22	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	41	230	41	34	225	24	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	23	17	37	34	19	24	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	29	21	47	38	21	27	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	41	34		86			97
C (m) (veh/h)	1282	1205		393			473
v/c	0.03	0.03		0.22			0.21
95% queue length	0.10	0.09		0.82			0.76
Control Delay (s/veh)	7.9	8.1		16.7			14.6
LOS	A	A		C			B
Approach Delay (s/veh)	--	--		16.7			14.6
Approach LOS	--	--		C			B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		252	18	19	258		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	296	21	20	283	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				43		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	45	0	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		20		60			
C (m) (veh/h)		1173		490			
v/c		0.02		0.12			
95% queue length		0.05		0.42			
Control Delay (s/veh)		8.1		13.4			
LOS		A		B			
Approach Delay (s/veh)	--	--	13.4				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	206	23	358	1	140	228	126	174	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			255	398		401			350	
Lane Group Capacity, c		714			525	518		710			538	
v/c Ratio, X		0.04			0.49	0.77		0.56			0.65	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			12.8	15.0		13.3			14.0	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			2.8	9.4		3.0			6.0	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.5			15.6	24.4		16.4			20.0
Lane Group LOS		B			B	C		B			C
Approach Delay		10.5			21.0			16.4			20.0
Approach LOS		B			C			B			C
Intersection Delay		19.2			$X_c = 0.71$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				347		3		365			387	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					397			397			450	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.62			0.41			0.48	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.8			9.4			9.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					3.8			1.1			1.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				20.6			10.5			10.8		
Lane Group LOS				C			B			B		
Approach Delay				20.6				10.5				10.8
Approach LOS				C				B				B
Intersection Delay	13.8			$X_c = 0.53$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		365	234	136	596		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	396	254	158	693	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		158					
C (m) (veh/h)		873					
v/c		0.18					
95% queue length		0.66					
Control Delay (s/veh)		10.0					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				97		178		410	77	169	410	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					316			530			674	
Lane Group Capacity, c					386			1056			670	
v/c Ratio, X					0.82			0.50			1.01	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.6			7.9			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					16.5			1.5			26.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				37.2			9.4			39.0
Lane Group LOS				<i>D</i>			<i>A</i>			<i>D</i>
Approach Delay				37.2	9.4			39.0		
Approach LOS				<i>D</i>	<i>A</i>			<i>D</i>		
Intersection Delay	28.3		$X_c = 0.95$		Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	141	63	73	36	172	43	83	551	131	95	438	67
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		16.6			21.5			16.3			21.2	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		322			273		91	749			659	
Lane Group Capacity, c		439			556		344	1051			977	
v/c Ratio, X		0.73			0.49		0.26	0.71			0.67	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.9			23.3		10.9	15.3			14.8	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		6.9			3.1		1.4	3.0			3.6	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	32.8	26.4	12.3	18.3	18.4
Lane Group LOS	C	C	B	B	B
Approach Delay	32.8	26.4	17.6	18.4	
Approach LOS	C	C	B	B	
Intersection Delay	21.3	$X_c = 0.72$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					512		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	575	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			87				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	103	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							103
C (m) (veh/h)							691
v/c							0.15
95% queue length							0.52
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	283							735			512	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	322							808			575	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.49							0.38			0.29	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.9							11.3			10.6	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.5							0.4			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.4						11.6			11.0
Lane Group LOS	C						B			B
Approach Delay	26.4						11.6		11.0	
Approach LOS	C						B		B	
Intersection Delay	14.2		$X_C = 0.42$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/25/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					362		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	416	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				182			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	195	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			195				
C (m) (veh/h)			589				
v/c			0.33				
95% queue length			1.44				
Control Delay (s/veh)			14.1				
LOS			B				
Approach Delay (s/veh)	--	--	14.1				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	91	144	135	187	462	54	86	117	305	47	40	39
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	108	294		208	559			546		59	99	
Lane Group Capacity, c	209	588		251	1124			886		380	358	
v/c Ratio, X	0.52	0.50		0.83	0.50			0.62		0.16	0.28	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	24.2	24.0		27.6	24.0			28.3		28.2	29.0	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	7.6	2.6		24.4	1.5			3.2		0.9	1.9	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	31.8	26.6		52.1	25.4			31.5		29.1	30.9	
Lane Group LOS	C	C		D	C			C		C	C	
Approach Delay	28.0			32.7				31.5		30.2		
Approach LOS	C			C				C		C		
Intersection Delay	31.1			$X_c = 0.61$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	10/25/2012				Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	0	85	31		126	89	0	
%Thrus Left Lane	50				50			
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	39	0	40		0	0	0	
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.88	0.88	0.90	0.90	0.78	0.78		
Flow Rate (veh/h)	47	83	188	50	50	51		
% Heavy Vehicles	42	42	4	4	3	3		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.7	0.4	0.4	0.1	0.3	-0.5		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.04	0.07	0.17	0.04	0.04	0.05		
hd, final value (s)	5.68	5.38	5.31	4.94	5.05	4.25		
x, final value	0.07	0.12	0.28	0.07	0.07	0.06		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	3.4	3.1	3.0	2.6	3.1	2.3		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	297	333	438	300	300	301		
Delay (s/veh)	8.83	8.84	10.04	8.00	8.43	7.53		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	8.84		9.61		7.97			
LOS	A		A		A			
Intersection Delay (s/veh)	9.04							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/25/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	60	101	307	130	266	18	226	151	86	22	302	219
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889			0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		556			460		260	273			603	
Lane Group Capacity, c		1177			980		308	801			1156	
v/c Ratio, X		0.47			0.47		0.84	0.34			0.52	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.6			17.6		22.2	16.4			18.1	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.2			1.5		23.7	1.2			1.7	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	18.8	19.1	45.9	17.5
Lane Group LOS	B	B	D	B
Approach Delay	18.8	19.1	31.4	19.8
Approach LOS	B	B	C	B
Intersection Delay	22.2	$X_c = 0.66$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	658	357	3	1	382	375	0	0	1	78	0	130
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	723	395			421	319		4		80	0	134
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.92	0.34			0.93	0.83		0.01		0.34	0.00	0.14
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	19.9	5.0			33.3	32.3		30.5		32.4	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	14.4	0.6			18.8	11.1		0.1		4.0	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	34.3	5.5			52.1	43.4		30.6		36.4	30.4	6.6
Lane Group LOS	C	A			D	D		C		D	C	A
Approach Delay	24.1			48.3			30.6			17.7		
Approach LOS	C			D			C			B		
Intersection Delay	32.1			$X_c = 0.84$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	398	36	0	2	62	69	4	3	0	62	6	691
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	437	40			127		7	5			76	423
Lane Group Capacity, c	709	1056			1536		518	732			541	593
v/c Ratio, X	0.62	0.04			0.08		0.01	0.01			0.14	0.71
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	16.3	11.5			11.7		16.9	16.9			17.8	23.3
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	3.8	0.1			0.1		0.0	0.0			0.5	6.4

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	20.1	11.5			11.8		16.9	16.9			18.3	29.7
Lane Group LOS	C	B			B		B	B			B	C
Approach Delay	19.4		11.8		16.9		27.9					
Approach LOS	B		B		B		C					
Intersection Delay	22.3		$X_c = 0.66$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 12-1 PM MD Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	142		24				4	205			390	349
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	161		20					232			923	
Lane Group Capacity, c	537		465					1667			1930	
v/c Ratio, X	0.30		0.04					0.14			0.48	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.5		14.2					7.6			9.3	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.4		0.2					0.2			0.6	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.9		14.4					7.7			9.9	
Lane Group LOS	B		B					A			A	
Approach Delay	16.6						7.7			9.9		
Approach LOS	B						A			A		
Intersection Delay	10.4			$X_c = 0.41$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		23					
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	41	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			345				
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	383	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					383		
C (m) (veh/h)					1033		
v/c					0.37		
95% queue length					1.73		
Control Delay (s/veh)					10.5		
LOS					B		
Approach Delay (s/veh)	--	--	10.5				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	544	1		1			22	194	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		287	287		1				269		
Lane Group Capacity, c		784		612	587		424				2033		
v/c Ratio, X		0.01		0.47	0.49		0.00				0.13		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.7	12.8		10.2				10.8		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.5	2.8		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		15.2	15.7		10.2				10.9	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.4			10.2			10.9		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		14.0		$X_c = 0.31$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	53	30	110	57	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	30	76	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	95		216				84	71	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.08		0.19				0.07	0.06	
hd, final value (s)	4.59		4.57				4.74	4.45	
x, final value	0.12		0.27				0.11	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.6		2.6				2.7	2.5	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	345		466				334	321	
Delay (s/veh)	8.22		9.29				8.33	7.88	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.22		9.29				8.13		
LOS	A		A				A		
Intersection Delay (s/veh)	8.68								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	39	45	0		0	126	22		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	41	61	86		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	97		191		80	132			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		-0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.17		0.07	0.12			
hd, final value (s)	4.87		4.48		4.72	4.16			
x, final value	0.13		0.24		0.10	0.15			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	2.9		2.5		2.7	2.2			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	347		441		330	382			
Delay (s/veh)	8.60		8.87		8.27	7.91			
LOS	A		A		A	A			
Approach: Delay (s/veh)	8.60		8.87		8.05				
LOS	A		A		A				
Intersection Delay (s/veh)	8.47								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	145	114	109	2	37	5	508	37	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		284	63		46			564				
Lane Group Capacity, c		545	560		531			2287				
v/c Ratio, X		0.52	0.11		0.09			0.25				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		15.5	13.2		13.1			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		3.5	0.4		0.3			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		19.0	13.6		13.4			9.3			
Lane Group LOS		B	B		B			A			
Approach Delay		18.0			13.4			9.3			
Approach LOS		B			B			A			
Intersection Delay		12.7			$X_c = 0.36$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	77		39				14	283			333	31
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		143						341			400	
Lane Group Capacity, c		460						1092			1173	
v/c Ratio, X		0.31						0.31			0.34	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.6						6.8			7.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.7						0.7			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.3					7.6			7.7	
Lane Group LOS		B					A			A	
Approach Delay		19.3					7.6			7.7	
Approach LOS		B					A			A	
Intersection Delay		9.5			$X_c = 0.33$		Intersection LOS			A	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	78		52				43	317			282	105
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		151						413			410	
Lane Group Capacity, c		555						843			1023	
v/c Ratio, X		0.27						0.49			0.40	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						9.9			9.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.2						1.8			1.1	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		15.9					11.8			10.5	
Lane Group LOS		B					B			B	
Approach Delay	15.9						11.8		10.5		
Approach LOS	B						B		B		
Intersection Delay	11.9			$X_C = 0.40$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	9		102	222	455	123	61	164			341	31
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	12		132		919		69	184			413	
Lane Group Capacity, c	222		220		1339		171	593			690	
v/c Ratio, X	0.05		0.60		0.69		0.40	0.31			0.60	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		24.5		19.1		15.4	14.9			16.7	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		9.7		2.1		6.5	1.3			3.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		34.2		21.2		21.9	16.1			19.8	
Lane Group LOS	C		C		C		C	B			B	
Approach Delay	33.2		21.2		17.7		19.8					
Approach LOS	C		C		B		B					
Intersection Delay	21.4		$X_c = 0.63$		Intersection LOS		C					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	5	15	7				6	220	109	222	415	27	
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1	
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.932		0.785	0.785		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						2		0	2		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		43						376		247	491		
Lane Group Capacity, c		456						920		560	1017		
v/c Ratio, X		0.09						0.41		0.44	0.48		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		15.8						7.8		8.0	8.3		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.4						1.3		2.0	1.3		

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.2						9.1		10.0	9.6	
Lane Group LOS		B						A		A	A	
Approach Delay		16.2						9.1			9.7	
Approach LOS		B						A			A	
Intersection Delay		9.7		$X_c = 0.35$			Intersection LOS			A		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	7	17	257	25	102	21	228	165	87	316	19
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		37		306	30	121		465			469	
Lane Group Capacity, c		626		548	763	563		688			642	
v/c Ratio, X		0.06		0.56	0.04	0.21		0.68			0.73	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.5	14.1	15.4		19.9			20.6	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.4	0.1	0.7		3.5			4.8	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.4	21.9	14.2	16.1	23.3	25.3	
Lane Group LOS	B	C	B	B	C	C	
Approach Delay	14.4	19.8	23.3	25.3			
Approach LOS	B	B	C	C			
Intersection Delay	22.6	$X_c = 0.64$	Intersection LOS	C			

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		143	182	55	163		185		57				
% Heavy Vehicles, %HV		8	8	16	16		2		2				
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.961			0.989			0.922					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		312			239			275					
Lane Group Capacity, c		913			794			571					
v/c Ratio, X		0.34			0.30			0.48					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.6			13.2			20.7					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.0			1.0			2.7					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay		14.5			14.2			23.3			
Lane Group LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Approach Delay		14.5			14.2			23.3			
Approach LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Intersection Delay		17.4			$X_c = 0.40$			Intersection LOS			<i>B</i>

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	35	196	35	31	205	22	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	41	230	41	34	225	24	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	23	17	37	34	19	24	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	29	21	47	38	21	27	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	41	34	86			97	
C (m) (veh/h)	1282	1205	393			473	
v/c	0.03	0.03	0.22			0.21	
95% queue length	0.10	0.09	0.82			0.76	
Control Delay (s/veh)	7.9	8.1	16.7			14.6	
LOS	A	A	C			B	
Approach Delay (s/veh)	--	--	16.7			14.6	
Approach LOS	--	--	C			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		252	18	19	258		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	296	21	20	283	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				43		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	45	0	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		20		60			
C (m) (veh/h)		1173		490			
v/c		0.02		0.12			
95% queue length		0.05		0.42			
Control Delay (s/veh)		8.1		13.4			
LOS		A		B			
Approach Delay (s/veh)	--	--	13.4				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	206	23	358	1	140	228	126	174	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			255	398		401			350	
Lane Group Capacity, c		714			525	518		710			538	
v/c Ratio, X		0.04			0.49	0.77		0.56			0.65	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			12.8	15.0		13.3			14.0	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			2.8	9.4		3.0			6.0	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.5			15.6	24.4		16.4			20.0
Lane Group LOS		B			B	C		B			C
Approach Delay		10.5			21.0			16.4			20.0
Approach LOS		B			C			B			C
Intersection Delay		19.2			$X_c = 0.71$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				347		3		365			387	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					397			397			450	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.62			0.41			0.48	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.8			9.4			9.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					3.8			1.1			1.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				20.6			10.5			10.8		
Lane Group LOS				C			B			B		
Approach Delay				20.6				10.5				10.8
Approach LOS				C				B				B
Intersection Delay	13.8			$X_c = 0.53$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		365	234	136	596		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	396	254	158	693	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		158					
C (m) (veh/h)		873					
v/c		0.18					
95% queue length		0.66					
Control Delay (s/veh)		10.0					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				97		178		410	77	169	410	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					316			530			674	
Lane Group Capacity, c					386			1056			670	
v/c Ratio, X					0.82			0.50			1.01	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.6			7.9			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					16.5			1.5			26.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				37.2			9.4			39.0
Lane Group LOS				<i>D</i>			<i>A</i>			<i>D</i>
Approach Delay				37.2	9.4			39.0		
Approach LOS				<i>D</i>	<i>A</i>			<i>D</i>		
Intersection Delay	28.3		$X_c = 0.95$		Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	141	63	73	36	172	43	83	551	131	95	438	67
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		322			273		91	749			659	
Lane Group Capacity, c		439			556		344	1051			977	
v/c Ratio, X		0.73			0.49		0.26	0.71			0.67	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.9			23.3		10.9	15.3			14.8	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		6.9			3.1		1.4	3.0			3.6	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	32.8	26.4	12.3	18.3	18.4
Lane Group LOS	C	C	B	B	B
Approach Delay	32.8	26.4	17.6		18.4
Approach LOS	C	C	B		B
Intersection Delay	21.3	$X_c = 0.72$	Intersection LOS		C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					512		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	575	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			87				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	103	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							103
C (m) (veh/h)							691
v/c							0.15
95% queue length							0.52
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	283							735			512	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	322							808			575	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.49							0.38			0.29	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.9							11.3			10.6	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.5							0.4			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.4						11.6			11.0
Lane Group LOS	C						B			B
Approach Delay	26.4						11.6		11.0	
Approach LOS	C						B		B	
Intersection Delay	14.2		$X_C = 0.42$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/25/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					362		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	416	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				182			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	195	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			195				
C (m) (veh/h)			589				
v/c			0.33				
95% queue length			1.44				
Control Delay (s/veh)			14.1				
LOS			B				
Approach Delay (s/veh)	--	--	14.1				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	91	144	135	187	462	54	86	117	305	47	40	39
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	108	294		208	559			546		59	99	
Lane Group Capacity, c	209	588		251	1124			886		380	358	
v/c Ratio, X	0.52	0.50		0.83	0.50			0.62		0.16	0.28	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	24.2	24.0		27.6	24.0			28.3		28.2	29.0	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	7.6	2.6		24.4	1.5			3.2		0.9	1.9	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	31.8	26.6		52.1	25.4			31.5		29.1	30.9	
Lane Group LOS	C	C		D	C			C		C	C	
Approach Delay	28.0			32.7				31.5		30.2		
Approach LOS	C			C				C		C		
Intersection Delay	31.1			$X_c = 0.61$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/25/2012			Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	85	31	126	89	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	39	0	40	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.88	0.88	0.90	0.90	0.78	0.78		
Flow Rate (veh/h)	47	83	188	50	50	51		
% Heavy Vehicles	42	42	4	4	3	3		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.7	0.4	0.4	0.1	0.3	-0.5		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.04	0.07	0.17	0.04	0.04	0.05		
hd, final value (s)	5.68	5.38	5.31	4.94	5.05	4.25		
x, final value	0.07	0.12	0.28	0.07	0.07	0.06		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	3.4	3.1	3.0	2.6	3.1	2.3		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	297	333	438	300	300	301		
Delay (s/veh)	8.83	8.84	10.04	8.00	8.43	7.53		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	8.84		9.61		7.97			
LOS	A		A		A			
Intersection Delay (s/veh)	9.04							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/25/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	60	101	307	130	266	18	226	151	86	22	302	219
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889			0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		556			460		260	273			603	
Lane Group Capacity, c		1177			980		308	801			1156	
v/c Ratio, X		0.47			0.47		0.84	0.34			0.52	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.6			17.6		22.2	16.4			18.1	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.2			1.5		23.7	1.2			1.7	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	18.8	19.1	45.9	17.5
Lane Group LOS	B	B	D	B
Approach Delay	18.8	19.1	31.4	19.8
Approach LOS	B	B	C	B
Intersection Delay	22.2	$X_c = 0.66$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	658	357	3	1	382	375	0	0	1	78	0	130
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	723	395			421	319		4		80	0	134
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.92	0.34			0.93	0.83		0.01		0.34	0.00	0.14
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	19.9	5.0			33.3	32.3		30.5		32.4	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	14.4	0.6			18.8	11.1		0.1		4.0	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	34.3	5.5			52.1	43.4		30.6		36.4	30.4	6.6
Lane Group LOS	C	A			D	D		C		D	C	A
Approach Delay	24.1			48.3			30.6			17.7		
Approach LOS	C			D			C			B		
Intersection Delay	32.1			$X_c = 0.84$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	398	36	0	2	62	69	4	3	0	62	6	691
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	437	40			127		7	5			76	423
Lane Group Capacity, c	709	1056			1536		518	732			541	593
v/c Ratio, X	0.62	0.04			0.08		0.01	0.01			0.14	0.71
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	16.3	11.5			11.7		16.9	16.9			17.8	23.3
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	3.8	0.1			0.1		0.0	0.0			0.5	6.4

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	20.1	11.5			11.8		16.9	16.9			18.3	29.7
Lane Group LOS	C	B			B		B	B			B	C
Approach Delay	19.4		11.8		16.9		27.9					
Approach LOS	B		B		B		C					
Intersection Delay	22.3		$X_c = 0.66$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 12-1 PM MD Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	142		24				4	205			390	349
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	161		20					232			923	
Lane Group Capacity, c	537		465					1667			1930	
v/c Ratio, X	0.30		0.04					0.14			0.48	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.5		14.2					7.6			9.3	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.4		0.2					0.2			0.6	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.9		14.4					7.7			9.9	
Lane Group LOS	B		B					A			A	
Approach Delay	16.6						7.7			9.9		
Approach LOS	B						A			A		
Intersection Delay	10.4			$X_c = 0.41$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		23					
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	41	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			345				
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	383	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					383		
C (m) (veh/h)					1033		
v/c					0.37		
95% queue length					1.73		
Control Delay (s/veh)					10.5		
LOS					B		
Approach Delay (s/veh)	--	--	10.5				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	544	1		1			22	194	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		287	287		1				269		
Lane Group Capacity, c		784		612	587		424				2033		
v/c Ratio, X		0.01		0.47	0.49		0.00				0.13		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.7	12.8		10.2				10.8		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.5	2.8		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		15.2	15.7		10.2				10.9	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.4			10.2			10.9		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		14.0		$X_c = 0.31$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	53	30	110	57	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	30	76	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	95		216				84	71	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.08		0.19				0.07	0.06	
hd, final value (s)	4.59		4.57				4.74	4.45	
x, final value	0.12		0.27				0.11	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.6		2.6				2.7	2.5	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	345		466				334	321	
Delay (s/veh)	8.22		9.29				8.33	7.88	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.22		9.29				8.13		
LOS	A		A				A		
Intersection Delay (s/veh)	8.68								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	39	45	0		0	126	22		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	41	61	86		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	97		191		80	132			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		-0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.17		0.07	0.12			
hd, final value (s)	4.87		4.48		4.72	4.16			
x, final value	0.13		0.24		0.10	0.15			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	2.9		2.5		2.7	2.2			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	347		441		330	382			
Delay (s/veh)	8.60		8.87		8.27	7.91			
LOS	A		A		A	A			
Approach: Delay (s/veh)	8.60		8.87		8.05				
LOS	A		A		A				
Intersection Delay (s/veh)	8.47								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	145	114	109	2	37	5	508	37	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		284	63		46			564				
Lane Group Capacity, c		545	560		531			2287				
v/c Ratio, X		0.52	0.11		0.09			0.25				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		15.5	13.2		13.1			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		3.5	0.4		0.3			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		19.0	13.6		13.4			9.3			
Lane Group LOS		B	B		B			A			
Approach Delay		18.0			13.4			9.3			
Approach LOS		B			B			A			
Intersection Delay		12.7			$X_c = 0.36$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	77		39				14	283			333	31
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		143						341			400	
Lane Group Capacity, c		460						1092			1173	
v/c Ratio, X		0.31						0.31			0.34	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.6						6.8			7.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.7						0.7			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.3					7.6			7.7	
Lane Group LOS		B					A			A	
Approach Delay		19.3					7.6			7.7	
Approach LOS		B					A			A	
Intersection Delay		9.5		$X_c = 0.33$		Intersection LOS			A		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	78		52				43	317			282	105
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		151						413			410	
Lane Group Capacity, c		555						843			1023	
v/c Ratio, X		0.27						0.49			0.40	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						9.9			9.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.2						1.8			1.1	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		15.9					11.8			10.5	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay	15.9						11.8		10.5		
Approach LOS	<i>B</i>						<i>B</i>		<i>B</i>		
Intersection Delay	11.9			$X_c = 0.40$			Intersection LOS		<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	9		102	222	455	123	61	164			341	31
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	12		132		919		69	184			413	
Lane Group Capacity, c	222		220		1339		171	593			690	
v/c Ratio, X	0.05		0.60		0.69		0.40	0.31			0.60	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		24.5		19.1		15.4	14.9			16.7	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		9.7		2.1		6.5	1.3			3.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		34.2		21.2		21.9	16.1			19.8	
Lane Group LOS	C		C		C		C	B			B	
Approach Delay	33.2		21.2		17.7		19.8					
Approach LOS	C		C		B		B					
Intersection Delay	21.4		$X_c = 0.63$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	5	15	7				6	220	109	222	415	27
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.932		0.785	0.785	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	2	
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		43						376		247	491	
Lane Group Capacity, c		456						920		560	1017	
v/c Ratio, X		0.09						0.41		0.44	0.48	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		15.8						7.8		8.0	8.3	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.4						1.3		2.0	1.3	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		16.2					9.1		10.0	9.6	
Lane Group LOS		B					A		A	A	
Approach Delay		16.2					9.1		9.7		
Approach LOS		B					A		A		
Intersection Delay		9.7		$X_c = 0.35$		Intersection LOS		A			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	7	17	257	25	102	21	228	165	87	316	19
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		37		306	30	121		465			469	
Lane Group Capacity, c		626		548	763	563		688			642	
v/c Ratio, X		0.06		0.56	0.04	0.21		0.68			0.73	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.5	14.1	15.4		19.9			20.6	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.4	0.1	0.7		3.5			4.8	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.4	21.9	14.2	16.1	23.3	25.3
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.4	19.8	23.3	25.3		
Approach LOS	B	B	C	C		
Intersection Delay	22.6	$X_c = 0.64$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		143	182	55	163		185		57				
% Heavy Vehicles, %HV		8	8	16	16		2		2				
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.961			0.989			0.922					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		312			239			275					
Lane Group Capacity, c		913			794			571					
v/c Ratio, X		0.34			0.30			0.48					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.6			13.2			20.7					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.0			1.0			2.7					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay	14.5	14.2	23.3	
Lane Group LOS	B	B	C	
Approach Delay	14.5	14.2	23.3	
Approach LOS	B	B	C	
Intersection Delay	17.4	$X_c = 0.40$	Intersection LOS	B

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	28	200	66	56	198	27	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	32	229	75	69	244	33	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	33	18	28	55	17	33	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	38	20	32	67	20	40	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	32	69		127			90
C (m) (veh/h)	1263	1218		354			363
v/c	0.03	0.06		0.36			0.25
95% queue length	0.08	0.18		1.59			0.96
Control Delay (s/veh)	7.9	8.1		20.8			18.2
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		20.8			18.2
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		269	18	14	268		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	309	20	17	330	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				28		26	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	41	0	38	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		17		79			
C (m) (veh/h)		1203		507			
v/c		0.01		0.16			
95% queue length		0.04		0.55			
Control Delay (s/veh)		8.0		13.4			
LOS		A		B			
Approach Delay (s/veh)	--	--	13.4				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	10	2	236	20	234	4	136	277	174	236	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		14			312	285		448			477	
Lane Group Capacity, c		725			534	523		743			497	
v/c Ratio, X		0.02			0.58	0.54		0.60			0.96	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			13.5	13.2		13.6			17.0	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.0			4.3	3.8		3.3			31.5	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			17.8	17.0		17.0			48.5	
Lane Group LOS		<i>B</i>			<i>B</i>	<i>B</i>		<i>B</i>			<i>D</i>	
Approach Delay		10.3			17.4			17.0			48.5	
Approach LOS		<i>B</i>			<i>B</i>			<i>B</i>			<i>D</i>	
Intersection Delay		26.9			$X_c = 0.77$			Intersection LOS			<i>C</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				511		4		413			473	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					613			444			550	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.91			0.43			0.58	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.2			9.6			10.5	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					8.8			1.2			0.9	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				28.0			10.8			11.4	
Lane Group LOS				C			B			B	
Approach Delay				28.0				10.8			
Approach LOS				C				B			
Intersection Delay	17.5		$X_c = 0.71$		Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		413	254	182	802		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	444	273	211	932	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		211					
C (m) (veh/h)		815					
v/c		0.26					
95% queue length		1.03					
Control Delay (s/veh)		11.0					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				130		142		512	101	118	669	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					324			660			915	
Lane Group Capacity, c					398			1095			784	
v/c Ratio, X					0.81			0.60			1.17	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.6			8.6			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					15.1			2.0			77.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				35.7			10.5			90.0		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				35.7				10.5				90.0
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	53.1			$X_c = 1.05$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	94	44	102	33	188	42	56	540	104	108	514	49
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		273			325		62	707			760	
Lane Group Capacity, c		464			568		301	1066			1021	
v/c Ratio, X		0.59			0.57		0.21	0.66			0.74	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		24.3			24.1		10.5	14.6			15.7	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.7			4.1		1.2	2.5			4.7	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	27.0	28.2	11.7	17.1
Lane Group LOS	C	C	B	B
Approach Delay	27.0	28.2	16.7	20.4
Approach LOS	C	C	B	C
Intersection Delay	21.1	$X_c = 0.68$	Intersection LOS	C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 NO-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					601		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	690	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			69				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	88	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							88
C (m) (veh/h)							593
v/c							0.15
95% queue length							0.52
Control Delay (s/veh)							12.1
LOS							B
Approach Delay (s/veh)	--	--				12.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	368							676			601	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	404							751			691	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.60							0.35			0.35	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.0							11.0			11.0	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	3.9							0.3			0.5	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	28.9						11.4			11.5
Lane Group LOS	C						B			B
Approach Delay	28.9						11.4		11.5	
Approach LOS	C						B		B	
Intersection Delay	15.2		$X_C = 0.44$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					472		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	530	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				458			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	492	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			492				
C (m) (veh/h)			508				
v/c			0.97				
95% queue length			12.62				
Control Delay (s/veh)			61.0				
LOS			F				
Approach Delay (s/veh)	--	--	61.0				
Approach LOS	--	--	F				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	105	166	190	239	319	43	116	89	253	45	43	54
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	119	354		266	428			466		59	95	
Lane Group Capacity, c	270	583		240	1149			902		384	367	
v/c Ratio, X	0.44	0.61		1.11	0.37			0.52		0.15	0.26	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	23.4	25.1		29.5	22.2			27.4		28.2	28.9	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	3.6	3.3		88.7	0.9			2.1		0.8	1.7	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	27.1	28.4		118.2	23.1			29.5		29.0	30.5	
Lane Group LOS	C	C		F	C			C		C	C	
Approach Delay	28.0			59.5				29.5		30.0		
Approach LOS	C			E				C		C		
Intersection Delay	40.8			$X_c = 0.69$				Intersection LOS		D		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	98	28	143	129	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	12	0	67	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.87	0.87	0.91	0.91	0.76	0.76		
Flow Rate (veh/h)	56	88	227	71	15	88		
% Heavy Vehicles	7	7	1	1	1	1		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1	-0.1	0.4	0.0	0.2	-0.6		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.05	0.08	0.20	0.06	0.01	0.08		
hd, final value (s)	5.13	4.87	5.23	4.88	5.13	4.34		
x, final value	0.08	0.12	0.33	0.10	0.02	0.11		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	2.8	2.6	2.9	2.6	3.1	2.3		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	306	338	477	321	265	338		
Delay (s/veh)	8.27	8.23	10.48	8.10	8.25	7.85		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	8.25		9.92		7.91			
LOS	A		A		A			
Intersection Delay (s/veh)	9.10							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	51	114	274	120	284	9	202	99	121	21	304	120
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.884			0.927		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		499			503		217	236			473	
Lane Group Capacity, c		1198			1042		385	792			1296	
v/c Ratio, X		0.42			0.48		0.56	0.30			0.36	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.0			17.7		18.5	16.0			16.6	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		0.9			1.5		5.9	1.0			0.8	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	18.0	19.2	24.4	17.0
Lane Group LOS	B	B	C	B
Approach Delay	18.0	19.2	20.5	17.4
Approach LOS	B	B	C	B
Intersection Delay	18.7	$X_c = 0.52$	Intersection LOS	B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	723	316	4	3	325	371	2	3	2	99	0	275
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	803	355			345	391		17		124	0	344
Lane Group Capacity, c	839	1205			584	497		148		204	283	862
v/c Ratio, X	0.96	0.29			0.59	0.79		0.11		0.61	0.00	0.40
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	15.2	4.1			26.2	28.3		32.7		35.4	32.1	10.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	17.3	0.4			2.8	8.0		1.6		12.8	0.0	1.4

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	32.5	4.5			29.0	36.3		34.2		48.2	32.1	11.8
Lane Group LOS	C	A			C	D		C		D	C	B
Approach Delay	23.9			32.9			34.2			21.4		
Approach LOS	C			C			C			C		
Intersection Delay	26.3			$X_c = 0.86$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	382	35	0	1	35	42	3	3	2	60	1	661
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	424	39			72			16			65	390
Lane Group Capacity, c	755	1066			1561			1220			550	616
v/c Ratio, X	0.56	0.04			0.05			0.01			0.12	0.63
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.6	11.5			11.5			16.9			17.6	22.3
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	2.9	0.1			0.1			0.0			0.4	4.6

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	18.5	11.5			11.6			16.9			18.0	26.9
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	17.9		11.6		16.9		25.6					
Approach LOS	<i>B</i>		<i>B</i>		<i>B</i>		<i>C</i>					
Intersection Delay	20.9		$X_c = 0.59$		Intersection LOS		<i>C</i>					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 5-6 PM Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	142		23				2	252			391	270
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	163		20					288			727	
Lane Group Capacity, c	547		473					1732			1949	
v/c Ratio, X	0.30		0.04					0.17			0.37	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.5		14.2					7.7			8.7	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.4		0.2					0.2			0.4	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.8		14.4					7.9			9.1	
Lane Group LOS	B		B					A			A	
Approach Delay	16.6						7.9			9.1		
Approach LOS	B						A			A		
Intersection Delay	10.0			$X_c = 0.34$			Intersection LOS			A		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		23					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	32	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			390				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	443	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					443		
C (m) (veh/h)					1045		
v/c					0.42		
95% queue length					2.15		
Control Delay (s/veh)					11.0		
LOS					B		
Approach Delay (s/veh)	--	--	11.0				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	445	2		4			22	215	4	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		250	252		5				264		
Lane Group Capacity, c		784		600	576		426				2050		
v/c Ratio, X		0.01		0.42	0.44		0.01				0.13		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.4	12.5		10.3				10.8		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.1	2.4		0.0				0.1		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	14.4	14.9	10.3	10.9
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	14.6	10.3	10.9	
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Intersection Delay	13.3	$X_c = 0.28$	Intersection LOS	<i>B</i>	

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	103	37	117	81	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	23	86	24			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.78				0.91	0.91	
Flow Rate (veh/h)	174		253				72	73	
% Heavy Vehicles	2		3				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.6				0.3	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.4	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.15		0.22				0.06	0.06	
hd, final value (s)	4.42		4.62				4.98	4.69	
x, final value	0.21		0.32				0.10	0.10	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.4		2.6				3.0	2.7	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	424		503				322	323	
Delay (s/veh)	8.62		9.82				8.53	8.18	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.62		9.82				8.35		
LOS	A		A				A		
Intersection Delay (s/veh)	9.09								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	<i>KM</i>	Intersection	<i>Sharrotts Rd @ Veterans Rd E</i>
Agency/Co.	<i>AECOM</i>	Jurisdiction	<i>Staten Island, NY</i>
Date Performed	<i>10/24/2012</i>	Analysis Year	<i>2015 No-Action Conditions</i>
Analysis Time Period	<i>Weekday 5-6 PM Peak Period</i>		

Project ID *Charleston EIS*East/West Street: *Sharrotts Road*North/South Street: *Veterans Road East*

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	<i>37</i>	<i>88</i>	<i>0</i>	<i>0</i>	<i>135</i>	<i>19</i>
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	<i>63</i>	<i>97</i>	<i>138</i>	<i>0</i>	<i>0</i>	<i>0</i>
%Thrus Left Lane	<i>50</i>					

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LT</i>		<i>TR</i>		<i>LT</i>	<i>TR</i>		
PHF	<i>0.80</i>		<i>0.78</i>		<i>0.99</i>	<i>0.99</i>		
Flow Rate (veh/h)	<i>155</i>		<i>197</i>		<i>111</i>	<i>188</i>		
% Heavy Vehicles	<i>2</i>		<i>5</i>		<i>1</i>	<i>1</i>		
No. Lanes	<i>1</i>		<i>1</i>		<i>2</i>		<i>0</i>	
Geometry Group	<i>2</i>		<i>2</i>		<i>1</i>			
Duration, T	<i>0.25</i>							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	<i>0.3</i>		<i>0.0</i>		<i>0.6</i>	<i>0.0</i>		
Prop. Right-Turns	<i>0.0</i>		<i>0.1</i>		<i>0.0</i>	<i>0.7</i>		
Prop. Heavy Vehicle	<i>0.0</i>		<i>0.0</i>		<i>0.0</i>	<i>0.0</i>		
hLT-adj	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>		
hRT-adj	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>		
hHV-adj	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>		
hadj, computed	<i>0.1</i>		<i>0.0</i>		<i>0.1</i>	<i>-0.4</i>		

Departure Headway and Service Time

hd, initial value (s)	<i>3.20</i>		<i>3.20</i>		<i>3.20</i>	<i>3.20</i>		
x, initial	<i>0.14</i>		<i>0.18</i>		<i>0.10</i>	<i>0.17</i>		
hd, final value (s)	<i>4.91</i>		<i>4.78</i>		<i>4.90</i>	<i>4.34</i>		
x, final value	<i>0.21</i>		<i>0.26</i>		<i>0.15</i>	<i>0.23</i>		
Move-up time, m (s)	<i>2.0</i>		<i>2.0</i>		<i>2.0</i>			
Service Time, t _s (s)	<i>2.9</i>		<i>2.8</i>		<i>2.9</i>	<i>2.3</i>		

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	<i>405</i>		<i>447</i>		<i>361</i>	<i>438</i>		
Delay (s/veh)	<i>9.22</i>		<i>9.46</i>		<i>8.77</i>	<i>8.61</i>		
LOS	<i>A</i>		<i>A</i>		<i>A</i>	<i>A</i>		
Approach: Delay (s/veh)	<i>9.22</i>		<i>9.46</i>		<i>8.67</i>			
LOS	<i>A</i>		<i>A</i>		<i>A</i>			
Intersection Delay (s/veh)	<i>9.04</i>							
Intersection LOS	<i>A</i>							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	187	115	111	7	46	8	402	102	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		355	68		70			569				
Lane Group Capacity, c		524	560		533			2302				
v/c Ratio, X		0.68	0.12		0.13			0.25				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		16.6	13.2		13.3			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		6.9	0.4		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		23.5	13.7		13.8			9.3			
Lane Group LOS		C	B		B			A			
Approach Delay		21.9			13.8			9.3			
Approach LOS		C			B			A			
Intersection Delay		14.6			$X_c = 0.43$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	84		33				18	451			451	44
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		122						545			538	
Lane Group Capacity, c		453						1099			1127	
v/c Ratio, X		0.27						0.50			0.48	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.4						7.8			7.7	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.4						1.4			1.3	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	18.8			9.2		9.0
Lane Group LOS	B			A		A
Approach Delay	18.8			9.2		9.0
Approach LOS	B			A		A
Intersection Delay	10.1	$X_c = 0.42$		Intersection LOS		B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	143		83				22	427			388	135
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		283						523			546	
Lane Group Capacity, c		573						891			985	
v/c Ratio, X		0.49						0.59			0.55	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.0						10.6			10.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.8						2.4			2.0	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		18.7					13.0			12.4	
Lane Group LOS		B					B			B	
Approach Delay		18.7					13.0			12.4	
Approach LOS		B					B			B	
Intersection Delay		13.9			$X_c = 0.55$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		114	266	450	266	51	198			479	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		120		1115		55	215			557	
Lane Group Capacity, c	222		220		1329		124	616			670	
v/c Ratio, X	0.02		0.55		0.84		0.44	0.35			0.83	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		24.3		20.2		15.6	15.1			18.4	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		8.0		3.4		10.3	1.4			10.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		32.3		23.6		26.0	16.5			28.6	
Lane Group LOS	C		C		C		C	B			C	
Approach Delay	32.0		23.6		18.5		28.6					
Approach LOS	C		C		B		C					
Intersection Delay	24.8		$X_c = 0.78$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	9	20	6				8	240	118	308	502	50
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.922		0.564	0.564	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	7	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		56						398		331	581	
Lane Group Capacity, c		460						953		538	985	
v/c Ratio, X		0.12						0.42		0.62	0.59	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.0						7.9		9.2	9.0	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.5						1.2		3.0	1.5	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.5					9.1		12.1	10.5	
Lane Group LOS	B					A		B	B	
Approach Delay	16.5				9.1			11.1		
Approach LOS	B				A			B		
Intersection Delay	10.7		$X_c = 0.45$		Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	12	20	258	28	106	18	254	169	91	382	9
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		55		297	32	122		480			554	
Lane Group Capacity, c		632		560	792	585		727			640	
v/c Ratio, X		0.09		0.53	0.04	0.21		0.66			0.87	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.4		18.2	14.1	15.3		19.7			22.6	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.2	0.1	0.7		3.1			3.9	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.7	21.4	14.2	16.0	22.8	26.5
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.7	19.4	22.8	26.5		
Approach LOS	B	B	C	C		
Intersection Delay	22.8	$X_c = 0.70$	Intersection LOS	C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	0	1		0		0			
Lane Group		TR			LT			LR				
Volume, V (vph)		136	245	49	149		139		55			
% Heavy Vehicles, %HV		5	5	8	8		1		1			
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0			2.0			2.0				
Extension of Effective Green, e		2.0			2.0			2.0				
Arrival Type, AT		3			3			3				
Unit Extension, UE		3.0			3.0			3.0				
Filtering/Metering, I		0.927			0.989			0.917				
Initial Unmet Demand, Q _b		0.0			0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0	
Lane Width		16.0			16.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5			0			0				
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		393			239			289				
Lane Group Capacity, c		927			836			574				
v/c Ratio, X		0.42			0.29			0.50				
Total Green Ratio, g/C		0.50			0.50			0.39				
Uniform Delay, d ₁		14.3			13.1			20.9				
Progression Factor, PF		1.000			1.000			1.000				
Delay Calibration, k		0.50			0.50			0.50				
Incremental Delay, d ₂		1.3			0.9			2.9				
Initial Queue Delay, d ₃		0.0			0.0			0.0				

Control Delay		15.6			14.0			23.8			
Lane Group LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Approach Delay		15.6			14.0			23.8			
Approach LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Intersection Delay		17.7			$X_c = 0.46$			Intersection LOS			<i>B</i>

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	11	198	49	28	220	27	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	11	215	53	31	250	30	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	26	32	23	60	17	40	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	64	79	57	76	21	51	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	11	31		148			200
C (m) (veh/h)	1277	1278		390			436
v/c	0.01	0.02		0.38			0.46
95% queue length	0.03	0.07		1.74			2.35
Control Delay (s/veh)	7.8	7.9		19.8			20.1
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		19.8			20.1
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		240	12	16	287		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	260	13	18	326	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				9		19	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	12	0	25	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		18		37			
C (m) (veh/h)		1273		619			
v/c		0.01		0.06			
95% queue length		0.04		0.19			
Control Delay (s/veh)		7.9		11.2			
LOS		A		B			
Approach Delay (s/veh)	--	--	11.2				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	9	5	277	26	342	5	162	330	143	204	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		15			329	372		546			381	
Lane Group Capacity, c		724			550	539		746			466	
v/c Ratio, X		0.02			0.60	0.69		0.73			0.82	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			13.6	14.3		14.7			15.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			4.0	6.1		5.4			14.7	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.3			17.6	20.4		20.1			30.2
Lane Group LOS		B			B	C		C			C
Approach Delay		10.3			19.1			20.1			30.2
Approach LOS		B			B			C			C
Intersection Delay		21.9			$X_c = 0.75$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				493		8		488			487	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					582			536			535	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.86			0.52			0.52	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					18.7			10.2			10.1	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					7.1			1.6			0.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				25.8			11.8			10.8	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				25.8	11.8			10.8			
Approach LOS				<i>C</i>	<i>B</i>			<i>B</i>			
Intersection Delay	16.4		$X_c = 0.66$		Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		488	262	175	804		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	536	287	192	883	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		192					
C (m) (veh/h)		790					
v/c		0.24					
95% queue length		0.95					
Control Delay (s/veh)		11.0					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				104		186		547	90	130	649	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					326			700			856	
Lane Group Capacity, c					394			1130			763	
v/c Ratio, X					0.83			0.62			1.12	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.7			8.7			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					16.9			2.0			60.2	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				37.6			10.7			73.2		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>E</i>		
Approach Delay				37.6				10.7				73.2
Approach LOS				<i>D</i>				<i>B</i>				<i>E</i>
Intersection Delay	43.8			$X_c = 1.03$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	101	45	91	29	88	35	133	707	138	26	560	96
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		304			209		145	918			720	
Lane Group Capacity, c		520			544		318	1065			1276	
v/c Ratio, X		0.58			0.38		0.46	0.86			0.56	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		24.2			22.3		12.4	17.6			13.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.6			2.0		2.6	5.4			1.7	

Initial Queue Delay, d_3		0.0		0.0		0.0	0.0			0.0	
Control Delay		26.8		24.3		15.1	23.0			15.2	
Lane Group LOS		C		C		B	C			B	
Approach Delay		26.8		24.3		22.0				15.2	
Approach LOS		C		C		C				B	
Intersection Delay		20.7		$X_c = 0.75$		Intersection LOS				C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description Charleston EIS							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					625		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	679	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			55				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	62	0	0	0	
Percent Heavy Vehicles	0	0	5	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							62
C (m) (veh/h)							696
v/c							0.09
95% queue length							0.29
Control Delay (s/veh)							10.7
LOS							B
Approach Delay (s/veh)	--	--				10.7	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	381							843			625	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	443							878			679	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.66							0.41			0.35	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.7							11.5			11.0	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	5.1							0.3			0.5	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	30.8						11.9			11.5	
Lane Group LOS	C						B			B	
Approach Delay	30.8						11.9		11.5		
Approach LOS	C						B		B		
Intersection Delay	15.9		$X_C = 0.51$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					560		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	602	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				228			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	253	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			253				
C (m) (veh/h)			464				
v/c			0.55				
95% queue length			3.21				
Control Delay (s/veh)			21.7				
LOS			C				
Approach Delay (s/veh)	--	--	21.7				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD					Analysis Year	2015 No-Action Conditions					
	Peak					Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	96	185	202	263	429	46	126	183	280	43	96	89
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	103	365		274	595			654		51	220	
Lane Group Capacity, c	195	584		221	1196			929		384	362	
v/c Ratio, X	0.53	0.63		1.24	0.50			0.70		0.13	0.61	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	24.3	25.3		29.5	23.3			29.2		28.1	31.5	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	7.0	3.5		137.7	1.3			4.5		0.6	6.4	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	31.3	28.8		167.2	24.7			33.6		28.7	37.8	
Lane Group LOS	C	C		F	C			C		C	D	
Approach Delay	29.3			69.6				33.6		36.1		
Approach LOS	C			E				C		D		
Intersection Delay	46.9			$X_c = 0.90$				Intersection LOS		D		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	0	193	32	107	173	0			
%Thrus Left Lane	50			50					
Approach	Northbound				Southbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	55	0	92	0	0	0			
%Thrus Left Lane									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	T	TR	LT	T	L	R			
PHF	0.85	0.85	0.91	0.91	0.89	0.89			
Flow Rate (veh/h)	112	151	211	95	61	103			
% Heavy Vehicles	0	0	0	1	1	1			
No. Lanes	2		2		2		0		
Geometry Group	5		5		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0	0.0	0.6	0.0	1.0	0.0			
Prop. Right-Turns	0.0	0.2	0.0	0.0	0.0	1.0			
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0			
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2			
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.0	-0.2	0.3	0.0	0.2	-0.6			
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20			
x, initial	0.10	0.13	0.19	0.08	0.05	0.09			
hd, final value (s)	5.23	5.06	5.45	5.19	5.41	4.61			
x, final value	0.16	0.21	0.32	0.14	0.09	0.13			
Move-up time, m (s)	2.3		2.3		2.0				
Service Time, t _s (s)	2.9	2.8	3.2	2.9	3.4	2.6			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	362	401	461	345	311	353			
Delay (s/veh)	8.95	9.12	10.70	8.72	8.95	8.31			
LOS	A	A	B	A	A	A			
Approach: Delay (s/veh)	9.05		10.08		8.55				
LOS	A		B		A				
Intersection Delay (s/veh)	9.37								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	79	123	264	161	357	24	229	200	112	25	521	162
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.830			0.865		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		507			695		244	343			761	
Lane Group Capacity, c		1016			1040		260	817			1328	
v/c Ratio, X		0.50			0.67		0.94	0.42			0.57	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.8			19.8		23.8	17.1			18.6	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.5			3.0		42.0	1.6			1.8	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	19.3	22.7	65.8	18.7
Lane Group LOS	B	C	E	B
Approach Delay	19.3	22.7	38.3	20.4
Approach LOS	B	C	D	C
Intersection Delay	24.9	$X_c = 0.80$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	849	375			420	521				166		206
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	875	391			447	414				180		224
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	0.98	0.33			0.97	1.06				0.31		0.23
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	20.9	4.9			33.7	34.0				32.2		6.7
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	16.8	0.4			12.1	36.9				1.4		0.6
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

Control Delay	37.6	5.3			45.8	70.9				33.6		7.3
Lane Group LOS	<i>D</i>	<i>A</i>			<i>D</i>	<i>E</i>				<i>C</i>		<i>A</i>
Approach Delay	27.6			57.9						19.0		
Approach LOS	<i>C</i>			<i>E</i>						<i>B</i>		
Intersection Delay	36.6			$X_c = 0.87$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	497	45	0	0	45	56	0	0	1	81	0	896
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	518	47			95			1			87	535
Lane Group Capacity, c	738	1066			1633			1184			546	622
v/c Ratio, X	0.70	0.04			0.06			0.00			0.16	0.86
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	17.3	11.5			11.6			16.8			17.9	25.3
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	5.3	0.1			0.1			0.0			0.5	12.7

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	22.7	11.6			11.7			16.8			18.4	37.9
Lane Group LOS	C	B			B			B			B	D
Approach Delay	21.8				11.7				16.8		35.2	
Approach LOS	C				B				B		D	
Intersection Delay	27.5				$X_c = 0.77$				Intersection LOS		C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Saturday 12:45-1:45 PM					Jurisdiction	Staten Island, NY					
	Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	250		34				5	256			508	461
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	294		29					297			1089	
Lane Group Capacity, c	573		497					1702			1948	
v/c Ratio, X	0.51		0.06					0.17			0.56	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	16.7		14.3					7.7			9.9	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	2.9		0.2					0.2			0.6	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	19.6		14.5					7.9			10.4	
Lane Group LOS	B		B					A			B	
Approach Delay	19.2						7.9			10.4		
Approach LOS	B						A			B		
Intersection Delay	11.6			$X_c = 0.54$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2015 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		28					
Peak-Hour Factor, PHF	1.00	0.69	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	40	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			501				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	569	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					569		
C (m) (veh/h)					1037		
v/c					0.55		
95% queue length					3.44		
Control Delay (s/veh)					12.6		
LOS					B		
Approach Delay (s/veh)	--	--	12.6				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	685	2		5			27	285	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		565	190		6				351		
Lane Group Capacity, c		784		606	582		394				2034		
v/c Ratio, X		0.01		0.93	0.33		0.02				0.17		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		16.7	11.8		10.3				11.0		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		22.7	1.4		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	39.4	13.3	10.3	11.2
Lane Group LOS	B	D	B	B	B
Approach Delay	10.2	32.9	10.3	11.2	
Approach LOS	B	C	B	B	
Intersection Delay	25.8	$X_c = 0.55$	Intersection LOS	C	

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	75	34	163	58	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	17	116	22			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT		TR
PHF	0.75		0.85				0.89	0.89	
Flow Rate (veh/h)	145		259				84	89	
% Heavy Vehicles	2		4				0	0	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.2		0.2				0.0	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.13		0.23				0.07	0.08	
hd, final value (s)	4.48		4.70				4.90	4.69	
x, final value	0.18		0.34				0.11	0.12	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.5		2.7				2.9	2.7	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	395		509				334	339	
Delay (s/veh)	8.46		10.08				8.53	8.31	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.46		10.08				8.42		
LOS	A		B				A		
Intersection Delay (s/veh)	9.17								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2015 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	25	66	0		0	169	23		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	51	103	135		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.75		0.85		0.95		0.95		
Flow Rate (veh/h)	121		225		106		196		
% Heavy Vehicles	2		2		1		1		
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.3		0.0		0.5	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.1		-0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20		3.20		
x, initial	0.11		0.20		0.09		0.17		
hd, final value (s)	4.94		4.68		4.87		4.33		
x, final value	0.17		0.29		0.14		0.24		
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	2.9		2.7		2.9		2.3		
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	371		475		356		446		
Delay (s/veh)	8.92		9.61		8.68		8.66		
LOS	A		A		A		A		
Approach: Delay (s/veh)	8.92		9.61		8.67				
LOS	A		A		A				
Intersection Delay (s/veh)	9.04								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	211	179	137	0	61	15	625	63	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		520	95		91			743				
Lane Group Capacity, c		532	560		557			2267				
v/c Ratio, X		0.98	0.17		0.16			0.33				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.3	13.5		13.4			9.5				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		33.8	0.7		0.6			0.4				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		53.0	14.1		14.1			9.9			
Lane Group LOS		D	B		B			A			
Approach Delay		47.0			14.1			9.9			
Approach LOS		D			B			A			
Intersection Delay		25.9			$X_c = 0.60$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	128		54				17	388			367	59
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		198						440			468	
Lane Group Capacity, c		461						1111			1185	
v/c Ratio, X		0.43						0.40			0.39	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.2						7.3			7.3	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.7						1.0			0.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	20.9					8.3			8.2
Lane Group LOS	C					A			A
Approach Delay	20.9					8.3	8.2		
Approach LOS	C					A	A		
Intersection Delay	10.5	$X_c = 0.41$				Intersection LOS		B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	138		65				42	412			371	150
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		271						494			551	
Lane Group Capacity, c		575						859			1029	
v/c Ratio, X		0.47						0.58			0.54	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.8						10.5			10.2	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.6						2.4			1.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		18.4					12.9			12.1	
Lane Group LOS		B					B			B	
Approach Delay		18.4					12.9			12.1	
Approach LOS		B					B			B	
Intersection Delay		13.7			$X_c = 0.53$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		119	285	596	168	79	218			404	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		168		1206		86	237			468	
Lane Group Capacity, c	222		220		1338		148	621			707	
v/c Ratio, X	0.12		0.76		0.90		0.58	0.38			0.66	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		25.1		20.7		16.5	15.3			17.1	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.8		16.4		5.3		14.4	1.6			3.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.7		41.5		26.0		30.9	16.9			20.7	
Lane Group LOS	C		D		C		C	B			C	
Approach Delay	39.0		26.0		20.6		20.7					
Approach LOS	D		C		C		C					
Intersection Delay	25.2		$X_c = 0.77$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	30	32	5				3	267	143	305	457	46
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.899		0.644	0.644	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0		0	0	
Min. Time for Pedestrians, G _p		17.5		16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		88						448		339	547	
Lane Group Capacity, c		462						966		508	1024	
v/c Ratio, X		0.19						0.46		0.67	0.53	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.3						8.2		9.6	8.6	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.9						1.4		4.4	1.3	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.2					9.6		14.0	9.9	
Lane Group LOS		B					A		B	A	
Approach Delay		17.2					9.6		11.5		
Approach LOS		B					A		B		
Intersection Delay		11.2		$X_c = 0.51$		Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	7	7	18	284	18	102	12	262	234	83	365	7
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		37		316	20	113		552			506	
Lane Group Capacity, c		628		580	808	601		735			635	
v/c Ratio, X		0.06		0.54	0.02	0.19		0.75			0.80	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.3	14.0	15.2		20.8			21.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.2	0.1	0.6		3.7			5.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.4	21.6	14.1	15.8	24.5	26.8
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.4	19.8	24.5	26.8		
Approach LOS	B	B	C	C		
Intersection Delay	23.6	$X_c = 0.67$	Intersection LOS	C		

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	AECOM					Area Type	All other areas								
Date Performed	3/31/13					Jurisdiction	Staten Island, NY								
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 No-Action Conditions								
						Project ID	Charleston EIS								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		147	232	71	138		147		55						
% Heavy Vehicles, %HV		2	2	4	4		1		1						
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.927			0.992			0.953							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		2			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		414			235			238							
Lane Group Capacity, c		970			759			575							
v/c Ratio, X		0.43			0.31			0.41							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		14.3			13.3			20.0							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		1.3			1.1			2.1							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay		15.6			14.4			22.1			
Lane Group LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Approach Delay		15.6			14.4			22.1			
Approach LOS		<i>B</i>			<i>B</i>			<i>C</i>			
Intersection Delay		17.0			$X_c = 0.42$			Intersection LOS			<i>B</i>

No-Action

2020

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	27	152	42	29	155	39	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	
Hourly Flow Rate, HFR (veh/h)	32	183	50	34	186	46	
Percent Heavy Vehicles	20	--	--	13	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	9	8	8	62	10	28	
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.95	0.95	0.95	
Hourly Flow Rate, HFR (veh/h)	11	10	10	65	10	29	
Percent Heavy Vehicles	32	32	32	5	5	5	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	32	34	104			31	
C (m) (veh/h)	1237	1272	468			434	
v/c	0.03	0.03	0.22			0.07	
95% queue length	0.08	0.08	0.84			0.23	
Control Delay (s/veh)	8.0	7.9	14.9			13.9	
LOS	A	A	B			B	
Approach Delay (s/veh)	--	--	14.9			13.9	
Approach LOS	--	--	B			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		205	32	20	203		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	246	38	24	244	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				8		16	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.69	1.00	0.69	
Hourly Flow Rate, HFR (veh/h)	0	0	0	11	0	23	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		24		34			
C (m) (veh/h)		1228		651			
v/c		0.02		0.05			
95% queue length		0.06		0.16			
Control Delay (s/veh)		8.0		10.8			
LOS		A		B			
Approach Delay (s/veh)	--	--	10.8				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group	LTR			LT R			LTR			LTR		
Volume, V (vph)	3	6	1	167	34	272	0	146	302	85	116	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			231	313		527			241	
Lane Group Capacity, c		718			540	514		702			402	
v/c Ratio, X		0.02			0.43	0.61		0.75			0.60	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.4	13.7		14.9			13.6	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.0			2.3	5.0		6.5			6.5	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			14.7	18.7		21.3			20.1	
Lane Group LOS		B			B	B		C			C	
Approach Delay		10.3			17.0			21.3			20.1	
Approach LOS		B			B			C			C	
Intersection Delay		19.2			$X_c = 0.68$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				268		5		443			285	
% Heavy Vehicles, %HV				8		8		11			15	
Peak-Hour Factor, PHF				0.88		0.88		0.85			0.87	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.893			0.886			0.754	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			3	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					311			521			328	
Lane Group Capacity, c					632			970			925	
v/c Ratio, X					0.49			0.54			0.35	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					15.9			10.3			9.1	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					2.4			1.9			0.8	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				18.4			12.1			9.9	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>A</i>	
Approach Delay				18.4	12.1			9.9			
Approach LOS				<i>B</i>	<i>B</i>			<i>A</i>			
Intersection Delay	13.2		$X_c = 0.52$			Intersection LOS			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		443	224	120	433		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.87	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	521	263	137	497	0	
Percent Heavy Vehicles	0	--	--	20	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		137					
C (m) (veh/h)		760					
v/c		0.18					
95% queue length		0.65					
Control Delay (s/veh)		10.8					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				62		153		512	74	98	328	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.980			0.769			0.842	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					232			689			490	
Lane Group Capacity, c					383			1032			722	
v/c Ratio, X					0.61			0.67			0.68	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					19.2			9.1			9.2	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					6.8			2.6			4.3	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				26.1			11.7			13.5	
Lane Group LOS				C			B			B	
Approach Delay				26.1				11.7			
Approach LOS				C				B			
Intersection Delay	14.7		$X_c = 0.66$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	72	27	73	24	133	46	66	660	112	65	360	29
% Heavy Vehicles, %HV	1	1	1	1	1	1	3	3	3	9	9	9
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.94	0.94	0.94	0.92	0.92	0.92	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.810			1.000		0.649	0.649			0.986	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	6
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		191			216		72	839			533	
Lane Group Capacity, c		551			575		403	1048			972	
v/c Ratio, X		0.35			0.38		0.18	0.80			0.55	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.0			22.2		10.3	16.6			13.3	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.4			1.9		0.6	4.3			2.2	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	23.4	24.1	11.0	20.8
Lane Group LOS	C	C	B	C
Approach Delay	23.4	24.1	20.0	15.5
Approach LOS	C	C	C	B
Intersection Delay	19.6	$X_c = 0.64$	Intersection LOS	B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					394		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	469	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			59				
Peak-Hour Factor, PHF	1.00	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	84	0	0	0	
Percent Heavy Vehicles	0	0	60	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							84
C (m) (veh/h)							681
v/c							0.12
95% queue length							0.42
Control Delay (s/veh)							11.0
LOS							B
Approach Delay (s/veh)	--	--				11.0	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	284							778			394	
% Heavy Vehicles, %HV	2							1			3	
Peak-Hour Factor, PHF	0.90							0.91			0.84	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.665			0.986	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	316							855			469	
Lane Group Capacity, c	669							2156			1951	
v/c Ratio, X	0.47							0.40			0.24	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.7							11.4			10.3	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.4							0.4			0.3	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.1						11.8			10.5	
Lane Group LOS	C						B			B	
Approach Delay	26.1						11.8			10.5	
Approach LOS	C						B			B	
Intersection Delay	14.2			$X_C = 0.43$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					394		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	414	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				287			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	308	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			308				
C (m) (veh/h)			583				
v/c			0.53				
95% queue length			3.08				
Control Delay (s/veh)			17.9				
LOS			C				
Approach Delay (s/veh)	--	--	17.9				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	50	141	164	203	343	79	84	230	138	67	27	27
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	57	303		233	480			485		100	80	
Lane Group Capacity, c	241	577		240	1087			894		376	354	
v/c Ratio, X	0.24	0.53		0.97	0.44			0.54		0.27	0.23	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	21.7	24.2		29.6	23.5			27.6		28.9	28.7	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	1.8	2.7		50.5	1.3			2.4		1.7	1.5	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	23.5	26.9		80.0	24.7			30.0		30.6	30.1	
Lane Group LOS	C	C		F	C			C		C	C	
Approach Delay	26.4			42.8				30.0		30.4		
Approach LOS	C			D				C		C		
Intersection Delay	34.5			$X_c = 0.64$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	64	18	33	75	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	11	0	22	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.80	0.80	0.78	0.78	0.86	0.86		
Flow Rate (veh/h)	39	61	89	48	12	25		
% Heavy Vehicles	21	21	1	1	5	5		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.5	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.2	0.2	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.4	0.1	0.3	0.0	0.3	-0.5		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.03	0.05	0.08	0.04	0.01	0.02		
hd, final value (s)	5.04	4.79	4.91	4.67	4.73	3.93		
x, final value	0.05	0.08	0.12	0.06	0.02	0.03		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	2.7	2.5	2.6	2.4	2.7	1.9		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	289	311	339	298	262	275		
Delay (s/veh)	8.03	7.91	8.29	7.68	7.80	7.04		
LOS	A	A	A	A	A	A		
Approach: Delay (s/veh)	7.96		8.08		7.29			
LOS	A		A		A			
Intersection Delay (s/veh)	7.93							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	54	29	250	115	269	6	256	74	51	11	98	102
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.916			0.953		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		378			448		294	144			277	
Lane Group Capacity, c		1080			1113		488	788			1191	
v/c Ratio, X		0.35			0.40		0.60	0.18			0.23	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		16.4			16.9		19.0	15.1			15.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		0.8			1.0		5.4	0.5			0.5	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	17.3	17.9	24.4	15.6	15.9
Lane Group LOS	B	B	C	B	B
Approach Delay	17.3	17.9	21.5	15.9	
Approach LOS	B	B	C	B	
Intersection Delay	18.4	$X_c = 0.50$	Intersection LOS		B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	802	241	19	4	275	237	4	16	6	107	5	145
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.86	0.86	0.86	0.37	0.37	0.37	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.603	0.603			0.932	0.932		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	881	286			325	276		70		123	6	167
Lane Group Capacity, c	847	1158			449	383		355		226	320	952
v/c Ratio, X	1.04	0.25			0.72	0.72		0.20		0.54	0.02	0.18
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	18.3	4.6			31.2	31.2		31.5		33.7	30.5	6.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	35.1	0.3			9.1	10.4		1.2		9.1	0.1	0.4

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	53.5	4.9			40.3	41.6		32.8		42.8	30.6	6.8
Lane Group LOS	D	A			D	D		C		D	C	A
Approach Delay	41.6			40.9			32.8			22.2		
Approach LOS	D			D			C			C		
Intersection Delay	38.4			$X_c = 0.99$			Intersection LOS			D		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	329	26	0	14	68	38	6	22	11	15	38	412
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	362	29			153			86			70	338
Lane Group Capacity, c	699	1066			1543			1234			673	616
v/c Ratio, X	0.52	0.03			0.10			0.07			0.10	0.55
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.2	11.4			11.8			17.3			17.5	21.4
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	2.7	0.0			0.1			0.1			0.3	3.4

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	17.8	11.5			12.0			17.4			17.8	24.8
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	17.4			12.0			17.4			23.6		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	19.0			$X_c = 0.53$			Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 8-9 AM Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	82		4				4	87			386	219
% Heavy Vehicles, %HV	11		11				2	2			1	1
Peak-Hour Factor, PHF	0.80		0.80				0.80	0.80			0.81	0.81
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.995		0.995					1.000			0.752	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	4	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	102		0					114			747	
Lane Group Capacity, c	532		461					1679			1983	
v/c Ratio, X	0.19		0.00					0.07			0.38	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	14.9		14.0					7.3			8.7	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	0.8		0.0					0.1			0.4	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	15.7		14.0					7.3			9.1	
Lane Group LOS	B		B					A			A	
Approach Delay	15.7							7.3		9.1		
Approach LOS	B							A		A		
Intersection Delay	9.6			$X_c = 0.31$				Intersection LOS		A		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		25					
Peak-Hour Factor, PHF	1.00	0.82	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	30	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			165				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	211	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					211		
C (m) (veh/h)					1047		
v/c					0.20		
95% queue length					0.75		
Control Delay (s/veh)					9.3		
LOS					A		
Approach Delay (s/veh)	--	--	9.3				
Approach LOS	--	--	A				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	420	1		3			24	185	0	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		266	267		4				258		
Lane Group Capacity, c		776		600	576		428				2052		
v/c Ratio, X		0.01		0.44	0.46		0.01				0.13		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.5	12.7		10.2				10.8		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.3	2.6		0.0				0.1		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		14.8	15.3		10.3				10.9	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.1			10.3			10.9		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		13.7		$X_c = 0.29$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY				
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions				
Analysis Time Period	Weekday 8-9 AM Peak Period									
Project ID <i>Charleston EIS</i>										
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	57	22	120	72	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	11	67	28				
%Thrus Left Lane	50									
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT		TR	
PHF	0.80		0.79				0.81		0.81	
Flow Rate (veh/h)	98		242				53		75	
% Heavy Vehicles	24		5				3		3	
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.6				0.2	0.0		
Prop. Right-Turns	0.3		0.0				0.0	0.5		
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	0.2		0.2				0.1	-0.2		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20		3.20	
x, initial	0.09		0.22				0.05		0.07	
hd, final value (s)	4.72		4.53				4.81		4.48	
x, final value	0.13		0.30				0.07		0.09	
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.7		2.5				2.8		2.5	
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	348		492				303		325	
Delay (s/veh)	8.41		9.49				8.17		7.95	
LOS	A		A				A		A	
Approach: Delay (s/veh)	8.41		9.49				8.04			
LOS	A		A				A			
Intersection Delay (s/veh)	8.87									
Intersection LOS	A									

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	6	61	0		0	134	18		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	58	53	46		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.80		0.79		0.87		0.87		
Flow Rate (veh/h)	83		191		95		83		
% Heavy Vehicles	17		7		1		1		
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.1		0.0		0.7	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.6			
Prop. Heavy Vehicle	0.2		0.1		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.2	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.07		0.17		0.08	0.07			
hd, final value (s)	4.82		4.45		4.71	4.19			
x, final value	0.11		0.24		0.12	0.10			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	2.8		2.5		2.7	2.2			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	333		441		345		333		
Delay (s/veh)	8.43		8.82		8.37		7.64		
LOS	A		A		A		A		
Approach: Delay (s/veh)	8.43		8.82		8.03				
LOS	A		A		A				
Intersection Delay (s/veh)	8.44								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	79	65	45	6	40	1	381	77	1			
% Heavy Vehicles, %HV	3	3	3	1	1	1	3	3	3			
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.76	0.76	0.76			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.998			0.978				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	21	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		182	30		60			603				
Lane Group Capacity, c		540	549		546			2253				
v/c Ratio, X		0.34	0.05		0.11			0.27				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		14.4	12.9		13.2			9.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		1.7	0.2		0.4			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		16.1	13.1		13.6			9.5			
Lane Group LOS		B	B		B			A			
Approach Delay		15.6			13.6			9.5			
Approach LOS		B			B			A			
Intersection Delay		11.3			$X_c = 0.30$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	48		18				16	351			478	31
% Heavy Vehicles, %HV	1		1				7	7			9	9
Peak-Hour Factor, PHF	0.74		0.74				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.997						0.936			0.808	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		89						412			592	
Lane Group Capacity, c		462						1012			1102	
v/c Ratio, X		0.19						0.41			0.54	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.0						7.3			8.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.9						1.1			1.5	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	17.9			8.5		9.6
Lane Group LOS	B			A		A
Approach Delay	17.9			8.5		9.6
Approach LOS	B			A		A
Intersection Delay	9.9	$X_c = 0.43$		Intersection LOS		A

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	56		53				30	379			292	122
% Heavy Vehicles, %HV	14		14				8	8			11	11
Peak-Hour Factor, PHF	0.80		0.80				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.982						0.866			0.927	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	17
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		136						460			462	
Lane Group Capacity, c		502						805			928	
v/c Ratio, X		0.27						0.57			0.50	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						10.5			10.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						2.5			1.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		16.0					13.0			11.8	
Lane Group LOS		B					B			B	
Approach Delay	16.0						13.0		11.8		
Approach LOS	B						B		B		
Intersection Delay	12.9			$X_c = 0.45$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		58	244	395	164	42	200			475	22
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		73		912		47	222			654	
Lane Group Capacity, c	218		216		1314		120	575			661	
v/c Ratio, X	0.02		0.34		0.69		0.39	0.39			0.99	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		23.6		19.2		15.3	15.3			19.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		4.1		2.2		8.8	1.9			16.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		27.7		21.4		24.2	17.2			36.5	
Lane Group LOS	C		C		C		C	B			D	
Approach Delay	27.4		21.4		18.4		36.5					
Approach LOS	C		C		B		D					
Intersection Delay	26.4		$X_c = 0.76$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	12	39	11				5	230	98	260	485	32
% Heavy Vehicles, %HV	3	3	3				7	7	7	7	7	7
Peak-Hour Factor, PHF	0.85	0.85	0.85				0.90	0.90	0.90	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.941		0.518	0.518	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7		0	2	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		73						371		325	646	
Lane Group Capacity, c		457						897		532	960	
v/c Ratio, X		0.16						0.41		0.61	0.67	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.1						7.9		9.1	9.6	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.7						1.3		2.7	2.0	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		16.9					9.2		11.9	11.6	
Lane Group LOS		B					A		B	B	
Approach Delay		16.9					9.2		11.7		
Approach LOS		B					A		B		
Intersection Delay		11.3		$X_c = 0.50$		Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	20	13	172	16	105	10	183	131	139	301	9
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		60		195	18	119		360			591	
Lane Group Capacity, c		648		548	777	574		691			547	
v/c Ratio, X		0.09		0.36	0.02	0.21		0.52			1.08	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.5		16.5	14.0	15.3		18.1			25.0	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		1.7	0.1	0.8		2.4			39.8	

Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.8		18.2	14.1	16.1		20.5			64.8	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>	<i>B</i>		<i>C</i>			<i>E</i>	
Approach Delay		14.8		17.2				20.5			64.8	
Approach LOS		<i>B</i>		<i>B</i>				<i>C</i>			<i>E</i>	
Intersection Delay		38.9		$X_c = 0.72$				Intersection LOS			<i>D</i>	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/28/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		140	189	77	172		225		75				
% Heavy Vehicles, %HV		10	10	13	13		9		9				
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.964			0.986			0.858					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		310			280			319					
Lane Group Capacity, c		893			769			534					
v/c Ratio, X		0.35			0.36			0.60					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.6			13.8			21.9					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.0			1.3			4.2					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay		14.6			15.1			26.1			
Lane Group LOS		B			B			C			
Approach Delay		14.6			15.1			26.1			
Approach LOS		B			B			C			
Intersection Delay		18.8			$X_C = 0.47$			Intersection LOS			B

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	37	215	36	32	227	23	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	43	252	42	35	249	25	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	24	17	39	35	19	25	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	30	21	50	39	21	28	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	43	35		88			101
C (m) (veh/h)	1255	1182		363			443
v/c	0.03	0.03		0.24			0.23
95% queue length	0.11	0.09		0.93			0.87
Control Delay (s/veh)	8.0	8.1		18.1			15.5
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		18.1			15.5
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	37	215	36	32	227	23	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	43	252	42	35	249	25	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	24	17	39	35	19	25	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	30	21	50	39	21	28	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	43	35		88			101
C (m) (veh/h)	1255	1182		363			443
v/c	0.03	0.03		0.24			0.23
95% queue length	0.11	0.09		0.93			0.87
Control Delay (s/veh)	8.0	8.1		18.1			15.5
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		18.1			15.5
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		273	18	19	282		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	321	21	20	309	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				44		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	46	0	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		20		61			
C (m) (veh/h)		1148		460			
v/c		0.02		0.13			
95% queue length		0.05		0.45			
Control Delay (s/veh)		8.2		14.0			
LOS		A		B			
Approach Delay (s/veh)	--	--	14.0				
Approach LOS	--	--	B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		273	18	19	282		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	321	21	20	309	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				44		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	46	0	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0		0
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		20		61			
C (m) (veh/h)		1148		460			
v/c		0.02		0.13			
95% queue length		0.05		0.45			
Control Delay (s/veh)		8.2		14.0			
LOS		A		B			
Approach Delay (s/veh)	--	--	14.0				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	232	24	382	1	146	261	144	181	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			285	424		444			378	
Lane Group Capacity, c		713			524	518		708			486	
v/c Ratio, X		0.04			0.54	0.82		0.63			0.78	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.2	15.5		13.8			15.1	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			3.6	12.1		3.9			11.6	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.5			16.8	27.6		17.7			26.7
Lane Group LOS		B			B	C		B			C
Approach Delay		10.5			23.2			17.7			26.7
Approach LOS		B			C			B			C
Intersection Delay		22.3			$X_c = 0.80$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	232	24	382	1	146	261	144	181	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			285	424		444			378	
Lane Group Capacity, c		713			524	518		708			486	
v/c Ratio, X		0.04			0.54	0.82		0.63			0.78	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.2	15.5		13.8			15.1	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			3.6	12.1		3.9			11.6	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.5			16.8	27.6		17.7			26.7
Lane Group LOS		B			B	C		B			C
Approach Delay		10.5			23.2			17.7			26.7
Approach LOS		B			C			B			C
Intersection Delay		22.3			$X_c = 0.80$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				359		3		402			420	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					411			437			488	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.64			0.45			0.52	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.9			9.7			10.1	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					4.1			1.3			1.1	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				21.1			11.0			11.3	
Lane Group LOS				C			B			B	
Approach Delay				21.1				11.0			
Approach LOS				C				B			
Intersection Delay	14.2			$X_c = 0.56$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				359		3		402			420	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					411			437			488	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.64			0.45			0.52	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.9			9.7			10.1	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					4.1			1.3			1.1	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				21.1			11.0			11.3	
Lane Group LOS				C			B			B	
Approach Delay				21.1				11.0			
Approach LOS				C				B			
Intersection Delay	14.2		$X_c = 0.56$		Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		402	242	141	637		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	436	263	163	740	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		163					
C (m) (veh/h)		836					
v/c		0.19					
95% queue length		0.72					
Control Delay (s/veh)		10.3					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		402	242	141	637		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	436	263	163	740	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		163					
C (m) (veh/h)		836					
v/c		0.19					
95% queue length		0.72					
Control Delay (s/veh)		10.3					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				102		197		436	80	185	434	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					343			561			720	
Lane Group Capacity, c					386			1056			629	
v/c Ratio, X					0.89			0.53			1.14	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.1			8.1			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					23.8			1.7			74.9	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				45.0			9.7			87.9
Lane Group LOS				<i>D</i>			<i>A</i>			<i>F</i>
Approach Delay				45.0	9.7			87.9		
Approach LOS				<i>D</i>	<i>A</i>			<i>F</i>		
Intersection Delay	51.8		$X_c = 1.06$		Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				102		197		436	80	185	434	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					343			561			720	
Lane Group Capacity, c					386			1056			629	
v/c Ratio, X					0.89			0.53			1.14	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.1			8.1			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					23.8			1.7			74.9	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				45.0			9.7			87.9
Lane Group LOS				<i>D</i>			<i>A</i>			<i>F</i>
Approach Delay				45.0	9.7			87.9		
Approach LOS				<i>D</i>	<i>A</i>			<i>F</i>		
Intersection Delay	51.8		$X_c = 1.06$		Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0	
Lane Group		LTR			LTR		L	TR			LTR		
Volume, V (vph)	144	65	85	39	177	64	98	571	137	115	453	69	
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14	
Lane Width		16.0			15.0		10.0	14.0			10.5		
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y	
Parking Maneuvers, N _m						5						5	
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		342			304		108	778			700		
Lane Group Capacity, c		423			550		324	1050			903		
v/c Ratio, X		0.81			0.55		0.33	0.74			0.78		
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54		
Uniform Delay, d ₁		26.8			23.9		11.4	15.7			16.2		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		10.4			4.0		2.0	3.5			6.3		

Initial Queue Delay, d_3		0.0		0.0		0.0	0.0			0.0	
Control Delay		37.2		27.9		13.4	19.1			22.5	
Lane Group LOS		D		C		B	B			C	
Approach Delay		37.2		27.9		18.4				22.5	
Approach LOS		D		C		B				C	
Intersection Delay		23.9		$X_c = 0.79$		Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	144	65	85	39	177	64	98	571	137	115	453	69
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		16.6			21.5			16.3			21.2	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		342			304		108	778			700	
Lane Group Capacity, c		423			550		324	1050			903	
v/c Ratio, X		0.81			0.55		0.33	0.74			0.78	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		26.8			23.9		11.4	15.7			16.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		10.4			4.0		2.0	3.5			6.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	37.2	27.9	13.4	19.1	22.5
Lane Group LOS	D	C	B	B	C
Approach Delay	37.2	27.9	18.4	22.5	
Approach LOS	D	C	B	C	
Intersection Delay	23.9	$X_c = 0.79$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					545		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	612	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			91				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	108	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							108
C (m) (veh/h)							675
v/c							0.16
95% queue length							0.57
Control Delay (s/veh)							11.3
LOS							B
Approach Delay (s/veh)	--	--				11.3	
Approach LOS	--	--				B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					545		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	612	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			91				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	108	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							108
C (m) (veh/h)							675
v/c							0.16
95% queue length							0.57
Control Delay (s/veh)							11.3
LOS							B
Approach Delay (s/veh)	--	--				11.3	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	292							778			545	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	332							855			612	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.50							0.40			0.31	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.0							11.4			10.8	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.7							0.4			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.7						11.8			11.2	
Lane Group LOS	C						B			B	
Approach Delay	26.7						11.8		11.2		
Approach LOS	C						B		B		
Intersection Delay	14.4		$X_C = 0.44$		Intersection LOS		B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	292							778			545	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	332							855			612	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.50							0.40			0.31	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.0							11.4			10.8	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.7							0.4			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.7						11.8			11.2	
Lane Group LOS	C						B			B	
Approach Delay	26.7						11.8		11.2		
Approach LOS	C						B		B		
Intersection Delay	14.4		$X_C = 0.44$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/25/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					383		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	440	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				188			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	202	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			202				
C (m) (veh/h)			571				
v/c			0.35				
95% queue length			1.59				
Control Delay (s/veh)			14.7				
LOS			B				
Approach Delay (s/veh)	--	--	14.7				
Approach LOS	--	--	B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/25/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					383		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	440	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				188			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	202	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			202				
C (m) (veh/h)			571				
v/c			0.35				
95% queue length			1.59				
Control Delay (s/veh)			14.7				
LOS			B				
Approach Delay (s/veh)	--	--	14.7				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	94	149	140	195	478	83	88	228	316	148	49	40
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	112	306		217	609			680		185	111	
Lane Group Capacity, c	188	587		242	1116			903		380	360	
v/c Ratio, X	0.60	0.52		0.90	0.55			0.75		0.49	0.31	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	25.0	24.2		28.5	24.4			29.7		30.5	29.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	11.4	2.8		34.4	1.8			5.8		4.3	2.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	36.3	27.0		62.9	26.2			35.5		34.9	31.4	
Lane Group LOS	D	C		E	C			D		C	C	
Approach Delay	29.5			35.9				35.5		33.6		
Approach LOS	C			D				D		C		
Intersection Delay	34.2			$X_c = 0.74$				Intersection LOS		C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	94	149	140	195	478	83	88	228	316	148	49	40
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	112	306		217	609			680		185	111	
Lane Group Capacity, c	188	587		242	1116			903		380	360	
v/c Ratio, X	0.60	0.52		0.90	0.55			0.75		0.49	0.31	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	25.0	24.2		28.5	24.4			29.7		30.5	29.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	11.4	2.8		34.4	1.8			5.8		4.3	2.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	36.3	27.0		62.9	26.2			35.5		34.9	31.4	
Lane Group LOS	D	C		E	C			D		C	C	
Approach Delay	29.5			35.9			35.5			33.6		
Approach LOS	C			D			D			C		
Intersection Delay	34.2			$X_c = 0.74$			Intersection LOS			C		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/25/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	0	115	32	130	126	0			
%Thrus Left Lane	50			50					
Approach	Northbound				Southbound				
Movement	L	T	R	L	T	R			
Volume (veh/h)	40	0	41	0	0	0			
%Thrus Left Lane									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	T	TR	LT	T	L	R			
PHF	0.88	0.88	0.90	0.90	0.78	0.78			
Flow Rate (veh/h)	64	101	214	70	51	52			
% Heavy Vehicles	42	42	4	4	3	3			
No. Lanes	2		2		2		0		
Geometry Group	5		5		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0			
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0			
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0			
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2			
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.7	0.5	0.4	0.1	0.3	-0.5			
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20			
x, initial	0.06	0.09	0.19	0.06	0.05	0.05			
hd, final value (s)	5.74	5.49	5.33	4.99	5.23	4.43			
x, final value	0.10	0.15	0.32	0.10	0.07	0.06			
Move-up time, m (s)	2.3		2.3		2.0				
Service Time, t _s (s)	3.4	3.2	3.0	2.7	3.2	2.4			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	314	351	464	320	301	302			
Delay (s/veh)	9.09	9.18	10.48	8.23	8.65	7.74			
LOS	A	A	B	A	A	A			
Approach: Delay (s/veh)	9.15		9.92		8.19				
LOS	A		A		A				
Intersection Delay (s/veh)	9.37								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/25/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	0	115	32		130	126	0		
%Thrus Left Lane	50				50				
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	40	0	41		0	0	0		
%Thrus Left Lane									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	T	TR	LT	T	L	R			
PHF	0.88	0.88	0.90	0.90	0.78	0.78			
Flow Rate (veh/h)	64	101	214	70	51	52			
% Heavy Vehicles	42	42	4	4	3	3			
No. Lanes	2		2		2		0		
Geometry Group	5		5		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0	0.0	0.7	0.0	1.0	0.0			
Prop. Right-Turns	0.0	0.4	0.0	0.0	0.0	1.0			
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0			
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2			
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.7	0.5	0.4	0.1	0.3	-0.5			
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20			
x, initial	0.06	0.09	0.19	0.06	0.05	0.05			
hd, final value (s)	5.74	5.49	5.33	4.99	5.23	4.43			
x, final value	0.10	0.15	0.32	0.10	0.07	0.06			
Move-up time, m (s)	2.3		2.3		2.0				
Service Time, t _s (s)	3.4	3.2	3.0	2.7	3.2	2.4			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	314	351	464	320	301	302			
Delay (s/veh)	9.09	9.18	10.48	8.23	8.65	7.74			
LOS	A	A	B	A	A	A			
Approach: Delay (s/veh)	9.15		9.92		8.19				
LOS	A		A		A				
Intersection Delay (s/veh)	9.37								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/25/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	62	104	420	134	274	18	261	156	88	23	312	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889		0.932	0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		698		149	324		300	280			624	
Lane Group Capacity, c		1224		256	812		298	801			1154	
v/c Ratio, X		0.57		0.58	0.40		1.01	0.35			0.54	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		18.6		18.7	16.9		25.0	16.4			18.3	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.7		8.7	1.4		53.8	1.2			1.8	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		20.3		27.5	18.2		78.8	17.6			20.1	
Lane Group LOS		C		C	B		E	B			C	
Approach Delay		20.3		21.1			49.3				20.1	
Approach LOS		C		C			D				C	
Intersection Delay		27.5		$X_c = 0.79$			Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/25/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	62	104	420	134	274	18	261	156	88	23	312	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889		0.932	0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		698		149	324		300	280			624	
Lane Group Capacity, c		1224		256	812		298	801			1154	
v/c Ratio, X		0.57		0.58	0.40		1.01	0.35			0.54	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		18.6		18.7	16.9		25.0	16.4			18.3	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.7		8.7	1.4		53.8	1.2			1.8	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		20.3		27.5	18.2		78.8	17.6			20.1	
Lane Group LOS		C		C	B		E	B			C	
Approach Delay		20.3		21.1			49.3				20.1	
Approach LOS		C		C			D				C	
Intersection Delay		27.5		$X_c = 0.79$			Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	681	386	3	1	411	473	0	0	1	90	0	135
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	748	427			453	426		4		93	0	139
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.96	0.37			1.00	1.11		0.01		0.40	0.00	0.15
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	20.7	5.1			34.0	34.0		30.5		32.7	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	18.7	0.7			32.4	69.1		0.1		5.0	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	39.4	5.8			66.4	103.1		30.6		37.8	30.4	6.6
Lane Group LOS	D	A			E	F		C		D	C	A
Approach Delay	27.2			84.2			30.6			19.1		
Approach LOS	C			F			C			B		
Intersection Delay	48.3			$X_c = 0.90$			Intersection LOS			D		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	681	386	3	1	411	473	0	0	1	90	0	135
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	748	427			453	426		4		93	0	139
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.96	0.37			1.00	1.11		0.01		0.40	0.00	0.15
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	20.7	5.1			34.0	34.0		30.5		32.7	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	18.7	0.7			32.4	69.1		0.1		5.0	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	39.4	5.8			66.4	103.1		30.6		37.8	30.4	6.6
Lane Group LOS	D	A			E	F		C		D	C	A
Approach Delay	27.2			84.2			30.6			19.1		
Approach LOS	C			F			C			B		
Intersection Delay	48.3			$X_c = 0.90$			Intersection LOS			D		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	439	38	0	2	65	71	4	3	0	65	6	816
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	482	42			132		7	5			79	562
Lane Group Capacity, c	706	1056			1537		516	732			538	593
v/c Ratio, X	0.68	0.04			0.09		0.01	0.01			0.15	0.95
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	17.1	11.5			11.8		16.9	16.9			17.8	26.6
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	5.1	0.1			0.1		0.0	0.0			0.5	24.1

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	22.2	11.5			11.9		16.9	16.9			18.3	50.8
Lane Group LOS	C	B			B		B	B			B	D
Approach Delay	21.3				11.9		16.9			46.8		
Approach LOS	C				B		B			D		
Intersection Delay	32.8				$X_c = 0.80$		Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	439	38	0	2	65	71	4	3	0	65	6	816
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	482	42			132		7	5			79	562
Lane Group Capacity, c	706	1056			1537		516	732			538	593
v/c Ratio, X	0.68	0.04			0.09		0.01	0.01			0.15	0.95
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	17.1	11.5			11.8		16.9	16.9			17.8	26.6
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	5.1	0.1			0.1		0.0	0.0			0.5	24.1

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	22.2	11.5			11.9		16.9	16.9			18.3	50.8
Lane Group LOS	C	B			B		B	B			B	D
Approach Delay	21.3				11.9		16.9				46.8	
Approach LOS	C				B		B				D	
Intersection Delay	32.8				$X_c = 0.80$		Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 12-1 PM MD Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	174		25				4	211			402	394
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	198		22					238			994	
Lane Group Capacity, c	537		465					1665			1923	
v/c Ratio, X	0.37		0.05					0.14			0.52	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.9		14.2					7.6			9.6	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.9		0.2					0.2			0.7	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	17.8		14.4					7.7			10.2	
Lane Group LOS	B		B					A			B	
Approach Delay	17.4						7.7			10.2		
Approach LOS	B						A			B		
Intersection Delay	10.9			$X_c = 0.46$			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 12-1 PM MD Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	174		25				4	211			402	394
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	198		22					238			994	
Lane Group Capacity, c	537		465					1665			1923	
v/c Ratio, X	0.37		0.05					0.14			0.52	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.9		14.2					7.6			9.6	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.9		0.2					0.2			0.7	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	17.8		14.4					7.7			10.2	
Lane Group LOS	B		B					A			B	
Approach Delay	17.4						7.7			10.2		
Approach LOS	B						A			B		
Intersection Delay	10.9			$X_c = 0.46$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		24					
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	43	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			383				
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	425	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					425		
C (m) (veh/h)					1030		
v/c					0.41		
95% queue length					2.05		
Control Delay (s/veh)					10.9		
LOS					B		
Approach Delay (s/veh)	--	--	10.9				
Approach LOS	--	--	B				

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		24						
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	43	0	0	0	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	0	0		
Configuration		T						
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			383					
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	425	0	0	0		
Percent Heavy Vehicles	0	0	1	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					425			
C (m) (veh/h)					1030			
v/c					0.41			
95% queue length					2.05			
Control Delay (s/veh)					10.9			
LOS					B			
Approach Delay (s/veh)	--	--	10.9					
Approach LOS	--	--	B					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	564	1		1			23	232	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		297	298		1				319		
Lane Group Capacity, c		784		612	587		403				2035		
v/c Ratio, X		0.01		0.49	0.51		0.00				0.16		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.8	12.9		10.2				10.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.7	3.1		0.0				0.2		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		15.5	16.0		10.2				11.1	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.8			10.2			11.1		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		14.1		$X_c = 0.33$			Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	564	1		1			23	232	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		297	298		1				319		
Lane Group Capacity, c		784		612	587		403				2035		
v/c Ratio, X		0.01		0.49	0.51		0.00				0.16		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.8	12.9		10.2				10.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.7	3.1		0.0				0.2		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		15.5	16.0		10.2				11.1	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.8			10.2			11.1		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		14.1		$X_c = 0.33$			Intersection LOS			<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	55	31	145	59	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	31	79	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	99		264				86	73	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.09		0.23				0.08	0.06	
hd, final value (s)	4.66		4.61				4.87	4.59	
x, final value	0.13		0.34				0.12	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.6				2.9	2.6	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		514				336	323	
Delay (s/veh)	8.35		9.94				8.51	8.06	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.35		9.94				8.30		
LOS	A		A				A		
Intersection Delay (s/veh)	9.14								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	55	31	145	59	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	31	79	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	99		264				86	73	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.09		0.23				0.08	0.06	
hd, final value (s)	4.66		4.61				4.87	4.59	
x, final value	0.13		0.34				0.12	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.6				2.9	2.6	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		514				336	323	
Delay (s/veh)	8.35		9.94				8.51	8.06	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.35		9.94				8.30		
LOS	A		A				A		
Intersection Delay (s/veh)	9.14								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	40	46	0		0	162	23		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	42	64	88		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	99		239		83	136			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.21		0.07	0.12			
hd, final value (s)	4.95		4.53		4.84	4.29			
x, final value	0.14		0.30		0.11	0.16			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.0		2.5		2.8	2.3			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		489		333	386			
Delay (s/veh)	8.73		9.46		8.45	8.11			
LOS	A		A		A	A			
Approach: Delay (s/veh)	8.73		9.46		8.24				
LOS	A		A		A				
Intersection Delay (s/veh)	8.85								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	40	46	0		0	162	23		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	42	64	88		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT	TR			
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	99		239		83	136			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.21		0.07	0.12			
hd, final value (s)	4.95		4.53		4.84	4.29			
x, final value	0.14		0.30		0.11	0.16			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.0		2.5		2.8	2.3			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		489		333	386			
Delay (s/veh)	8.73		9.46		8.45	8.11			
LOS	A		A		A	A			
Approach: Delay (s/veh)	8.73		9.46		8.24				
LOS	A		A		A				
Intersection Delay (s/veh)	8.85								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	149	144	113	2	39	5	526	39	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		322	67		48			584				
Lane Group Capacity, c		554	560		531			2287				
v/c Ratio, X		0.58	0.12		0.09			0.26				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		15.9	13.2		13.1			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		4.4	0.4		0.3			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		20.3	13.7		13.4			9.4			
Lane Group LOS		C	B		B			A			
Approach Delay		19.2			13.4			9.4			
Approach LOS		B			B			A			
Intersection Delay		13.3			$X_c = 0.39$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	149	144	113	2	39	5	526	39	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		322	67		48			584				
Lane Group Capacity, c		554	560		531			2287				
v/c Ratio, X		0.58	0.12		0.09			0.26				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		15.9	13.2		13.1			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		4.4	0.4		0.3			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		20.3	13.7		13.4			9.4			
Lane Group LOS		C	B		B			A			
Approach Delay		19.2			13.4			9.4			
Approach LOS		B			B			A			
Intersection Delay		13.3			$X_c = 0.39$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	107		40				14	294			347	32
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		181						354			416	
Lane Group Capacity, c		462						1093			1173	
v/c Ratio, X		0.39						0.32			0.35	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.0						6.9			7.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.4						0.8			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		20.4					7.7			7.9	
Lane Group LOS		C					A			A	
Approach Delay		20.4					7.7			7.9	
Approach LOS		C					A			A	
Intersection Delay		10.2			$X_c = 0.37$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	107		40				14	294			347	32
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		181						354			416	
Lane Group Capacity, c		462						1093			1173	
v/c Ratio, X		0.39						0.32			0.35	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.0						6.9			7.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.4						0.8			0.8	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		20.4					7.7			7.9	
Lane Group LOS		C					A			A	
Approach Delay		20.4					7.7			7.9	
Approach LOS		C					A			A	
Intersection Delay		10.2			$X_c = 0.37$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	80		54				44	356			294	140
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		156						460			461	
Lane Group Capacity, c		554						840			1015	
v/c Ratio, X		0.28						0.55			0.45	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						10.3			9.7	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						2.3			1.4	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		16.0					12.6			11.1	
Lane Group LOS		B					B			B	
Approach Delay	16.0						12.6		11.1		
Approach LOS	B						B		B		
Intersection Delay	12.5			$X_c = 0.44$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	80		54				44	356			294	140
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		156						460			461	
Lane Group Capacity, c		554						840			1015	
v/c Ratio, X		0.28						0.55			0.45	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						10.3			9.7	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						2.3			1.4	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		16.0					12.6			11.1	
Lane Group LOS		B					B			B	
Approach Delay	16.0						12.6		11.1		
Approach LOS	B						B		B		
Intersection Delay	12.5			$X_c = 0.44$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		106	231	471	127	64	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		138		953		72	192			429	
Lane Group Capacity, c	222		220		1339		162	593			690	
v/c Ratio, X	0.06		0.63		0.71		0.44	0.32			0.62	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		24.6		19.3		15.7	14.9			16.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		10.7		2.4		8.1	1.4			3.4	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		35.3		21.7		23.7	16.3			20.3	
Lane Group LOS	C		D		C		C	B			C	
Approach Delay	34.3		21.7		18.3		20.3					
Approach LOS	C		C		B		C					
Intersection Delay	21.9		$X_c = 0.66$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		106	231	471	127	64	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		138		953		72	192			429	
Lane Group Capacity, c	222		220		1339		162	593			690	
v/c Ratio, X	0.06		0.63		0.71		0.44	0.32			0.62	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		24.6		19.3		15.7	14.9			16.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		10.7		2.4		8.1	1.4			3.4	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		35.3		21.7		23.7	16.3			20.3	
Lane Group LOS	C		D		C		C	B			C	
Approach Delay	34.3		21.7		18.3		20.3					
Approach LOS	C		C		B		C					
Intersection Delay	21.9		$X_c = 0.66$		Intersection LOS		C					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	5	15	8				6	230	116	230	433	28	
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1	
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.932		0.785	0.785		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						2		0	2		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		45						395		256	512		
Lane Group Capacity, c		454						920		546	1017		
v/c Ratio, X		0.10						0.43		0.47	0.50		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		15.9						8.0		8.2	8.4		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.4						1.4		2.3	1.4		

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.3					9.3		10.4	9.8	
Lane Group LOS	B					A		B	A	
Approach Delay	16.3					9.3		10.0		
Approach LOS	B					A		B		
Intersection Delay	10.0				$X_c = 0.37$		Intersection LOS		B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	5	15	8				6	230	116	230	433	28
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.932		0.785	0.785	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	2	
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		45						395		256	512	
Lane Group Capacity, c		454						920		546	1017	
v/c Ratio, X		0.10						0.43		0.47	0.50	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		15.9						8.0		8.2	8.4	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.4						1.4		2.3	1.4	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.3					9.3		10.4	9.8	
Lane Group LOS	B					A		B	A	
Approach Delay	16.3					9.3		10.0		
Approach LOS	B					A		B		
Intersection Delay	10.0				$X_c = 0.37$		Intersection LOS		B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	8	17	266	26	106	22	241	171	91	330	19
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		38		317	31	126		488			489	
Lane Group Capacity, c		628		547	763	563		687			628	
v/c Ratio, X		0.06		0.58	0.04	0.22		0.71			0.78	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.7	14.1	15.4		20.3			21.2	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.7	0.1	0.8		4.1			6.2	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.5	22.4	14.2	16.2	24.3	27.4
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.5	20.2	24.3	27.4		
Approach LOS	B	C	C	C		
Intersection Delay	23.8	$X_c = 0.68$	Intersection LOS	C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	8	17	266	26	106	22	241	171	91	330	19
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		38		317	31	126		488			489	
Lane Group Capacity, c		628		547	763	563		687			628	
v/c Ratio, X		0.06		0.58	0.04	0.22		0.71			0.78	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.7	14.1	15.4		20.3			21.2	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.7	0.1	0.8		4.1			6.2	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.5	22.4	14.2	16.2	24.3	27.4
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.5	20.2	24.3	27.4		
Approach LOS	B	C	C	C		
Intersection Delay	23.8	$X_c = 0.68$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		160	189	90	183		192		86				
% Heavy Vehicles, %HV		8	8	16	16		2		2				
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.961			0.989			0.922					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		339			300			316					
Lane Group Capacity, c		915			694			567					
v/c Ratio, X		0.37			0.43			0.56					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.8			14.4			21.5					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.1			1.9			3.6					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay		14.9			16.3			25.1			
Lane Group LOS		B			B			C			
Approach Delay		14.9			16.3			25.1			
Approach LOS		B			B			C			
Intersection Delay		18.7			$X_C = 0.49$			Intersection LOS			B

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	AECOM					Area Type	All other areas								
Date Performed	3/31/13					Jurisdiction	Staten Island, NY								
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 No-Action Conditions								
						Project ID	Charleston EIS								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		160	189	90	183		192		86						
% Heavy Vehicles, %HV		8	8	16	16		2		2						
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.961			0.989			0.922							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		5			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		339			300			316							
Lane Group Capacity, c		915			694			567							
v/c Ratio, X		0.37			0.43			0.56							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		13.8			14.4			21.5							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		1.1			1.9			3.6							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay		14.9			16.3			25.1			
Lane Group LOS		B			B			C			
Approach Delay		14.9			16.3			25.1			
Approach LOS		B			B			C			
Intersection Delay		18.7			$X_C = 0.49$			Intersection LOS			B

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	29	229	69	58	230	28	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	33	263	79	71	283	34	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	34	18	29	57	17	34	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	39	20	33	70	20	41	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	33	71		131			92
C (m) (veh/h)	1221	1179		311			321
v/c	0.03	0.06		0.42			0.29
95% queue length	0.08	0.19		2.00			1.16
Control Delay (s/veh)	8.0	8.2		24.7			20.7
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		24.7			20.7
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		300	18	14	302		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	344	20	17	372	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				29		27	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	42	0	39	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		17		81			
C (m) (veh/h)		1167		467			
v/c		0.01		0.17			
95% queue length		0.04		0.62			
Control Delay (s/veh)		8.1		14.3			
LOS		A		B			
Approach Delay (s/veh)	--	--	14.3				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	11	2	278	20	263	4	141	325	203	246	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			363	321		505			523	
Lane Group Capacity, c		727			531	523		740			449	
v/c Ratio, X		0.02			0.68	0.61		0.68			1.16	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.3	13.7		14.3			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			6.5	5.0		4.7			96.0	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.4			20.8	18.7		18.9			113.5	
Lane Group LOS		<i>B</i>			<i>C</i>	<i>B</i>		<i>B</i>			<i>F</i>	
Approach Delay		10.4			19.8			18.9			113.5	
Approach LOS		<i>B</i>			<i>B</i>			<i>B</i>			<i>F</i>	
Intersection Delay		47.8			$X_c = 0.92$			Intersection LOS			<i>D</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				528		4		467			526	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					634			502			612	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.95			0.49			0.64	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.5			9.9			11.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					11.8			1.5			1.1	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				31.3			11.5			12.2	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				31.3	11.5			12.2			
Approach LOS				<i>C</i>	<i>B</i>			<i>B</i>			
Intersection Delay	18.9		$X_c = 0.76$		Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		467	263	189	866		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	502	282	219	1006	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		219					
C (m) (veh/h)		768					
v/c		0.29					
95% queue length		1.18					
Control Delay (s/veh)		11.5					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				135		168		550	106	142	711	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					361			705			992	
Lane Group Capacity, c					397			1096			699	
v/c Ratio, X					0.91			0.64			1.42	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.3			8.9			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					25.3			2.3			189.6	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				46.6			11.2			202.6	
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>	
Approach Delay				46.6	11.2			202.6			
Approach LOS				<i>D</i>	<i>B</i>			<i>F</i>			
Intersection Delay	109.7			$X_c = 1.26$			Intersection LOS			<i>F</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	97	49	123	35	196	73	78	559	110	143	532	50
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		306			375		86	735			821	
Lane Group Capacity, c		441			565		275	1065			936	
v/c Ratio, X		0.69			0.66		0.31	0.69			0.88	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.4			25.1		11.3	15.0			17.9	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		4.5			6.0		2.3	2.9			11.0	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	29.9	31.1	13.5	17.8
Lane Group LOS	C	C	B	B
Approach Delay	29.9	31.1	17.4	28.8
Approach LOS	C	C	B	C
Intersection Delay	25.3	$X_c = 0.81$	Intersection LOS	C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					653		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	750	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			71				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	91	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							91
C (m) (veh/h)							569
v/c							0.16
95% queue length							0.57
Control Delay (s/veh)							12.5
LOS							B
Approach Delay (s/veh)	--	--				12.5	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	380							728			653	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	418							809			751	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.62							0.38			0.38	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.2							11.2			11.2	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	4.2							0.4			0.5	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	29.4						11.6			11.8	
Lane Group LOS	C						B			B	
Approach Delay	29.4						11.6		11.8		
Approach LOS	C						B		B		
Intersection Delay	15.4			$X_C = 0.47$			Intersection LOS		B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year				
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)					504			
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	566	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	0	0	0	1	0		
Configuration					T			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				474				
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	0		
Percent Heavy Vehicles	0	0	60	3	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	0	1	0	0		
Configuration				L				
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration			L					
v (veh/h)			509					
C (m) (veh/h)			484					
v/c			1.05					
95% queue length			15.47					
Control Delay (s/veh)			84.5					
LOS			F					
Approach Delay (s/veh)	--	--	84.5					
Approach LOS	--	--	F					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	109	172	197	248	330	86	119	263	267	222	59	56
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	124	368		276	493			671		292	119	
Lane Group Capacity, c	239	583		240	1132			925		384	370	
v/c Ratio, X	0.52	0.63		1.15	0.44			0.73		0.76	0.32	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	24.2	25.3		29.5	22.8			29.4		32.8	29.3	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	5.6	3.6		103.2	1.2			4.9		12.9	2.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	29.7	29.0		132.7	23.9			34.3		45.6	31.5	
Lane Group LOS	C	C		F	C			C		D	C	
Approach Delay	29.2			63.0				34.3		41.6		
Approach LOS	C			E				C		D		
Intersection Delay	43.9			$X_c = 0.91$				Intersection LOS		D		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	148	29	148	186	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	13	0	69	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.87	0.87	0.91	0.91	0.76	0.76		
Flow Rate (veh/h)	85	118	264	102	17	90		
% Heavy Vehicles	7	7	1	1	1	1		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.6	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.3	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1	-0.1	0.3	0.0	0.2	-0.6		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.08	0.10	0.23	0.09	0.02	0.08		
hd, final value (s)	5.23	5.03	5.28	4.97	5.39	4.59		
x, final value	0.12	0.16	0.39	0.14	0.03	0.11		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	2.9	2.7	3.0	2.7	3.4	2.6		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	335	368	514	352	267	340		
Delay (s/veh)	8.66	8.72	11.26	8.48	8.53	8.19		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	8.70		10.49		8.24			
LOS	A		B		A			
Intersection Delay (s/veh)	9.60							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	53	117	466	123	294	10	252	102	124	22	315	124
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.884		0.927	0.927		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		723		150	371		271	243			490	
Lane Group Capacity, c		1241		247	824		375	793			1295	
v/c Ratio, X		0.58		0.61	0.45		0.72	0.31			0.38	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		18.7		19.0	17.4		20.5	16.1			16.7	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.8		9.9	1.6		11.5	1.0			0.8	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		20.5		28.9	19.0		31.9	17.1			17.5	
Lane Group LOS		C		C	B		C	B			B	
Approach Delay		20.5		21.9			24.9				17.5	
Approach LOS		C		C			C				B	
Intersection Delay		21.2		$X_c = 0.66$			Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	750	354	4	3	366	536	2	3	2	118	0	286
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	833	397			388	564		17		147	0	357
Lane Group Capacity, c	806	1206			584	497		148		204	283	862
v/c Ratio, X	1.03	0.33			0.66	1.13		0.11		0.72	0.00	0.41
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	18.2	4.2			26.9	31.0		32.7		36.1	32.1	10.5
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	35.3	0.5			3.9	76.4		1.6		19.7	0.0	1.5

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	53.5	4.7			30.8	107.4		34.2		55.8	32.1	12.0
Lane Group LOS	D	A			C	F		C		E	C	B
Approach Delay	37.7			76.2			34.2			24.8		
Approach LOS	D			E			C			C		
Intersection Delay	48.8			$X_c = 1.02$			Intersection LOS			D		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	437	37	0	1	37	43	3	3	2	62	1	866
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	486	41			75			16			67	609
Lane Group Capacity, c	753	1066			1563			1220			548	616
v/c Ratio, X	0.65	0.04			0.05			0.01			0.12	0.99
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	16.6	11.5			11.5			16.9			17.6	27.3
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	4.0	0.1			0.1			0.0			0.4	32.4

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	20.7	11.5			11.6			16.9			18.1	59.7
Lane Group LOS	<i>C</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>E</i>
Approach Delay	19.9			11.6			16.9			55.6		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>E</i>		
Intersection Delay	38.1			$X_c = 0.80$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Weekday 5-6 PM Peak Period					Jurisdiction	Staten Island, NY					
						Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	194		24				2	260			403	331
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	223		21					297			807	
Lane Group Capacity, c	547		473					1731			1936	
v/c Ratio, X	0.41		0.04					0.17			0.42	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	16.1		14.2					7.7			8.9	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	2.2		0.2					0.2			0.5	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	18.3		14.4					7.9			9.5	
Lane Group LOS	B		B					A			A	
Approach Delay	17.9						7.9			9.5		
Approach LOS	B						A			A		
Intersection Delay	10.6			$X_c = 0.41$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		24					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	33	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			449				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	510	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					510		
C (m) (veh/h)					1043		
v/c					0.49		
95% queue length					2.76		
Control Delay (s/veh)					11.7		
LOS					B		
Approach Delay (s/veh)	--	--	11.7				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	461	2		4			23	273	4	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 25.0	G =	G =	G =	G =	G = 25.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		259	261		5				329		
Lane Group Capacity, c		784		600	576		399				2052		
v/c Ratio, X		0.01		0.43	0.45		0.01				0.16		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.4	12.6		10.3				10.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.2	2.5		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	14.7	15.1	10.3	11.1
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	14.9	10.3	11.1	
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Intersection Delay	13.4	$X_c = 0.31$	Intersection LOS	<i>B</i>	

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY				
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions				
Analysis Time Period	Weekday 5-6 PM Peak Period									
Project ID <i>Charleston EIS</i>										
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	106	39	172	84	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	24	89	25				
%Thrus Left Lane				50						
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT	TR		
PHF	0.80		0.78				0.91	0.91		
Flow Rate (veh/h)	180		327				74	76		
% Heavy Vehicles	2		3				1	1		
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.7				0.4	0.0		
Prop. Right-Turns	0.3		0.0				0.0	0.4		
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	-0.1		0.2				0.1	-0.2		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20	3.20		
x, initial	0.16		0.29				0.07	0.07		
hd, final value (s)	4.54		4.67				5.18	4.89		
x, final value	0.23		0.42				0.11	0.10		
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.5		2.7				3.2	2.9		
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	430		577				324	326		
Delay (s/veh)	8.87		11.08				8.79	8.46		
LOS	A		B				A	A		
Approach: Delay (s/veh)	8.87		11.08				8.62			
LOS	A		B				A			
Intersection Delay (s/veh)	9.91									
Intersection LOS	A									

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	39	91	0		0	191	19	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	66	100	142		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT		TR	
PHF	0.80		0.78		0.99		0.99	
Flow Rate (veh/h)	161		268		116		193	
% Heavy Vehicles	2		5		1		1	
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.3		0.0		0.6	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		0.0		0.1	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.14		0.24		0.10	0.17		
hd, final value (s)	5.06		4.86		5.11	4.55		
x, final value	0.23		0.36		0.16	0.24		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	3.1		2.9		3.1	2.5		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	411		518		366		443	
Delay (s/veh)	9.54		10.59		9.11		9.01	
LOS	A		B		A		A	
Approach: Delay (s/veh)	9.54		10.59		9.05			
LOS	A		B		A			
Intersection Delay (s/veh)	9.72							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	193	165	115	8	47	9	416	106	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		421	73		73			588				
Lane Group Capacity, c		539	560		526			2302				
v/c Ratio, X		0.78	0.13		0.14			0.26				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		17.4	13.3		13.3			9.1				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		10.7	0.5		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		28.2	13.8		13.9			9.4			
Lane Group LOS		C	B		B			A			
Approach Delay		26.0			13.9			9.4			
Approach LOS		C			B			A			
Intersection Delay		16.8			$X_c = 0.48$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	133		34				18	469			472	45
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		174						566			562	
Lane Group Capacity, c		456						1099			1127	
v/c Ratio, X		0.38						0.52			0.50	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.0						8.0			7.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.4						1.5			1.4	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	20.3				9.5			9.3
Lane Group LOS	C				A			A
Approach Delay	20.3				9.5			9.3
Approach LOS	C				A			A
Intersection Delay	10.8		$X_c = 0.47$		Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	147		86				23	490			406	191
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		291						597			626	
Lane Group Capacity, c		573						890			976	
v/c Ratio, X		0.51						0.67			0.64	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.1						11.3			11.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.9						3.4			2.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.0					14.6			13.9	
Lane Group LOS		B					B			B	
Approach Delay	19.0						14.6		13.9		
Approach LOS	B						B		B		
Intersection Delay	15.2			$X_c = 0.61$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		119	284	466	276	53	207			501	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		125		1167		58	225			582	
Lane Group Capacity, c	222		220		1329		124	616			670	
v/c Ratio, X	0.02		0.57		0.88		0.47	0.37			0.87	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		24.4		20.5		15.8	15.2			18.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		8.7		4.5		11.3	1.6			12.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		33.1		25.0		27.1	16.7			31.4	
Lane Group LOS	C		C		C		C	B			C	
Approach Delay	32.8		25.0		18.9		31.4					
Approach LOS	C		C		B		C					
Intersection Delay	26.4		$X_c = 0.82$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	10	20	6				9	250	127	319	533	52
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.922		0.564	0.564	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	7	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		58						420		343	616	
Lane Group Capacity, c		460						950		523	986	
v/c Ratio, X		0.13						0.44		0.66	0.62	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.0						8.0		9.5	9.3	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.6						1.4		3.6	1.7	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.5					9.4		13.1	11.0	
Lane Group LOS	B					A		B	B	
Approach Delay	16.5				9.4			11.7		
Approach LOS	B				A			B		
Intersection Delay	11.2		$X_c = 0.48$		Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	13	20	267	29	110	18	269	175	94	409	10
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		57		307	33	126		502			589	
Lane Group Capacity, c		634		560	792	585		727			632	
v/c Ratio, X		0.09		0.55	0.04	0.22		0.69			0.93	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.5		18.4	14.2	15.4		20.0			23.7	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.5	0.1	0.8		3.6			7.2	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.7	21.8	14.2	16.1	23.6	30.9
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.7	19.8	23.6	30.9		
Approach LOS	B	B	C	C		
Intersection Delay	24.9	$X_c = 0.74$	Intersection LOS	C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	0	1		0		0			
Lane Group		TR			LT			LR				
Volume, V (vph)		162	253	106	179		144		105			
% Heavy Vehicles, %HV		5	5	8	8		1		1			
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0			2.0			2.0				
Extension of Effective Green, e		2.0			2.0			2.0				
Arrival Type, AT		3			3			3				
Unit Extension, UE		3.0			3.0			3.0				
Filtering/Metering, I		0.927			0.989			0.917				
Initial Unmet Demand, Q _b		0.0			0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0	
Lane Width		16.0			16.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5			0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		435			344			372				
Lane Group Capacity, c		931			605			567				
v/c Ratio, X		0.47			0.57			0.66				
Total Green Ratio, g/C		0.50			0.50			0.39				
Uniform Delay, d ₁		14.7			15.7			22.6				
Progression Factor, PF		1.000			1.000			1.000				
Delay Calibration, k		0.50			0.50			0.50				
Incremental Delay, d ₂		1.6			3.8			5.4				
Initial Queue Delay, d ₃		0.0			0.0			0.0				

Control Delay		16.2			19.5			27.9			
Lane Group LOS		B			B			C			
Approach Delay		16.2			19.5			27.9			
Approach LOS		B			B			C			
Intersection Delay		21.0			$X_c = 0.61$			Intersection LOS			C

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	12	229	50	29	257	28	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	13	248	54	32	292	31	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	27	33	24	62	17	41	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	67	82	59	79	21	52	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	13	32		152			208
C (m) (veh/h)	1231	1242		337			388
v/c	0.01	0.03		0.45			0.54
95% queue length	0.03	0.08		2.25			3.05
Control Delay (s/veh)	8.0	8.0		24.2			24.5
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		24.2			24.5
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		271	13	16	327		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	294	14	18	371	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				109		19	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	145	0	25	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		18		170			
C (m) (veh/h)		1236		423			
v/c		0.01		0.40			
95% queue length		0.04		1.90			
Control Delay (s/veh)		8.0		19.1			
LOS		A		C			
Approach Delay (s/veh)	--	--		19.1			
Approach LOS	--	--		C			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	10	5	325	27	377	5	168	390	176	213	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			382	410		619			427	
Lane Group Capacity, c		726			548	539		743			400	
v/c Ratio, X		0.02			0.70	0.76		0.83			1.07	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.4	14.9		15.6			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			6.1	8.4		9.3			64.1	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.4			20.5	23.3		24.9			81.6	
Lane Group LOS		<i>B</i>			<i>C</i>	<i>C</i>		<i>C</i>			<i>F</i>	
Approach Delay		10.4		22.0		24.9		81.6				
Approach LOS		<i>B</i>		<i>C</i>		<i>C</i>		<i>F</i>				
Intersection Delay		36.6		$X_c = 0.91$		Intersection LOS		<i>D</i>				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				510		9		555			544	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					603			610			598	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.89			0.59			0.58	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.0			10.7			10.6	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					9.0			2.2			0.9	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				27.9			12.9			11.5	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				27.9				12.9			
Approach LOS				<i>C</i>				<i>B</i>			
Intersection Delay	17.4		$X_c = 0.71$		Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		555	270	181	871		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	609	296	198	957	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		198					
C (m) (veh/h)		735					
v/c		0.27					
95% queue length		1.09					
Control Delay (s/veh)		11.7					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				108		218		591	94	155	692	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					366			752			930	
Lane Group Capacity, c					393			1130			676	
v/c Ratio, X					0.93			0.67			1.38	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.5			9.0			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					29.7			2.5			171.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				51.2			11.5			184.7
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>
Approach Delay				51.2	11.5			184.7		
Approach LOS				<i>D</i>	<i>B</i>			<i>F</i>		
Intersection Delay	97.2		$X_c = 1.23$			Intersection LOS		<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	103	49	114	31	92	73	161	732	144	58	579	99
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		341			268		175	953			778	
Lane Group Capacity, c		487			538		293	1065			1016	
v/c Ratio, X		0.70			0.50		0.60	0.89			0.77	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.5			23.3		13.8	18.2			16.0	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		4.5			3.3		5.0	7.0			5.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	29.9	26.6	18.8	25.2
Lane Group LOS	C	C	B	C
Approach Delay	29.9	26.6	24.2	21.3
Approach LOS	C	C	C	C
Intersection Delay	24.4	$X_c = 0.82$	Intersection LOS	C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					678		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	736	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			57				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	64	0	0	0	
Percent Heavy Vehicles	0	0	5	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							64
C (m) (veh/h)							671
v/c							0.10
95% queue length							0.32
Control Delay (s/veh)							10.9
LOS							B
Approach Delay (s/veh)	--	--				10.9	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	394							908			678	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	458							946			737	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.68							0.44			0.38	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.9							11.8			11.2	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	5.6							0.4			0.5	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	31.5						12.2			11.8
Lane Group LOS	C						B			B
Approach Delay	31.5						12.2		11.8	
Approach LOS	C						B		B	
Intersection Delay	16.2		$X_C = 0.53$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					597		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	641	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				236			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	262	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			262				
C (m) (veh/h)			441				
v/c			0.59				
95% queue length			3.76				
Control Delay (s/veh)			24.4				
LOS			C				
Approach Delay (s/veh)	--	--	24.4				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD					Analysis Year	2020 No-Action Conditions					
	Peak					Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	99	191	209	273	444	100	130	402	293	241	115	93
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	106	378		284	683			917		287	248	
Lane Group Capacity, c	161	584		211	1179			950		384	364	
v/c Ratio, X	0.66	0.65		1.35	0.58			0.97		0.75	0.68	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	25.6	25.5		29.5	24.2			32.1		32.6	32.1	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	13.8	3.9		181.4	1.9			21.9		10.8	8.5	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	39.5	29.4		210.9	26.0			54.0		43.5	40.6	
Lane Group LOS	<i>D</i>	<i>C</i>		<i>F</i>	<i>C</i>			<i>D</i>		<i>D</i>	<i>D</i>	
Approach Delay	31.6			80.3				54.0		42.1		
Approach LOS	<i>C</i>			<i>F</i>				<i>D</i>		<i>D</i>		
Intersection Delay	56.8			$X_c = 1.06$				Intersection LOS		<i>E</i>		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	0	252	33	111	244	0		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	57	0	95	0	0	0		
%Thrus Left Lane								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	T	TR	LT	T	L	R		
PHF	0.85	0.85	0.91	0.91	0.89	0.89		
Flow Rate (veh/h)	148	186	255	134	64	106		
% Heavy Vehicles	0	0	0	1	1	1		
No. Lanes	2		2		2		0	
Geometry Group	5		5		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0	0.0	0.5	0.0	1.0	0.0		
Prop. Right-Turns	0.0	0.2	0.0	0.0	0.0	1.0		
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0		
hLT-adj	0.5	0.5	0.5	0.5	0.2	0.2		
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.0	-0.1	0.2	0.0	0.2	-0.6		
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20		
x, initial	0.13	0.17	0.23	0.12	0.06	0.09		
hd, final value (s)	5.38	5.24	5.55	5.32	5.72	4.92		
x, final value	0.22	0.27	0.39	0.20	0.10	0.14		
Move-up time, m (s)	2.3		2.3		2.0			
Service Time, t _s (s)	3.1	2.9	3.2	3.0	3.7	2.9		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	398	436	505	384	314	356		
Delay (s/veh)	9.60	9.86	11.79	9.34	9.36	8.75		
LOS	A	A	B	A	A	A		
Approach: Delay (s/veh)	9.75		10.94		8.98			
LOS	A		B		A			
Intersection Delay (s/veh)	10.12							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	82	127	476	165	369	25	291	207	115	26	540	168
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.830		0.865	0.865		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		744		212	505		310	353			790	
Lane Group Capacity, c		1156		240	828		247	818			1327	
v/c Ratio, X		0.64		0.88	0.61		1.26	0.43			0.60	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		19.5		22.9	19.1		25.0	17.2			18.9	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.3		30.9	2.9		143.5	1.7			2.0	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	21.8	53.8	21.9	168.5	18.8	20.9	
Lane Group LOS	C	D	C	F	B	C	
Approach Delay	21.8	31.4	88.8	20.9			
Approach LOS	C	C	F	C			
Intersection Delay	39.1	$X_c = 1.07$	Intersection LOS	D			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	881	421			465	709				192		214
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	908	439			495	608				209		233
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	1.01	0.38			1.08	1.55				0.36		0.24
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	10.5	5.1			34.0	34.0				32.5		6.7
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	25.1	0.5			42.2	252.0				1.8		0.6
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

Control Delay	35.7	5.6			76.2	286.0				34.3		7.3
Lane Group LOS	<i>D</i>	<i>A</i>			<i>E</i>	<i>F</i>				<i>C</i>		<i>A</i>
Approach Delay	25.9			191.8						20.1		
Approach LOS	<i>C</i>			<i>F</i>						<i>C</i>		
Intersection Delay	88.3			$X_c = 1.03$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	567	46	0	0	46	58	0	0	1	84	0	1128
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	591	48			99			1			90	785
Lane Group Capacity, c	736	1066			1633			1184			544	622
v/c Ratio, X	0.80	0.05			0.06			0.00			0.17	1.26
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	18.8	11.5			11.6			16.8			18.0	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	8.8	0.1			0.1			0.0			0.6	128.8

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	27.6	11.6			11.7			16.8			18.5	156.3
Lane Group LOS	<i>C</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>F</i>
Approach Delay	26.4			11.7			16.8			142.2		
Approach LOS	<i>C</i>			<i>B</i>			<i>B</i>			<i>F</i>		
Intersection Delay	88.2			$X_c = 1.00$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd					
Agency or Co.	AECOM						W					
Date Performed	10/24/2012					Area Type	All other areas					
Time Period	Saturday 12:45-1:45 PM					Jurisdiction	Staten Island, NY					
	Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	312		36				5	263			524	542
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	367		32					305			1198	
Lane Group Capacity, c	573		497					1699			1937	
v/c Ratio, X	0.64		0.06					0.18			0.62	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	17.6		14.3					7.7			10.3	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	4.8		0.2					0.2			0.7	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	22.4		14.5					8.0			11.0	
Lane Group LOS	C		B					A			B	
Approach Delay	21.8						8.0			11.0		
Approach LOS	C						A			B		
Intersection Delay	12.8			$X_c = 0.63$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		29						
Peak-Hour Factor, PHF	1.00	0.69	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	42	0	0	0	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	0	0	0	
Configuration		T						
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			570					
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	647	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	1	0	0	0	0	
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					647			
C (m) (veh/h)					1034			
v/c					0.63			
95% queue length					4.58			
Control Delay (s/veh)					14.1			
LOS					B			
Approach Delay (s/veh)	--	--	14.1					
Approach LOS	--	--	B					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	709	2		5			28	358	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		584	197		6				434		
Lane Group Capacity, c		784		606	582		350				2036		
v/c Ratio, X		0.01		0.96	0.34		0.02				0.21		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		17.1	11.9		10.3				11.2		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		28.1	1.5		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	45.2	13.4	10.4		11.4
Lane Group LOS	B	D	B	B		B
Approach Delay	10.2	37.2		10.4		11.4
Approach LOS	B	D		B		B
Intersection Delay	27.8	$X_c = 0.59$		Intersection LOS		C

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	77	36	231	60	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	17	120	23			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.75		0.85				0.89	0.89	
Flow Rate (veh/h)	150		341				86	92	
% Heavy Vehicles	2		4				0	0	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.2		0.2				0.0	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.13		0.30				0.08	0.08	
hd, final value (s)	4.61		4.75				5.12	4.91	
x, final value	0.19		0.45				0.12	0.13	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.6		2.8				3.1	2.9	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	400		591				336	342	
Delay (s/veh)	8.70		11.58				8.83	8.62	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.70		11.58				8.72		
LOS	A		B				A		
Intersection Delay (s/veh)	10.17								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012				Analysis Year	2020 No-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	26	69	0		0	237	24		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	53	107	139		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.75		0.85		0.95		0.95		
Flow Rate (veh/h)	126		306		110		202		
% Heavy Vehicles	2		2		1		1		
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.3		0.0		0.5	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.1		-0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20		3.20		
x, initial	0.11		0.27		0.10		0.18		
hd, final value (s)	5.11		4.77		5.09		4.56		
x, final value	0.18		0.41		0.16		0.26		
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.1		2.8		3.1		2.6		
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	376		556		360		452		
Delay (s/veh)	9.21		10.98		9.03		9.11		
LOS	A		B		A		A		
Approach: Delay (s/veh)	9.21		10.98		9.08				
LOS	A		B		A				
Intersection Delay (s/veh)	9.88								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	218	238	142	0	64	15	648	66	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		608	101		95			771				
Lane Group Capacity, c		544	560		558			2267				
v/c Ratio, X		1.12	0.18		0.17			0.34				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.5	13.5		13.5			9.6				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		75.0	0.7		0.7			0.4				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		94.5	14.2		14.1			10.0			
Lane Group LOS		F	B		B			A			
Approach Delay		83.1			14.1			10.0			
Approach LOS		F			B			A			
Intersection Delay		43.1			$X_c = 0.67$			Intersection LOS			D

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	185		56				17	404			384	61
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		262						457			489	
Lane Group Capacity, c		464						1111			1186	
v/c Ratio, X		0.56						0.41			0.41	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.0						7.3			7.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		4.6						1.1			1.0	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	23.6					8.4			8.3
Lane Group LOS	C					A			A
Approach Delay	23.6					8.4			8.3
Approach LOS	C					A			A
Intersection Delay	11.7		$X_C = 0.46$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	142		67				43	482			388	218
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		278						571			644	
Lane Group Capacity, c		575						853			1018	
v/c Ratio, X		0.48						0.67			0.63	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.9						11.3			11.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.7						3.5			2.7	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		18.6					14.8			13.7	
Lane Group LOS		B					B			B	
Approach Delay		18.6					14.8			13.7	
Approach LOS		B					B			B	
Intersection Delay		15.0			$X_c = 0.59$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		123	302	617	173	82	229			422	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		173		1255		89	249			488	
Lane Group Capacity, c	222		220		1338		138	621			707	
v/c Ratio, X	0.12		0.79		0.94		0.64	0.40			0.69	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		25.2		21.0		17.0	15.4			17.3	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.8		18.2		7.7		19.4	1.8			4.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.7		43.3		28.7		36.4	17.2			21.4	
Lane Group LOS	C		D		C		D	B			C	
Approach Delay	40.7		28.7		22.2		21.4					
Approach LOS	D		C		C		C					
Intersection Delay	27.2		$X_c = 0.80$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	31	33	5				3	279	153	316	482	47
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.899		0.644	0.644	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0		0	0	
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		91						472		351	576	
Lane Group Capacity, c		462						965		492	1024	
v/c Ratio, X		0.20						0.49		0.71	0.56	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.3						8.3		10.0	8.8	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		1.0						1.6		5.6	1.4	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.3					9.9		15.6	10.2	
Lane Group LOS		B					A		B	B	
Approach Delay		17.3					9.9		12.3		
Approach LOS		B					A		B		
Intersection Delay		11.8		$X_c = 0.54$		Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/24/2012					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	8	18	294	18	106	13	279	242	86	387	8
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		41		327	20	118		580			535	
Lane Group Capacity, c		631		578	808	601		735			620	
v/c Ratio, X		0.06		0.57	0.02	0.20		0.79			0.86	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.6	14.0	15.2		21.4			22.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.5	0.1	0.6		4.5			8.1	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.5	22.1	14.1	15.9	25.9	30.6
Lane Group LOS	B	C	B	B	C	C
Approach Delay	14.5	20.2	25.9	30.6		
Approach LOS	B	C	C	C		
Intersection Delay	25.5	$X_c = 0.71$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 No-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		176	240	141	172		152		112				
% Heavy Vehicles, %HV		2	2	4	4		1		1				
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.992			0.953					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		2			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		462			351			311					
Lane Group Capacity, c		975			536			567					
v/c Ratio, X		0.47			0.65			0.55					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		14.7			16.7			21.4					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.5			6.1			3.6					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay	16.3	22.8	25.0	
Lane Group LOS	B	C	C	
Approach Delay	16.3	22.8	25.0	
Approach LOS	B	C	C	
Intersection Delay	20.7	$X_c = 0.61$	Intersection LOS	C

With-Action

2015

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	26	145	41	28	148	37	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	
Hourly Flow Rate, HFR (veh/h)	31	174	49	33	178	44	
Percent Heavy Vehicles	20	--	--	13	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	8	7	7	60	9	27	
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.95	0.95	0.95	
Hourly Flow Rate, HFR (veh/h)	10	8	8	63	9	28	
Percent Heavy Vehicles	32	32	32	5	5	5	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	31	33		100			26
C (m) (veh/h)	1248	1283		488			444
v/c	0.02	0.03		0.20			0.06
95% queue length	0.08	0.08		0.76			0.19
Control Delay (s/veh)	8.0	7.9		14.3			13.6
LOS	A	A		B			B
Approach Delay (s/veh)	--	--		14.3			13.6
Approach LOS	--	--		B			B

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		196	31	20	195		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	236	37	24	234	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				7		16	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.69	1.00	0.69	
Hourly Flow Rate, HFR (veh/h)	0	0	0	10	0	23	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		24		33			
C (m) (veh/h)		1240		671			
v/c		0.02		0.05			
95% queue length		0.06		0.15			
Control Delay (s/veh)		8.0		10.6			
LOS		A		B			
Approach Delay (s/veh)	--	--	10.6				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	3	6	1	156	33	264	0	139	278	82	112	8
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			217	303		491			232	
Lane Group Capacity, c		719			541	514		702			433	
v/c Ratio, X		0.02			0.40	0.59		0.70			0.54	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.3	13.5		14.4			13.1	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.0		2.1	4.6		5.1		4.7	
Initial Queue Delay, d_3		0.0		0.0	0.0		0.0		0.0	
Control Delay		10.3		14.3	18.1		19.5		17.8	
Lane Group LOS		B		B	B		B		B	
Approach Delay	10.3	16.6			19.5			17.8		
Approach LOS	B	B			B			B		
Intersection Delay	17.9	$X_c = 0.64$			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				259		5		413			270	
% Heavy Vehicles, %HV				8		8		11			15	
Peak-Hour Factor, PHF				0.88		0.88		0.85			0.87	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.893			0.886			0.754	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			3	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					300			486			310	
Lane Group Capacity, c					632			970			925	
v/c Ratio, X					0.47			0.50			0.34	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					15.8			10.0			9.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					2.3			1.6			0.7	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>18.1</i>			<i>11.6</i>			<i>9.7</i>	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>A</i>	
Approach Delay				<i>18.1</i>				<i>11.6</i>			
Approach LOS				<i>B</i>				<i>B</i>			
Intersection Delay	<i>12.9</i>			$X_c = 0.49$			Intersection LOS			<i>B</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		413	216	116	413		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.87	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	485	254	133	474	0	
Percent Heavy Vehicles	0	--	--	20	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		133					
C (m) (veh/h)		791					
v/c		0.17					
95% queue length		0.60					
Control Delay (s/veh)		10.5					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				58		133		494	72	88	318	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.980			0.769			0.842	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					205			666			467	
Lane Group Capacity, c					384			1032			773	
v/c Ratio, X					0.53			0.65			0.60	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					18.8			8.9			8.6	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					5.1			2.4			2.9	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>23.9</i>			<i>11.3</i>			<i>11.5</i>	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				<i>23.9</i>				<i>11.3</i>			
Approach LOS				<i>C</i>				<i>B</i>			
Intersection Delay	<i>13.3</i>			$X_c = 0.61$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	69	25	64	21	124	20	52	649	108	53	355	28
% Heavy Vehicles, %HV	1	1	1	1	1	1	3	3	3	9	9	9
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.94	0.94	0.94	0.92	0.92	0.92	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.810			1.000		0.649	0.649			0.986	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	6
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		176			175		57	822			512	
Lane Group Capacity, c		563			585		415	1049			1047	
v/c Ratio, X		0.31			0.30		0.14	0.78			0.49	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		21.7			21.6		10.1	16.3			12.7	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		1.2			1.3		0.4	3.9			1.6	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		22.8			22.9		10.5	20.2			14.3	
Lane Group LOS		C			C		B	C			B	
Approach Delay		22.8			22.9			19.5			14.3	
Approach LOS		C			C			B			B	
Intersection Delay		18.7			$X_c = 0.60$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description Charleston EIS							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					378		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	450	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			57				
Peak-Hour Factor, PHF	1.00	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	81	0	0	0	
Percent Heavy Vehicles	0	0	60	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							81
C (m) (veh/h)							689
v/c							0.12
95% queue length							0.40
Control Delay (s/veh)							10.9
LOS							B
Approach Delay (s/veh)	--	--				10.9	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	275							738			378	
% Heavy Vehicles, %HV	2							1			3	
Peak-Hour Factor, PHF	0.90							0.91			0.84	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l _i	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.665			0.986	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	306							811			450	
Lane Group Capacity, c	669							2156			1951	
v/c Ratio, X	0.46							0.38			0.23	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.6							11.2			10.2	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.2							0.3			0.3	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	25.8						11.6			10.5	
Lane Group LOS	C						B			B	
Approach Delay	25.8						11.6			10.5	
Approach LOS	C						B			B	
Intersection Delay	14.0			$X_c = 0.41$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					378		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	397	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				278			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	298	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			298				
C (m) (veh/h)			597				
v/c			0.50				
95% queue length			2.78				
Control Delay (s/veh)			16.9				
LOS			C				
Approach Delay (s/veh)	--	--	16.9				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	72	136	158	195	331	51	81	213	132	5	26	40
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	82	291		224	434			458		7	99	
Lane Group Capacity, c	263	577		249	1096			894		376	348	
v/c Ratio, X	0.31	0.50		0.90	0.40			0.51		0.02	0.28	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	22.3	24.0		28.6	23.0			27.4		27.3	29.1	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	2.4	2.4		35.5	1.1			2.1		0.1	2.0	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	24.7	26.5		64.1	24.1			29.5		27.4	31.1	
Lane Group LOS	C	C		E	C			C		C	C	
Approach Delay	26.1			37.7				29.5		30.8		
Approach LOS	C			D				C		C		
Intersection Delay	32.2			$X_c = 0.61$				Intersection LOS		C		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection			
Agency/Co.	AECOM				Jurisdiction		Staten Island, NY	
Date Performed	1/24/13				Analysis Year		2015 With-Action Conditions	
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	38	51	28		32	50	35	
%Thrus Left Lane	50				50			
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	13	16	21		21	52	16	
%Thrus Left Lane					50			
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.80	0.80	0.78	0.78	0.86	1.00	0.92	0.92
Flow Rate (veh/h)	78	66	73	76	33	21	50	45
% Heavy Vehicles	21	21	1	1	5	0	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.6	0.0	0.6	0.0	0.5	0.0	0.4	0.0
Prop. Right-Turns	0.0	0.5	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.7	-0.0	0.3	-0.4	0.3	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.07	0.06	0.06	0.07	0.03	0.02	0.04	0.04
hd, final value (s)	5.68	5.01	5.32	4.64	5.63	4.62	5.50	5.01
x, final value	0.12	0.09	0.11	0.10	0.05	0.03	0.08	0.06
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	3.4	2.7	3.0	2.3	3.3	2.3	3.2	2.7
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	328	316	323	326	283	271	300	295
Delay (s/veh)	9.17	8.22	8.66	7.84	8.64	7.45	8.65	8.05
LOS	A	A	A	A	A	A	A	A
Approach: Delay (s/veh)	8.73		8.24		8.18		8.37	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.42							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	52	28	180	111	260	6	222	91	50	10	157	99
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.916			0.953		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		296			434		255	162			350	
Lane Group Capacity, c		1071			1172		447	795			1217	
v/c Ratio, X		0.28			0.37		0.57	0.20			0.29	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		15.8			16.6		18.6	15.3			15.9	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		0.6			0.9		5.2	0.6			0.6	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		16.4			17.5		23.8	15.9			16.5	
Lane Group LOS		B			B		C	B			B	
Approach Delay		16.4			17.5			20.7			16.5	
Approach LOS		B			B			C			B	
Intersection Delay		17.9			$X_c = 0.47$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	767	227	19	4	263	232	4	16	6	103	5	140
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.86	0.86	0.86	0.37	0.37	0.37	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.603	0.603			0.932	0.932		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	843	270			311	270		70		118	6	161
Lane Group Capacity, c	858	1157			449	383		355		226	320	952
v/c Ratio, X	0.98	0.23			0.69	0.70		0.20		0.52	0.02	0.17
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	17.3	4.5			30.9	31.0		31.5		33.5	30.5	6.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	20.0	0.3			8.0	9.7		1.2		8.4	0.1	0.4
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	37.3	4.8			38.9	40.8		32.8		41.9	30.6	6.8
Lane Group LOS	D	A			D	D		C		D	C	A
Approach Delay	29.4			39.8			32.8			21.8		
Approach LOS	C			D			C			C		
Intersection Delay	31.4			$X_c = 0.87$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	311	25	0	14	66	36	6	21	10	15	36	398
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	342	27			148			82			67	320
Lane Group Capacity, c	702	1066			1545			1235			671	616
v/c Ratio, X	0.49	0.03			0.10			0.07			0.10	0.52
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	14.9	11.4			11.8			17.3			17.5	21.1
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	2.3	0.0			0.1			0.1			0.3	3.0
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	17.2	11.4			11.9			17.4			17.8	24.1
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	16.8			11.9			17.4			23.0		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	18.5			$X_c = 0.50$			Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	88		4				4	84			373	224
% Heavy Vehicles, %HV	11		11				2	2			1	1
Peak-Hour Factor, PHF	0.80		0.80				0.80	0.80			0.81	0.81
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.995		0.995					1.000			0.752	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	4	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	110		0					110			737	
Lane Group Capacity, c	532		461					1678			1979	
v/c Ratio, X	0.21		0.00					0.07			0.37	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.0		14.0					7.3			8.7	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	0.9		0.0					0.1			0.4	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	15.9		14.0					7.3			9.1	
Lane Group LOS	<i>B</i>		<i>B</i>					<i>A</i>			<i>A</i>	
Approach Delay	15.9							7.3		9.1		
Approach LOS	<i>B</i>							<i>A</i>		<i>A</i>		
Intersection Delay	9.7		$X_c = 0.31$			Intersection LOS			<i>A</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		24					
Peak-Hour Factor, PHF	1.00	0.82	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	29	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			169				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	216	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					216		
C (m) (veh/h)					1049		
v/c					0.21		
95% queue length					0.77		
Control Delay (s/veh)					9.3		
LOS					A		
Approach Delay (s/veh)	--	--	9.3				
Approach LOS	--	--	A				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/15/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	421	1		3			23	176	0	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		267	267		4				245		
Lane Group Capacity, c		776		600	575		434				2052		
v/c Ratio, X		0.01		0.44	0.46		0.01				0.12		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.5	12.7		10.2				10.7		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.3	2.6		0.0				0.1		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	14.9	15.3	10.3	10.9
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	15.1	10.3	10.9	
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Intersection Delay	13.7	$X_c = 0.29$	Intersection LOS	<i>B</i>	

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	55	21	114	70	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	10	65	27			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.79				0.81	0.81	
Flow Rate (veh/h)	94		232				51	73	
% Heavy Vehicles	24		5				3	3	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.6				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.5	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.2		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.08		0.21				0.05	0.06	
hd, final value (s)	4.69		4.51				4.77	4.45	
x, final value	0.12		0.29				0.07	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.5				2.8	2.4	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	344		482				301	323	
Delay (s/veh)	8.35		9.35				8.11	7.89	
LOS	A		A				A	A	
Approach: Delay (s/veh)	8.35		9.35				7.98		
LOS	A		A				A		
Intersection Delay (s/veh)	8.76								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	6	59	0		0	127	18	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	56	51	45		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.79		0.87	0.87		
Flow Rate (veh/h)	80		182		92	80		
% Heavy Vehicles	17		7		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.1		0.0		0.7	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.6		
Prop. Heavy Vehicle	0.2		0.1		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.3		0.0		0.2	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.07		0.16		0.08	0.07		
hd, final value (s)	4.80		4.43		4.68	4.15		
x, final value	0.11		0.22		0.12	0.09		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.8		2.4		2.7	2.2		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	330		432		342	330		
Delay (s/veh)	8.37		8.70		8.31	7.57		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.37		8.70		7.97			
LOS	A		A		A			
Intersection Delay (s/veh)	8.35							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	77	65	53	6	39	1	383	74	1			
% Heavy Vehicles, %HV	3	3	3	1	1	1	3	3	3			
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.76	0.76	0.76			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.998			0.978				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	21	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		179	41		58			602				
Lane Group Capacity, c		543	549		545			2253				
v/c Ratio, X		0.33	0.07		0.11			0.27				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		14.3	13.0		13.2			9.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		1.6	0.3		0.4			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		15.9	13.3		13.6			9.5			
Lane Group LOS		B	B		B			A			
Approach Delay		15.5			13.6			9.5			
Approach LOS		B			B			A			
Intersection Delay		11.2			$X_c = 0.29$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	48		18				16	333			461	30
% Heavy Vehicles, %HV	1		1				7	7			9	9
Peak-Hour Factor, PHF	0.74		0.74				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.997						0.936			0.808	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		89						392			571	
Lane Group Capacity, c		462						1011			1102	
v/c Ratio, X		0.19						0.39			0.52	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.0						7.2			8.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		0.9						1.1			1.4	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		17.9					8.3			9.4	
Lane Group LOS		B					A			A	
Approach Delay		17.9					8.3			9.4	
Approach LOS		B					A			A	
Intersection Delay		9.7		$X_C = 0.41$		Intersection LOS			A		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	54		51				29	363			281	116
% Heavy Vehicles, %HV	14		14				8	8			11	11
Peak-Hour Factor, PHF	0.80		0.80				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.982						0.866			0.927	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	17
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		131						441			442	
Lane Group Capacity, c		502						806			929	
v/c Ratio, X		0.26						0.55			0.48	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.6						10.3			9.8	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.2						2.3			1.6	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		15.8					12.6			11.5	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		15.8					12.6			11.5	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		12.5		$X_c = 0.43$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		65	234	382	158	56	187			458	21
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		82		880		62	208			631	
Lane Group Capacity, c	218		216		1314		120	575			661	
v/c Ratio, X	0.02		0.38		0.67		0.52	0.36			0.95	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		23.7		19.0		16.1	15.2			19.6	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		4.9		2.0		14.2	1.7			10.9	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		28.6		21.0		30.3	16.8			30.5	
Lane Group LOS	C		C		C		C	B			C	
Approach Delay	28.3		21.0		19.9		30.5					
Approach LOS	C		C		B		C					
Intersection Delay	24.4		$X_c = 0.74$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	11	37	10				5	231	84	251	475	31
% Heavy Vehicles, %HV	3	3	3				7	7	7	7	7	7
Peak-Hour Factor, PHF	0.85	0.85	0.85				0.90	0.90	0.90	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _l		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.941		0.518	0.518	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7		0	2	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		69						356		314	633	
Lane Group Capacity, c		457						901		542	960	
v/c Ratio, X		0.15						0.40		0.58	0.66	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.1						7.8		8.9	9.5	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.7						1.2		2.3	1.9	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.8						9.0		11.3	11.4	
Lane Group LOS		B						A		B	B	
Approach Delay		16.8						9.0		11.3		
Approach LOS		B						A		B		
Intersection Delay		11.0		$X_c = 0.49$				Intersection LOS		B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/15/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	7	20	12	166	16	101	9	176	127	134	297	8	
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7	
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		2		0	0	2		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =	G =	G = 40.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25								Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		57		189	18	115		347			578		
Lane Group Capacity, c		652		549	777	574		692			557		
v/c Ratio, X		0.09		0.34	0.02	0.20		0.50			1.04		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.5		16.4	14.0	15.2		17.9			25.0		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.3		1.7	0.1	0.8		2.2			22.9	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.7		18.1	14.1	16.0		20.1			47.9	
Lane Group LOS		B		B	B	B		C			D	
Approach Delay		14.7		17.1				20.1			47.9	
Approach LOS		B		B				C			D	
Intersection Delay		31.4		$X_c = 0.69$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/28/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		134	182	71	165		217		70				
% Heavy Vehicles, %HV		10	10	13	13		9		9				
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.964			0.986			0.858					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		296			265			305					
Lane Group Capacity, c		894			785			534					
v/c Ratio, X		0.33			0.34			0.57					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.5			13.5			21.6					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.0			1.1			3.8					

Initial Queue Delay, d_3		0.0			0.0			0.0			
Control Delay		14.4			14.7			25.4			
Lane Group LOS		B			B			C			
Approach Delay		14.4			14.7			25.4			
Approach LOS		B			B			C			
Intersection Delay		18.4			$X_C = 0.44$			Intersection LOS			B

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	35	223	35	31	235	22	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	41	262	41	34	258	24	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	23	17	37	34	19	24	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	29	21	47	38	21	27	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	41	34	86			97	
C (m) (veh/h)	1247	1172	358			434	
v/c	0.03	0.03	0.24			0.22	
95% queue length	0.10	0.09	0.92			0.85	
Control Delay (s/veh)	8.0	8.2	18.2			15.7	
LOS	A	A	C			C	
Approach Delay (s/veh)	--	--	18.2			15.7	
Approach LOS	--	--	C			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		279	18	19	288		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	328	21	20	316	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				43		15	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	45	0	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		20		60			
C (m) (veh/h)		1141		454			
v/c		0.02		0.13			
95% queue length		0.05		0.45			
Control Delay (s/veh)		8.2		14.1			
LOS		A		B			
Approach Delay (s/veh)	--	--	14.1				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	229	23	385	1	140	254	156	174	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			280	428		429			384	
Lane Group Capacity, c		713			524	518		707			483	
v/c Ratio, X		0.04			0.53	0.83		0.61			0.80	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.1	15.6		13.7			15.3	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.1			3.4	12.6		3.6			12.7	
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.5			16.6	28.2		17.2			28.0	
Lane Group LOS		B			B	C		B			C	
Approach Delay		10.5			23.6			17.2			28.0	
Approach LOS		B			C			B			C	
Intersection Delay		22.7			$X_c = 0.81$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				347		3		391			410	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					397			425			477	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.62			0.44			0.50	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.8			9.6			10.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					3.8			1.2			1.1	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>20.6</i>			<i>10.8</i>			<i>11.1</i>	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				<i>20.6</i>				<i>10.8</i>			
Approach LOS				<i>C</i>				<i>B</i>			
Intersection Delay	<i>13.9</i>			$X_c = 0.55$			Intersection LOS			<i>B</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		391	234	136	619		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	424	254	158	719	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		158					
C (m) (veh/h)		852					
v/c		0.19					
95% queue length		0.68					
Control Delay (s/veh)		10.2					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				97		178		436	77	169	433	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					316			558			700	
Lane Group Capacity, c					386			1057			659	
v/c Ratio, X					0.82			0.53			1.06	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.6			8.0			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					16.5			1.7			43.1	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>37.2</i>			<i>9.7</i>			<i>56.1</i>		
Lane Group LOS				<i>D</i>			<i>A</i>			<i>E</i>		
Approach Delay				<i>37.2</i>				<i>9.7</i>				<i>56.1</i>
Approach LOS				<i>D</i>				<i>A</i>				<i>E</i>
Intersection Delay	<i>35.9</i>			$X_c = 0.98$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	141	63	73	36	172	43	83	577	131	95	461	67
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		322			273		91	778			685	
Lane Group Capacity, c		439			556		331	1052			959	
v/c Ratio, X		0.73			0.49		0.27	0.74			0.71	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.9			23.3		11.0	15.6			15.3	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	6.9	3.1	1.5	3.5	4.4
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	32.8	26.4	12.5	19.1	19.7
Lane Group LOS	C	C	B	B	B
Approach Delay	32.8	26.4	18.4	19.7	
Approach LOS	C	C	B	B	
Intersection Delay	22.0	$X_c = 0.74$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					535		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	601	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			87				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	103	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							103
C (m) (veh/h)							680
v/c							0.15
95% queue length							0.53
Control Delay (s/veh)							11.2
LOS							B
Approach Delay (s/veh)	--	--				11.2	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	283							761			535	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	322							836			601	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.49							0.39			0.31	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.9							11.4			10.7	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.5							0.4			0.4	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	26.4						11.8			11.1	
Lane Group LOS	C						B			B	
Approach Delay	26.4						11.8			11.1	
Approach LOS	C						B			B	
Intersection Delay	14.2		$X_C = 0.43$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					380		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	436	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				182			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	195	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			195				
C (m) (veh/h)			574				
v/c			0.34				
95% queue length			1.50				
Control Delay (s/veh)			14.5				
LOS			B				
Approach Delay (s/veh)	--	--	14.5				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	147	144	135	187	462	55	86	337	305	48	58	88
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	175	294		208	560			782		60	182	
Lane Group Capacity, c	208	588		251	1123			915		380	351	
v/c Ratio, X	0.84	0.50		0.83	0.50			0.85		0.16	0.52	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	27.8	24.0		27.6	24.0			30.8		28.2	30.8	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	28.2	2.6		24.4	1.5			10.0		0.9	5.3	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	56.0	26.6		52.1	25.5			40.8		29.1	36.1	
Lane Group LOS	<i>E</i>	<i>C</i>		<i>D</i>	<i>C</i>			<i>D</i>		<i>C</i>	<i>D</i>	
Approach Delay	37.6			32.7				40.8		34.3		
Approach LOS	<i>D</i>			<i>C</i>				<i>D</i>		<i>C</i>		
Intersection Delay	36.7			$X_c = 0.76$				Intersection LOS		<i>D</i>		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	1/24/13			Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	97	98	59	126	102	90		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	43	41	40	78	188	58		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.88	0.88	0.90	0.90	0.78	0.78	0.92	0.92
Flow Rate (veh/h)	165	122	196	156	107	51	186	165
% Heavy Vehicles	42	42	4	4	3	3	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.7	0.0	0.7	0.0	0.5	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.5	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	1.0	0.3	0.4	-0.4	0.3	-0.6	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.15	0.11	0.17	0.14	0.10	0.05	0.17	0.15
hd, final value (s)	7.61	6.89	6.94	6.13	7.26	6.31	6.86	6.37
x, final value	0.35	0.23	0.38	0.27	0.22	0.09	0.35	0.29
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	5.3	4.6	4.6	3.8	5.0	4.0	4.6	4.1
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	415	372	446	406	357	301	436	415
Delay (s/veh)	14.33	11.68	13.79	11.04	11.95	9.62	13.28	11.67
LOS	B	B	B	B	B	A	B	B
Approach: Delay (s/veh)	13.21		12.57		11.20		12.52	
LOS	B		B		B		B	
Intersection Delay (s/veh)	12.53							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	60	101	308	130	266	18	227	196	86	22	518	219
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889			0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		558			460		261	324			843	
Lane Group Capacity, c		1177			980		204	808			1182	
v/c Ratio, X		0.47			0.47		1.28	0.40			0.71	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.6			17.6		25.0	16.9			20.3	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	1.2	1.5	157.9	1.5	3.7
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	18.8	19.1	182.9	18.4	24.0
Lane Group LOS	B	B	F	B	C
Approach Delay	18.8	19.1	91.8	24.0	
Approach LOS	B	B	F	C	
Intersection Delay	38.1	$X_c = 0.88$	Intersection LOS	D	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 Witho-Action Conditions					
Project ID						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	658	383	3	1	405	569	0	0	1	99	0	130
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	723	424			446	532		4		102	0	134
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.92	0.37			0.99	1.39		0.01		0.44	0.00	0.14
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	20.0	5.1			33.9	34.0		30.5		33.0	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	14.4	0.7			28.7	183.8		0.1		5.9	0.0	0.3
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	34.4	5.7			62.6	217.8		30.6		38.9	30.4	6.6
Lane Group LOS	C	A			E	F		C		D	C	A
Approach Delay	23.8			147.0			30.6			20.5		
Approach LOS	C			F			C			C		
Intersection Delay	74.4			$X_c = 0.99$			Intersection LOS			E		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	444	36	0	2	62	69	4	3	0	62	6	908
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	488	40			127		7	5			76	664
Lane Group Capacity, c	709	1056			1536		518	732			541	593
v/c Ratio, X	0.69	0.04			0.08		0.01	0.01			0.14	1.12
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	17.2	11.5			11.7		16.9	16.9			17.8	27.5
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50

Incremental Delay, d_2	5.2	0.1			0.1		0.0	0.0			0.5	72.6
Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	22.4	11.5			11.8		16.9	16.9			18.3	100.1
Lane Group LOS	C	B			B		B	B			B	F
Approach Delay	21.5		11.8		16.9		91.7					
Approach LOS	C		B		B		F					
Intersection Delay	57.5		$X_c = 0.88$		Intersection LOS		E					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	233		24				4	205			390	452
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	265		20					232			1052	
Lane Group Capacity, c	537		465					1663			1910	
v/c Ratio, X	0.49		0.04					0.14			0.55	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	16.6		14.2					7.6			9.8	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	3.1		0.2					0.2			0.8	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	19.7		14.4					7.7			10.6	
Lane Group LOS	B		B					A			B	
Approach Delay	19.4						7.7			10.6		
Approach LOS	B						A			B		
Intersection Delay	11.8			$X_c = 0.53$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	23							
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	41	0	0	0	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	0		0	
Configuration	T							
Upstream Signal	0			0				
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	436							
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	484	0	0	0		
Percent Heavy Vehicles	0	0	1	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage	0			0				
RT Channelized			0				0	
Lanes	0	0	1	0	0		0	
Configuration	R							
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					484			
C (m) (veh/h)					1033			
v/c					0.47			
95% queue length					2.55			
Control Delay (s/veh)					11.5			
LOS					B			
Approach Delay (s/veh)	--	--	11.5					
Approach LOS	--	--	B					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/15/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	578	1		1			22	262	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		304	305		1				354		
Lane Group Capacity, c		784		612	587		388				2036		
v/c Ratio, X		0.01		0.50	0.52		0.00				0.17		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.9	13.0		10.2				11.0		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.8	3.2		0.0				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	15.7	16.2	10.2		11.2
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>		<i>B</i>
Approach Delay	10.2	16.0	10.2	11.2		
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>		
Intersection Delay	14.2	$X_c = 0.35$	Intersection LOS	<i>B</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	53	30	178	57	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	30	76	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	95		305				84	71	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.08		0.27				0.07	0.06	
hd, final value (s)	4.71		4.61				4.96	4.67	
x, final value	0.12		0.39				0.12	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.6				3.0	2.7	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	345		555				334	321	
Delay (s/veh)	8.38		10.53				8.61	8.14	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.38		10.53				8.40		
LOS	A		B				A		
Intersection Delay (s/veh)	9.57								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	39	45	0		0	194	22		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	41	61	86		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT	TR			
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	97		279		80	132			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.25		0.07	0.12			
hd, final value (s)	5.00		4.53		4.94	4.38			
x, final value	0.13		0.35		0.11	0.16			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.0		2.5		2.9	2.4			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	347		529		330	382			
Delay (s/veh)	8.77		9.97		8.54	8.21			
LOS	A		A		A	A			
Approach: Delay (s/veh)	8.77		9.97		8.34				
LOS	A		A		A				
Intersection Delay (s/veh)	9.18								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	145	175	139	2	37	5	542	37	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _l		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		351	96		46			599				
Lane Group Capacity, c		566	560		530			2286				
v/c Ratio, X		0.62	0.17		0.09			0.26				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		16.2	13.5		13.1			9.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		5.0	0.7		0.3			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		21.2	14.1		13.4			9.4			
Lane Group LOS		C	B		B			A			
Approach Delay		19.7			13.4			9.4			
Approach LOS		B			B			A			
Intersection Delay		13.8			$X_c = 0.41$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	138		39				14	283			333	31
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		218						341			400	
Lane Group Capacity, c		465						1092			1173	
v/c Ratio, X		0.47						0.31			0.34	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.4						6.8			7.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		3.3						0.7			0.8	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		21.7					7.6			7.7	
Lane Group LOS		C					A			A	
Approach Delay	21.7					7.6		7.7			
Approach LOS	C					A		A			
Intersection Delay	10.9		$X_C = 0.38$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	78		52				43	378			282	173
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		151						483			485	
Lane Group Capacity, c		555						844			1006	
v/c Ratio, X		0.27						0.57			0.48	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						10.5			9.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.2						2.5			1.6	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		15.9					13.1			11.5	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		15.9					13.1			11.5	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		12.7		$X_c = 0.45$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	9		132	222	455	123	95	164			341	31
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	12		171		919		107	184			413	
Lane Group Capacity, c	222		220		1339		171	593			690	
v/c Ratio, X	0.05		0.78		0.69		0.63	0.31			0.60	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		25.1		19.1		16.8	14.9			16.7	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		19.8		2.1		15.1	1.3			3.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		44.9		21.2		32.0	16.1			19.8	
Lane Group LOS	C		D		C		C	B			B	
Approach Delay	43.5		21.2		22.0		19.8					
Approach LOS	D		C		C		B					
Intersection Delay	23.3		$X_c = 0.67$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	5	15	7				6	254	109	222	445	27
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _i		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.932		0.785	0.785	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	2	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		43						414		247	524	
Lane Group Capacity, c		456						925		532	1018	
v/c Ratio, X		0.09						0.45		0.46	0.51	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		15.8						8.1		8.2	8.5	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.4						1.5		2.3	1.5	

Initial Queue Delay, d_3	0.0					0.0		0.0	0.0	
Control Delay	16.2					9.5		10.4	9.9	
Lane Group LOS	B					A		B	A	
Approach Delay	16.2				9.5			10.1		
Approach LOS	B				A			B		
Intersection Delay	10.1		$X_c = 0.37$		Intersection LOS			B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/15/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _i	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	5	7	17	257	25	102	21	262	165	87	346	19	
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I _i		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		2		0	0	2		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =	G =	G = 40.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		37		306	30	121		503			502		
Lane Group Capacity, c		626		548	763	563		692			635		
v/c Ratio, X		0.06		0.56	0.04	0.21		0.73			0.79		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.3		18.5	14.1	15.4		20.5			21.4		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.2		3.4	0.1	0.7		4.3			6.5	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.4		21.9	14.2	16.1		24.9			27.9	
Lane Group LOS		B		C	B	B		C			C	
Approach Delay		14.4		19.8				24.9			27.9	
Approach LOS		B		B				C			C	
Intersection Delay		24.1		$X_c = 0.67$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		170	182	123	193		185		118				
% Heavy Vehicles, %HV		8	8	16	16		2		2				
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.961			0.989			0.922					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		342			347			344					
Lane Group Capacity, c		918			611			564					
v/c Ratio, X		0.37			0.57			0.61					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.8			15.7			22.0					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.1			3.8			4.5					

Initial Queue Delay, d_3		0.0		0.0		0.0			
Control Delay		14.9		19.5		26.5			
Lane Group LOS		B		B		C			
Approach Delay		14.9		19.5		26.5			
Approach LOS		B		B		C			
Intersection Delay		20.3		$X_c = 0.59$		Intersection LOS			C

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	28	227	66	56	226	27	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	32	260	75	69	279	33	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	33	18	28	55	17	33	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	38	20	32	67	20	40	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	32	69	127			90	
C (m) (veh/h)	1226	1186	319			328	
v/c	0.03	0.06	0.40			0.27	
95% queue length	0.08	0.19	1.84			1.09	
Control Delay (s/veh)	8.0	8.2	23.6			20.1	
LOS	A	A	C			C	
Approach Delay (s/veh)	--	--	23.6			20.1	
Approach LOS	--	--	C			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		296	18	14	296		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	340	20	17	365	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				28		26	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	41	0	38	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		17		79			
C (m) (veh/h)		1171		472			
v/c		0.01		0.17			
95% queue length		0.04		0.60			
Control Delay (s/veh)		8.1		14.2			
LOS		A		B			
Approach Delay (s/veh)	--	--	14.2				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	10	2	259	20	261	4	136	301	202	236	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		14			340	318		474			510	
Lane Group Capacity, c		725			533	523		741			464	
v/c Ratio, X		0.02			0.64	0.61		0.64			1.10	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			13.9	13.7		13.9			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.0		5.4	4.8		3.9			71.5	
Initial Queue Delay, d_3		0.0		0.0	0.0		0.0			0.0	
Control Delay		10.3		19.3	18.5		17.8			89.0	
Lane Group LOS		B		B	B		B			F	
Approach Delay		10.3		18.9			17.8			89.0	
Approach LOS		B		B			B			F	
Intersection Delay		40.1		$X_c = 0.87$			Intersection LOS			D	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				511		4		437			496	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					613			470			577	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.91			0.46			0.60	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.2			9.7			10.8	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					8.8			1.4			1.0	

Initial Queue Delay, d_3				0.0			0.0			0.0	
Control Delay				28.0			11.1			11.7	
Lane Group LOS				C			B			B	
Approach Delay				28.0				11.1			
Approach LOS				C				B			
Intersection Delay	17.5			$X_c = 0.73$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		437	254	182	825		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	469	273	211	959	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		211					
C (m) (veh/h)		797					
v/c		0.26					
95% queue length		1.06					
Control Delay (s/veh)		11.1					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				130		142		536	101	118	692	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					324			685			942	
Lane Group Capacity, c					398			1096			773	
v/c Ratio, X					0.81			0.63			1.22	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.6			8.7			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					15.1			2.2			99.8	

Initial Queue Delay, d_3				0.0			0.0			0.0		
Control Delay				35.7			10.9			112.8		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				35.7				10.9				112.8
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	64.2			$X_c = 1.09$			Intersection LOS			<i>E</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	94	44	102	33	188	42	56	564	104	108	537	49
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		273			325		62	734			786	
Lane Group Capacity, c		464			568		289	1067			1002	
v/c Ratio, X		0.59			0.57		0.21	0.69			0.78	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		24.3			24.1		10.6	14.9			16.3	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		2.7			4.1		1.3	2.8			5.9	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		27.0			28.2		11.9	17.7			22.2	
Lane Group LOS		C			C		B	B			C	
Approach Delay		27.0			28.2			17.3			22.2	
Approach LOS		C			C			B			C	
Intersection Delay		21.9			$X_c = 0.71$			Intersection LOS			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					624		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	717	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	<i>Raised curb</i>						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			69				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	88	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							88
C (m) (veh/h)							582
v/c							0.15
95% queue length							0.53
Control Delay (s/veh)							12.3
LOS							B
Approach Delay (s/veh)	--	--				12.3	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	368							700			624	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	404							778			717	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.60							0.36			0.36	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.0							11.1			11.1	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	3.9							0.4			0.5	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	28.9						11.5			11.6	
Lane Group LOS	C						B			B	
Approach Delay	28.9						11.5			11.6	
Approach LOS	C						B			B	
Intersection Delay	15.2		$X_c = 0.45$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					490		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	550	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				458			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	492	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			492				
C (m) (veh/h)			494				
v/c			1.00				
95% queue length			13.46				
Control Delay (s/veh)			68.5				
LOS			F				
Approach Delay (s/veh)	--	--	68.5				
Approach LOS	--	--	F				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	158	166	190	239	319	44	116	297	253	46	61	104
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	180	354		266	429			689		61	184	
Lane Group Capacity, c	270	583		240	1148			931		384	357	
v/c Ratio, X	0.67	0.61		1.11	0.37			0.74		0.16	0.52	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	25.7	25.1		29.5	22.2			29.5		28.2	30.7	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	8.8	3.3		88.7	0.9			5.3		0.9	5.1	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	34.5	28.4		118.2	23.1			34.8		29.1	35.8	
Lane Group LOS	C	C		F	C			C		C	D	
Approach Delay	30.4			59.5				34.8		34.1		
Approach LOS	C			E				C		C		
Intersection Delay	41.6			$X_c = 0.83$				Intersection LOS		D		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	1/24/13			Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	93	111	54	143	140	86		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	15	39	67	80	193	60		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.87	0.87	0.91	0.91	0.76	1.00	1.00	1.00
Flow Rate (veh/h)	169	126	233	170	70	67	176	157
% Heavy Vehicles	7	7	1	1	1	0	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.6	0.0	0.7	0.0	0.3	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.5	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.4	-0.2	0.4	-0.4	0.2	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.15	0.11	0.21	0.15	0.06	0.06	0.16	0.14
hd, final value (s)	6.93	6.27	6.71	5.98	7.12	6.27	6.86	6.36
x, final value	0.33	0.22	0.43	0.28	0.14	0.12	0.34	0.28
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	4.6	4.0	4.4	3.7	4.8	4.0	4.6	4.1
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	419	376	483	420	320	317	426	407
Delay (s/veh)	12.93	10.72	14.45	11.02	10.97	9.80	12.97	11.48
LOS	B	B	B	B	B	A	B	B
Approach: Delay (s/veh)	11.98		13.00		10.39		12.27	
LOS	B		B		B		B	
Intersection Delay (s/veh)	12.23							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	51	114	275	120	284	9	203	142	121	21	523	120
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.884			0.927		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		501			503		218	283			706	
Lane Group Capacity, c		1198			1040		265	804			1321	
v/c Ratio, X		0.42			0.48		0.82	0.35			0.53	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.1			17.7		21.9	16.5			18.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		1.0			1.5		24.2	1.2			1.6	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		18.0			19.2		46.1	17.7			19.8	
Lane Group LOS		B			B		D	B			B	
Approach Delay		18.0			19.2		30.0			19.8		
Approach LOS		B			B		C			B		
Intersection Delay		21.6			$X_c = 0.65$		Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	723	340	4	3	348	569	2	3	2	118	0	275
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	803	382			369	599		17		147	0	344
Lane Group Capacity, c	820	1205			584	497		148		204	283	862
v/c Ratio, X	0.98	0.32			0.63	1.21		0.11		0.72	0.00	0.40
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	16.8	4.2			26.6	31.0		32.7		36.1	32.1	10.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	21.5	0.5			3.4	104.6		1.6		19.7	0.0	1.4
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	38.3	4.6			30.0	135.6		34.2		55.8	32.1	11.8
Lane Group LOS	<i>D</i>	<i>A</i>			<i>C</i>	<i>F</i>		<i>C</i>		<i>E</i>	<i>C</i>	<i>B</i>
Approach Delay	27.5			95.3			34.2			25.0		
Approach LOS	<i>C</i>			<i>F</i>			<i>C</i>			<i>C</i>		
Intersection Delay	51.7			$X_c = 1.02$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	426	35	0	1	35	42	3	3	2	60	1	882
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _i	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	473	39			72			16			65	626
Lane Group Capacity, c	755	1066			1561			1220			550	616
v/c Ratio, X	0.63	0.04			0.05			0.01			0.12	1.02
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	16.4	11.5			11.5			16.9			17.6	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	3.7	0.1			0.1			0.0			0.4	39.2
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	20.1	11.5			11.6			16.9			18.0	66.7
Lane Group LOS	C	B			B			B			B	E
Approach Delay	19.5		11.6		16.9		62.1					
Approach LOS	B		B		B		E					
Intersection Delay	41.8		$X_c = 0.80$		Intersection LOS		D					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	235		23				2	252			391	367
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	270		20					288			833	
Lane Group Capacity, c	547		473					1731			1926	
v/c Ratio, X	0.49		0.04					0.17			0.43	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	16.6		14.2					7.7			9.0	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	3.1		0.2					0.2			0.6	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	19.7		14.4					7.9			9.6	
Lane Group LOS	<i>B</i>		<i>B</i>					<i>A</i>			<i>A</i>	
Approach Delay	19.3							7.9		9.6		
Approach LOS	<i>B</i>							<i>A</i>		<i>A</i>		
Intersection Delay	11.2			$X_c = 0.46$				Intersection LOS		<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/15/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		23					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	32	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			483				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	548	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					548		
C (m) (veh/h)					1045		
v/c					0.52		
95% queue length					3.15		
Control Delay (s/veh)					12.2		
LOS					B		
Approach Delay (s/veh)	--	--	12.2				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/15/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	477	2		4			22	280	4	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.25	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		268	270		5				336		
Lane Group Capacity, c		784		600	576		396				2053		
v/c Ratio, X		0.01		0.45	0.47		0.01				0.16		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.5	12.7		10.3				11.0		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.4	2.7		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	14.9	15.4	10.3	11.1
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	15.1	10.3	11.1	
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Intersection Delay	13.6	$X_c = 0.32$	Intersection LOS	<i>B</i>	

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	103	37	182	81	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	23	86	24			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.78				0.91	0.91	
Flow Rate (veh/h)	174		336				72	73	
% Heavy Vehicles	2		3				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.3	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.4	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.15		0.30				0.06	0.06	
hd, final value (s)	4.54		4.66				5.18	4.90	
x, final value	0.22		0.43				0.10	0.10	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.5		2.7				3.2	2.9	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	424		586				322	323	
Delay (s/veh)	8.81		11.19				8.78	8.44	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.81		11.19				8.61		
LOS	A		B				A		
Intersection Delay (s/veh)	9.99								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/15/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	37	88	0		0	200	19		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	63	97	138		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT	TR			
PHF	0.80		0.78		0.99	0.99			
Flow Rate (veh/h)	155		280		111	188			
% Heavy Vehicles	2		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.3		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.1		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.14		0.25		0.10	0.17			
hd, final value (s)	5.05		4.83		5.12	4.56			
x, final value	0.22		0.38		0.16	0.24			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.1		2.8		3.1	2.6			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	405		530		361	438			
Delay (s/veh)	9.45		10.72		9.08	8.98			
LOS	A		B		A	A			
Approach: Delay (s/veh)	9.45		10.72		9.01				
LOS	A		B		A				
Intersection Delay (s/veh)	9.76								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	187	177	142	7	46	8	434	102	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _l		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		428	105		70			605				
Lane Group Capacity, c		545	560		529			2300				
v/c Ratio, X		0.79	0.19		0.13			0.26				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		17.5	13.6		13.3			9.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		10.9	0.7		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		28.3	14.3		13.8			9.4			
Lane Group LOS		C	B		B			A			
Approach Delay		25.6			13.8			9.4			
Approach LOS		C			B			A			
Intersection Delay		16.8			$X_c = 0.48$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	146		33				18	451			451	44
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		186						545			538	
Lane Group Capacity, c		457						1099			1127	
v/c Ratio, X		0.41						0.50			0.48	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		18.1						7.8			7.7	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.6						1.4			1.3	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		20.7					9.2			9.0	
Lane Group LOS		C					A			A	
Approach Delay	20.7					9.2		9.0			
Approach LOS	C					A		A			
Intersection Delay	10.8		$X_C = 0.47$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	143		83				22	489			388	200
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		283						595			617	
Lane Group Capacity, c		573						892			973	
v/c Ratio, X		0.49						0.67			0.63	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.0						11.3			11.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.8						3.3			2.8	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		18.7					14.6			13.8	
Lane Group LOS		B					B			B	
Approach Delay	18.7					14.6		13.8			
Approach LOS	B					B		B			
Intersection Delay	15.0		$X_c = 0.60$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		145	266	450	266	83	198			479	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		153		1115		90	215			557	
Lane Group Capacity, c	222		220		1329		124	616			670	
v/c Ratio, X	0.02		0.70		0.84		0.73	0.35			0.83	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		24.8		20.2		17.6	15.1			18.4	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		14.3		3.4		28.9	1.4			10.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		39.2		23.6		46.5	16.5			28.6	
Lane Group LOS	C		D		C		D	B			C	
Approach Delay	38.8		23.6		25.4		28.6					
Approach LOS	D		C		C		C					
Intersection Delay	26.3		$X_c = 0.81$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	9	20	6				8	272	118	308	533	50
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _l		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.922		0.564	0.564	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	7	
Min. Time for Pedestrians, G _p		17.5		16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		56						433		331	614	
Lane Group Capacity, c		460						957		514	986	
v/c Ratio, X		0.12						0.45		0.64	0.62	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.0						8.1		9.4	9.2	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.5						1.4		3.5	1.7	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.5						9.5		12.9	10.9	
Lane Group LOS		B						A		B	B	
Approach Delay		16.5						9.5		11.6		
Approach LOS		B						A		B		
Intersection Delay		11.2			$X_c = 0.47$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	12	20	258	28	106	18	286	169	91	413	9
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p	11.5			11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		55		297	32	122		515			590	
Lane Group Capacity, c		632		560	792	585		731			633	
v/c Ratio, X		0.09		0.53	0.04	0.21		0.70			0.93	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.4		18.2	14.1	15.3		20.2			23.7	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.3		3.2	0.1	0.7		3.8			7.2	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.7		21.4	14.2	16.0		24.0			30.9	
Lane Group LOS		B		C	B	B		C			C	
Approach Delay		14.7		19.4				24.0			30.9	
Approach LOS		B		B				C			C	
Intersection Delay		25.0		$X_c = 0.73$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		163	245	114	177		139		117				
% Heavy Vehicles, %HV		5	5	8	8		1		1				
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.989			0.917					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		426			350			382					
Lane Group Capacity, c		933			590			565					
v/c Ratio, X		0.46			0.59			0.68					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		14.6			16.0			22.8					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.5			4.3			5.9					

Initial Queue Delay, d_3		0.0		0.0		0.0			
Control Delay		16.1		20.3		28.7			
Lane Group LOS		B		C		C			
Approach Delay		16.1		20.3		28.7			
Approach LOS		B		C		C			
Intersection Delay		21.5		$X_c = 0.63$		Intersection LOS			C

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description Charleston EIS							
East/West Street: Sharrotts Road				North/South Street: Arthur Kill Road			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	11	234	49	28	261	27	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	11	254	53	31	296	30	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	26	32	23	60	17	40	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	64	79	57	76	21	51	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	11	31		148			200
C (m) (veh/h)	1228	1237		341			389
v/c	0.01	0.03		0.43			0.51
95% queue length	0.03	0.08		2.12			2.83
Control Delay (s/veh)	8.0	8.0		23.4			23.6
LOS	A	A		C			C
Approach Delay (s/veh)	--	--	23.4			23.6	
Approach LOS	--	--	C			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		276	12	16	328		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	299	13	18	372	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				9		19	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	12	0	25	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		18		37			
C (m) (veh/h)		1232		570			
v/c		0.01		0.06			
95% queue length		0.04		0.21			
Control Delay (s/veh)		8.0		11.8			
LOS		A		B			
Approach Delay (s/veh)	--	--	11.8				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	9	5	308	26	378	5	162	366	184	204	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		15			363	411		585			426	
Lane Group Capacity, c		724			549	539		744			409	
v/c Ratio, X		0.02			0.66	0.76		0.79			1.04	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.1	15.0		15.2			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.1			5.3	8.4		7.1			55.7	
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			19.3	23.4		22.3			73.2	
Lane Group LOS		B			B	C		C			E	
Approach Delay		10.3			21.5			22.3			73.2	
Approach LOS		B			C			C			E	
Intersection Delay		33.9			$X_c = 0.90$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				493		8		524			518	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					582			576			569	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.86			0.56			0.55	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					18.7			10.4			10.4	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					7.1			1.9			0.8	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>25.8</i>			<i>12.4</i>			<i>11.2</i>	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				<i>25.8</i>				<i>12.4</i>			
Approach LOS				<i>C</i>				<i>B</i>			
Intersection Delay	<i>16.5</i>		$X_c = 0.68$		Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		524	262	175	835		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	575	287	192	917	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		192					
C (m) (veh/h)		763					
v/c		0.25					
95% queue length		0.99					
Control Delay (s/veh)		11.3					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				104		186		583	90	130	680	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					326			740			890	
Lane Group Capacity, c					394			1131			745	
v/c Ratio, X					0.83			0.65			1.19	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.7			9.0			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					16.9			2.3			91.4	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>37.6</i>			<i>11.3</i>			<i>104.4</i>		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				<i>37.6</i>				<i>11.3</i>				<i>104.4</i>
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	<i>58.1</i>			$X_c = 1.08$			Intersection LOS			<i>E</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	101	45	91	29	88	35	133	743	138	26	591	96
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		304			209		145	958			753	
Lane Group Capacity, c		520			544		304	1067			1222	
v/c Ratio, X		0.58			0.38		0.48	0.90			0.62	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		24.2			22.3		12.6	18.3			14.1	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	2.6	2.0	3.0	7.2	2.2
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	26.8	24.3	15.6	25.5	16.3
Lane Group LOS	C	C	B	C	B
Approach Delay	26.8	24.3	24.2	16.3	
Approach LOS	C	C	C	B	
Intersection Delay	22.0	$X_c = 0.78$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description Charleston EIS							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					656		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	713	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			55				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	62	0	0	0	
Percent Heavy Vehicles	0	0	5	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							62
C (m) (veh/h)							681
v/c							0.09
95% queue length							0.30
Control Delay (s/veh)							10.8
LOS							B
Approach Delay (s/veh)	--	--				10.8	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	381							879			656	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	443							916			713	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.66							0.43			0.37	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.7							11.7			11.2	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	5.1							0.4			0.5	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	30.8						12.0			11.7	
Lane Group LOS	C						B			B	
Approach Delay	30.8						12.0			11.7	
Approach LOS	C						B			B	
Intersection Delay	15.9		$X_c = 0.52$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/13/16			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					585		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	629	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				228			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	253	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			253				
C (m) (veh/h)			448				
v/c			0.56				
95% queue length			3.41				
Control Delay (s/veh)			23.0				
LOS			C				
Approach Delay (s/veh)	--	--	23.0				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD Peak					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	173	185	202	263	429	48	126	486	280	45	121	156
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	186	365		274	597			991		54	330	
Lane Group Capacity, c	195	584		221	1196			957		384	357	
v/c Ratio, X	0.95	0.63		1.24	0.50			1.04		0.14	0.92	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	29.3	25.3		29.5	23.4			32.5		28.1	34.3	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	43.3	3.5		137.7	1.3			38.7		0.7	28.6	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	72.6	28.8		167.2	24.7			71.2		28.8	62.9	
Lane Group LOS	<i>E</i>	<i>C</i>		<i>F</i>	<i>C</i>			<i>E</i>		<i>C</i>	<i>E</i>	
Approach Delay	43.6			69.5				71.2		58.1		
Approach LOS	<i>D</i>			<i>E</i>				<i>E</i>		<i>E</i>		
Intersection Delay	63.4			$X_c = 1.09$				Intersection LOS		<i>E</i>		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	138	206	58	107	187	128		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	59	59	92	111	269	83		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.89	0.85	0.91	0.91	0.89	0.89	0.92	0.92
Flow Rate (veh/h)	270	189	219	243	132	103	265	236
% Heavy Vehicles	0	0	0	1	1	1	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.6	0.0	0.5	0.0	0.5	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.4	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.3	-0.3	0.3	-0.4	0.3	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.24	0.17	0.19	0.22	0.12	0.09	0.24	0.21
hd, final value (s)	7.95	7.41	7.95	7.29	8.45	7.50	7.90	7.40
x, final value	0.60	0.39	0.48	0.49	0.31	0.21	0.58	0.49
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	5.7	5.1	5.6	5.0	6.1	5.2	5.6	5.1
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	443	439	442	481	382	353	446	474
Delay (s/veh)	21.72	14.76	17.86	16.86	14.89	12.23	20.98	16.88
LOS	C	B	C	C	B	B	C	C
Approach: Delay (s/veh)	18.85		17.33		13.73		19.05	
LOS	C		C		B		C	
Intersection Delay (s/veh)	17.76							
Intersection LOS	C							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	79	123	266	161	357	24	231	262	112	25	815	162
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.830			0.865		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		509			695		246	411			1077	
Lane Group Capacity, c		1016			1038		137	825			1348	
v/c Ratio, X		0.50			0.67		1.80	0.50			0.80	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		17.9			19.8		25.0	17.8			21.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	1.5	3.0	385.6	2.1	5.0
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	19.3	22.8	410.6	20.0	26.6
Lane Group LOS	B	C	F	B	C
Approach Delay	19.3	22.8	166.2	26.6	
Approach LOS	B	C	F	C	
Intersection Delay	55.6	$X_c = 1.23$	Intersection LOS	E	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	849	411			451	786				194		206
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, l _i	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	875	428			480	688				211		224
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	0.98	0.37			1.04	1.76				0.37		0.23
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	17.4	5.1			34.0	34.0				32.5		6.7
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	16.8	0.5			30.3	343.7				1.8		0.6

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0				0.0		0.0
Control Delay	34.2	5.5			64.3	377.7				34.3		7.3
Lane Group LOS	C	A			E	F				C		A
Approach Delay	24.8			248.9						20.4		
Approach LOS	C			F						C		
Intersection Delay	114.2			$X_C = 1.09$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	561	45	0	0	45	56	0	0	1	81	0	1192
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	584	47			95			1			87	854
Lane Group Capacity, c	738	1066			1633			1184			546	622
v/c Ratio, X	0.79	0.04			0.06			0.00			0.16	1.37
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	18.6	11.5			11.6			16.8			17.9	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	8.2	0.1			0.1			0.0			0.5	176.5
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	26.8	11.6			11.7			16.8			18.4	204.0
Lane Group LOS	C	B			B			B			B	F
Approach Delay	25.7			11.7			16.8			186.9		
Approach LOS	C			B			B			F		
Intersection Delay	115.8			$X_c = 1.05$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	374		34				5	256			508	603
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	440		29					297			1249	
Lane Group Capacity, c	573		497					1695			1927	
v/c Ratio, X	0.77		0.06					0.18			0.65	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	18.5		14.3					7.7			10.5	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	8.6		0.2					0.2			0.8	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	27.1		14.5					7.9			11.4	
Lane Group LOS	C		B					A			B	
Approach Delay	26.3							7.9		11.4		
Approach LOS	C							A		B		
Intersection Delay	14.3			$X_c = 0.69$				Intersection LOS		B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		28					
Peak-Hour Factor, PHF	1.00	0.69	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	40	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			625				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	710	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					710		
C (m) (veh/h)					1037		
v/c					0.68		
95% queue length					5.71		
Control Delay (s/veh)					15.6		
LOS					C		
Approach Delay (s/veh)	--	--	15.6				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/16/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	732	2		5			27	380	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		603	203		6				458		
Lane Group Capacity, c		784		606	582		337				2036		
v/c Ratio, X		0.01		1.00	0.35		0.02				0.22		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		17.4	11.9		10.3				11.3		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		34.8	1.6		0.1				0.3		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		52.3	13.5		10.4				11.5	
Lane Group LOS		<i>B</i>		<i>D</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		42.5			10.4			11.5		
Approach LOS		<i>B</i>		<i>D</i>			<i>B</i>			<i>B</i>		
Intersection Delay		31.1		$X_c = 0.61$			Intersection LOS			C		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/16/13				Analysis Year	2015 With-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	75	34	258	58	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	17	116	22			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.75		0.85				0.89	0.89	
Flow Rate (veh/h)	145		371				84	89	
% Heavy Vehicles	2		4				0	0	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.2		0.2				0.0	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.13		0.33				0.07	0.08	
hd, final value (s)	4.64		4.74				5.18	4.98	
x, final value	0.19		0.49				0.12	0.12	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.6		2.7				3.2	3.0	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	395		621				334	339	
Delay (s/veh)	8.70		12.19				8.90	8.67	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.70		12.19				8.78		
LOS	A		B				A		
Intersection Delay (s/veh)	10.60								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	1/16/13				Analysis Year	2015 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	25	66	0		0	264	23	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	51	103	135		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT		TR	
PHF	0.75		0.85		0.95	0.95		
Flow Rate (veh/h)	121		337		106	196		
% Heavy Vehicles	2		2		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.3		0.0		0.5	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		-0.0		0.1	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.11		0.30		0.09	0.17		
hd, final value (s)	5.13		4.75		5.16	4.62		
x, final value	0.17		0.45		0.15	0.25		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	3.1		2.8		3.2	2.6		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	371		587		356	446		
Delay (s/veh)	9.20		11.51		9.08	9.16		
LOS	A		B		A	A		
Approach: Delay (s/veh)	9.20		11.51		9.13			
LOS	A		B		A			
Intersection Delay (s/veh)	10.20							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	211	262	178	0	61	15	672	63	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _i		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only		06	07	08			
Timing	G = 21.0	G =	G =	G =	G = 29.0		G =	G =	G =			
	Y = 5	Y =	Y =	Y =	Y = 5		Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		630	149		91			794				
Lane Group Capacity, c		553	560		557			2266				
v/c Ratio, X		1.14	0.27		0.16			0.35				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.5	14.0		13.4			9.6				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		82.8	1.2		0.6			0.4				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		102.3	15.1		14.1			10.1			
Lane Group LOS		<i>F</i>	<i>B</i>		<i>B</i>			<i>B</i>			
Approach Delay		85.6			14.1			10.1			
Approach LOS		<i>F</i>			<i>B</i>			<i>B</i>			
Intersection Delay		45.6			$X_c = 0.68$			Intersection LOS			<i>D</i>

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	211		54				17	388			367	59
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		288						440			468	
Lane Group Capacity, c		465						1111			1185	
v/c Ratio, X		0.62						0.40			0.39	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.3						7.3			7.3	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		5.7						1.0			0.9	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		25.0					8.3			8.2	
Lane Group LOS		C					A			A	
Approach Delay		25.0					8.3			8.2	
Approach LOS		C					A			A	
Intersection Delay		12.3			$X_C = 0.47$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	138		65				42	495			371	245
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		271						584			655	
Lane Group Capacity, c		575						854			1012	
v/c Ratio, X		0.47						0.68			0.65	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.8						11.4			11.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.6						3.8			2.9	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		18.4					15.2			14.0	
Lane Group LOS		B					B			B	
Approach Delay	18.4					15.2		14.0			
Approach LOS	B					B		B			
Intersection Delay	15.2		$X_c = 0.60$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		160	285	596	168	126	218			404	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I _l	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		225		1206		137	237			468	
Lane Group Capacity, c	222		220		1338		148	621			707	
v/c Ratio, X	0.12		1.02		0.90		0.93	0.38			0.66	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		26.0		20.7		19.3	15.3			17.1	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.8		57.3		5.3		53.5	1.6			3.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.7		83.3		26.0		72.7	16.9			20.7	
Lane Group LOS	<i>C</i>		<i>F</i>		<i>C</i>		<i>E</i>	<i>B</i>			<i>C</i>	
Approach Delay	76.9		26.0		37.4		20.7					
Approach LOS	<i>E</i>		<i>C</i>		<i>D</i>		<i>C</i>					
Intersection Delay	32.4		$X_c = 0.93$		Intersection LOS		<i>C</i>					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions					
Project ID						Charleston EIS						
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	30	32	5				3	314	143	305	498	46
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _i		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.899		0.644	0.644	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0		0	0	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		88						499		339	592	
Lane Group Capacity, c		462						971		475	1025	
v/c Ratio, X		0.19						0.51		0.71	0.58	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.3						8.5		10.0	8.9	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.9						1.7		5.8	1.5	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.2					10.2		15.8	10.4	
Lane Group LOS		B					B		B	B	
Approach Delay		17.2					10.2		12.4		
Approach LOS		B					B		B		
Intersection Delay		12.0		$X_c = 0.54$		Intersection LOS		B			

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/16/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	7	7	18	284	18	102	12	309	234	83	406	7	
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1	
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		0		0	0	0		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =		G = 40.0		G =	G =		G =		
	Y = 5	Y =	Y =	Y =		Y = 5		Y =	Y =		Y =		
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		37		316	20	113		603			551		
Lane Group Capacity, c		628		580	808	601		741			626		
v/c Ratio, X		0.06		0.54	0.02	0.19		0.81			0.88		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.3		18.3	14.0	15.2		21.8			22.8		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.2		3.2	0.1	0.6		5.2			9.0	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.4		21.6	14.1	15.8		26.9			31.9	
Lane Group LOS		B		C	B	B		C			C	
Approach Delay		14.4		19.8				26.9			31.9	
Approach LOS		B		B				C			C	
Intersection Delay		26.3		$X_c = 0.71$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		183	232	166	179		147		138				
% Heavy Vehicles, %HV		2	2	4	4		1		1				
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.992			0.953					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		2			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		461			388			335					
Lane Group Capacity, c		978			505			564					
v/c Ratio, X		0.47			0.77			0.59					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		14.7			18.3			21.9					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.5			10.6			4.3					

Initial Queue Delay, d_3		0.0		0.0		0.0				
Control Delay		16.2		28.9		26.2				
Lane Group LOS		B		C		C				
Approach Delay		16.2		28.9		26.2				
Approach LOS		B		C		C				
Intersection Delay		23.2		$X_c = 0.69$		Intersection LOS				C

With-Action

2020

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	27	186	42	29	255	39	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	
Hourly Flow Rate, HFR (veh/h)	32	224	50	34	307	46	
Percent Heavy Vehicles	20	--	--	13	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	9	8	8	46	10	28	
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.95	0.95	0.95	
Hourly Flow Rate, HFR (veh/h)	11	10	10	48	10	29	
Percent Heavy Vehicles	32	32	32	5	5	5	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	32	34	87			31	
C (m) (veh/h)	1113	1228	388			342	
v/c	0.03	0.03	0.22			0.09	
95% queue length	0.09	0.09	0.85			0.30	
Control Delay (s/veh)	8.3	8.0	16.9			16.6	
LOS	A	A	C			C	
Approach Delay (s/veh)	--	--	16.9			16.6	
Approach LOS	--	--	C			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		216	89	102	205		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	260	107	122	246	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				150		39	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.69	1.00	0.69	
Hourly Flow Rate, HFR (veh/h)	0	0	0	217	0	56	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		122		273			
C (m) (veh/h)		1144		359			
v/c		0.11		0.76			
95% queue length		0.36		6.08			
Control Delay (s/veh)		8.5		40.7			
LOS		A		E			
Approach Delay (s/veh)	--	--	40.7				
Approach LOS	--	--	E				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	3	6	1	174	34	227	0	205	304	117	160	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			239	261		599			328	
Lane Group Capacity, c		717			539	514		710			304	
v/c Ratio, X		0.02			0.44	0.51		0.84			1.08	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.5	12.9		15.7			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.0			2.5	3.3		10.6			74.2	
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			15.0	16.3		26.3			91.7	
Lane Group LOS		B			B	B		C			F	
Approach Delay		10.3			15.7			26.3			91.7	
Approach LOS		B			B			C			F	
Intersection Delay		37.4			$X_c = 0.79$			Intersection LOS			D	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				268		5		504			335	
% Heavy Vehicles, %HV				8		8		11			15	
Peak-Hour Factor, PHF				0.88		0.88		0.85			0.87	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.893			0.886			0.754	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			3	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					311			593			385	
Lane Group Capacity, c					632			970			925	
v/c Ratio, X					0.49			0.61			0.42	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					15.9			10.8			9.5	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					2.4			2.5			1.0	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>18.4</i>			<i>13.4</i>			<i>10.5</i>	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>B</i>	
Approach Delay				<i>18.4</i>				<i>13.4</i>			
Approach LOS				<i>B</i>				<i>B</i>			
Intersection Delay	<i>13.7</i>			$X_c = 0.56$			Intersection LOS			<i>B</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/28/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		504	224	120	483		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.87	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	592	263	137	555	0	
Percent Heavy Vehicles	0	--	--	20	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		137					
C (m) (veh/h)		713					
v/c		0.19					
95% queue length		0.71					
Control Delay (s/veh)		11.2					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/28/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				62		177		549	74	119	358	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.980			0.769			0.842	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					257			733			548	
Lane Group Capacity, c					382			1033			637	
v/c Ratio, X					0.67			0.71			0.86	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					19.7			9.4			11.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					9.0			3.2			12.3	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>28.6</i>			<i>12.6</i>			<i>23.3</i>		
Lane Group LOS				<i>C</i>			<i>B</i>			<i>C</i>		
Approach Delay				<i>28.6</i>				<i>12.6</i>				<i>23.3</i>
Approach LOS				<i>C</i>				<i>B</i>				<i>C</i>
Intersection Delay	<i>19.1</i>			$X_c = 0.80$			Intersection LOS			<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	72	27	94	24	133	46	88	675	112	65	369	30
% Heavy Vehicles, %HV	1	1	1	1	1	1	3	3	3	9	9	9
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.94	0.94	0.94	0.92	0.92	0.92	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.810			1.000		0.649	0.649			0.986	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	6
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		214			216		96	856			545	
Lane Group Capacity, c		558			574		397	1049			960	
v/c Ratio, X		0.38			0.38		0.24	0.82			0.57	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.3			22.2		10.8	16.8			13.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		1.6			1.9		0.9	4.7			2.4	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		23.9			24.1		11.7	21.5			15.9	
Lane Group LOS		C			C		B	C			B	
Approach Delay		23.9			24.1			20.5			15.9	
Approach LOS		C			C			C			B	
Intersection Delay		20.0			$X_c = 0.65$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description Charleston EIS							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					405		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	482	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			59				
Peak-Hour Factor, PHF	1.00	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	84	0	0	0	
Percent Heavy Vehicles	0	0	60	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							84
C (m) (veh/h)							674
v/c							0.12
95% queue length							0.42
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	284							793			405	
% Heavy Vehicles, %HV	2							1			3	
Peak-Hour Factor, PHF	0.90							0.91			0.84	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.665			0.986	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	316							871			482	
Lane Group Capacity, c	669							2156			1951	
v/c Ratio, X	0.47							0.40			0.25	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	23.7							11.5			10.3	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.4							0.4			0.3	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	26.1						11.8			10.6	
Lane Group LOS	C						B			B	
Approach Delay	26.1						11.8			10.6	
Approach LOS	C						B			B	
Intersection Delay	14.2		$X_c = 0.43$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					404		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	425	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				287			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	308	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			308				
C (m) (veh/h)			575				
v/c			0.54				
95% queue length			3.16				
Control Delay (s/veh)			18.2				
LOS			C				
Approach Delay (s/veh)	--	--	18.2				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	74	164	169	203	285	80	119	324	138	68	32	41
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	84	335		233	415			624		101	109	
Lane Group Capacity, c	272	579		217	1081			903		376	350	
v/c Ratio, X	0.31	0.58		1.07	0.38			0.69		0.27	0.31	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	22.3	24.8		30.0	22.9			29.0		29.0	29.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	2.3	3.3		81.4	1.0			4.3		1.7	2.3	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	24.6	28.1		111.4	23.9			33.4		30.7	31.5	
Lane Group LOS	C	C		F	C			C		C	C	
Approach Delay	27.4			55.4			33.4			31.1		
Approach LOS	C			E			C			C		
Intersection Delay	39.3			$X_c = 0.74$			Intersection LOS			D		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	38	69	28	33	84	35		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R		
Volume (veh/h)	14	16	22	21	52	16		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.80	0.80	0.78	0.78	0.86	0.86	1.00	1.00
Flow Rate (veh/h)	89	77	95	97	34	25	47	42
% Heavy Vehicles	21	21	1	1	5	5	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.5	0.0	0.4	0.0	0.5	0.0	0.4	0.0
Prop. Right-Turns	0.0	0.4	0.0	0.5	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.6	0.0	0.2	-0.3	0.3	-0.6	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.08	0.07	0.08	0.09	0.03	0.02	0.04	0.04
hd, final value (s)	5.69	5.12	5.29	4.76	5.80	4.86	5.66	5.17
x, final value	0.14	0.11	0.14	0.13	0.05	0.03	0.07	0.06
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	3.4	2.8	3.0	2.5	3.5	2.6	3.4	2.9
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	339	327	345	347	284	275	297	292
Delay (s/veh)	9.32	8.44	8.85	8.15	8.83	7.73	8.81	8.20
LOS	A	A	A	A	A	A	A	A
Approach: Delay (s/veh)	8.91		8.50		8.36		8.53	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.62							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	54	28	275	115	203	6	264	93	51	11	160	102
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.916			0.953		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		406			372		303	166			359	
Lane Group Capacity, c		1104			1058		441	794			1216	
v/c Ratio, X		0.37			0.35		0.69	0.21			0.30	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		16.6			16.5		20.0	15.3			16.0	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		0.9			0.9		8.5	0.6			0.6	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		17.5			17.3		28.4	15.9			16.6	
Lane Group LOS		B			B		C	B			B	
Approach Delay		17.5			17.3		24.0				16.6	
Approach LOS		B			B		C				B	
Intersection Delay		19.2			$X_c = 0.53$		Intersection LOS				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	802	256	19	4	284	315	4	16	6	119	5	146
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.86	0.86	0.86	0.37	0.37	0.37	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.603	0.603			0.932	0.932		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	881	302			335	366		70		137	6	168
Lane Group Capacity, c	839	1158			449	383		355		226	320	952
v/c Ratio, X	1.05	0.26			0.75	0.96		0.20		0.61	0.02	0.18
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	18.7	4.6			31.4	33.5		31.5		34.1	30.5	6.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	38.4	0.3			10.1	34.5		1.2		11.5	0.1	0.4
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	57.2	4.9			41.5	68.1		32.8		45.6	30.6	6.8
Lane Group LOS	<i>E</i>	<i>A</i>			<i>D</i>	<i>E</i>		<i>C</i>		<i>D</i>	<i>C</i>	<i>A</i>
Approach Delay	43.8			55.4			32.8			24.4		
Approach LOS	<i>D</i>			<i>E</i>			<i>C</i>			<i>C</i>		
Intersection Delay	44.4			$X_c = 1.03$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	356	26	0	14	68	38	6	22	11	15	38	499
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	391	29			153			86			70	453
Lane Group Capacity, c	699	1066			1543			1234			673	616
v/c Ratio, X	0.56	0.03			0.10			0.07			0.10	0.74
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.6	11.4			11.8			17.3			17.5	23.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	3.1	0.0			0.1			0.1			0.3	7.4
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	18.7	11.5			12.0			17.4			17.8	31.0
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	18.2			12.0			17.4			29.2		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	22.2			$X_c = 0.64$			Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	108		4				4	86			320	263
% Heavy Vehicles, %HV	11		11				2	2			1	1
Peak-Hour Factor, PHF	0.80		0.80				0.80	0.80			0.81	0.81
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.995		0.995					1.000			0.752	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	4	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	135		0					112			720	
Lane Group Capacity, c	532		461					1680			1956	
v/c Ratio, X	0.25		0.00					0.07			0.37	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.2		14.0					7.3			8.7	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.1		0.0					0.1			0.4	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	16.4		14.0					7.3			9.1	
Lane Group LOS	<i>B</i>		<i>B</i>					<i>A</i>			<i>A</i>	
Approach Delay	16.4							7.3		9.1		
Approach LOS	<i>B</i>							<i>A</i>		<i>A</i>		
Intersection Delay	9.9		$X_c = 0.32$			Intersection LOS			<i>A</i>			

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	1/23/13			Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		238						
Peak-Hour Factor, PHF	1.00	0.82	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	290	0	0	0	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	0	0	0	
Configuration		T						
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			190					
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	243	0	0	0		
Percent Heavy Vehicles	0	0	1	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	1	0	0	0	0	
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					243			
C (m) (veh/h)					752			
v/c					0.32			
95% queue length					1.40			
Control Delay (s/veh)					12.1			
LOS					B			
Approach Delay (s/veh)	--	--	12.1					
Approach LOS	--	--	B					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		214	0	384	191		3			24	200	64	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.80	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		267		486	242		4				356		
Lane Group Capacity, c		776		402	794		387				1987		
v/c Ratio, X		0.34		1.21	0.30		0.01				0.18		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		11.9		17.5	11.7		10.3				11.0		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		1.2		114.8	1.0		0.0				0.2		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		13.1		132.3	12.7		10.3				11.2	
Lane Group LOS		<i>B</i>		<i>F</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		13.1		92.6			10.3			11.2		
Approach LOS		<i>B</i>		<i>F</i>			<i>B</i>			<i>B</i>		
Intersection Delay		55.3		$X_c = 0.69$			Intersection LOS			<i>E</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/23/13				Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	57	22	183	72	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	11	83	12			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.79				0.81	0.81	
Flow Rate (veh/h)	98		322				63	65	
% Heavy Vehicles	24		5				3	3	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.2	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.2	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.2		0.2				0.1	-0.1	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.09		0.29				0.06	0.06	
hd, final value (s)	4.83		4.57				5.00	4.82	
x, final value	0.13		0.41				0.09	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.8		2.6				3.0	2.8	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	348		572				313	315	
Delay (s/veh)	8.56		10.68				8.47	8.28	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.56		10.68				8.38		
LOS	A		B				A		
Intersection Delay (s/veh)	9.76								
Intersection LOS	A								

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	1/16/13				Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	6	61	0		0	197	18	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	58	53	46		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.79		0.87	0.87		
Flow Rate (veh/h)	83		271		95	83		
% Heavy Vehicles	17		7		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.1		0.0		0.7	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.6		
Prop. Heavy Vehicle	0.2		0.1		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.3		0.1		0.2	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.07		0.24		0.08	0.07		
hd, final value (s)	4.94		4.49		4.90	4.38		
x, final value	0.11		0.34		0.13	0.10		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	2.9		2.5		2.9	2.4		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	333		521		345	333		
Delay (s/veh)	8.57		9.77		8.63	7.87		
LOS	A		A		A	A		
Approach: Delay (s/veh)	8.57		9.77		8.28			
LOS	A		A		A			
Intersection Delay (s/veh)	9.08							
Intersection LOS	A							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	79	196	153	6	73	1	501	77	1			
% Heavy Vehicles, %HV	3	3	3	1	1	1	3	3	3			
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.76	0.76	0.76			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _l		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.998			0.978				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	21	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		348	167		101			761				
Lane Group Capacity, c		585	549		548			2250				
v/c Ratio, X		0.59	0.30		0.18			0.34				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		16.0	14.2		13.5			9.6				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		4.4	1.4		0.7			0.4				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		20.4	15.6		14.3			10.0			
Lane Group LOS		C	B		B			A			
Approach Delay		18.9			14.3			10.0			
Approach LOS		B			B			A			
Intersection Delay		13.6			$X_c = 0.45$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	179		18				16	351			478	64
% Heavy Vehicles, %HV	1		1				7	7			9	9
Peak-Hour Factor, PHF	0.74		0.74				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.997						0.936			0.808	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		266						412			630	
Lane Group Capacity, c		470						1010			1093	
v/c Ratio, X		0.57						0.41			0.58	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.0						7.3			8.4	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		4.9						1.1			1.8	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		23.9					8.5			10.2	
Lane Group LOS		C					A			B	
Approach Delay		23.9					8.5			10.2	
Approach LOS		C					A			B	
Intersection Delay		12.4		$X_C = 0.57$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	56		53				30	510			325	185
% Heavy Vehicles, %HV	14		14				8	8			11	11
Peak-Hour Factor, PHF	0.80		0.80				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.982						0.866			0.927	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	17
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		136						607			573	
Lane Group Capacity, c		502						808			918	
v/c Ratio, X		0.27						0.75			0.62	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						12.0			10.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						5.6			3.0	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		16.0					17.6			13.9	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		16.0					17.6			13.9	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		15.8		$X_c = 0.56$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		166	244	467	164	90	200			475	22
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		210		994		100	222			654	
Lane Group Capacity, c	218		216		1319		120	575			661	
v/c Ratio, X	0.02		0.97		0.75		0.83	0.39			0.99	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		25.9		19.6		18.5	15.3			19.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		53.7		2.9		44.5	1.9			16.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		79.6		22.5		63.0	17.2			36.5	
Lane Group LOS	C		E		C		E	B			D	
Approach Delay	78.5		22.5		31.4		36.5					
Approach LOS	E		C		C		D					
Intersection Delay	33.5		$X_c = 0.90$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	12	39	11				5	278	98	330	523	32
% Heavy Vehicles, %HV	3	3	3				7	7	7	7	7	7
Peak-Hour Factor, PHF	0.85	0.85	0.85				0.90	0.90	0.90	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _l		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.941		0.518	0.518	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7		0	2	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		73						424		412	694	
Lane Group Capacity, c		457						902		496	960	
v/c Ratio, X		0.16						0.47		0.83	0.72	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.1						8.2		11.2	10.1	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.7						1.7		8.3	2.5	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.9						9.8		19.5	12.6	
Lane Group LOS		B						A		B	B	
Approach Delay		16.9						9.8			15.2	
Approach LOS		B						A			B	
Intersection Delay		13.8		$X_c = 0.60$			Intersection LOS			B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/16/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	8	20	13	172	16	105	10	231	131	139	339	9	
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7	
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		2		0	0	2		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =		G =		G =		G = 40.0	G =		G =		
	Y = 5	Y =		Y =		Y =		Y = 5	Y =		Y =		
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		60		195	18	119		414			641		
Lane Group Capacity, c		648		548	777	574		699			531		
v/c Ratio, X		0.09		0.36	0.02	0.21		0.59			1.21		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.5		16.5	14.0	15.3		18.9			25.0		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.3		1.7	0.1	0.8		3.1			95.0	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.8		18.2	14.1	16.1		22.0			120.0	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>	<i>B</i>		<i>C</i>			<i>F</i>	
Approach Delay		14.8		17.2				22.0			120.0	
Approach LOS		<i>B</i>		<i>B</i>				<i>C</i>			<i>F</i>	
Intersection Delay		64.0		$X_c = 0.78$				Intersection LOS			<i>E</i>	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/28/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		174	189	173	215		282		151				
% Heavy Vehicles, %HV		10	10	13	13		9		9				
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.964			0.986			0.858					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		347			436			461					
Lane Group Capacity, c		900			557			529					
v/c Ratio, X		0.39			0.78			0.87					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.9			18.5			25.4					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.2			10.4			15.6					

Initial Queue Delay, d_3		0.0		0.0		0.0			
Control Delay		15.1		28.9		41.0			
Lane Group LOS		B		C		D			
Approach Delay		15.1		28.9		41.0			
Approach LOS		B		C		D			
Intersection Delay		29.5		$X_c = 0.82$		Intersection LOS			C

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	37	253	36	32	275	23	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	43	297	42	35	302	25	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	24	17	39	27	19	25	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	30	21	50	30	21	28	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	43	35		79			101
C (m) (veh/h)	1200	1136		327			389
v/c	0.04	0.03		0.24			0.26
95% queue length	0.11	0.10		0.93			1.02
Control Delay (s/veh)	8.1	8.3		19.5			17.5
LOS	A	A		C			C
Approach Delay (s/veh)	--	--		19.5			17.5
Approach LOS	--	--		C			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		310	44	25	316		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	364	51	27	347	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				148		16	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	157	0	17	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		27		174			
C (m) (veh/h)		1077		369			
v/c		0.03		0.47			
95% queue length		0.08		2.43			
Control Delay (s/veh)		8.4		23.2			
LOS		A		C			
Approach Delay (s/veh)	--	--		23.2			
Approach LOS	--	--		C			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	255	24	391	1	175	272	233	192	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			310	434		487			495	
Lane Group Capacity, c		713			523	518		711			418	
v/c Ratio, X		0.04			0.59	0.84		0.68			1.18	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.6	15.7		14.3			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.1			4.3	13.4		5.0			104.8	
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.5			17.9	29.1		19.2			122.3	
Lane Group LOS		B			B	C		B			F	
Approach Delay		10.5			24.4			19.2			122.3	
Approach LOS		B			C			B			F	
Intersection Delay		50.4			$X_c = 1.01$			Intersection LOS			D	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				359		3		441			454	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					411			479			528	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.64			0.49			0.56	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.9			10.0			10.4	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					4.1			1.5			1.4	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>	
Control Delay				<i>21.1</i>			<i>11.5</i>			<i>11.8</i>	
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>	
Approach Delay				<i>21.1</i>				<i>11.5</i>			
Approach LOS				<i>C</i>				<i>B</i>			
Intersection Delay	<i>14.4</i>			$X_c = 0.59$			Intersection LOS			<i>B</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		441	242	141	671		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	479	263	163	780	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		163					
C (m) (veh/h)		805					
v/c		0.20					
95% queue length		0.75					
Control Delay (s/veh)		10.6					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				102		199		473	80	186	467	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					346			601			759	
Lane Group Capacity, c					385			1058			612	
v/c Ratio, X					0.90			0.57			1.24	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.2			8.3			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					25.2			1.9			115.2	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>46.4</i>			<i>10.3</i>			<i>128.2</i>		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				<i>46.4</i>				<i>10.3</i>				<i>128.2</i>
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	<i>70.1</i>			$X_c = 1.13$			Intersection LOS			<i>E</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	144	65	86	39	177	64	99	607	137	115	485	70
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		343			304		109	818			737	
Lane Group Capacity, c		424			550		308	1052			880	
v/c Ratio, X		0.81			0.55		0.35	0.78			0.84	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		26.8			23.9		11.6	16.2			17.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2		10.4			4.0		2.3	4.2			9.1	
Initial Queue Delay, d_3		0.0			0.0		0.0	0.0			0.0	
Control Delay		37.2			27.9		13.9	20.4			26.3	
Lane Group LOS		<i>D</i>			<i>C</i>		<i>B</i>	<i>C</i>			<i>C</i>	
Approach Delay		37.2			27.9			19.6			26.3	
Approach LOS		<i>D</i>			<i>C</i>			<i>B</i>			<i>C</i>	
Intersection Delay		25.4			$X_c = 0.83$			Intersection LOS			<i>C</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year			
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					578		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	649	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			91				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	108	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							108
C (m) (veh/h)							659
v/c							0.16
95% queue length							0.58
Control Delay (s/veh)							11.5
LOS							B
Approach Delay (s/veh)	--	--				11.5	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	292							814			578	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	332							895			649	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.50							0.42			0.33	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.0							11.6			10.9	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.7							0.4			0.4	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	26.7						12.0			11.4	
Lane Group LOS	C						B			B	
Approach Delay	26.7						12.0			11.4	
Approach LOS	C						B			B	
Intersection Delay	14.4			$X_C = 0.45$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					410		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	471	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				188			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	202	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			202				
C (m) (veh/h)			548				
v/c			0.37				
95% queue length			1.69				
Control Delay (s/veh)			15.4				
LOS			C				
Approach Delay (s/veh)	--	--	15.4				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	150	250	148	195	446	84	178	448	316	149	67	89
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	179	436		217	575			1013		186	195	
Lane Group Capacity, c	202	597		150	1114			924		380	353	
v/c Ratio, X	0.89	0.73		1.45	0.52			1.10		0.49	0.55	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	28.4	26.4		30.0	24.2			32.5		30.5	31.0	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	35.2	6.6		232.2	1.6			59.5		4.4	6.0	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	63.6	33.1		262.2	25.7			92.0		34.9	37.1	
Lane Group LOS	<i>E</i>	<i>C</i>		<i>F</i>	<i>C</i>			<i>F</i>		<i>C</i>	<i>D</i>	
Approach Delay	41.9			90.5				92.0		36.0		
Approach LOS	<i>D</i>			<i>F</i>				<i>F</i>		<i>D</i>		
Intersection Delay	73.0			$X_c = 1.09$				Intersection LOS		<i>E</i>		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection			
Agency/Co.	AECOM				Jurisdiction		Staten Island, NY	
Date Performed	1/24/13				Analysis Year		2020 With-Action Conditions	
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	97	128	60		130	139	90	
%Thrus Left Lane	50				50			
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	44	41	41		78	188	58	
%Thrus Left Lane					50			
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.88	0.88	0.90	0.90	0.78	0.78	1.00	1.00
Flow Rate (veh/h)	182	140	220	177	108	52	172	152
% Heavy Vehicles	42	42	4	4	3	3	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.6	0.0	0.7	0.0	0.5	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.5	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	1.0	0.4	0.4	-0.3	0.3	-0.6	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.16	0.12	0.20	0.16	0.10	0.05	0.15	0.14
hd, final value (s)	7.62	6.98	6.94	6.22	7.44	6.48	7.07	6.57
x, final value	0.39	0.27	0.42	0.31	0.22	0.09	0.34	0.28
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	5.3	4.7	4.6	3.9	5.1	4.2	4.8	4.3
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	432	390	470	427	358	302	422	402
Delay (s/veh)	15.03	12.27	14.66	11.64	12.27	9.85	13.33	11.78
LOS	C	B	B	B	B	A	B	B
Approach: Delay (s/veh)	13.83		13.31		11.48		12.60	
LOS	B		B		B		B	
Intersection Delay (s/veh)	13.02							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	62	117	509	134	223	18	281	201	88	23	528	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889		0.932	0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		819		149	268		323	332			864	
Lane Group Capacity, c		1246		206	811		196	808			1180	
v/c Ratio, X		0.66		0.72	0.33		1.65	0.41			0.73	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		19.6		20.5	16.3		25.0	17.0			20.6	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	

Incremental Delay, d_2	2.4	18.5	1.0	313.3	1.5	4.0
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	22.0	39.0	17.3	338.3	18.5	24.6
Lane Group LOS	C	D	B	F	B	C
Approach Delay	22.0	25.1	176.2	24.6		
Approach LOS	C	C	F	C		
Intersection Delay	60.0	$X_c = 1.19$	Intersection LOS	E		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	681	422	3	1	443	746	0	0	1	119	0	136
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.93	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	748	467			477	726		4		123	0	140
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.96	0.40			1.06	1.90		0.01		0.53	0.00	0.15
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	18.0	5.3			34.0	34.0		30.5		33.6	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	18.7	0.8			47.7	408.4		0.1		8.3	0.0	0.3
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	36.7	6.0			81.7	442.4		30.6		41.9	30.4	6.6
Lane Group LOS	D	A			F	F		C		D	C	A
Approach Delay	24.9			299.4			30.6			23.1		
Approach LOS	C			F			C			C		
Intersection Delay	147.7			$X_c = 1.16$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	504	38	0	2	65	71	4	3	0	65	6	1120
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	42			132		7	5			79	900
Lane Group Capacity, c	706	1056			1537		516	732			538	593
v/c Ratio, X	0.78	0.04			0.09		0.01	0.01			0.15	1.52
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	18.5	11.5			11.8		16.9	16.9			17.8	27.5
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50

Incremental Delay, d_2	8.2	0.1			0.1		0.0	0.0			0.5	240.6
Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	26.8	11.5			11.9		16.9	16.9			18.3	268.1
Lane Group LOS	C	B			B		B	B			B	F
Approach Delay	25.7		11.9		16.9		248.0					
Approach LOS	C		B		B		F					
Intersection Delay	151.2		$X_C = 1.11$		Intersection LOS		F					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	265		25				4	224			351	497
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	301		22					253			1060	
Lane Group Capacity, c	537		465					1664			1894	
v/c Ratio, X	0.56		0.05					0.15			0.56	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	17.0		14.2					7.6			9.9	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	4.1		0.2					0.2			0.8	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	21.1		14.4					7.8			10.7	
Lane Group LOS	C		B					A			B	
Approach Delay	20.7							7.8		10.7		
Approach LOS	C							A		B		
Intersection Delay	12.2		$X_c = 0.56$			Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/23/13			Analysis Year			
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		61					
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	110	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			487				
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	541	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					541		
C (m) (veh/h)					946		
v/c					0.57		
95% queue length					3.73		
Control Delay (s/veh)					13.8		
LOS					B		
Approach Delay (s/veh)	--	--	13.8				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		38	0	553	83		1			23	294	30	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		152		582	87		1				433		
Lane Group Capacity, c		784		521	810		347				2011		
v/c Ratio, X		0.19		1.12	0.11		0.00				0.22		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		11.1		17.5	10.7		10.2				11.2		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.6		75.3	0.3		0.0				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	11.7	92.8	10.9	10.2	11.5	
Lane Group LOS	<i>B</i>	<i>F</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Approach Delay	11.7	82.2	10.2	11.5		
Approach LOS	<i>B</i>	<i>F</i>	<i>B</i>	<i>B</i>		
Intersection Delay	49.2	$X_c = 0.67$	Intersection LOS	<i>D</i>		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/23/13				Analysis Year	2020 With Action Conditions			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	55	31	229	59	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	31	87	12			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	99		373				91	68	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.4	0.0	
Prop. Right-Turns	0.4		0.0				0.0	0.2	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.1	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.09		0.33				0.08	0.06	
hd, final value (s)	4.82		4.65				5.14	4.93	
x, final value	0.13		0.48				0.13	0.09	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.8		2.7				3.1	2.9	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		623				341	318	
Delay (s/veh)	8.56		11.91				8.91	8.44	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.56		11.91				8.71		
LOS	A		B				A		
Intersection Delay (s/veh)	10.57								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/16/13				Analysis Year				
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	40	46	0		0	246	23		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	42	64	88		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT		TR		
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	99		348		83	136			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.09		0.31		0.07	0.12			
hd, final value (s)	5.12		4.58		5.11	4.55			
x, final value	0.14		0.44		0.12	0.17			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.1		2.6		3.1	2.6			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	349		598		333	386			
Delay (s/veh)	8.96		11.18		8.79	8.50			
LOS	A		B		A	A			
Approach: Delay (s/veh)	8.96		11.18		8.61				
LOS	A		B		A				
Intersection Delay (s/veh)	10.00+								
Intersection LOS	B								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	149	236	163	2	54	5	582	39	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _i		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		423	122		64			642				
Lane Group Capacity, c		575	560		533			2286				
v/c Ratio, X		0.74	0.22		0.12			0.28				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		17.1	13.7		13.2			9.3				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		8.1	0.9		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		25.2	14.6		13.7			9.6			
Lane Group LOS		C	B		B			A			
Approach Delay		22.8			13.7			9.6			
Approach LOS		C			B			A			
Intersection Delay		15.6			$X_c = 0.47$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	199		40				14	294			347	47
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		295						354			433	
Lane Group Capacity, c		467						1091			1167	
v/c Ratio, X		0.63						0.32			0.37	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.4						6.9			7.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		6.2						0.8			0.9	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		25.6					7.7			8.0	
Lane Group LOS		C					A			A	
Approach Delay		25.6					7.7			8.0	
Approach LOS		C					A			A	
Intersection Delay		12.7			$X_C = 0.45$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	80		54				44	448			309	224
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		156						566			571	
Lane Group Capacity, c		554						841			1000	
v/c Ratio, X		0.28						0.67			0.57	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.7						11.3			10.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						3.9			2.3	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		16.0					15.2			12.8	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		16.0					15.2			12.8	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		14.2		$X_c = 0.52$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		156	231	477	127	114	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		203		960		128	192			429	
Lane Group Capacity, c	222		220		1340		162	593			690	
v/c Ratio, X	0.06		0.92		0.72		0.79	0.32			0.62	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		25.7		19.3		18.1	14.9			16.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		38.5		2.4		29.8	1.4			3.4	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		64.2		21.7		47.9	16.3			20.3	
Lane Group LOS	C		E		C		D	B			C	
Approach Delay	61.7		21.7		28.9		20.3					
Approach LOS	E		C		C		C					
Intersection Delay	27.1		$X_c = 0.79$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	5	15	8				6	280	116	236	477	28
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _i		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.932		0.785	0.785	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	2	
Min. Time for Pedestrians, G _p		17.5		16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		45						452		262	561	
Lane Group Capacity, c		454						926		506	1018	
v/c Ratio, X		0.10						0.49		0.52	0.55	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		15.9						8.3		8.5	8.7	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.4						1.7		3.0	1.7	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.3						10.0		11.5	10.4	
Lane Group LOS		B						B		B	B	
Approach Delay		16.3						10.0		10.7		
Approach LOS		B						B		B		
Intersection Delay		10.7			$X_c = 0.40$			Intersection LOS		B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/16/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	5	8	17	266	26	106	22	291	171	91	374	19	
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		2		0	0	2		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =	G =	G = 40.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25								Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		38		317	31	126		544			538		
Lane Group Capacity, c		628		547	763	563		693			617		
v/c Ratio, X		0.06		0.58	0.04	0.22		0.78			0.87		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.3		18.7	14.1	15.4		21.3			22.7		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.2		3.7	0.1	0.8		5.8			10.9	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.5		22.4	14.2	16.2		27.1			33.5	
Lane Group LOS		B		C	B	B		C			C	
Approach Delay		14.5		20.2				27.1			33.5	
Approach LOS		B		C				C			C	
Intersection Delay		26.9		$X_c = 0.73$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		198	189	189	226		196		174				
% Heavy Vehicles, %HV		8	8	16	16		2		2				
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.961			0.989			0.922					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		381			456			421					
Lane Group Capacity, c		921			515			559					
v/c Ratio, X		0.41			0.89			0.75					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		14.2			20.2			23.8					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.3			19.4			8.4					

Initial Queue Delay, d_3		0.0		0.0		0.0			
Control Delay		15.5		39.6		32.2			
Lane Group LOS		B		D		C			
Approach Delay		15.5		39.6		32.2			
Approach LOS		B		D		C			
Intersection Delay		29.8		$X_c = 0.83$		Intersection LOS			C

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	29	270	69	58	289	28	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	33	310	79	71	356	34	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	34	18	29	45	17	34	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	39	20	33	55	20	41	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	33	71		116			92
C (m) (veh/h)	1147	1132		269			267
v/c	0.03	0.06		0.43			0.34
95% queue length	0.09	0.20		2.05			1.48
Control Delay (s/veh)	8.2	8.4		28.2			25.4
LOS	A	A		D			D
Approach Delay (s/veh)	--	--		28.2		25.4	
Approach LOS	--	--		D		D	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		338	63	33	330		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	388	72	40	407	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				111		31	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	163	0	45	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		40		208			
C (m) (veh/h)		1075		325			
v/c		0.04		0.64			
95% queue length		0.12		4.15			
Control Delay (s/veh)		8.5		33.8			
LOS		A		D			
Approach Delay (s/veh)	--	--	33.8				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	11	2	301	20	300	4	183	328	287	262	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			391	366		554			640	
Lane Group Capacity, c		727			530	523		746			392	
v/c Ratio, X		0.02			0.74	0.70		0.74			1.63	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.7	14.4		14.8			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.1			8.3	7.1		6.1			296.1
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.4			23.1	21.5		20.9			313.6
Lane Group LOS		B			C	C		C			F
Approach Delay		10.4			22.3			20.9			313.6
Approach LOS		B			C			C			F
Intersection Delay		116.6			$X_C = 1.18$			Intersection LOS			F

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				528		4		512			565	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					634			551			657	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.95			0.54			0.69	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.5			10.3			11.4	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					11.8			1.9			1.4	

Initial Queue Delay, d_3					0.0			0.0			0.0	
Control Delay					31.3			12.1			12.8	
Lane Group LOS					C			B			B	
Approach Delay				31.3			12.1			12.8		
Approach LOS				C			B			B		
Intersection Delay	19.0			$X_c = 0.79$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		512	263	189	905		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	550	282	219	1052	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		219					
C (m) (veh/h)		736					
v/c		0.30					
95% queue length		1.25					
Control Delay (s/veh)		11.9					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				135		175		588	106	145	747	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					369			746			1038	
Lane Group Capacity, c					397			1098			673	
v/c Ratio, X					0.93			0.68			1.54	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.4			9.2			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					28.4			2.7			244.9	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>49.9</i>			<i>11.9</i>			<i>257.9</i>		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				<i>49.9</i>				<i>11.9</i>				<i>257.9</i>
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	<i>137.0</i>			$X_c = 1.35$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	97	49	126	35	196	73	82	593	110	143	564	53
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		309			375		90	773			861	
Lane Group Capacity, c		442			565		259	1067			912	
v/c Ratio, X		0.70			0.66		0.35	0.72			0.94	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.5			25.1		11.5	15.4			19.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	4.6	6.0	2.8	3.3	18.4
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	30.1	31.1	14.4	18.8	37.6
Lane Group LOS	C	C	B	B	D
Approach Delay	30.1	31.1	18.3	37.6	
Approach LOS	C	C	B	D	
Intersection Delay	28.7	$X_c = 0.85$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					688		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	790	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			71				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	91	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							91
C (m) (veh/h)							553
v/c							0.16
95% queue length							0.58
Control Delay (s/veh)							12.8
LOS							B
Approach Delay (s/veh)	--	--				12.8	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	380							762			688	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	418							847			791	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.62							0.39			0.40	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.2							11.4			11.4	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	4.2							0.4			0.6	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	29.4						11.8			12.0	
Lane Group LOS	C						B			B	
Approach Delay	29.4						11.8			12.0	
Approach LOS	C						B			B	
Intersection Delay	15.4		$X_c = 0.48$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/16/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					532		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	597	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				474			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			509				
C (m) (veh/h)			464				
v/c			1.10				
95% queue length			16.91				
Control Delay (s/veh)			100.2				
LOS			F				
Approach Delay (s/veh)	--	--	100.2				
Approach LOS	--	--	F				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	162	264	206	248	326	87	205	471	267	223	77	106
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	184	483		276	491			986		293	208	
Lane Group Capacity, c	240	591		240	1132			942		384	360	
v/c Ratio, X	0.77	0.82		1.15	0.43			1.05		0.76	0.58	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	26.9	27.5		29.5	22.7			32.5		32.8	31.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	15.1	8.6		103.2	1.2			42.3		13.0	6.4	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	42.0	36.1		132.7	23.9			74.8		45.8	37.7	
Lane Group LOS	<i>D</i>	<i>D</i>		<i>F</i>	<i>C</i>			<i>E</i>		<i>D</i>	<i>D</i>	
Approach Delay	37.7			63.0				74.8		42.4		
Approach LOS	<i>D</i>			<i>E</i>				<i>E</i>		<i>D</i>		
Intersection Delay	57.7			$X_c = 1.01$				Intersection LOS		<i>E</i>		

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	1/24/13				Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	93	161	55		148	197	86	
%Thrus Left Lane	50				50			
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	16	39	69		80	193	60	
%Thrus Left Lane					50			
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.87	0.87	0.91	0.91	0.76	0.76	1.00	1.00
Flow Rate (veh/h)	197	156	269	202	72	90	176	157
% Heavy Vehicles	7	7	1	1	1	1	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.5	0.0	0.6	0.0	0.3	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.4	0.0	0.5	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.4	-0.2	0.3	-0.3	0.2	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.18	0.14	0.24	0.18	0.06	0.08	0.16	0.14
hd, final value (s)	7.15	6.60	6.93	6.30	7.52	6.67	7.25	6.75
x, final value	0.39	0.29	0.52	0.35	0.15	0.17	0.35	0.29
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	4.9	4.3	4.6	4.0	5.2	4.4	4.9	4.5
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	447	406	507	452	322	340	426	407
Delay (s/veh)	14.38	11.93	16.82	12.40	11.54	10.70	13.88	12.25
LOS	B	B	C	B	B	B	B	B
Approach: Delay (s/veh)	13.30		14.92		11.08		13.11	
LOS	B		B		B		B	
Intersection Delay (s/veh)	13.56							
Intersection LOS	B							

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/23/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0	
Lane Group		LTR		DefL	TR		DefL	TR			LTR		
Volume, V (vph)	53	120	557	123	272	10	271	145	124	22	534	124	
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2	
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I ₁		2.0		2.0	2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0		
Arrival Type, AT		3		3	3		3	3			3		
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.884		0.927	0.927		1.000	1.000			1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0	0			0		
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =	G =	G = 40.0	G =	G =	G =	G =	G =		
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =		
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		829		150	344		291	289			723		
Lane Group Capacity, c		1250		204	824		258	804			1320		
v/c Ratio, X		0.66		0.74	0.42		1.13	0.36			0.55		
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44		
Uniform Delay, d ₁		19.7		20.6	17.1		25.0	16.5			18.4		
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50		

Incremental Delay, d_2	2.5	19.5	1.4	94.9	1.3	1.6
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	22.2	40.2	18.5	119.9	17.8	20.0
Lane Group LOS	C	D	B	F	B	B
Approach Delay	22.2	25.1	69.0	20.0		
Approach LOS	C	C	E	B		
Intersection Delay	32.5	$X_c = 0.93$	Intersection LOS	C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	750	388	4	3	398	814	2	3	2	146	0	289
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _i	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	833	435			422	857		17		182	0	361
Lane Group Capacity, c	780	1206			584	497		148		204	283	862
v/c Ratio, X	1.07	0.36			0.72	1.72		0.11		0.89	0.00	0.42
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	19.6	4.4			27.5	31.0		32.7		37.3	32.1	10.6
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

Incremental Delay, d_2	46.8	0.6			5.0	331.5		1.6		40.0	0.0	1.5
Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	66.5	4.9			32.6	362.5		34.2		77.3	32.1	12.1
Lane Group LOS	<i>E</i>	<i>A</i>			<i>C</i>	<i>F</i>		<i>C</i>		<i>E</i>	<i>C</i>	<i>B</i>
Approach Delay	45.4			253.6			34.2			33.9		
Approach LOS	<i>D</i>			<i>F</i>			<i>C</i>			<i>C</i>		
Intersection Delay	129.0			$X_c = 1.25$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	499	37	0	1	37	43	3	3	2	62	1	1176
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	41			75			16			67	938
Lane Group Capacity, c	753	1066			1563			1220			548	616
v/c Ratio, X	0.74	0.04			0.05			0.01			0.12	1.52
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	17.8	11.5			11.5			16.9			17.6	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	6.0	0.1			0.1			0.0			0.4	242.9
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	23.8	11.5			11.6			16.9			18.1	270.4
Lane Group LOS	C	B			B			B			B	F
Approach Delay	23.0			11.6			16.9			253.6		
Approach LOS	C			B			B			F		
Intersection Delay	159.5			$X_c = 1.08$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	287		24				2	263			381	428
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l _i	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	330		21					301			889	
Lane Group Capacity, c	547		473					1731			1912	
v/c Ratio, X	0.60		0.04					0.17			0.46	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	17.3		14.2					7.7			9.2	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	4.7		0.2					0.2			0.6	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	22.1		14.4					7.9			9.9	
Lane Group LOS	C		B					A			A	
Approach Delay	21.6							7.9		9.9		
Approach LOS	C							A		A		
Intersection Delay	12.2		$X_c = 0.52$			Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/23/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		94					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	132	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			545				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	619	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					619		
C (m) (veh/h)					920		
v/c					0.67		
95% queue length					5.40		
Control Delay (s/veh)					16.6		
LOS					C		
Approach Delay (s/veh)	--	--	16.6				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		71	0	475	76		4			23	334	38	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.50	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		142		534	85		5				434		
Lane Group Capacity, c		784		521	794		346				2028		
v/c Ratio, X		0.18		1.02	0.11		0.01				0.21		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		11.0		17.5	10.7		10.3				11.2		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.5		45.6	0.3		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	11.5	63.1	11.0	10.3	11.4	
Lane Group LOS	B	E	B	B	B	
Approach Delay	11.5	55.9	10.3	11.4		
Approach LOS	B	E	B	B		
Intersection Delay	34.4	$X_c = 0.62$	Intersection LOS	C		

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/23/13				Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	106	39	255	84	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	24	101	13			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.78				0.91	0.91	
Flow Rate (veh/h)	180		433				80	70	
% Heavy Vehicles	2		3				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.3	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.2	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.1		0.2				0.1	-0.1	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.16		0.38				0.07	0.06	
hd, final value (s)	4.70		4.72				5.44	5.25	
x, final value	0.24		0.57				0.12	0.10	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.7				3.4	3.3	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	430		683				330	320	
Delay (s/veh)	9.14		13.74				9.19	8.85	
LOS	A		B				A	A	
Approach: Delay (s/veh)	9.14		13.74				9.03		
LOS	A		B				A		
Intersection Delay (s/veh)	11.73								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	1/16/13				Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	39	91	0		0	274	19		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	66	100	142		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT	TR			
PHF	0.80		0.78		0.99	0.99			
Flow Rate (veh/h)	161		375		116	193			
% Heavy Vehicles	2		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.3		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.1		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.14		0.33		0.10	0.17			
hd, final value (s)	5.25		4.92		5.39	4.83			
x, final value	0.24		0.51		0.17	0.26			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.3		2.9		3.4	2.8			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	411		625		366	443			
Delay (s/veh)	9.86		12.99		9.52	9.51			
LOS	A		B		A	A			
Approach: Delay (s/veh)	9.86		12.99		9.51				
LOS	A		B		A				
Intersection Delay (s/veh)	11.12								
Intersection LOS	B								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	193	270	176	8	66	9	486	106	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _l		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only		06	07	08			
Timing	G = 21.0	G =	G =	G =	G = 29.0		G =	G =	G =			
	Y = 5	Y =	Y =	Y =	Y = 5		Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		545	145		95			667				
Lane Group Capacity, c		559	560		528			2299				
v/c Ratio, X		0.97	0.26		0.18			0.29				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.2	13.9		13.5			9.3				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		32.4	1.1		0.7			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		51.6	15.1		14.3			9.6			
Lane Group LOS		D	B		B			A			
Approach Delay		43.9			14.3			9.6			
Approach LOS		D			B			A			
Intersection Delay		26.2			$X_c = 0.58$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	238		34				18	469			472	64
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		283						566			583	
Lane Group Capacity, c		459						1098			1122	
v/c Ratio, X		0.62						0.52			0.52	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.3						8.0			8.0	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		6.0						1.5			1.5	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		25.3					9.5			9.5	
Lane Group LOS		C					A			A	
Approach Delay	25.3						9.5		9.5		
Approach LOS	C						A		A		
Intersection Delay	12.6		$X_C = 0.55$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	147		86				23	595			425	274
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		291						719			737	
Lane Group Capacity, c		573						890			965	
v/c Ratio, X		0.51						0.81			0.76	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.1						12.6			12.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.9						6.6			5.1	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		19.0					19.2			17.2	
Lane Group LOS		B					B			B	
Approach Delay		19.0					19.2			17.2	
Approach LOS		B					B			B	
Intersection Delay		18.3			$X_c = 0.69$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		180	284	485	276	104	207			501	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		189		1188		113	225			582	
Lane Group Capacity, c	222		220		1330		124	616			670	
v/c Ratio, X	0.02		0.86		0.89		0.91	0.37			0.87	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		25.4		20.6		19.2	15.2			18.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		29.0		5.1		57.1	1.6			12.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		54.5		25.7		76.2	16.7			31.4	
Lane Group LOS	C		D		C		E	B			C	
Approach Delay	53.8		25.7		36.6		31.4					
Approach LOS	D		C		D		C					
Intersection Delay	31.1		$X_c = 0.89$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	10	20	6				9	301	127	333	580	52
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _l		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.922		0.564	0.564	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2		0	7	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		58						475		358	667	
Lane Group Capacity, c		460						956		487	986	
v/c Ratio, X		0.13						0.50		0.74	0.68	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.0						8.4		10.2	9.7	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.6						1.7		5.5	2.1	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.5						10.1		15.7	11.8	
Lane Group LOS		B						B		B	B	
Approach Delay		16.5						10.1		13.2		
Approach LOS		B						B		B		
Intersection Delay		12.3		$X_c = 0.53$			Intersection LOS		B			

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/16/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0	
Lane Group		LTR		L	T	R		LTR			LTR		
Volume, V (vph)	6	13	20	267	29	110	18	320	175	94	456	10	
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2	
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0		
Arrival Type, AT		3		3	3	3		3			3		
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0		
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229		
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0	
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N	
Parking Maneuvers, N _m			5			5							
Buses Stopping, N _b		2		0	0	2		0			0		
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =	G =	G =	G =	G = 40.0	G =	G =	G =	G =	G =	G =	
	Y = 5	Y =	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		57		307	33	126		558			643		
Lane Group Capacity, c		634		560	792	585		733			620		
v/c Ratio, X		0.09		0.55	0.04	0.22		0.76			1.04		
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44		
Uniform Delay, d ₁		14.5		18.4	14.2	15.4		21.0			25.0		
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50		

Incremental Delay, d_2		0.3		3.5	0.1	0.8		5.0			27.8	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.7		21.8	14.2	16.1		26.0			52.8	
Lane Group LOS		B		C	B	B		C			D	
Approach Delay		14.7		19.8				26.0			52.8	
Approach LOS		B		B				C			D	
Intersection Delay		33.9		$X_c = 0.79$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		203	253	208	224		158		200				
% Heavy Vehicles, %HV		5	5	8	8		1		1				
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.989			0.917					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		485			521			535					
Lane Group Capacity, c		939			470			560					
v/c Ratio, X		0.52			1.11			0.96					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		15.2			22.5			26.7					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.9			74.3			26.9					

Initial Queue Delay, d_3		0.0		0.0		0.0				
Control Delay		17.0		96.8		53.7				
Lane Group LOS		<i>B</i>		<i>F</i>		<i>D</i>				
Approach Delay	17.0		96.8		53.7					
Approach LOS	<i>B</i>		<i>F</i>		<i>D</i>					
Intersection Delay	56.7		$X_c = 1.04$		Intersection LOS		<i>E</i>			

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	12	282	50	29	328	28	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	13	306	54	32	372	31	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	27	33	24	52	17	41	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	67	82	59	66	21	52	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	13	32		139			208
C (m) (veh/h)	1150	1182		278			320
v/c	0.01	0.03		0.50			0.65
95% queue length	0.03	0.08		2.61			4.27
Control Delay (s/veh)	8.2	8.1		30.2			34.9
LOS	A	A		D			D
Approach Delay (s/veh)	--	--		30.2			34.9
Approach LOS	--	--		D			D

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		321	55	27	377		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	348	59	30	428	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				119		22	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	158	0	29	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		30		187			
C (m) (veh/h)		1136		341			
v/c		0.03		0.55			
95% queue length		0.08		3.13			
Control Delay (s/veh)		8.3		27.7			
LOS		A		D			
Approach Delay (s/veh)	--	--	27.7				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	10	5	356	27	434	5	216	400	296	229	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			416	472		682			577	
Lane Group Capacity, c		726			547	539		748			324	
v/c Ratio, X		0.02			0.76	0.88		0.91			1.78	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.9	16.1		16.5			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

Incremental Delay, d_2		0.1			8.2	15.6		15.4			363.6
Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0
Control Delay		10.4			23.2	31.7		31.9			381.1
Lane Group LOS		B			C	C		C			F
Approach Delay		10.4			27.7			31.9			381.1
Approach LOS		B			C			C			F
Intersection Delay		123.2			$X_c = 1.33$			Intersection LOS			F

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				510		9		613			592	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					603			674			651	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.89			0.66			0.63	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.0			11.2			11.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					9.0			2.9			1.1	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>27.9</i>			<i>14.0</i>			<i>12.1</i>		
Lane Group LOS				<i>C</i>			<i>B</i>			<i>B</i>		
Approach Delay				<i>27.9</i>				<i>14.0</i>				<i>12.1</i>
Approach LOS				<i>C</i>				<i>B</i>				<i>B</i>
Intersection Delay	<i>17.7</i>			$X_c = 0.75$			Intersection LOS			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	3/31/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		613	270	181	919		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	673	296	198	1009	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT					
v (veh/h)		198					
C (m) (veh/h)		695					
v/c		0.28					
95% queue length		1.17					
Control Delay (s/veh)		12.2					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	3/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				108		222		645	94	157	735	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, l ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					370			812			981	
Lane Group Capacity, c					393			1132			643	
v/c Ratio, X					0.94			0.72			1.53	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.5			9.5			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					31.5			3.1			238.7	

Initial Queue Delay, d_3				<i>0.0</i>			<i>0.0</i>			<i>0.0</i>		
Control Delay				<i>53.1</i>			<i>12.6</i>			<i>251.7</i>		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				<i>53.1</i>				<i>12.6</i>				<i>251.7</i>
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	<i>128.0</i>			$X_c = 1.34$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	103	49	116	31	92	73	163	783	144	58	622	101
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		344			268		177	1008			827	
Lane Group Capacity, c		488			538		272	1067			959	
v/c Ratio, X		0.70			0.50		0.65	0.94			0.86	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.5			23.3		14.5	19.2			17.6	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	

Incremental Delay, d_2	4.5	3.3	6.6	11.2	9.7
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	30.1	26.6	21.1	30.4	27.3
Lane Group LOS	C	C	C	C	C
Approach Delay	30.1	26.6	29.0	27.3	
Approach LOS	C	C	C	C	
Intersection Delay	28.4	$X_c = 0.85$	Intersection LOS	C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/17/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description Charleston EIS							
East/West Street: South Bridge Street Right Turn				North/South Street: Page Avenue			
Intersection Orientation: North-South				Study Period (hrs): 0.25			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					723		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	785	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			57				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	64	0	0	0	
Percent Heavy Vehicles	0	0	5	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							64
C (m) (veh/h)							650
v/c							0.10
95% queue length							0.33
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	394							959			723	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, l ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	458							999			786	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.68							0.47			0.40	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.9							12.0			11.5	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	5.6							0.4			0.6	

Initial Queue Delay, d_3	0.0						0.0			0.0	
Control Delay	31.5						12.4			12.0	
Lane Group LOS	C						B			B	
Approach Delay	31.5						12.4			12.0	
Approach LOS	C						B			B	
Intersection Delay	16.2		$X_c = 0.55$		Intersection LOS		B				

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/17/13633			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					633		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	680	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				236			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	262	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			262				
C (m) (veh/h)			418				
v/c			0.63				
95% queue length			4.15				
Control Delay (s/veh)			27.0				
LOS			D				
Approach Delay (s/veh)	--	--	27.0				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD					Analysis Year	2020 With-Action Conditions					
	Peak					Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	176	318	221	273	443	102	266	705	293	243	140	160
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	189	528		284	685			1405		289	357	
Lane Group Capacity, c	160	594		97	1179			966		384	359	
v/c Ratio, X	1.18	0.89		2.93	0.58			1.45		0.75	0.99	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	30.0	28.4		29.5	24.2			32.5		32.7	34.9	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	117.3	13.4		892.3	1.9			210.3		11.1	42.6	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	147.3	41.8		921.8	26.1			242.8		43.8	77.5	
Lane Group LOS	F	D		F	C			F		D	E	
Approach Delay	69.6			288.6			242.8			62.4		
Approach LOS	E			F			F			E		
Intersection Delay	190.3			$X_c = 1.93$			Intersection LOS			F		

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	10/24/2012			Analysis Year	2020 With-Action Conditions			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R	L	R
Volume (veh/h)	138	265	59	111	258	128		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R	L	R
Volume (veh/h)	61	59	95	111	269	83		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.85	0.85	0.91	0.91	0.89	0.89	0.92	0.92
Flow Rate (veh/h)	317	225	262	281	134	106	265	236
% Heavy Vehicles	0	0	0	1	1	1	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.5	0.0	0.5	0.0	0.5	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.3	0.0	0.5	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.3	-0.2	0.2	-0.3	0.3	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.28	0.20	0.23	0.25	0.12	0.09	0.24	0.21
hd, final value (s)	8.21	7.74	8.20	7.64	8.92	7.97	8.33	7.84
x, final value	0.72	0.48	0.60	0.60	0.33	0.23	0.61	0.51
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	5.9	5.4	5.9	5.3	6.6	5.7	6.0	5.5
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	433	456	432	463	384	356	424	449
Delay (s/veh)	29.53	17.47	22.33	21.00	16.00	13.09	23.37	18.53
LOS	D	C	C	C	C	B	C	C
Approach: Delay (s/veh)	24.52		21.64		14.71		21.09	
LOS	C		C		B		C	
Intersection Delay (s/veh)	21.44							
Intersection LOS	C							

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/23/13					Jurisdiction	SFR NB-DefL = 2050						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	2	0	
Lane Group		LTR		DefL	TR		DefL	TR			LTR		
Volume, V (vph)	82	139	593	165	339	25	322	269	115	26	834	168	
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0		
Arrival Type, AT		3		3	3		3	3			3		
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.830		0.865	0.865		1.000	1.000			1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0	0			0		
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8			
Phasing	EW Perm	02		03		04		NS Perm	06		07		08
Timing	G = 40.0	G =		G =		G =		G = 40.0	G =		G =		G =
	Y = 5	Y =		Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0							
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		885		212	467		343	422			1106		
Lane Group Capacity, c		1182		186	828		128	825			1346		
v/c Ratio, X		0.75		1.14	0.56		2.68	0.51			0.82		
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44		
Uniform Delay, d ₁		20.8		25.0	18.5		25.0	18.0			21.9		
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50		

Incremental Delay, d_2	3.6	104.1	2.4	777.7	2.3	5.8
Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	24.5	129.1	20.9	802.7	20.2	27.6
Lane Group LOS	C	F	C	F	C	C
Approach Delay	24.5	54.7	371.1	27.6		
Approach LOS	C	D	F	C		
Intersection Delay	108.7	$X_c = 1.92$	Intersection LOS	F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	881	472			508	1077				233		216
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	908	492			540	988				253		235
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	1.01	0.42			1.17	2.53				0.44		0.24
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	21.4	5.4			34.0	34.0				33.0		6.8
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	25.1	0.6			82.7	688.4				2.4		0.6

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0				0.0		0.0
Control Delay	46.5	5.9			116.7	722.4				35.4		7.4
Lane Group LOS	D	A			F	F				D		A
Approach Delay	32.2			508.3						21.9		
Approach LOS	C			F						C		
Intersection Delay	243.7			$X_C = 1.32$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	660	46	0	0	46	58	0	0	1	84	0	1539
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	688	48			99			1			90	1227
Lane Group Capacity, c	736	1066			1633			1184			544	622
v/c Ratio, X	0.93	0.05			0.06			0.00			0.17	1.97
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	21.1	11.5			11.6			16.8			18.0	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

Incremental Delay, d_2	20.1	0.1			0.1			0.0			0.6	442.7
Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	41.2	11.6			11.7			16.8			18.5	470.2
Lane Group LOS	<i>D</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>F</i>
Approach Delay	39.3			11.7			16.8			439.3		
Approach LOS	<i>D</i>			<i>B</i>			<i>B</i>			<i>F</i>		
Intersection Delay	282.7			$X_c = 1.39$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/23/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	436		36				5	275			494	684
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	513		32					319			1324	
Lane Group Capacity, c	573		497					1695			1915	
v/c Ratio, X	0.90		0.06					0.19			0.69	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	19.6		14.3					7.8			10.9	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	17.5		0.2					0.2			1.0	

Initial Queue Delay, d_3	0.0		0.0					0.0			0.0	
Control Delay	37.0		14.5					8.0			11.9	
Lane Group LOS	<i>D</i>		<i>B</i>					<i>A</i>			<i>B</i>	
Approach Delay	35.7							8.0		11.9		
Approach LOS	<i>D</i>							<i>A</i>		<i>B</i>		
Intersection Delay	17.3		$X_c = 0.77$			Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	1/23/13			Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		93					
Peak-Hour Factor, PHF	1.00	0.75	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	124	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			706				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	802	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					802		
C (m) (veh/h)					932		
v/c					0.86		
95% queue length					11.03		
Control Delay (s/veh)					27.3		
LOS					D		
Approach Delay (s/veh)	--	--	27.3				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N _l		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		65	0	728	89		5			28	451	37	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.75	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, I _l		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		87		800	98		6				580		
Lane Group Capacity, c		784		562	802		283				2016		
v/c Ratio, X		0.11		1.42	0.12		0.02				0.29		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.7		17.5	10.8		10.3				11.6		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.3		200.5	0.3		0.1				0.4		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	11.0	218.0	11.1	10.4	12.0	
Lane Group LOS	<i>B</i>	<i>F</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Approach Delay	11.0	195.4	10.4	12.0		
Approach LOS	<i>B</i>	<i>F</i>	<i>B</i>	<i>B</i>		
Intersection Delay	116.8	$X_c = 0.86$	Intersection LOS	<i>F</i>		

ALL-WAY STOP CONTROL ANALYSIS										
General Information					Site Information					
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W				
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY				
Date Performed	1/28/13				Analysis Year	2020 With-Action Conditions				
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd									
Project ID <i>Charleston EIS</i>										
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>					
Volume Adjustments and Site Characteristics										
Approach	Eastbound					Westbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	77	36	350	60	0				
%Thrus Left Lane										
Approach	Northbound					Southbound				
Movement	L	T	R	L	T	R				
Volume (veh/h)	0	0	0	17	130	13				
%Thrus Left Lane				50						
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Configuration	TR		LT				LT	TR		
PHF	0.75		0.85				0.89	0.89		
Flow Rate (veh/h)	150		481				92	87		
% Heavy Vehicles	2		4				0	0		
No. Lanes	1		1		0		2			
Geometry Group	2		2				1			
Duration, T	0.25									
Saturation Headway Adjustment Worksheet										
Prop. Left-Turns	0.0		0.9				0.2	0.0		
Prop. Right-Turns	0.3		0.0				0.0	0.2		
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7		
hadj, computed	-0.2		0.2				0.0	-0.1		
Departure Headway and Service Time										
hd, initial value (s)	3.20		3.20				3.20	3.20		
x, initial	0.13		0.43				0.08	0.08		
hd, final value (s)	4.83		4.81				5.48	5.34		
x, final value	0.20		0.64				0.14	0.13		
Move-up time, m (s)	2.0		2.0				2.0			
Service Time, t _s (s)	2.8		2.8				3.5	3.3		
Capacity and Level of Service										
	Eastbound		Westbound		Northbound		Southbound			
	L1	L2	L1	L2	L1	L2	L1	L2		
Capacity (veh/h)	400		731				342	337		
Delay (s/veh)	9.04		16.03				9.37	9.13		
LOS	A		C				A	A		
Approach: Delay (s/veh)	9.04		16.03				9.26			
LOS	A		C				A			
Intersection Delay (s/veh)	13.24									
Intersection LOS	B									

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	1/17/13				Analysis Year	2020 With-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	26	69	0		0	356	24	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	53	107	139		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT		TR	
PHF	0.75		0.85		0.95	0.95		
Flow Rate (veh/h)	126		446		110	202		
% Heavy Vehicles	2		2		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.3		0.0		0.5	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		-0.0		0.1	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.11		0.40		0.10	0.18		
hd, final value (s)	5.36		4.84		5.46	4.92		
x, final value	0.19		0.60		0.17	0.28		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	3.4		2.8		3.5	2.9		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	376		696		360	452		
Delay (s/veh)	9.59		14.84		9.55	9.79		
LOS	A		B		A	A		
Approach: Delay (s/veh)	9.59		14.84		9.70			
LOS	A		B		A			
Intersection Delay (s/veh)	12.28							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	218	367	213	0	88	15	730	66	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, I _i		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		780	196		124			859				
Lane Group Capacity, c		549	560		561			2266				
v/c Ratio, X		1.42	0.35		0.22			0.38				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.5	14.4		13.7			9.8				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		199.8	1.7		0.9			0.5				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		219.3	16.2		14.6			10.3			
Lane Group LOS		F	B		B			B			
Approach Delay		178.5			14.6			10.3			
Approach LOS		F			B			B			
Intersection Delay		94.4			$X_c = 0.82$			Intersection LOS			F

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	314		56				17	404			384	85
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		402						457			515	
Lane Group Capacity, c		467						1110			1179	
v/c Ratio, X		0.86						0.41			0.44	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		20.9						7.3			7.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		17.5						1.1			1.1	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		38.4					8.4			8.6	
Lane Group LOS		D					A			A	
Approach Delay		38.4					8.4			8.6	
Approach LOS		D					A			A	
Intersection Delay		17.3		$X_c = 0.57$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	142		67				43	611			412	337
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, l ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		278						711			801	
Lane Group Capacity, c		575						781			1004	
v/c Ratio, X		0.48						0.91			0.80	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		15.9						13.8			12.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		2.7						14.6			6.0	

Initial Queue Delay, d_3		0.0					0.0			0.0	
Control Delay		18.6					28.4			18.5	
Lane Group LOS		B					C			B	
Approach Delay		18.6					28.4			18.5	
Approach LOS		B					C			B	
Intersection Delay		22.4			$X_c = 0.74$		Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		194	302	628	173	153	229			422	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		273		1268		166	249			488	
Lane Group Capacity, c	222		220		1339		138	621			707	
v/c Ratio, X	0.12		1.24		0.95		1.20	0.40			0.69	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		26.0		21.1		20.0	15.4			17.3	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.8		133.0		8.5		138.2	1.8			4.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.7		159.0		29.6		158.2	17.2			21.4	
Lane Group LOS	C		F		C		F	B			C	
Approach Delay	146.9		29.6		73.6		21.4					
Approach LOS	F		C		E		C					
Intersection Delay	49.6		$X_C = 1.11$		Intersection LOS		D					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/17/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _i	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	31	33	5				3	350	153	327	542	47
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, I _i		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.899		0.644	0.644	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0		0	0	
Min. Time for Pedestrians, G _p	17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		91						549		363	642	
Lane Group Capacity, c		462						972		445	1025	
v/c Ratio, X		0.20						0.56		0.82	0.63	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.3						8.8		11.0	9.3	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		1.0						2.1		10.3	1.9	

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.3					11.0		21.3	11.1	
Lane Group LOS		B					B		C	B	
Approach Delay		17.3					11.0		14.8		
Approach LOS		B					B		B		
Intersection Delay		13.7		$X_c = 0.61$		Intersection LOS		B			

HCS+™ DETAILED REPORT														
General Information						Site Information								
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave							
Agency or Co.	AECOM					Area Type	All other areas							
Date Performed	1/17/13					Jurisdiction	Staten Island, NY							
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions							
						Project ID	Charleston EIS							
Volume and Timing Input														
	EB			WB			NB			SB				
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0		
Lane Group		LTR		L	T	R		LTR			LTR			
Volume, V (vph)	8	8	18	294	18	106	13	350	242	86	447	8		
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1		
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90		
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P		
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0			
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0			
Arrival Type, AT		3		3	3	3		3			3			
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0			
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500			
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0			
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0		
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0			
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N		
Parking Maneuvers, N _m			5			5								
Buses Stopping, N _b		0		0	0	0		0			0			
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3				
Phasing	EW Perm	02		03		04		NS Perm	06		07		08	
Timing	G = 40.0	G =		G =		G =		G = 40.0	G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5	Y =		Y =		Y =	
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination														
	EB			WB			NB			SB				
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Adjusted Flow Rate, v		41		327	20	118		657			602			
Lane Group Capacity, c		631		578	808	601		743			605			
v/c Ratio, X		0.06		0.57	0.02	0.20		0.88			1.00			
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44			
Uniform Delay, d ₁		14.3		18.6	14.0	15.2		22.9			24.9			
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000			
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50			

Incremental Delay, d_2		0.2		3.5	0.1	0.6		8.2			24.7	
Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		14.5		22.1	14.1	15.9		31.1			49.6	
Lane Group LOS		B		C	B	B		C			D	
Approach Delay		14.5		20.2				31.1			49.6	
Approach LOS		B		C				C			D	
Intersection Delay		34.2		$X_c = 0.78$				Intersection LOS			C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	3/31/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		229	240	283	234		160		233				
% Heavy Vehicles, %HV		2	2	4	4		1		1				
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, l ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.992			0.953					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		2			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		530			581			462					
Lane Group Capacity, c		984			435			558					
v/c Ratio, X		0.54			1.34			0.83					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		15.4			22.5			24.8					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		2.0			165.9			12.7					

Initial Queue Delay, d_3		0.0			0.0			0.0			
Control Delay		17.4			188.4			37.5			
Lane Group LOS		<i>B</i>			<i>F</i>			<i>D</i>			
Approach Delay		17.4			188.4			37.5			
Approach LOS		<i>B</i>			<i>F</i>			<i>D</i>			
Intersection Delay		86.4			$X_C = 1.11$			Intersection LOS			<i>F</i>

Mitigation

2015

Weekday

AM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	04/02/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				58		133		494	72	88	318	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.980			0.769		0.842	0.842	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					205			666		101	366	
Lane Group Capacity, c					384			1032		293	911	
v/c Ratio, X					0.53			0.65		0.34	0.40	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					18.8			8.9		7.0	7.3	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					5.1			2.4		2.7	1.1	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				23.9			11.3		9.7	8.4	
Lane Group LOS				C			B		A	A	
Approach Delay				23.9				11.3			
Approach LOS				C				B			
Intersection Delay	12.3			$X_c = 0.61$				Intersection LOS			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	7	20	12	166	16	101	9	176	127	134	297	8
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		57		189	18	115		347			578	
Lane Group Capacity, c		635		534	758	559		710			575	
v/c Ratio, X		0.09		0.35	0.02	0.21		0.49			1.01	
Total Green Ratio, g/C		0.43		0.43	0.43	0.43		0.46			0.46	
Uniform Delay, d ₁		15.0		17.1	14.6	15.9		17.2			24.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		1.8	0.1	0.8		2.0			12.5	

Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		15.3		18.8	14.7	16.7		19.2			37.0	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>	<i>B</i>		<i>B</i>			<i>D</i>	
Approach Delay		15.3		17.8				19.2			37.0	
Approach LOS		<i>B</i>		<i>B</i>				<i>B</i>			<i>D</i>	
Intersection Delay		26.6		$X_c = 0.69$				Intersection LOS			<i>C</i>	

Weekday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				97		178		436	77	169	433	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.944			0.880		0.500	0.500	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					316			558		197	503	
Lane Group Capacity, c					386			1057		354	933	
v/c Ratio, X					0.82			0.53		0.56	0.54	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					20.6			8.0		8.2	8.1	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					16.5			1.7		3.1	1.1	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				37.2			9.7		11.4	9.2	
Lane Group LOS				D			A		B	A	
Approach Delay				37.2				9.7			
Approach LOS				D				A			
Intersection Delay	15.3		$X_c = 0.64$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	147	144	135	187	462	55	86	337	305	48	58	88
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 32.0	G =	G =	G =	G = 25.0	G = 18.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	175	294		208	560			782		60	182	
Lane Group Capacity, c	230	627		277	1198			915		342	316	
v/c Ratio, X	0.76	0.47		0.75	0.47			0.85		0.18	0.58	
Total Green Ratio, g/C	0.36	0.36		0.36	0.36			0.28		0.20	0.20	
Uniform Delay, d ₁	25.6	22.4		25.5	22.4			30.8		29.8	32.5	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	18.2	2.2		15.9	1.2			10.0		1.1	7.3	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	43.8	24.6		41.4	23.6			40.8		30.9	39.9	
Lane Group LOS	D	C		D	C			D		C	D	
Approach Delay	31.8			28.4				40.8		37.7		
Approach LOS	C			C				D		D		
Intersection Delay	34.4			$X_c = 0.75$				Intersection LOS		C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	60	101	308	130	266	18	227	196	86	22	518	219
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889			0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	NB Only	07	08				
Timing	G = 36.0	G =	G =	G =	G = 34.0	G = 5.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		558			460		261	324			843	
Lane Group Capacity, c		1057			871		348	889			1004	
v/c Ratio, X		0.53			0.53		0.75	0.36			0.84	
Total Green Ratio, g/C		0.40			0.40		0.49	0.49			0.38	
Uniform Delay, d ₁		20.5			20.5		30.5	14.3			25.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.7			2.1		13.8	1.2			8.4	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	22.2	22.7	44.3	15.5
Lane Group LOS	C	C	D	B
Approach Delay	22.2	22.7	28.3	33.9
Approach LOS	C	C	C	C
Intersection Delay	27.8	$X_c = 0.92$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	658	383	3	1	405	569	0	0	1	99	0	130
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	WB Only	04	NS Perm	06	07	08				
Timing	G = 35.0	G = 14.0	G = 11.0	G =	G = 14.0	G =	G =	G =				
	Y = 3	Y = 3	Y = 5	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	723	424			446	532		4		102	0	134
Lane Group Capacity, c	745	941			574	488		287		204	280	887
v/c Ratio, X	0.97	0.45			0.78	1.09		0.01		0.50	0.00	0.15
Total Green Ratio, g/C	0.58	0.58			0.31	0.31		0.16		0.16	0.16	0.60
Uniform Delay, d ₁	20.6	10.8			28.2	31.0		32.2		34.8	32.1	7.9
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	22.0	1.1			5.6	57.7		0.1		8.5	0.0	0.4

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	42.6	12.0			33.8	88.7		32.2		43.3	32.1	8.3
Lane Group LOS	D	B			C	F		C		D	C	A
Approach Delay	31.3			63.7			32.2			23.4		
Approach LOS	C			E			C			C		
Intersection Delay	43.9			$X_c = 0.94$			Intersection LOS			D		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2015 MitiAction Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	444	36	0	2	62	69	4	3	0	62	6	908
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 38.0	G =	G =	G =	G = 42.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	488	40			127		7	5			76	664
Lane Group Capacity, c	599	891			1297		621	878			654	711
v/c Ratio, X	0.81	0.04			0.10		0.01	0.01			0.12	0.93
Total Green Ratio, g/C	0.42	0.42			0.42		0.47	0.47			0.47	0.47
Uniform Delay, d ₁	22.9	15.3			15.7		12.9	12.8			13.5	22.7
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	11.2	0.1			0.2		0.0	0.0			0.3	19.3

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	34.1	15.4			15.8		12.9	12.8			13.9	42.0
Lane Group LOS	C	B			B		B	B			B	D
Approach Delay	32.7				15.8		12.9			39.1		
Approach LOS	C				B		B			D		
Intersection Delay	34.4				$X_c = 0.88$		Intersection LOS			C		

Weekday

PM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	10	2	259	20	261	4	136	301	202	236	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 23.0	G =	G =	G =	G = 27.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		14			340	318		474			510	
Lane Group Capacity, c		666			490	481		801			521	
v/c Ratio, X		0.02			0.69	0.66		0.59			0.98	
Total Green Ratio, g/C		0.38			0.38	0.38		0.45			0.45	
Uniform Delay, d ₁		11.5			15.5	15.3		12.4			16.2	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			7.4	6.5		2.9			34.5	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		11.6			22.9	21.8		15.3			50.8	
Lane Group LOS		B			C	C		B			D	
Approach Delay		11.6			22.4			15.3			50.8	
Approach LOS		B			C			B			D	
Intersection Delay		29.0			$X_c = 0.85$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				130		142		536	101	118	692	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.901			0.798		0.114	0.114	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					324			685		137	805	
Lane Group Capacity, c					398			1096		293	941	
v/c Ratio, X					0.81			0.63		0.47	0.86	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					20.6			8.7		7.7	10.9	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					15.1			2.2		0.6	1.3	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				35.7			10.9		8.3	12.2	
Lane Group LOS				<i>D</i>			<i>B</i>		<i>A</i>	<i>B</i>	
Approach Delay				35.7	10.9			11.6			
Approach LOS				<i>D</i>	<i>B</i>			<i>B</i>			
Intersection Delay	15.4		$X_c = 0.84$		Intersection LOS			<i>B</i>			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	51	114	275	120	284	9	203	142	121	21	523	120
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.884			0.927		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		501			503		218	283			706	
Lane Group Capacity, c		1167			1012		276	825			1354	
v/c Ratio, X		0.43			0.50		0.79	0.34			0.52	
Total Green Ratio, g/C		0.43			0.43		0.46	0.46			0.46	
Uniform Delay, d ₁		17.8			18.4		20.8	15.8			17.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		1.0			1.6		20.2	1.1			1.4	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	18.8	20.0	41.0	16.9
Lane Group LOS	B	C	D	B
Approach Delay	18.8	20.0	27.4	18.9
Approach LOS	B	C	C	B
Intersection Delay	21.1	$X_c = 0.65$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	10/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	723	340	4	3	348	569	2	3	2	118	0	275
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 27.0	G =	G =	G = 15.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	803	382			369	599		17		147	0	344
Lane Group Capacity, c	806	1187			563	480		167		218	303	879
v/c Ratio, X	1.00	0.32			0.66	1.25		0.10		0.67	0.00	0.39
Total Green Ratio, g/C	0.72	0.72			0.30	0.30		0.17		0.17	0.17	0.59
Uniform Delay, d ₁	17.8	4.5			27.4	31.5		31.8		35.2	31.3	9.9
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	25.4	0.5			3.9	122.7		1.2		15.5	0.0	1.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	43.2	5.0			31.3	154.2		33.0		50.7	31.3	11.2
Lane Group LOS	D	A			C	F		C		D	C	B
Approach Delay	30.9			107.4			33.0			23.0		
Approach LOS	C			F			C			C		
Intersection Delay	57.3			$X_c = 1.02$			Intersection LOS			E		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	426	35	0	1	35	42	3	3	2	60	1	882
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 41.0	G =	G =	G =	G = 39.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	473	39			72			16			65	626
Lane Group Capacity, c	687	971			1422			1362			616	686
v/c Ratio, X	0.69	0.04			0.05			0.01			0.11	0.91
Total Green Ratio, g/C	0.46	0.46			0.46			0.43			0.43	0.43
Uniform Delay, d ₁	19.4	13.6			13.7			14.5			15.1	23.9
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	5.3	0.1			0.1			0.0			0.3	17.7

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	24.7	13.7			13.7			14.5			15.5	41.6
Lane Group LOS	C	B			B			B			B	D
Approach Delay	23.9				13.7				14.5		39.1	
Approach LOS	C				B				B		D	
Intersection Delay	31.4				$X_c = 0.80$				Intersection LOS		C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		145	266	450	266	83	198			479	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 16.0	G =	G =	G = 21.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		153		1115		90	215			557	
Lane Group Capacity, c	222		220		1250		126	647			703	
v/c Ratio, X	0.02		0.70		0.89		0.71	0.33			0.79	
Total Green Ratio, g/C	0.13		0.13		0.27		0.35	0.35			0.35	
Uniform Delay, d ₁	22.6		24.8		21.2		16.9	14.3			17.5	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		14.3		5.4		27.4	1.3			7.8	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		39.2		26.5		44.3	15.6			25.3	
Lane Group LOS	C		D		C		D	B			C	
Approach Delay	38.8		26.5		24.1		25.3					
Approach LOS	D		C		C		C					
Intersection Delay	26.8		$X_c = 0.81$		Intersection LOS		C					

Saturday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	9	5	308	26	378	5	162	366	184	204	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 23.0	G =	G =	G =	G = 27.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		15			363	411		585			426	
Lane Group Capacity, c		666			505	496		803			467	
v/c Ratio, X		0.02			0.72	0.83		0.73			0.91	
Total Green Ratio, g/C		0.38			0.38	0.38		0.45			0.45	
Uniform Delay, d ₁		11.5			15.7	16.7		13.5			15.4	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			7.3	12.8		5.0			24.7	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		11.6			23.1	29.5		18.5			40.1	
Lane Group LOS		B			C	C		B			D	
Approach Delay		11.6			26.5			18.5			40.1	
Approach LOS		B			C			B			D	
Intersection Delay		27.0			$X_c = 0.87$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				104		186		583	90	130	680	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.940			0.789		0.272	0.272	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					326			740		143	747	
Lane Group Capacity, c					394			1131		280	977	
v/c Ratio, X					0.83			0.65		0.51	0.76	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					20.7			9.0		7.9	9.9	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					16.9			2.3		1.8	1.6	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				37.6			11.3		9.7	11.5	
Lane Group LOS				<i>D</i>			<i>B</i>		<i>A</i>	<i>B</i>	
Approach Delay				37.6	11.3			11.3			
Approach LOS				<i>D</i>	<i>B</i>			<i>B</i>			
Intersection Delay	15.7			$X_c = 0.78$			Intersection LOS		<i>B</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/29/13					Jurisdiction	SFR NB-DefL = 2050						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0	
Lane Group		LTR			LTR		DefL	TR			LTR		
Volume, V (vph)	79	123	266	161	357	24	231	262	112	25	815	162	
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.830			0.865		1.000	1.000			1.000		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0			12.0		13.0	13.0			9.5		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	NB Only	07	08					
Timing	G = 31.0	G =	G =	G =	G = 39.0	G = 5.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		509			695		246	411			1077		
Lane Group Capacity, c		723			783		349	1011			1313		
v/c Ratio, X		0.70			0.89		0.70	0.41			0.82		
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.43		
Uniform Delay, d ₁		25.5			27.9		31.1	12.0			22.4		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		4.7			12.6		11.3	1.2			5.8		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	30.3	40.4	42.4	13.2	28.3
Lane Group LOS	C	D	D	B	C
Approach Delay	30.3	40.4	24.1		28.3
Approach LOS	C	D	C		C
Intersection Delay	30.6	$X_c = 1.15$	Intersection LOS		C

HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	KM			Intersection	Boscombe Ave & OBX Off-Ramp		
Agency or Co.	AECOM			Area Type	All other areas		
Date Performed	4/10/13			Jurisdiction	Sat Flw Rate EB-L = 2100		
Time Period	Saturday 12:45-1:45 PM Peak Pd			Analysis Year	2015 Miti-Action Conditions		
				Project ID	Charleston EIS		

Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	849	411			451	786				194		206
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 23.0	G =	G =	G = 15.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					

Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	875	428			480	688				211		224
Lane Group Capacity, c	895	1189			481	764				540		945
v/c Ratio, X	0.98	0.36			1.00	0.90				0.39		0.24
Total Green Ratio, g/C	0.73	0.72			0.26	0.48				0.17		0.63
Uniform Delay, d ₁	21.0	4.7			33.5	21.5				33.4		7.1
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	16.8	0.4			16.7	3.5				2.1		0.6
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0
Control Delay	37.8	5.1			50.2	25.0				35.5		7.7
Lane Group LOS	D	A			D	C				D		A
Approach Delay	27.0			35.4						21.2		
Approach LOS	C			D						C		
Intersection Delay	29.5			X _c = 0.95			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	561	45	0	0	45	56	0	0	1	81	0	1192
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G = 9.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	584	47			95			1			87	854
Lane Group Capacity, c	823	1066			1125			1184			546	871
v/c Ratio, X	0.71	0.04			0.08			0.00			0.16	0.98
Total Green Ratio, g/C	0.50	0.50			0.34			0.39			0.39	0.54
Uniform Delay, d ₁	18.6	11.5			19.9			16.8			17.9	20.0
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	5.0	0.1			0.1			0.0			0.5	23.9

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	23.6	11.6			20.1			16.8			18.4	43.9
Lane Group LOS	C	B			C			B			B	D
Approach Delay	22.7				20.1				16.8		41.6	
Approach LOS	C				C				B		D	
Intersection Delay	33.2				$X_c = 0.99$				Intersection LOS		C	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/29/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	732	2		5			27	380	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 26.0	G =	G =	G =	G = 24.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		603	203		6				458		
Lane Group Capacity, c		815		630	606		321				1955		
v/c Ratio, X		0.00		0.96	0.33		0.02				0.23		
Total Green Ratio, g/C		0.43		0.43	0.43		0.40				0.40		
Uniform Delay, d ₁		9.7		16.5	11.3		10.9				11.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		26.2	1.4		0.1				0.3		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		9.7		42.7	12.7		11.0				12.2	
Lane Group LOS		A		D	B		B				B	
Approach Delay		9.7		35.2			11.0			12.2		
Approach LOS		A		D			B			B		
Intersection Delay		26.7		$X_c = 0.61$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	211	262	178	0	61	15	672	63	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 24.0	G =	G =	G =	G = 26.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		630	149		91			794				
Lane Group Capacity, c		632	640		636			2032				
v/c Ratio, X		1.00	0.23		0.14			0.39				
Total Green Ratio, g/C		0.40	0.40		0.40			0.43				
Uniform Delay, d ₁		18.0	11.9		11.5			11.6				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		35.0	0.8		0.5			0.5				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		52.9	12.8		11.9			12.1			
Lane Group LOS		<i>D</i>	<i>B</i>		<i>B</i>			<i>B</i>			
Approach Delay		45.3			11.9			12.1			
Approach LOS		<i>D</i>			<i>B</i>			<i>B</i>			
Intersection Delay		27.6			$X_c = 0.68$			Intersection LOS			<i>C</i>

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/29/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2015 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		160	285	596	168	126	218			404	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, l	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	NB Only	07	08				
Timing	G = 5.0	G = 16.0	G =	G =	G = 15.0	G = 4.0	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		225		1206		137	237			468	
Lane Group Capacity, c	139		385		1259		390	745			530	
v/c Ratio, X	0.19		0.58		0.96		0.35	0.32			0.88	
Total Green Ratio, g/C	0.08		0.23		0.27		0.40	0.40			0.25	
Uniform Delay, d ₁	25.6		20.4		21.7		19.7	12.4			21.7	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	2.2		4.6		10.2		2.3	1.0			14.9	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	27.8		25.0		31.9		22.0	13.4			36.6	
Lane Group LOS	C		C		C		C	B			D	
Approach Delay	25.3		31.9			16.5			36.6			
Approach LOS	C		C			B			D			
Intersection Delay	29.6		$X_c = 0.85$			Intersection LOS			C			

Mitigation

2020

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/4/13			Analysis Year	2020 Miti-Action Conditions		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		216	89	102	205		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	260	107	122	246	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				150		39	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.70	
Hourly Flow Rate, HFR (veh/h)	0	0	0	200	0	55	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT	L		R		
v (veh/h)		122	200		55		
C (m) (veh/h)		1144	317		731		
v/c		0.11	0.63		0.08		
95% queue length		0.36	4.02		0.24		
Control Delay (s/veh)		8.5	33.9		10.3		
LOS		A	D		B		
Approach Delay (s/veh)	--	--	28.8				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	3	6	1	174	34	227	0	205	304	117	160	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887	0.887	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4	0	0	3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			239	261		241	358	134	194	
Lane Group Capacity, c		717			539	514		702	606	399	676	
v/c Ratio, X		0.02			0.44	0.51		0.34	0.59	0.34	0.29	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42	0.42	0.42	0.42	
Uniform Delay, d ₁		10.3			12.5	12.9		11.9	13.5	11.9	11.6	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.0			2.5	3.3		1.2	3.7	2.3	1.1	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/3/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				62		177		549	74	119	358	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.980			0.769		0.842	0.842	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					257			733		137	411	
Lane Group Capacity, c					382			1033		265	911	
v/c Ratio, X					0.67			0.71		0.52	0.45	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					19.7			9.4		8.0	7.6	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					9.0			3.2		6.0	1.4	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/10/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	74	164	169	203	285	80	119	324	138	68	32	41
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 32.0	G =	G =	G =	G = 25.0	G = 18.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	84	335		233	415			624		101	109	
Lane Group Capacity, c	295	618		242	1153			903		339	315	
v/c Ratio, X	0.28	0.54		0.96	0.36			0.69		0.30	0.35	
Total Green Ratio, g/C	0.36	0.36		0.36	0.36			0.28		0.20	0.20	
Uniform Delay, d ₁	20.8	23.2		28.4	21.4			29.0		30.6	30.9	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	1.9	2.7		48.4	0.9			4.3		2.2	3.0	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR			LTR		DefL	TR		L	TR	
Volume, V (vph)	54	28	275	115	203	6	264	93	51	11	160	102
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3			3		3	3		3	3	
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.916	0.916			0.953		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	61	345			372		303	166		14	345	
Lane Group Capacity, c	390	1286			1101		450	794		476	1285	
v/c Ratio, X	0.16	0.27			0.34		0.67	0.21		0.03	0.27	
Total Green Ratio, g/C	0.44	0.44			0.44		0.44	0.44		0.44	0.44	
Uniform Delay, d ₁	14.9	15.8			16.3		19.8	15.3		14.1	15.8	
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	0.8	0.5			0.8		7.8	0.6		0.1	0.5	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N _l	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	356	26	0	14	68	38	6	22	11	15	38	499
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I _l	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	391	29			153			86			70	453
Lane Group Capacity, c	699	1066			1543			1234			673	616
v/c Ratio, X	0.56	0.03			0.10			0.07			0.10	0.74
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.6	11.4			11.8			17.3			17.5	23.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		214	0	384	191		3			24	200	64	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.80	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 30.0	G =	G =	G =	G = 20.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		267		486	242		4				356		
Lane Group Capacity, c		932		522	953		298				1590		
v/c Ratio, X		0.29		0.93	0.25		0.01				0.22		
Total Green Ratio, g/C		0.50		0.50	0.50		0.33				0.33		
Uniform Delay, d ₁		8.8		14.0	8.6		13.4				14.4		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.8		25.2	0.6		0.1				0.3		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		166	244	467	164	90	200			475	22
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	NB Only	07	08				
Timing	G = 10.0	G = 15.0	G =	G =	G = 12.0	G = 5.0	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 3	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		210		994		100	222			654	
Lane Group Capacity, c	273		540		1164		401	575			669	
v/c Ratio, X	0.01		0.39		0.85		0.25	0.39			0.98	
Total Green Ratio, g/C	0.17		0.33		0.25		0.33	0.33			0.20	
Uniform Delay, d ₁	20.9		15.3		21.5		19.3	15.3			23.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		2.1		6.0		1.4	1.9			14.2	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	20	13	172	16	105	10	231	131	139	339	9
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G =	G =	G =	G = 43.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		60		195	18	119		414			641	
Lane Group Capacity, c		597		504	719	531		753			587	
v/c Ratio, X		0.10		0.39	0.03	0.22		0.55			1.09	
Total Green Ratio, g/C		0.41		0.41	0.41	0.41		0.48			0.48	
Uniform Delay, d ₁		16.3		18.6	15.8	17.2		16.6			23.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		2.2	0.1	0.9		2.5			44.4	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		174	189	173	215		282		151			
% Heavy Vehicles, %HV		10	10	13	13		9		9			
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.964		0.986	0.986			0.858				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		347		194	242			461				
Lane Group Capacity, c		900		377	813			529				
v/c Ratio, X		0.39		0.51	0.30			0.87				
Total Green Ratio, g/C		0.50		0.50	0.50			0.39				
Uniform Delay, d ₁		13.9		15.1	13.2			25.4				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		1.2		4.9	0.9			15.6				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/4/13			Analysis Year	2020 Miti-Action Conditions		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		310	44	25	316		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	364	51	27	347	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				148		16	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	157	0	17	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		1
Configuration				L			R
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT	L		R		
v (veh/h)		27	157		17		
C (m) (veh/h)		1077	352		663		
v/c		0.03	0.45		0.03		
95% queue length		0.08	2.21		0.08		
Control Delay (s/veh)		8.4	23.2		10.6		
LOS		A	C		B		
Approach Delay (s/veh)	--	--	22.0				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	1	17	6	255	24	391	1	175	272	233	192	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931	0.931	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3	0	0	4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			310	434		191	296	271	224	
Lane Group Capacity, c		713			523	518		704	606	440	703	
v/c Ratio, X		0.04			0.59	0.84		0.27	0.49	0.62	0.32	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42	0.42	0.42	0.42	
Uniform Delay, d ₁		10.4			13.6	15.7		11.5	12.8	13.7	11.8	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.1			4.3	13.4		0.9	2.6	6.3	1.2	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/3/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				102		199		473	80	186	467	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.944			0.880		0.500	0.500	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					346			601		216	543	
Lane Group Capacity, c					385			1058		332	933	
v/c Ratio, X					0.90			0.57		0.65	0.58	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					21.2			8.3		8.9	8.4	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					25.2			1.9		4.9	1.3	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/10/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	150	250	148	195	446	84	178	448	316	149	67	89
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 33.0	G =	G =	G =	G = 25.0	G = 17.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	179	436		217	575			1013		186	195	
Lane Group Capacity, c	234	657		187	1225			924		323	300	
v/c Ratio, X	0.76	0.66		1.16	0.47			1.10		0.58	0.65	
Total Green Ratio, g/C	0.37	0.37		0.37	0.37			0.28		0.19	0.19	
Uniform Delay, d ₁	25.1	23.9		28.5	21.8			32.5		33.2	33.7	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	18.3	4.5		113.3	1.2			59.5		7.2	10.3	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	62	117	509	134	223	18	281	201	88	23	528	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.889	0.889		0.932	0.932		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0		12.0	12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	WB Only	03	04	SB Only	NS Perm	NB Only	08				
Timing	G = 25.0	G = 5.0	G =	G =	G = 13.0	G = 14.0	G = 14.0	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 3	Y = 3	Y = 5	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	74	745		149	268		323	332		26	838	
Lane Group Capacity, c	278	865		277	669		468	627		325	943	
v/c Ratio, X	0.27	0.86		0.54	0.40		0.69	0.53		0.08	0.89	
Total Green Ratio, g/C	0.28	0.28		0.37	0.37		0.34	0.34		0.33	0.33	
Uniform Delay, d ₁	25.3	30.9		33.3	21.2		29.1	23.7		20.6	28.4	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	2.1	9.9		6.8	1.7		8.1	3.2		0.5	12.2	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/16/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	681	422	3	1	443	746	0	0	1	119	0	136
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.93	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 23.0	G =	G =	G = 15.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	748	467			477	726		4		123	0	140
Lane Group Capacity, c	783	1176			471	401		307		219	300	936
v/c Ratio, X	0.96	0.40			1.01	1.81		0.01		0.56	0.00	0.15
Total Green Ratio, g/C	0.72	0.72			0.26	0.26		0.17		0.17	0.17	0.63
Uniform Delay, d ₁	21.2	4.9			33.5	33.5		31.3		34.5	31.3	6.7
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 Mitih-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	504	38	0	2	65	71	4	3	0	65	6	1120
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 28.0	G = 12.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	42			132		7	5			79	900
Lane Group Capacity, c	818	1056			955		516	732			538	881
v/c Ratio, X	0.68	0.04			0.14		0.01	0.01			0.15	1.02
Total Green Ratio, g/C	0.50	0.50			0.31		0.39	0.39			0.39	0.58
Uniform Delay, d ₁	18.5	11.5			22.3		16.9	16.9			17.8	19.0
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		38	0	553	83		1			23	294	30	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 29.0	G =	G =	G =	G = 21.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		152		582	87		1				433		
Lane Group Capacity, c		909		621	940		279				1689		
v/c Ratio, X		0.17		0.94	0.09		0.00				0.26		
Total Green Ratio, g/C		0.48		0.48	0.48		0.35				0.35		
Uniform Delay, d ₁		8.7		14.6	8.4		12.7				13.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.4		23.3	0.2		0.0				0.4		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		156	231	477	127	114	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 10.0	G = 15.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		203		960		128	192			429	
Lane Group Capacity, c	278		275		1182		289	593			1163	
v/c Ratio, X	0.05		0.74		0.81		0.44	0.32			0.37	
Total Green Ratio, g/C	0.17		0.17		0.25		0.33	0.33			0.33	
Uniform Delay, d ₁	21.0		23.8		21.2		15.6	14.9			15.2	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.3		13.7		4.5		4.6	1.4			0.7	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		198	189	189	226		196		174			
% Heavy Vehicles, %HV		8	8	16	16		2		2			
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.961		0.989	0.989			0.922				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		381		208	248			421				
Lane Group Capacity, c		921		347	792			559				
v/c Ratio, X		0.41		0.60	0.31			0.75				
Total Green Ratio, g/C		0.50		0.50	0.50			0.39				
Uniform Delay, d ₁		14.2		16.1	13.3			23.8				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		1.3		7.4	1.0			8.4				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/4/13			Analysis Year	2020 Miti-Action Conditions		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		338	63	33	330		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	388	72	40	407	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				111		31	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	163	0	45	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT	L		R		
v (veh/h)		40	163		45		
C (m) (veh/h)		1075	287		619		
v/c		0.04	0.57		0.07		
95% queue length		0.12	3.26		0.23		
Control Delay (s/veh)		8.5	32.8		11.3		
LOS		A	D		B		
Approach Delay (s/veh)	--	--	28.2				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group		LTR			LT	R		LT	R	L	TR	
Volume, V (vph)	1	11	2	301	20	300	4	183	328	287	262	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919	0.919	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3	0	0	2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			391	366		201	353	334	306	
Lane Group Capacity, c		727			530	523		741	641	436	709	
v/c Ratio, X		0.02			0.74	0.70		0.27	0.55	0.77	0.43	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42	0.42	0.42	0.42	
Uniform Delay, d ₁		10.3			14.7	14.4		11.5	13.2	15.0	12.4	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.1			8.3	7.1		0.8	3.1	12.1	1.9	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				135		175		588	106	145	747	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.901			0.798		0.114	0.114	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					369			746		169	869	
Lane Group Capacity, c					397			1098		267	941	
v/c Ratio, X					0.93			0.68		0.63	0.92	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					21.4			9.2		8.8	11.8	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					28.4			2.7		1.3	2.5	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	53	120	557	123	272	10	271	145	124	22	534	124
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.884	0.884		0.927	0.927		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0		12.0	12.0		13.0	13.0		12.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	WB Only	03	04	SB Only	NS Perm	NB Only	08				
Timing	G = 25.0	G = 5.0	G =	G =	G = 13.0	G = 14.0	G = 14.0	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 3	Y = 3	Y = 5	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	60	769		150	344		291	289		23	700	
Lane Group Capacity, c	252	864		280	679		490	623		399	1053	
v/c Ratio, X	0.24	0.89		0.54	0.51		0.59	0.46		0.06	0.66	
Total Green Ratio, g/C	0.28	0.28		0.37	0.37		0.34	0.34		0.33	0.33	
Uniform Delay, d ₁	25.1	31.2		33.5	22.2		27.6	23.0		20.4	25.7	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	2.0	12.0		6.7	2.5		5.2	2.5		0.3	3.3	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	499	37	0	1	37	43	3	3	2	62	1	1176
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 29.0	G = 11.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	41			75			16			67	938
Lane Group Capacity, c	845	1066			1007			1220			548	897
v/c Ratio, X	0.66	0.04			0.07			0.01			0.12	1.05
Total Green Ratio, g/C	0.50	0.50			0.32			0.39			0.39	0.57
Uniform Delay, d ₁	17.8	11.5			21.2			16.9			17.6	19.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	3.8	0.1			0.1			0.0			0.4	41.8

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	1/31/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	T		L				LTR		
Volume, V (vph)		71	0	475	76		4			23	334	38	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.50	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 27.0	G =	G =	G =	G = 23.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		142		534	85		5				434		
Lane Group Capacity, c		846		571	858		312				1866		
v/c Ratio, X		0.17		0.94	0.10		0.02				0.23		
Total Green Ratio, g/C		0.45		0.45	0.45		0.38				0.38		
Uniform Delay, d ₁		9.8		15.7	9.5		11.5				12.5		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.4		24.4	0.2		0.1				0.3		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	193	270	176	8	66	9	486	106	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 22.0	G =	G =	G =	G = 28.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		545	145		95			667				
Lane Group Capacity, c		586	586		556			2220				
v/c Ratio, X		0.93	0.25		0.17			0.30				
Total Green Ratio, g/C		0.37	0.37		0.37			0.47				
Uniform Delay, d ₁		18.3	13.2		12.8			9.9				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		23.4	1.0		0.7			0.3				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		180	284	485	276	104	207			501	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 9.0	G = 16.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		189		1188		113	225			582	
Lane Group Capacity, c	250		248		1251		226	616			1141	
v/c Ratio, X	0.02		0.76		0.95		0.50	0.37			0.51	
Total Green Ratio, g/C	0.15		0.15		0.27		0.33	0.33			0.33	
Uniform Delay, d ₁	21.7		24.5		21.6		16.0	15.2			16.1	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		17.0		9.5		7.2	1.6			1.4	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	13	20	267	29	110	18	320	175	94	456	10
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		57		307	33	126		558			643	
Lane Group Capacity, c		618		545	773	570		752			644	
v/c Ratio, X		0.09		0.56	0.04	0.22		0.74			1.00	
Total Green Ratio, g/C		0.43		0.43	0.43	0.43		0.46			0.46	
Uniform Delay, d ₁		15.1		19.1	14.7	16.0		20.2			24.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.8	0.1	0.8		4.4			16.6	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/4/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		203	253	208	224		158		200			
% Heavy Vehicles, %HV		5	5	8	8		1		1			
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.927		0.989	0.989			0.917				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 34.0	G = 5.0	G =	G =	G = 38.0	G =	G =	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		485		251	270			535				
Lane Group Capacity, c		709		336	794			608				
v/c Ratio, X		0.68		0.75	0.34			0.88				
Total Green Ratio, g/C		0.38		0.47	0.47			0.42				
Uniform Delay, d ₁		23.5		30.8	15.2			23.9				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		4.9		13.9	1.2			15.5				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/3/13			Analysis Year	2020 Miti-Action Conditions		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		321	55	27	377		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	348	59	30	428	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				119		22	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	158	0	29	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT	L		R		
v (veh/h)		30	158		29		
C (m) (veh/h)		1136	313		667		
v/c		0.03	0.50		0.04		
95% queue length		0.08	2.69		0.14		
Control Delay (s/veh)		8.3	27.7		10.6		
LOS		A	D		B		
Approach Delay (s/veh)	--	--	25.0				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	0	10	5	356	27	434	5	216	400	296	229	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861	0.861	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2	0	0	2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			416	472		242	440	325	252	
Lane Group Capacity, c		726			547	539		744	641	429	737	
v/c Ratio, X		0.02			0.76	0.88		0.33	0.69	0.76	0.34	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42	0.42	0.42	0.42	
Uniform Delay, d ₁		10.3			14.9	16.1		11.8	14.3	14.9	11.9	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.1			8.2	15.6		1.0	5.1	11.8	1.3	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/2/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				108		222		645	94	157	735	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.940			0.789		0.272	0.272	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					370			812		173	808	
Lane Group Capacity, c					393			1132		252	977	
v/c Ratio, X					0.94			0.72		0.69	0.83	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					21.5			9.5		9.2	10.6	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					31.5			3.1		4.1	2.3	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				53.1			12.6		13.4	12.9	
Lane Group LOS				<i>D</i>			<i>B</i>		<i>B</i>	<i>B</i>	
Approach Delay				53.1	12.6			13.0			
Approach LOS				<i>D</i>	<i>B</i>			<i>B</i>			
Intersection Delay	19.7			$X_c = 0.86$			Intersection LOS		<i>B</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	82	139	593	165	339	25	322	269	115	26	834	168
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.830	0.830		0.865	0.865		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0		12.0	12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	WB Only	03	04	SB Only	NS Perm	NB Only	08				
Timing	G = 25.0	G = 6.0	G =	G =	G = 15.0	G = 14.0	G = 11.0	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 3	Y = 3	Y = 5	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	89	796		212	467		343	422		28	1078	
Lane Group Capacity, c	195	866		302	703		447	578		370	1150	
v/c Ratio, X	0.46	0.92		0.70	0.66		0.77	0.73		0.08	0.94	
Total Green Ratio, g/C	0.28	0.28		0.38	0.38		0.31	0.31		0.36	0.36	
Uniform Delay, d ₁	26.9	31.5		34.4	23.3		32.1	27.6		19.4	28.0	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	6.3	14.1		11.2	4.3		11.9	7.9		0.4	15.2	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/10/13					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	881	472			508	1077				233		216
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 24.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	908	492			540	988				253		235
Lane Group Capacity, c	895	1207			502	764				504		928
v/c Ratio, X	1.01	0.41			1.08	1.29				0.50		0.25
Total Green Ratio, g/C	0.74	0.73			0.27	0.48				0.16		0.62
Uniform Delay, d ₁	9.5	4.6			33.0	23.5				34.8		7.6
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	25.1	0.5			41.4	133.7				3.5		0.7
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	660	46	0	0	46	58	0	0	1	84	0	1539
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 26.0	G = 14.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	688	48			99			1			90	1227
Lane Group Capacity, c	853	1066			944			1184			544	959
v/c Ratio, X	0.81	0.05			0.10			0.00			0.17	1.28
Total Green Ratio, g/C	0.50	0.50			0.29			0.39			0.39	0.60
Uniform Delay, d ₁	20.7	11.5			23.5			16.8			18.0	18.0
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	7.8	0.1			0.2			0.0			0.6	132.7

HCS+™ DETAILED REPORT														
General Information						Site Information								
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W							
Agency or Co.	AECOM					Area Type	All other areas							
Date Performed	1/31/13					Jurisdiction	Staten Island, NY							
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions							
						Project ID	Charleston EIS							
Volume and Timing Input														
	EB			WB			NB			SB				
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Number of Lanes, N ₁		1	0	1	1		1			0	3	0		
Lane Group		TR		L	T		L				LTR			
Volume, V (vph)		65	0	728	89		5			28	451	37		
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2		
Peak-Hour Factor, PHF		0.75	0.25	0.91	0.91		0.88			0.89	0.89	0.89		
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P		
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0			
Arrival Type, AT		3		3	3		3				3			
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0			
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000			
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0		
Lane Width		12.0		13.0	13.0		10.0				11.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N		
Parking Maneuvers, N _m														
Buses Stopping, N _b		0		0	0		0				0			
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8			
Phasing	EW Perm	WB Only	03			04			NS Perm	06			07	08
Timing	G = 19.0	G = 7.0	G =	G =			G = 19.0			G =	G =			
	Y = 5	Y = 5	Y =	Y =			Y = 5			Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0							
Lane Group Capacity, Control Delay, and LOS Determination														
	EB			WB			NB			SB				
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Adjusted Flow Rate, v		87		800	98		6				580			
Lane Group Capacity, c		596		793	995		191				1532			
v/c Ratio, X		0.15		1.01	0.10		0.03				0.38			
Total Green Ratio, g/C		0.32		0.52	0.52		0.32				0.32			
Uniform Delay, d ₁		14.7		15.7	7.4		14.1				15.9			
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000			
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50			
Incremental Delay, d ₂		0.5		33.7	0.2		0.3				0.7			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	218	367	213	0	88	15	730	66	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 26.0	G =	G =	G =	G = 24.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		780	196		124			859				
Lane Group Capacity, c		695	693		695			1875				
v/c Ratio, X		1.12	0.28		0.18			0.46				
Total Green Ratio, g/C		0.43	0.43		0.43			0.40				
Uniform Delay, d ₁		17.0	11.0		10.4			13.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		72.9	1.0		0.6			0.8				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/2/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		194	302	628	173	153	229			422	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, l	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 12.0	G = 16.0	G =	G =	G = 17.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		273		1268		166	249			488	
Lane Group Capacity, c	334		330		1260		219	528			1009	
v/c Ratio, X	0.08		0.83		1.01		0.76	0.47			0.48	
Total Green Ratio, g/C	0.20		0.20		0.27		0.28	0.28			0.28	
Uniform Delay, d ₁	19.5		23.0		22.0		19.6	17.8			17.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.3		15.6		19.2		20.0	2.8			1.2	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	1/31/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	8	18	294	18	106	13	350	242	86	447	8
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		41		327	20	118		657			602	
Lane Group Capacity, c		615		563	788	586		763			630	
v/c Ratio, X		0.07		0.58	0.03	0.20		0.86			0.96	
Total Green Ratio, g/C		0.43		0.43	0.43	0.43		0.46			0.46	
Uniform Delay, d ₁		14.9		19.3	14.6	15.8		21.9			23.6	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.9	0.1	0.7		6.8			16.7	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/4/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		229	240	283	234		160		233			
% Heavy Vehicles, %HV		2	2	4	4		1		1			
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.927		0.992	0.992			0.953				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		2		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 30.0	G = 12.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		530		318	263			462				
Lane Group Capacity, c		656		408	883			558				
v/c Ratio, X		0.81		0.78	0.30			0.83				
Total Green Ratio, g/C		0.33		0.50	0.50			0.39				
Uniform Delay, d ₁		27.4		31.1	13.2			24.8				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		9.6		13.6	0.9			12.7				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Accident Data

Charleston Development (SI)

Data Source: NYS DMV/NYS DOT

Allentown Lane-Veterans Road West/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000449	2008	1	1	0	0	1	1
0000449	2009	3	1	0	0	1	1
0000449	2010	4	3	0	0	3	3
Total		8	5	0	0	5	5

North Bridge Street/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000447	2008	1	1	0	0	1	1
0000447	2009	0	0	0	0	0	0
0000447	2010	0	0	0	0	0	0
Total		1	1	0	0	1	1

Richmond Valley Road/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000291	2008	2	0	0	0	0	0
0000291	2009	0	0	0	0	0	0
0000291	2010	1	0	0	0	0	0
Total		3	0	0	0	0	0

Richmond Valley Road/Page Avenue

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000372	2008	0	0	0	0	0	0
0000372	2009	2	1	0	0	1	1
0000372	2010	0	0	0	0	0	0
Total		2	1	0	0	1	1

South Bridge Street/Page Avenue-Boscombe Avenue

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000492	2008	2	1	1	0	0	1
0000492	2009	0	0	0	0	0	0
0000492	2010	2	2	0	0	2	2
Total		4	3	1	0	2	3

Veterans Road West/Mohr Street-Korean War Veterans Parkway ramps

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000465	2008	0	0	0	0	0	0
0000465	2009	0	0	0	0	0	0
0000465	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Veterans Road West/Tyrellan Avenue

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000872	2008	2	1	0	0	1	1
0000872	2009	2	2	0	0	2	2
0000872	2010	1	1	0	0	2	2
Total		5	4	0	0	5	5

Note: There is one fatal crash in 2011 (7/23/2011, motor vehicle occupant, NYPD fatality data).

Boscombe Avenue/Outerbridge crossing on- & off-ramps

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000497	2008	0	0	0	0	0	0
0000497	2009	0	0	0	0	0	0
0000497	2010	0	0	0	0	0	0

Total	0	0	0	0	0	0	0
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Boscombe Avenue/Tyrellan Avenue

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000863	2008	0	0	0	0	0	0
0000863	2009	0	0	0	0	0	0
0000863	2010	1	0	0	0	0	0
Total		1	0	0	0	0	0

Bricktown Centre Road-Mohr Street/Veterans Road West

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
No node ID/No crash records							
Total							

Englewood Avenue/Veterans Road West

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000906	2008	0	0	0	0	0	0
0000906	2009	0	0	0	0	0	0
0000906	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Englewood Avenue/Veterans Road East

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000950	2008	1	1	0	0	1	1
0000950	2009	1	0	0	0	0	0
0000950	2010	0	0	0	0	0	0
Total		2	1	0	0	1	1

Englewood Avenue/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001048	2008	0	0	0	0	0	0
0001048	2009	0	0	0	0	0	0
0001048	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Sharrotts Road/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001079	2008	1	0	0	0	0	0
0001079	2009	2	1	1	0	0	1
0001079	2010	0	0	0	0	0	0
Total		3	1	1	0	0	1

Veterans Road East-Drumgoole Road West/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001039	2008	1	0	0	0	0	0
0001039	2009	0	0	0	0	0	0
0001039	2010	0	0	0	0	0	0
Total		1	0	0	0	0	0

South Service Road-Drumgoole Road East/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001146	2008	0	0	0	0	0	0
0001146	2009	1	0	0	0	0	0
0001146	2010	0	0	0	0	0	0
Total		1	0	0	0	0	0

Pleasant Plains Avenue-Amboy Road/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001136	2008	1	1	0	0	1	1
0001136	2009	1	0	0	0	0	0
0001136	2010	3	0	0	0	0	0
Total		5	1	0	0	1	1

Arthur Kill Road/Bloomingdale Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0001011	2008	1	0	0	0	0	0
0001011	2009	2	0	0	0	0	0
0001011	2010	0	0	0	0	0	0
Total		3	0	0	0	0	0

Sharrotts Road/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000481	2008	1	1	0	0	1	1
0000481	2009	2	1	0	0	1	1
0000481	2010	1	1	0	0	1	1
Total		4	3	0	0	3	3

Englewood Avenue/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000469	2008	0	0	0	0	0	0
0000469	2009	0	0	0	0	0	0
0000469	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

South Bridge Street/Arthur Kill Road

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000445	2008	0	0	0	0	0	0
0000445	2009	0	0	0	0	0	0
0000445	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Mohr Street/Tyrellan Avenue

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
No node ID/No crash records							
Total							

Sharrotts Road/Veterans Road West

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000910	2008	0	0	0	0	0	0
0000910	2009	0	0	0	0	0	0
0000910	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Sharrotts Road/Veterans Road East

NODE_ID	YEAR	# of Crashes	# of Injury Crashes	# of Injuries			Total
				Pedestrian	Bike	Motor Vehicle Occupant	
0000967	2008	0	0	0	0	0	0
0000967	2009	0	0	0	0	0	0
0000967	2010	0	0	0	0	0	0
Total		0	0	0	0	0	0

Alternative 2
With-Action
2020

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	27	186	149	29	255	39	
Peak-Hour Factor, PHF	0.83	0.83	0.83	0.83	0.83	0.83	
Hourly Flow Rate, HFR (veh/h)	32	224	179	34	307	46	
Percent Heavy Vehicles	20	--	--	13	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	9	8	8	118	10	28	
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.95	0.95	0.95	
Hourly Flow Rate, HFR (veh/h)	11	10	10	124	10	29	
Percent Heavy Vehicles	32	32	32	5	5	5	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	32	34		163			31
C (m) (veh/h)	1113	1099		310			302
v/c	0.03	0.03		0.53			0.10
95% queue length	0.09	0.10		2.88			0.34
Control Delay (s/veh)	8.3	8.4		28.8			18.3
LOS	A	A		D			C
Approach Delay (s/veh)	--	--		28.8			18.3
Approach LOS	--	--		D			C

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		216	178	158	221		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	260	214	190	266	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				147		146	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.70	
Hourly Flow Rate, HFR (veh/h)	0	0	0	196	0	208	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		190		404			
C (m) (veh/h)		1043		336			
v/c		0.18		1.20			
95% queue length		0.66		17.27			
Control Delay (s/veh)		9.2		150.0			
LOS		A		F			
Approach Delay (s/veh)	--	--		150.0			
Approach LOS	--	--		F			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	3	6	1	174	34	382	0	224	313	119	255	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			239	439		632			440	
Lane Group Capacity, c		717			539	514		711			308	
v/c Ratio, X		0.02			0.44	0.85		0.89			1.43	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.5	15.8		16.2			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.0			2.5	15.6		14.0			210.7	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.3			15.0	31.4		30.2			228.2	
Lane Group LOS		<i>B</i>			<i>B</i>	<i>C</i>		<i>C</i>			<i>F</i>	
Approach Delay		10.3			25.6			30.2			228.2	
Approach LOS		<i>B</i>			<i>C</i>			<i>C</i>			<i>F</i>	
Intersection Delay		77.8			$X_c = 1.14$			Intersection LOS			<i>E</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				268		5		532			431	
% Heavy Vehicles, %HV				8		8		11			15	
Peak-Hour Factor, PHF				0.88		0.88		0.85			0.87	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.893			0.886			0.754	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			3	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					311			626			495	
Lane Group Capacity, c					632			970			925	
v/c Ratio, X					0.49			0.65			0.54	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					15.9			11.1			10.2	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					2.4			2.9			1.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				18.4			14.0			11.9	
Lane Group LOS				<i>B</i>			<i>B</i>			<i>B</i>	
Approach Delay				18.4	14.0			11.9			
Approach LOS				<i>B</i>	<i>B</i>			<i>B</i>			
Intersection Delay	14.2	$X_c = 0.58$			Intersection LOS			<i>B</i>			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		532	224	190	509		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.87	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	625	263	218	585	0	
Percent Heavy Vehicles	0	--	--	20	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L					
v (veh/h)		218					
C (m) (veh/h)		692					
v/c		0.32					
95% queue length		1.35					
Control Delay (s/veh)		12.6					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				62		204		549	74	144	358	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.980			0.769			0.842	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only		02	03	04	NS Perm	06	07	08			
Timing	G = 16.0		G =	G =	G =	G = 34.0	G =	G =	G =			
	Y = 5		Y =	Y =	Y =	Y = 5	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					286			733			577	
Lane Group Capacity, c					381			1033			569	
v/c Ratio, X					0.75			0.71			1.01	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					20.2			9.4			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					12.5			3.2			38.2	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				32.7			12.6			51.2		
Lane Group LOS				<i>C</i>			<i>B</i>			<i>D</i>		
Approach Delay				32.7				12.6				51.2
Approach LOS				<i>C</i>				<i>B</i>				<i>D</i>
Intersection Delay	30.2			$X_c = 0.93$			Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group	LTR			LTR			L	TR		LTR		
Volume, V (vph)	72	53	94	24	161	46	88	675	112	65	369	30
% Heavy Vehicles, %HV	1	1	1	1	1	1	3	3	3	9	9	9
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.94	0.94	0.94	0.92	0.92	0.92	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.810			1.000		0.649	0.649			0.986	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	6
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		243			246		96	856			545	
Lane Group Capacity, c		560			577		397	1049			960	
v/c Ratio, X		0.43			0.43		0.24	0.82			0.57	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		22.7			22.7		10.8	16.8			13.5	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.0			2.3		0.9	4.7			2.4	

Initial Queue Delay, d_3		0.0		0.0		0.0	0.0			0.0	
Control Delay		24.7		25.0		11.7	21.5			15.9	
Lane Group LOS		C		C		B	C			B	
Approach Delay		24.7		25.0		20.5				15.9	
Approach LOS		C		C		C				B	
Intersection Delay		20.3		$X_c = 0.67$		Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	354							793			405	
% Heavy Vehicles, %HV	2							1			3	
Peak-Hour Factor, PHF	0.90							0.91			0.84	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.665			0.986	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	393							871			482	
Lane Group Capacity, c	669							2156			1951	
v/c Ratio, X	0.59							0.40			0.25	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.9							11.5			10.3	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	3.8							0.4			0.3	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	28.6						11.8			10.6	
Lane Group LOS	C						B			B	
Approach Delay	28.6						11.8			10.6	
Approach LOS	C						B			B	
Intersection Delay	15.3			$X_C = 0.47$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					405		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.84	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	482	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			59				
Peak-Hour Factor, PHF	1.00	1.00	0.70	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	84	0	0	0	
Percent Heavy Vehicles	0	0	60	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							84
C (m) (veh/h)							674
v/c							0.12
95% queue length							0.42
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	74	175	169	203	367	80	192	324	138	68	32	41
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	84	348		233	509			702		101	109	
Lane Group Capacity, c	228	581		208	1088			903		376	350	
v/c Ratio, X	0.37	0.60		1.12	0.47			0.78		0.27	0.31	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	22.8	25.0		30.0	23.7			29.9		29.0	29.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	3.5	3.5		97.8	1.4			6.5		1.7	2.3	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	26.3	28.5		127.8	25.1			36.5		30.7	31.5	
Lane Group LOS	C	C		F	C			D		C	C	
Approach Delay	28.1			57.3				36.5		31.1		
Approach LOS	C			E				D		C		
Intersection Delay	41.6			$X_c = 0.79$				Intersection LOS		D		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					404		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	425	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				287			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	308	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			308				
C (m) (veh/h)			575				
v/c			0.54				
95% queue length			3.16				
Control Delay (s/veh)			18.2				
LOS			C				
Approach Delay (s/veh)	--	--	18.2				
Approach LOS	--	--	C				

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/8/13				Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R	L	T	R	L	R	
Volume (veh/h)	38	69	28	33	84	35			
%Thrus Left Lane	50			50					
Approach	Northbound				Southbound				
Movement	L	T	R	L	T	R	L	R	
Volume (veh/h)	14	16	22	21	52	16			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT	TR	LT	TR	LT	R	LT	TR	
PHF	0.80	0.80	0.78	0.78	0.86	0.86	1.00	1.00	
Flow Rate (veh/h)	89	77	95	97	34	25	47	42	
% Heavy Vehicles	21	21	1	1	5	5	0	0	
No. Lanes	2		2		2		2		
Geometry Group	5		5		5		5		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5	0.0	0.4	0.0	0.5	0.0	0.4	0.0	
Prop. Right-Turns	0.0	0.4	0.0	0.5	0.0	1.0	0.0	0.4	
Prop. Heavy Vehicle	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0	
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	
hadj, computed	0.6	0.0	0.2	-0.3	0.3	-0.6	0.2	-0.3	
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	
x, initial	0.08	0.07	0.08	0.09	0.03	0.02	0.04	0.04	
hd, final value (s)	5.69	5.12	5.29	4.76	5.80	4.86	5.66	5.17	
x, final value	0.14	0.11	0.14	0.13	0.05	0.03	0.07	0.06	
Move-up time, m (s)	2.3		2.3		2.3		2.3		
Service Time, t _s (s)	3.4	2.8	3.0	2.5	3.5	2.6	3.4	2.9	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	339	327	345	347	284	275	297	292	
Delay (s/veh)	9.32	8.44	8.85	8.15	8.83	7.73	8.81	8.20	
LOS	A	A	A	A	A	A	A	A	
Approach: Delay (s/veh)	8.91		8.50		8.36		8.53		
LOS	A		A		A		A		
Intersection Delay (s/veh)	8.62								
Intersection LOS	A								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR			LTR		DefL	TR			LTR	
Volume, V (vph)	54	39	275	115	285	6	264	93	51	11	160	102
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.916			0.953		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p		18.6			16.6			20.1			19.8	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		418			467		303	166			359	
Lane Group Capacity, c		1087			1092		441	794			1216	
v/c Ratio, X		0.38			0.43		0.69	0.21			0.30	
Total Green Ratio, g/C		0.44			0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		16.8			17.1		20.0	15.3			16.0	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		0.9			1.2		8.5	0.6			0.6	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	17.7	18.3	28.4	15.9
Lane Group LOS	B	B	C	B
Approach Delay	17.7	18.3	24.0	16.6
Approach LOS	B	B	C	B
Intersection Delay	19.4	$X_c = 0.56$	Intersection LOS	B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	872	256	19	4	284	315	4	16	6	119	5	146
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.86	0.86	0.86	0.37	0.37	0.37	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.603	0.603			0.932	0.932		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	958	302			335	366		70		137	6	168
Lane Group Capacity, c	839	1158			449	383		355		226	320	952
v/c Ratio, X	1.14	0.26			0.75	0.96		0.20		0.61	0.02	0.18
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	18.7	4.6			31.4	33.5		31.5		34.1	30.5	6.4
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	72.9	0.3			10.1	34.5		1.2		11.5	0.1	0.4

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	91.7	4.9			41.5	68.1		32.8		45.6	30.6	6.8
Lane Group LOS	F	A			D	E		C		D	C	A
Approach Delay	70.9			55.4			32.8			24.4		
Approach LOS	E			E			C			C		
Intersection Delay	58.9			$X_c = 1.23$			Intersection LOS			E		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Hour					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	356	26	0	14	68	38	6	22	11	15	38	499
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.72	0.72	0.72	0.45	0.45	0.45	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.970	0.970			1.000			1.000			0.974	0.974
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	9	0	0	0	0	0	155
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	391	29			153			86			70	453
Lane Group Capacity, c	699	1066			1543			1234			673	616
v/c Ratio, X	0.56	0.03			0.10			0.07			0.10	0.74
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	15.6	11.4			11.8			17.3			17.5	23.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	3.1	0.0			0.1			0.1			0.3	7.4

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	18.7	11.5			12.0			17.4			17.8	31.0
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>C</i>
Approach Delay	18.2			12.0			17.4			29.2		
Approach LOS	<i>B</i>			<i>B</i>			<i>B</i>			<i>C</i>		
Intersection Delay	22.2			$X_C = 0.64$			Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	108		4				4	97			402	263
% Heavy Vehicles, %HV	11		11				2	2			1	1
Peak-Hour Factor, PHF	0.80		0.80				0.80	0.80			0.81	0.81
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.995		0.995					1.000			0.752	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	4	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	135		0					126			821	
Lane Group Capacity, c	532		461					1678			1973	
v/c Ratio, X	0.25		0.00					0.08			0.42	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	15.2		14.0					7.3			8.9	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	1.1		0.0					0.1			0.5	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	16.4		14.0					7.4			9.4	
Lane Group LOS	B		B					A			A	
Approach Delay	16.4						7.4			9.4		
Approach LOS	B						A			A		
Intersection Delay	10.0			$X_c = 0.35$			Intersection LOS			B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/8/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	446	1		3			24	220	0	
% Heavy Vehicles, %HV		2	2	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.80	0.25	0.79	0.79		0.80			0.81	0.81	0.81	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.981	0.981		0.998				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		1		283	283		4				302		
Lane Group Capacity, c		776		601	577		410				2054		
v/c Ratio, X		0.00		0.47	0.49		0.01				0.15		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.7	12.8		10.3				10.9		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.6	2.9		0.0				0.2		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		15.3	15.7		10.3				11.0	
Lane Group LOS		<i>B</i>		<i>B</i>	<i>B</i>		<i>B</i>				<i>B</i>	
Approach Delay		10.2		15.5			10.3			11.0		
Approach LOS		<i>B</i>		<i>B</i>			<i>B</i>			<i>B</i>		
Intersection Delay		13.9		$X_c = 0.32$			Intersection LOS			<i>B</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		25					
Peak-Hour Factor, PHF	1.00	0.82	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	30	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0	0	
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			201				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	257	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0	0	
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					257		
C (m) (veh/h)					1047		
v/c					0.25		
95% queue length					0.97		
Control Delay (s/veh)					9.6		
LOS					A		
Approach Delay (s/veh)	--	--	9.6				
Approach LOS	--	--	A				

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/8/13				Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Weekday 8-9 AM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	164	22	155	128	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	11	67	28			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.79				0.81	0.81	
Flow Rate (veh/h)	231		358				53	75	
% Heavy Vehicles	24		5				3	3	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.5				0.2	0.0	
Prop. Right-Turns	0.1		0.0				0.0	0.5	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.3		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.21		0.32				0.05	0.07	
hd, final value (s)	5.01		4.73				5.43	5.10	
x, final value	0.32		0.47				0.08	0.11	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	3.0		2.7				3.4	3.1	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	481		608				303	325	
Delay (s/veh)	10.36		11.85				8.90	8.71	
LOS	B		B				A	A	
Approach: Delay (s/veh)	10.36		11.85				8.79		
LOS	B		B				A		
Intersection Delay (s/veh)	10.82								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	2/8/13				Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	6	168	0		0	225	18	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	58	53	46		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.79		0.87	0.87		
Flow Rate (veh/h)	216		306		95	83		
% Heavy Vehicles	17		7		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0		0.0		0.7	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.6		
Prop. Heavy Vehicle	0.2		0.1		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.3		0.1		0.2	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.19		0.27		0.08	0.07		
hd, final value (s)	5.02		4.71		5.33	4.81		
x, final value	0.30		0.40		0.14	0.11		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	3.0		2.7		3.3	2.8		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	466		556		345	333		
Delay (s/veh)	10.18		10.82		9.20	8.41		
LOS	B		B		A	A		
Approach: Delay (s/veh)	10.18		10.82		8.83			
LOS	B		B		A			
Intersection Delay (s/veh)	10.11							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	79	89	57	6	45	1	401	77	1			
% Heavy Vehicles, %HV	3	3	3	1	1	1	3	3	3			
Peak-Hour Factor, PHF	0.79	0.79	0.79	0.79	0.79	0.79	0.76	0.76	0.76			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.998			0.978				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	21	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		213	46		66			630				
Lane Group Capacity, c		554	549		546			2253				
v/c Ratio, X		0.38	0.08		0.12			0.28				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		14.6	13.1		13.2			9.3				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		2.0	0.3		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		16.7	13.4		13.7			9.6			
Lane Group LOS		B	B		B			A			
Approach Delay		16.1			13.7			9.6			
Approach LOS		B			B			A			
Intersection Delay		11.6			$X_c = 0.32$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	72		18				16	351			478	36
% Heavy Vehicles, %HV	1		1				7	7			9	9
Peak-Hour Factor, PHF	0.74		0.74				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.997						0.936			0.808	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		121						412			598	
Lane Group Capacity, c		466						1012			1100	
v/c Ratio, X		0.26						0.41			0.54	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		17.3						7.3			8.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						1.1			1.6	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		18.7					8.5			9.7	
Lane Group LOS		B					A			A	
Approach Delay		18.7					8.5			9.7	
Approach LOS		B					A			A	
Intersection Delay		10.2			$X_c = 0.45$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	163		53				30	403			297	213
% Heavy Vehicles, %HV	14		14				8	8			11	11
Peak-Hour Factor, PHF	0.80		0.80				0.89	0.89			0.86	0.86
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.982						0.866			0.927	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	17
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		270						487			573	
Lane Group Capacity, c		513						799			911	
v/c Ratio, X		0.53						0.61			0.63	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.2						10.8			10.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		3.8						3.0			3.0	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.9					13.8			14.0	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		19.9					13.8			14.0	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		15.1		$X_c = 0.59$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		70	244	395	164	62	200			475	22
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		89		912		69	222			654	
Lane Group Capacity, c	218		216		1314		120	575			661	
v/c Ratio, X	0.02		0.41		0.69		0.57	0.39			0.99	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		23.8		19.2		16.5	15.3			19.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		5.6		2.2		17.6	1.9			16.6	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		29.4		21.4		34.1	17.2			36.5	
Lane Group LOS	C		C		C		C	B			D	
Approach Delay	29.1		21.4		21.2		36.5					
Approach LOS	C		C		C		D					
Intersection Delay	26.8		$X_c = 0.77$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0
Lane Group		LTR						LTR		L	TR	
Volume, V (vph)	12	39	11				5	250	98	260	497	32
% Heavy Vehicles, %HV	3	3	3				7	7	7	7	7	7
Peak-Hour Factor, PHF	0.85	0.85	0.85				0.90	0.90	0.90	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0	
Extension of Effective Green, e		2.0						2.0		2.0	2.0	
Arrival Type, AT		3						3		3	3	
Unit Extension, UE		3.0						3.0		3.0	3.0	
Filtering/Metering, I		1.000						0.941		0.518	0.518	
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0
Lane Width		13.0						16.0		12.0	12.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						7		0	2	
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9	
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		73						393		325	661	
Lane Group Capacity, c		457						899		516	960	
v/c Ratio, X		0.16						0.44		0.63	0.69	
Total Green Ratio, g/C		0.28						0.55		0.55	0.55	
Uniform Delay, d ₁		16.1						8.0		9.3	9.8	
Progression Factor, PF		1.000						1.000		1.000	1.000	
Delay Calibration, k		0.50						0.50		0.50	0.50	
Incremental Delay, d ₂		0.7						1.5		3.0	2.1	

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.9						9.5		12.3	11.9	
Lane Group LOS		B						A		B	B	
Approach Delay		16.9						9.5		12.0		
Approach LOS		B						A		B		
Intersection Delay		11.6		$X_c = 0.51$			Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	20	13	172	16	105	10	203	131	139	313	9
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		60		195	18	119		383			607	
Lane Group Capacity, c		648		548	777	574		695			539	
v/c Ratio, X		0.09		0.36	0.02	0.21		0.55			1.13	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.5		16.5	14.0	15.3		18.4			25.0	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		1.7	0.1	0.8		2.7			59.3	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.8	18.2	14.1	16.1	21.1	84.3
Lane Group LOS	B	B	B	B	C	F
Approach Delay	14.8	17.2	21.1	84.3		
Approach LOS	B	B	C	F		
Intersection Delay	47.7	$X_c = 0.74$	Intersection LOS	D		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	4/11/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		174	189	173	215		282		151				
% Heavy Vehicles, %HV		10	10	13	13		9		9				
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.964			0.986			0.858					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		347			436			461					
Lane Group Capacity, c		900			557			529					
v/c Ratio, X		0.39			0.78			0.87					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		13.9			18.5			25.4					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.2			10.4			15.6					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay	15.1	28.9	41.0	
Lane Group LOS	B	C	D	
Approach Delay	15.1	28.9	41.0	
Approach LOS	B	C	D	
Intersection Delay	29.5	$X_c = 0.82$	Intersection LOS	C

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charlestone EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	37	253	43	32	275	23	
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.91	0.91	0.91	
Hourly Flow Rate, HFR (veh/h)	43	297	50	35	302	25	
Percent Heavy Vehicles	8	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	24	17	39	38	19	25	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	30	21	50	43	21	28	
Percent Heavy Vehicles	1	1	1	10	10	10	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	43	35	92			101	
C (m) (veh/h)	1200	1128	307			387	
v/c	0.04	0.03	0.30			0.26	
95% queue length	0.11	0.10	1.23			1.03	
Control Delay (s/veh)	8.1	8.3	21.7			17.6	
LOS	A	A	C			C	
Approach Delay (s/veh)	--	--	21.7			17.6	
Approach LOS	--	--	C			C	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		310	29	28	324		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	364	34	30	356	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				54		24	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	57	0	25	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		30		82			
C (m) (veh/h)		1093		408			
v/c		0.03		0.20			
95% queue length		0.08		0.74			
Control Delay (s/veh)		8.4		16.0			
LOS		A		C			
Approach Delay (s/veh)	--	--	16.0				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	255	24	490	1	161	287	241	200	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			310	544		488			514	
Lane Group Capacity, c		713			523	518		708			418	
v/c Ratio, X		0.04			0.59	1.05		0.69			1.23	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.6	17.5		14.3			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			4.3	51.1		5.1			122.8	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.5			17.9	68.6		19.4			140.3	
Lane Group LOS		<i>B</i>			<i>B</i>	<i>E</i>		<i>B</i>			<i>F</i>	
Approach Delay		10.5		50.2			19.4		140.3			
Approach LOS		<i>B</i>		<i>D</i>			<i>B</i>		<i>F</i>			
Intersection Delay		66.2		$X_c = 1.14$			Intersection LOS		<i>E</i>			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	Philip Habib & Associates					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				359		3		443			461	
% Heavy Vehicles, %HV				6		6		11			12	
Peak-Hour Factor, PHF				0.88		0.88		0.92			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.858			0.858			0.567	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			4	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					411			482			536	
Lane Group Capacity, c					644			970			946	
v/c Ratio, X					0.64			0.50			0.57	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					16.9			10.0			10.5	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					4.1			1.6			1.4	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				21.1			11.5			11.9	
Lane Group LOS				C			B			B	
Approach Delay				21.1				11.5			
Approach LOS				C				B			
Intersection Delay	14.4			$X_c = 0.60$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		443	242	147	673		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	481	263	170	782	0	
Percent Heavy Vehicles	0	--	--	16	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L					
v (veh/h)		170					
C (m) (veh/h)		803					
v/c		0.21					
95% queue length		0.80					
Control Delay (s/veh)		10.7					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				102		201		473	80	188	467	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.944			0.880			0.500	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					348			601			762	
Lane Group Capacity, c					385			1058			610	
v/c Ratio, X					0.90			0.57			1.25	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.3			8.3			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					25.9			1.9			119.1	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				47.2			10.3			132.1	
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>	
Approach Delay				47.2	10.3			132.1			
Approach LOS				<i>D</i>	<i>B</i>			<i>F</i>			
Intersection Delay	72.0			$X_c = 1.14$			Intersection LOS		<i>E</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/8/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0	
Lane Group		LTR			LTR		L	TR			LTR		
Volume, V (vph)	144	67	86	39	179	64	99	607	137	115	485	70	
% Heavy Vehicles, %HV	3	3	3	2	2	2	2	2	2	6	6	6	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.92	0.92	0.92	0.91	0.91	0.91	0.89	0.89	0.89	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.649			1.000		0.732	0.732			0.973		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	14	
Lane Width		16.0			15.0		10.0	14.0			10.5		
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y	
Parking Maneuvers, N _m						5						5	
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		345			307		109	818			737		
Lane Group Capacity, c		423			550		308	1052			880		
v/c Ratio, X		0.82			0.56		0.35	0.78			0.84		
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54		
Uniform Delay, d ₁		26.9			23.9		11.6	16.2			17.2		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		10.8			4.1		2.3	4.2			9.1		

Initial Queue Delay, d_3		0.0		0.0		0.0	0.0			0.0	
Control Delay		37.7		28.0		13.9	20.4			26.3	
Lane Group LOS		D		C		B	C			C	
Approach Delay		37.7		28.0		19.6				26.3	
Approach LOS		D		C		B				C	
Intersection Delay		25.5		$X_c = 0.83$		Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	298							814			578	
% Heavy Vehicles, %HV	3							2			3	
Peak-Hour Factor, PHF	0.88							0.91			0.89	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.738			0.973	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	339							895			649	
Lane Group Capacity, c	662							2134			1951	
v/c Ratio, X	0.51							0.42			0.33	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	24.1							11.6			10.9	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	2.8							0.4			0.4	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	26.9						12.0			11.4
Lane Group LOS	C						B			B
Approach Delay	26.9						12.0		11.4	
Approach LOS	C						B		B	
Intersection Delay	14.5		$X_C = 0.45$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					578		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	649	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			91				
Peak-Hour Factor, PHF	1.00	1.00	0.84	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	108	0	0	0	
Percent Heavy Vehicles	0	0	29	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							108
C (m) (veh/h)							659
v/c							0.16
95% queue length							0.58
Control Delay (s/veh)							11.5
LOS							B
Approach Delay (s/veh)	--	--				11.5	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	150	273	148	195	539	84	184	448	316	149	67	89
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	179	463		217	678			1020		186	195	
Lane Group Capacity, c	161	599		131	1118			924		380	353	
v/c Ratio, X	1.11	0.77		1.66	0.61			1.10		0.49	0.55	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	30.0	26.9		30.0	25.1			32.5		30.5	31.0	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	98.8	8.1		324.7	2.3			62.3		4.4	6.0	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	128.8	35.1		354.7	27.3			94.8		34.9	37.1	
Lane Group LOS	F	D		F	C			F		C	D	
Approach Delay	61.2			106.7				94.8		36.0		
Approach LOS	E			F				F		D		
Intersection Delay	83.5			$X_c = 1.18$				Intersection LOS		F		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					410		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	471	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				188			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	202	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			202				
C (m) (veh/h)			548				
v/c			0.37				
95% queue length			1.69				
Control Delay (s/veh)			15.4				
LOS			C				
Approach Delay (s/veh)	--	--	15.4				
Approach LOS	--	--	C				

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	2/8/13				Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	97	128	60		130	139	90	
%Thrus Left Lane	50				50			
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	44	41	41		78	188	58	
%Thrus Left Lane					50			
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.88	0.88	0.90	0.90	0.78	0.78	1.00	1.00
Flow Rate (veh/h)	182	140	220	177	108	52	172	152
% Heavy Vehicles	42	42	4	4	3	3	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.6	0.0	0.7	0.0	0.5	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.5	0.0	0.6	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.4	0.4	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	1.0	0.4	0.4	-0.3	0.3	-0.6	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.16	0.12	0.20	0.16	0.10	0.05	0.15	0.14
hd, final value (s)	7.62	6.98	6.94	6.22	7.44	6.48	7.07	6.57
x, final value	0.39	0.27	0.42	0.31	0.22	0.09	0.34	0.28
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	5.3	4.7	4.6	3.9	5.1	4.2	4.8	4.3
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	432	390	470	427	358	302	422	402
Delay (s/veh)	15.03	12.27	14.66	11.64	12.27	9.85	13.33	11.78
LOS	C	B	B	B	B	A	B	B
Approach: Delay (s/veh)	13.83		13.31		11.48		12.60	
LOS	B		B		B		B	
Intersection Delay (s/veh)	13.02							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	62	140	509	134	316	18	281	201	88	23	528	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.889		0.932	0.932		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		847		149	371		323	332			864	
Lane Group Capacity, c		1232		196	813		196	808			1180	
v/c Ratio, X		0.69		0.76	0.46		1.65	0.41			0.73	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		20.0		21.0	17.4		25.0	17.0			20.6	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.8		22.5	1.7		313.3	1.5			4.0	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		22.8		43.4	19.1		338.3	18.5			24.6	
Lane Group LOS		C		D	B		F	B			C	
Approach Delay		22.8		26.1			176.2				24.6	
Approach LOS		C		C			F				C	
Intersection Delay		58.8		$X_c = 1.20$			Intersection LOS				E	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	687	422	3	1	443	746	0	0	1	119	0	136
% Heavy Vehicles, %HV	2	2	2	3	3	3	0	0	1	2	2	2
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.93	0.91	0.25	0.25	0.25	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.735	0.735			0.546	0.546		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	85	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	755	467			477	726		4		123	0	140
Lane Group Capacity, c	783	1158			451	383		328		233	320	952
v/c Ratio, X	0.96	0.40			1.06	1.90		0.01		0.53	0.00	0.15
Total Green Ratio, g/C	0.71	0.71			0.24	0.24		0.18		0.18	0.18	0.64
Uniform Delay, d ₁	18.2	5.3			34.0	34.0		30.5		33.6	30.4	6.3
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	20.2	0.8			47.7	408.4		0.1		8.3	0.0	0.3

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	38.4	6.0			81.7	442.4		30.6		41.9	30.4	6.6
Lane Group LOS	D	A			F	F		C		D	C	A
Approach Delay	26.0			299.4			30.6			23.1		
Approach LOS	C			F			C			C		
Intersection Delay	147.9			$X_c = 1.16$			Intersection LOS			F		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 With-Action Alt					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	504	38	0	2	65	71	4	3	0	65	6	1120
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	42			132		7	5			79	900
Lane Group Capacity, c	706	1056			1537		516	732			538	593
v/c Ratio, X	0.78	0.04			0.09		0.01	0.01			0.15	1.52
Total Green Ratio, g/C	0.50	0.50			0.50		0.39	0.39			0.39	0.39
Uniform Delay, d ₁	18.5	11.5			11.8		16.9	16.9			17.8	27.5
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	8.2	0.1			0.1		0.0	0.0			0.5	240.6

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	26.8	11.5			11.9		16.9	16.9			18.3	268.1
Lane Group LOS	C	B			B		B	B			B	F
Approach Delay	25.7		11.9		16.9		248.0					
Approach LOS	C		B		B		F					
Intersection Delay	151.2		$X_c = 1.11$		Intersection LOS		F					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	265		25				4	247			444	497
% Heavy Vehicles, %HV	10		10				4	4			2	2
Peak-Hour Factor, PHF	0.88		0.88				0.90	0.90			0.80	0.80
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.975		0.975					0.998			0.686	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	301		22					278			1176	
Lane Group Capacity, c	537		465					1663			1912	
v/c Ratio, X	0.56		0.05					0.17			0.62	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	17.0		14.2					7.7			10.3	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	4.1		0.2					0.2			1.0	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	21.1		14.4					7.9			11.3	
Lane Group LOS	C		B					A			B	
Approach Delay	20.7						7.9			11.3		
Approach LOS	C						A			B		
Intersection Delay	12.5			$X_c = 0.59$			Intersection LOS			B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/8/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	626	1		1			23	314	0	
% Heavy Vehicles, %HV		1	1	1	1		1			2	2	2	
Peak-Hour Factor, PHF		0.25	0.25	0.95	0.95		0.90			0.80	0.80	0.80	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.989				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		4		330	330		1				421		
Lane Group Capacity, c		784		612	587		352				2037		
v/c Ratio, X		0.01		0.54	0.56		0.00				0.21		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		13.2	13.3		10.2				11.2		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		3.3	3.8		0.0				0.2		

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0				0.0	
Control Delay		10.2		16.5	17.1		10.2				11.4	
Lane Group LOS		B		B	B		B				B	
Approach Delay		10.2		16.8			10.2			11.4		
Approach LOS		B		B			B			B		
Intersection Delay		14.7		$X_c = 0.38$			Intersection LOS			B		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/8/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		24					
Peak-Hour Factor, PHF	1.00	0.55	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	43	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			510				
Peak-Hour Factor, PHF	1.00	1.00	0.90	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	566	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					566		
C (m) (veh/h)					1030		
v/c					0.55		
95% queue length					3.45		
Control Delay (s/veh)					12.7		
LOS					B		
Approach Delay (s/veh)	--	--	12.7				
Approach LOS	--	--	B				

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/8/13				Analysis Year	2020 With Action Alt 2			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	62	31	227	62	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	31	79	20			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.86		0.77				0.80	0.80	
Flow Rate (veh/h)	108		374				86	73	
% Heavy Vehicles	17		4				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.8				0.4	0.0	
Prop. Right-Turns	0.3		0.0				0.0	0.3	
Prop. Heavy Vehicle	0.2		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.10		0.33				0.08	0.06	
hd, final value (s)	4.84		4.66				5.17	4.88	
x, final value	0.15		0.48				0.12	0.10	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.8		2.7				3.2	2.9	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	358		624				336	323	
Delay (s/veh)	8.66		11.96				8.90	8.42	
LOS	A		B				A	A	
Approach: Delay (s/veh)	8.66		11.96				8.68		
LOS	A		B				A		
Intersection Delay (s/veh)	10.59								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/8/13				Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Weekday 12-1 PM MD Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	40	53	0		0	247	23		
%Thrus Left Lane									
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	42	64	88		0	0	0		
%Thrus Left Lane	50								
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT		TR		LT	TR			
PHF	0.86		0.77		0.88	0.88			
Flow Rate (veh/h)	107		349		83	136			
% Heavy Vehicles	11		5		1	1			
No. Lanes	1		1		2		0		
Geometry Group	2		2		1				
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.4		0.0		0.6	0.0			
Prop. Right-Turns	0.0		0.1		0.0	0.7			
Prop. Heavy Vehicle	0.1		0.0		0.0	0.0			
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2			
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6			
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7			
hadj, computed	0.3		0.0		0.1	-0.4			
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20		3.20	3.20			
x, initial	0.10		0.31		0.07	0.12			
hd, final value (s)	5.12		4.60		5.14	4.58			
x, final value	0.15		0.45		0.12	0.17			
Move-up time, m (s)	2.0		2.0		2.0				
Service Time, t _s (s)	3.1		2.6		3.1	2.6			
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	357		599		333	386			
Delay (s/veh)	9.03		11.24		8.82	8.53			
LOS	A		B		A	A			
Approach: Delay (s/veh)	9.03		11.24		8.64				
LOS	A		B		A				
Intersection Delay (s/veh)	10.05								
Intersection LOS	B								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	149	229	155	2	53	5	574	39	2			
% Heavy Vehicles, %HV	1	1	1	5	5	5	1	1	1			
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.95	0.95	0.95	0.97	0.97	0.97			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.999			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	52	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		416	113		63			634				
Lane Group Capacity, c		574	560		533			2286				
v/c Ratio, X		0.72	0.20		0.12			0.28				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		17.0	13.6		13.2			9.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		7.8	0.8		0.5			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		24.7	14.4		13.7			9.5			
Lane Group LOS		C	B		B			A			
Approach Delay		22.5			13.7			9.5			
Approach LOS		C			B			A			
Intersection Delay		15.4			$X_c = 0.47$			Intersection LOS			B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	192		40				14	294			347	46
% Heavy Vehicles, %HV	1		1				2	2			2	2
Peak-Hour Factor, PHF	0.81		0.81				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.975						0.967			0.961	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		286						354			432	
Lane Group Capacity, c		467						1091			1168	
v/c Ratio, X		0.61						0.32			0.37	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.3						6.9			7.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		5.7						0.8			0.9	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		25.0					7.7			8.0	
Lane Group LOS		C					A			A	
Approach Delay		25.0					7.7			8.0	
Approach LOS		C					A			A	
Intersection Delay		12.4			$X_C = 0.45$		Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	87		54				44	441			308	225
% Heavy Vehicles, %HV	4		4				2	2			1	1
Peak-Hour Factor, PHF	0.86		0.86				0.87	0.87			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.986						0.905			0.957	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	14
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			2	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		164						558			570	
Lane Group Capacity, c		555						840			1000	
v/c Ratio, X		0.30						0.66			0.57	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		14.8						11.2			10.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		1.3						3.7			2.3	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		16.1					15.0			12.7	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay	16.1						15.0			12.7	
Approach LOS	<i>B</i>						<i>B</i>			<i>B</i>	
Intersection Delay	14.1			$X_C = 0.52$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		148	231	471	127	112	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		192		953		126	192			429	
Lane Group Capacity, c	222		220		1339		162	593			690	
v/c Ratio, X	0.06		0.87		0.71		0.78	0.32			0.62	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.7		25.5		19.3		18.0	14.9			16.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		30.4		2.4		28.4	1.4			3.4	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.1		55.9		21.7		46.4	16.3			20.3	
Lane Group LOS	C		E		C		D	B			C	
Approach Delay	53.8		21.7		28.2		20.3					
Approach LOS	D		C		C		C					
Intersection Delay	25.9		$X_c = 0.77$		Intersection LOS		C					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/8/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	5	15	8				6	278	116	230	475	28	
% Heavy Vehicles, %HV	2	2	2				6	6	6	1	1	1	
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.89	0.89	0.89	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.932		0.785	0.785		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	0	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						2		0	2		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		45						449		256	559		
Lane Group Capacity, c		454						925		508	1018		
v/c Ratio, X		0.10						0.49		0.50	0.55		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		15.9						8.3		8.4	8.7		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.4						1.7		2.8	1.7		

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.3						10.0		11.2	10.4	
Lane Group LOS		B						A		B	B	
Approach Delay		16.3						10.0		10.6		
Approach LOS		B						A		B		
Intersection Delay		10.6				$X_c = 0.40$		Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/8/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	5	8	17	266	26	106	22	289	171	91	372	19
% Heavy Vehicles, %HV	2	2	2	7	7	7	6	6	6	2	2	2
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.89	0.89	0.89	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.830	0.830	0.830		0.650			0.650	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		38		317	31	126		542			535	
Lane Group Capacity, c		628		547	763	563		693			618	
v/c Ratio, X		0.06		0.58	0.04	0.22		0.78			0.87	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.7	14.1	15.4		21.3			22.6	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.7	0.1	0.8		5.7			10.4	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.5	22.4	14.2	16.2	27.0	33.0	
Lane Group LOS	B	C	B	B	C	C	
Approach Delay	14.5	20.2	27.0	33.0			
Approach LOS	B	C	C	C			
Intersection Delay	26.7	$X_c = 0.72$	Intersection LOS	C			

HCS+™ DETAILED REPORT															
General Information						Site Information									
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd								
Agency or Co.	AECOM					Area Type	All other areas								
Date Performed	4/11/13					Jurisdiction	Staten Island, NY								
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 2								
						Project ID	Charleston EIS								
Volume and Timing Input															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Number of Lanes, N ₁		1	0	0	1		0		0						
Lane Group		TR			LT			LR							
Volume, V (vph)		198	189	189	226		196		174						
% Heavy Vehicles, %HV		8	8	16	16		2		2						
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88						
Pretimed (P) or Actuated (A)		P	P	P	P		P		P						
Start-up Lost Time, I ₁		2.0			2.0			2.0							
Extension of Effective Green, e		2.0			2.0			2.0							
Arrival Type, AT		3			3			3							
Unit Extension, UE		3.0			3.0			3.0							
Filtering/Metering, I		0.961			0.989			0.922							
Initial Unmet Demand, Q _b		0.0			0.0			0.0							
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0				
Lane Width		16.0			16.0			11.0							
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N			
Parking Maneuvers, N _m									5						
Buses Stopping, N _b		5			0			0							
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2				
Phasing	EW Perm	02		03		04		NB Only		06		07		08	
Timing	G = 45.0	G =		G =		G =		G = 35.0		G =		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5		Y =		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0								
Lane Group Capacity, Control Delay, and LOS Determination															
	EB			WB			NB			SB					
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
Adjusted Flow Rate, v		381			456			421							
Lane Group Capacity, c		921			515			559							
v/c Ratio, X		0.41			0.89			0.75							
Total Green Ratio, g/C		0.50			0.50			0.39							
Uniform Delay, d ₁		14.2			20.2			23.8							
Progression Factor, PF		1.000			1.000			1.000							
Delay Calibration, k		0.50			0.50			0.50							
Incremental Delay, d ₂		1.3			19.4			8.4							
Initial Queue Delay, d ₃		0.0			0.0			0.0							

Control Delay	15.5	39.6	32.2	
Lane Group LOS	B	D	C	
Approach Delay	15.5	39.6	32.2	
Approach LOS	B	D	C	
Intersection Delay	29.8	$X_c = 0.83$	Intersection LOS	C

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	29	270	88	58	289	28	
Peak-Hour Factor, PHF	0.87	0.87	0.87	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	33	310	101	71	356	34	
Percent Heavy Vehicles	6	--	--	9	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	34	18	29	68	17	34	
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.81	0.81	0.81	
Hourly Flow Rate, HFR (veh/h)	39	20	33	83	20	41	
Percent Heavy Vehicles	2	2	2	1	1	1	
Percent Grade (%)	0			0			
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR	LTR			LTR	
v (veh/h)	33	71	144			92	
C (m) (veh/h)	1147	1111	245			261	
v/c	0.03	0.06	0.59			0.35	
95% queue length	0.09	0.20	3.37			1.52	
Control Delay (s/veh)	8.2	8.5	38.7			26.1	
LOS	A	A	E			D	
Approach Delay (s/veh)	--	--	38.7			26.1	
Approach LOS	--	--	E			D	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		338	53	43	342		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	388	60	53	422	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				55		49	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	80	0	72	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		53		152			
C (m) (veh/h)		1086		369			
v/c		0.05		0.41			
95% queue length		0.15		1.96			
Control Delay (s/veh)		8.5		21.4			
LOS		A		C			
Approach Delay (s/veh)	--	--	21.4				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	11	2	301	20	380	4	167	349	300	279	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			391	463		559			674	
Lane Group Capacity, c		727			530	523		742			391	
v/c Ratio, X		0.02			0.74	0.89		0.75			1.72	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.7	16.2		14.9			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			8.3	18.3		6.4			336.3	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.4			23.1	34.5		21.3			353.8	
Lane Group LOS		B			C	C		C			F	
Approach Delay		10.4			29.2			21.3			353.8	
Approach LOS		B			C			C			F	
Intersection Delay		131.0			$X_c = 1.30$			Intersection LOS			F	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				528		4		517			583	
% Heavy Vehicles, %HV				2		2		5			12	
Peak-Hour Factor, PHF				0.84		0.84		0.93			0.86	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.373			0.921			0.340	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					634			556			678	
Lane Group Capacity, c					670			1026			954	
v/c Ratio, X					0.95			0.54			0.71	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.5			10.3			11.6	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					11.8			1.9			1.6	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				31.3			12.2			13.2	
Lane Group LOS				C			B			B	
Approach Delay				31.3				12.2			
Approach LOS				C				B			
Intersection Delay	19.0			$X_c = 0.81$			Intersection LOS			B	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		517	263	203	909		
Peak-Hour Factor, PHF	1.00	0.93	0.93	0.86	0.86	1.00	
Hourly Flow Rate, HFR (veh/h)	0	555	282	236	1056	0	
Percent Heavy Vehicles	0	--	--	18	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0		0
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L					
v (veh/h)		236					
C (m) (veh/h)		732					
v/c		0.32					
95% queue length		1.40					
Control Delay (s/veh)		12.2					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				135		180		588	106	149	747	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.901			0.798			0.114	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					375			746			1042	
Lane Group Capacity, c					397			1098			665	
v/c Ratio, X					0.94			0.68			1.57	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.6			9.2			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					31.0			2.7			256.0	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				52.6			11.9			269.0		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				52.6				11.9				269.0
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	142.8		$X_c = 1.37$		Intersection LOS			<i>F</i>				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0
Lane Group		LTR			LTR		L	TR			LTR	
Volume, V (vph)	97	53	126	35	201	73	82	593	110	143	564	53
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.81	0.81	0.81	0.91	0.91	0.91	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0	
Arrival Type, AT		3			3		3	3			3	
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.503			1.000		0.773	0.773			0.957	
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	10
Lane Width		16.0			15.0		10.0	14.0			10.5	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y
Parking Maneuvers, N _m						5						5
Buses Stopping, N _b		0			0		0	0			0	
Min. Time for Pedestrians, G _p	16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		313			381		90	773			861	
Lane Group Capacity, c		440			565		259	1067			912	
v/c Ratio, X		0.71			0.67		0.35	0.72			0.94	
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54	
Uniform Delay, d ₁		25.6			25.2		11.5	15.4			19.2	
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		4.9			6.3		2.8	3.3			18.4	

Initial Queue Delay, d_3		0.0		0.0		0.0	0.0			0.0	
Control Delay		30.5		31.5		14.4	18.8			37.6	
Lane Group LOS		C		C		B	B			D	
Approach Delay		30.5		31.5		18.3				37.6	
Approach LOS		C		C		B				D	
Intersection Delay		28.8		$X_c = 0.85$		Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	394							762			688	
% Heavy Vehicles, %HV	1							1			1	
Peak-Hour Factor, PHF	0.91							0.90			0.87	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.773			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	433							847			791	
Lane Group Capacity, c	675							2156			1990	
v/c Ratio, X	0.64							0.39			0.40	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	25.4							11.4			11.4	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	4.6							0.4			0.6	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	30.1						11.8			12.0
Lane Group LOS	C						B			B
Approach Delay	30.1						11.8		12.0	
Approach LOS	C						B		B	
Intersection Delay	15.7		$X_C = 0.49$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					688		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	790	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			71				
Peak-Hour Factor, PHF	1.00	1.00	0.78	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	91	0	0	0	
Percent Heavy Vehicles	0	0	55	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							91
C (m) (veh/h)							553
v/c							0.16
95% queue length							0.58
Control Delay (s/veh)							12.8
LOS							B
Approach Delay (s/veh)	--	--				12.8	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	162	298	206	248	388	87	223	471	267	223	77	106
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	184	522		276	566			1006		293	208	
Lane Group Capacity, c	207	594		240	1136			942		384	360	
v/c Ratio, X	0.89	0.88		1.15	0.50			1.07		0.76	0.58	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	28.4	28.3		29.5	23.3			32.5		32.8	31.2	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	30.3	12.5		103.2	1.5			49.2		13.0	6.4	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	58.8	40.8		132.7	24.8			81.7		45.8	37.7	
Lane Group LOS	<i>E</i>	<i>D</i>		<i>F</i>	<i>C</i>			<i>F</i>		<i>D</i>	<i>D</i>	
Approach Delay	45.5			60.2				81.7		42.4		
Approach LOS	<i>D</i>			<i>E</i>				<i>F</i>		<i>D</i>		
Intersection Delay	61.0			$X_c = 1.02$				Intersection LOS		<i>E</i>		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					532		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	597	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				474			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			509				
C (m) (veh/h)			464				
v/c			1.10				
95% queue length			16.91				
Control Delay (s/veh)			100.2				
LOS			F				
Approach Delay (s/veh)	--	--	100.2				
Approach LOS	--	--	F				

ALL-WAY STOP CONTROL ANALYSIS								
General Information				Site Information				
Analyst	KM			Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Bricktown Way</i>				North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R	L	T	R	L	R
Volume (veh/h)	93	161	55	148	197	86		
%Thrus Left Lane	50			50				
Approach	Northbound				Southbound			
Movement	L	T	R	L	T	R	L	R
Volume (veh/h)	16	39	69	80	193	60		
%Thrus Left Lane				50				
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT	TR	LT	TR	LT	R	LT	TR
PHF	0.87	0.87	0.91	0.91	0.76	0.76	1.00	1.00
Flow Rate (veh/h)	197	156	269	202	72	90	176	157
% Heavy Vehicles	7	7	1	1	1	1	0	0
No. Lanes	2		2		2		2	
Geometry Group	5		5		5		5	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.5	0.0	0.6	0.0	0.3	0.0	0.5	0.0
Prop. Right-Turns	0.0	0.4	0.0	0.5	0.0	1.0	0.0	0.4
Prop. Heavy Vehicle	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.0
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.4	-0.2	0.3	-0.3	0.2	-0.7	0.2	-0.3
Departure Headway and Service Time								
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20
x, initial	0.18	0.14	0.24	0.18	0.06	0.08	0.16	0.14
hd, final value (s)	7.15	6.60	6.93	6.30	7.52	6.67	7.25	6.75
x, final value	0.39	0.29	0.52	0.35	0.15	0.17	0.35	0.29
Move-up time, m (s)	2.3		2.3		2.3		2.3	
Service Time, t _s (s)	4.9	4.3	4.6	4.0	5.2	4.4	4.9	4.5
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	447	406	507	452	322	340	426	407
Delay (s/veh)	14.38	11.93	16.82	12.40	11.54	10.70	13.88	12.25
LOS	B	B	C	B	B	B	B	B
Approach: Delay (s/veh)	13.30		14.92		11.08		13.11	
LOS	B		B		B		B	
Intersection Delay (s/veh)	13.56							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	53	154	557	123	334	10	271	145	124	22	534	124
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.884		0.927	0.927		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		868		150	419		291	289			723	
Lane Group Capacity, c		1248		190	824		258	804			1320	
v/c Ratio, X		0.70		0.79	0.51		1.13	0.36			0.55	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		20.1		21.4	17.9		25.0	16.5			18.4	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		2.9		25.9	2.1		94.9	1.3			1.6	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		23.0		47.3	20.0		119.9	17.8			20.0	
Lane Group LOS		C		D	C		F	B			B	
Approach Delay		23.0		27.2			69.0				20.0	
Approach LOS		C		C			E				B	
Intersection Delay		32.8		$X_c = 0.96$			Intersection LOS				C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	0	1	1	0	1	0	1	1	1
Lane Group	L	TR			LT	R		LTR		L	LT	R
Volume, V (vph)	764	388	4	3	398	814	2	3	2	146	0	289
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.44	0.44	0.44	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0	2.0		2.0		2.0	2.0	2.0
Arrival Type, AT	3	3			3	3		3		3	3	3
Unit Extension, UE	3.0	3.0			3.0	3.0		3.0		3.0	3.0	3.0
Filtering/Metering, I	0.687	0.687			0.650	0.650		1.000		1.000	1.000	1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0	12.0		16.0		10.0	11.0	10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0		0		0	0	0
Min. Time for Pedestrians, G _p	14.6			24.6			20.9			20.1		
Phasing	EB Only	EW Perm	03	04	NS Perm	06	07	08				
Timing	G = 33.0	G = 28.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	849	435			422	857		17		182	0	361
Lane Group Capacity, c	780	1206			584	497		148		204	283	862
v/c Ratio, X	1.09	0.36			0.72	1.72		0.11		0.89	0.00	0.42
Total Green Ratio, g/C	0.73	0.73			0.31	0.31		0.16		0.16	0.16	0.58
Uniform Delay, d ₁	19.6	4.4			27.5	31.0		32.7		37.3	32.1	10.6
Progression Factor, PF	1.000	1.000			1.000	1.000		1.000		1.000	1.000	1.000
Delay Calibration, k	0.50	0.50			0.50	0.50		0.50		0.50	0.50	0.50
Incremental Delay, d ₂	54.1	0.6			5.0	331.5		1.6		40.0	0.0	1.5

Initial Queue Delay, d_3	0.0	0.0			0.0	0.0		0.0		0.0	0.0	0.0
Control Delay	73.8	4.9			32.6	362.5		34.2		77.3	32.1	12.1
Lane Group LOS	<i>E</i>	<i>A</i>			<i>C</i>	<i>F</i>		<i>C</i>		<i>E</i>	<i>C</i>	<i>B</i>
Approach Delay	50.4			253.6			34.2			33.9		
Approach LOS	<i>D</i>			<i>F</i>			<i>C</i>			<i>C</i>		
Intersection Delay	130.7			$X_c = 1.18$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	499	37	0	1	37	43	3	3	2	62	1	1176
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	41			75			16			67	938
Lane Group Capacity, c	753	1066			1563			1220			548	616
v/c Ratio, X	0.74	0.04			0.05			0.01			0.12	1.52
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	17.8	11.5			11.5			16.9			17.6	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	6.0	0.1			0.1			0.0			0.4	242.9

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	23.8	11.5			11.6			16.9			18.1	270.4
Lane Group LOS	<i>C</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>F</i>
Approach Delay	23.0			11.6			16.9			253.6		
Approach LOS	<i>C</i>			<i>B</i>			<i>B</i>			<i>F</i>		
Intersection Delay	159.5			$X_C = 1.08$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	9/2/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	287		24				2	297			443	428
% Heavy Vehicles, %HV	8		8				1	1			2	2
Peak-Hour Factor, PHF	0.87		0.87				0.88	0.88			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.974		0.974					0.995			0.791	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	6	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	330		21					340			957	
Lane Group Capacity, c	547		473					1731			1924	
v/c Ratio, X	0.60		0.04					0.20			0.50	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	17.3		14.2					7.8			9.4	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	4.7		0.2					0.3			0.7	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	22.1		14.4					8.1			10.2	
Lane Group LOS	C		B					A			B	
Approach Delay	21.6						8.1			10.2		
Approach LOS	C						A			B		
Intersection Delay	12.2			$X_c = 0.54$			Intersection LOS			B		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/9/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	520	2		4			23	351	4	
% Heavy Vehicles, %HV		1	1	3	3		1			1	1	1	
Peak-Hour Factor, PHF		0.50	0.25	0.89	0.89		0.88			0.91	0.91	0.91	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.983	0.983		0.988				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		2		292	294		5				415		
Lane Group Capacity, c		784		601	577		355				2055		
v/c Ratio, X		0.00		0.49	0.51		0.01				0.20		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		12.8	13.0		10.3				11.1		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		2.7	3.1		0.1				0.2		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	15.5	16.1	10.3	11.4
Lane Group LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	15.8	10.3	11.4	
Approach LOS	<i>B</i>	<i>B</i>	<i>B</i>	<i>B</i>	
Intersection Delay	14.0	$X_c = 0.36$	Intersection LOS	<i>B</i>	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		24					
Peak-Hour Factor, PHF	1.00	0.71	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	33	0	0	0	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	0		0
Configuration		T					
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			579				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	657	0	0	0	
Percent Heavy Vehicles	0	0	1	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration					R		
v (veh/h)					657		
C (m) (veh/h)					1043		
v/c					0.63		
95% queue length					4.66		
Control Delay (s/veh)					14.1		
LOS					B		
Approach Delay (s/veh)	--	--	14.1				
Approach LOS	--	--	B				

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/9/13				Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Weekday 5-6 PM Peak Period								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound					Westbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	125	39	250	95	0			
%Thrus Left Lane									
Approach	Northbound					Southbound			
Movement	L	T	R	L	T	R	L	T	R
Volume (veh/h)	0	0	0	24	89	25			
%Thrus Left Lane				50					
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	TR		LT				LT	TR	
PHF	0.80		0.78				0.91	0.91	
Flow Rate (veh/h)	204		441				74	76	
% Heavy Vehicles	2		3				1	1	
No. Lanes	1		1		0		2		
Geometry Group	2		2				1		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.0		0.7				0.4	0.0	
Prop. Right-Turns	0.2		0.0				0.0	0.4	
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0	
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2	
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6	
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7	
hadj, computed	-0.1		0.2				0.1	-0.2	
Departure Headway and Service Time									
hd, initial value (s)	3.20		3.20				3.20	3.20	
x, initial	0.18		0.39				0.07	0.07	
hd, final value (s)	4.74		4.75				5.52	5.24	
x, final value	0.27		0.58				0.11	0.11	
Move-up time, m (s)	2.0		2.0				2.0		
Service Time, t _s (s)	2.7		2.7				3.5	3.2	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	454		691				324	326	
Delay (s/veh)	9.46		14.14				9.23	8.89	
LOS	A		B				A	A	
Approach: Delay (s/veh)	9.46		14.14				9.06		
LOS	A		B				A		
Intersection Delay (s/veh)	11.98								
Intersection LOS	B								

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	2/9/13				Analysis Year	2020 With-Action Alt		
Analysis Time Period	Weekday 5-6 PM Peak Period							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	39	110	0		0	280	19	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	66	100	142		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT	TR		
PHF	0.80		0.78		0.99	0.99		
Flow Rate (veh/h)	185		382		116	193		
% Heavy Vehicles	2		5		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.3		0.0		0.6	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		0.0		0.1	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.16		0.34		0.10	0.17		
hd, final value (s)	5.27		4.97		5.48	4.91		
x, final value	0.27		0.53		0.18	0.26		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t _s (s)	3.3		3.0		3.5	2.9		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	435		632		366	443		
Delay (s/veh)	10.22		13.39		9.65	9.66		
LOS	B		B		A	A		
Approach: Delay (s/veh)	10.22		13.39		9.65			
LOS	B		B		A			
Intersection Delay (s/veh)	11.40							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	193	251	158	8	60	9	462	106	2			
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1			
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.87	0.87	0.87	0.89	0.89	0.89			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.999	0.999		0.996			0.980				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	53	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		522	124		88			640				
Lane Group Capacity, c		557	560		528			2300				
v/c Ratio, X		0.94	0.22		0.17			0.28				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		18.9	13.7		13.5			9.3				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		25.4	0.9		0.7			0.3				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		44.2	14.7		14.1			9.5			
Lane Group LOS		D	B		B			A			
Approach Delay		38.6			14.1			9.5			
Approach LOS		D			B			A			
Intersection Delay		23.5			$X_c = 0.56$			Intersection LOS			C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	219		34				18	469			472	58
% Heavy Vehicles, %HV	3		3				1	1			4	4
Peak-Hour Factor, PHF	0.96		0.96				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.984						0.880			0.893	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		263						566			576	
Lane Group Capacity, c		459						1098			1124	
v/c Ratio, X		0.57						0.52			0.51	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		19.0						8.0			7.9	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		5.0						1.5			1.5	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay	24.1			9.5		9.4
Lane Group LOS	C			A		A
Approach Delay	24.1			9.5		9.4
Approach LOS	C			A		A
Intersection Delay	12.2	$X_C = 0.53$		Intersection LOS		B

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	166		86				23	576			419	280
% Heavy Vehicles, %HV	1		1				1	1			3	3
Peak-Hour Factor, PHF	0.80		0.80				0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.911						0.834			0.880	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	21
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						2			7	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		314						697			737	
Lane Group Capacity, c		574						889			964	
v/c Ratio, X		0.55						0.78			0.76	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.3						12.3			12.1	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		3.4						5.8			5.1	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.7					18.1			17.2	
Lane Group LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Approach Delay		19.7					18.1			17.2	
Approach LOS		<i>B</i>					<i>B</i>			<i>B</i>	
Intersection Delay		18.0		$X_c = 0.69$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		162	284	466	276	99	207			501	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		171		1167		108	225			582	
Lane Group Capacity, c	222		220		1329		124	616			670	
v/c Ratio, X	0.02		0.78		0.88		0.87	0.37			0.87	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		25.1		20.5		18.8	15.2			18.8	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		20.2		4.5		49.3	1.6			12.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		45.3		25.0		68.1	16.7			31.4	
Lane Group LOS	C		D		C		E	B			C	
Approach Delay	44.8		25.0		33.4		31.4					
Approach LOS	D		C		C		C					
Intersection Delay	29.4		$X_c = 0.86$		Intersection LOS		C					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/9/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	10	20	6				9	296	127	319	576	52	
% Heavy Vehicles, %HV	2	2	2				2	2	2	2	2	2	
Peak-Hour Factor, PHF	0.63	0.63	0.63				0.92	0.92	0.92	0.93	0.93	0.93	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.922		0.564	0.564		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	12	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						2		0	7		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		58						470		343	662		
Lane Group Capacity, c		460						955		490	986		
v/c Ratio, X		0.13						0.49		0.70	0.67		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		16.0						8.3		9.9	9.6		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		0.6						1.7		4.7	2.1		

Initial Queue Delay, d_3		0.0						0.0		0.0	0.0	
Control Delay		16.5						10.0		14.6	11.7	
Lane Group LOS		B						B		B	B	
Approach Delay		16.5						10.0			12.7	
Approach LOS		B						B			B	
Intersection Delay		12.0			$X_c = 0.51$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	13	20	267	29	110	18	315	175	94	452	10
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		57		307	33	126		552			639	
Lane Group Capacity, c		634		560	792	585		732			622	
v/c Ratio, X		0.09		0.55	0.04	0.22		0.75			1.03	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.5		18.4	14.2	15.4		20.9			25.0	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.5	0.1	0.8		4.8			24.7	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.7	21.8	14.2	16.1	25.7	49.7
Lane Group LOS	B	C	B	B	C	D
Approach Delay	14.7	19.8	25.7	49.7		
Approach LOS	B	B	C	D		
Intersection Delay	32.7	$X_c = 0.79$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	4/11/13					Jurisdiction	Staten Island, NY						
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		203	253	208	224		158		200				
% Heavy Vehicles, %HV		5	5	8	8		1		1				
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.989			0.917					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		5			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		485			521			535					
Lane Group Capacity, c		939			470			560					
v/c Ratio, X		0.52			1.11			0.96					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		15.2			22.5			26.7					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		1.9			74.3			26.9					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay	17.0	96.8	53.7	
Lane Group LOS	<i>B</i>	<i>F</i>	<i>D</i>	
Approach Delay	17.0	96.8	53.7	
Approach LOS	<i>B</i>	<i>F</i>	<i>D</i>	
Intersection Delay	56.7	$X_C = 1.04$	Intersection LOS	<i>E</i>

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Sharrotts Rd & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Sharrotts Road</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)	12	282	64	29	328	28	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.88	0.88	0.88	
Hourly Flow Rate, HFR (veh/h)	13	306	69	32	372	31	
Percent Heavy Vehicles	3	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration	LTR			LTR			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	27	33	24	68	17	41	
Peak-Hour Factor, PHF	0.40	0.40	0.40	0.78	0.78	0.78	
Hourly Flow Rate, HFR (veh/h)	67	82	59	87	21	52	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	1	0	0	1		0
Configuration		LTR			LTR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration	LTR	LTR		LTR			LTR
v (veh/h)	13	32		160			208
C (m) (veh/h)	1150	1167		257			315
v/c	0.01	0.03		0.62			0.66
95% queue length	0.03	0.08		3.77			4.39
Control Delay (s/veh)	8.2	8.2		39.6			36.1
LOS	A	A		E			E
Approach Delay (s/veh)	--	--		39.6			36.1
Approach LOS	--	--		E			E

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		321	33	33	387		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	348	35	37	439	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	0	0	1	0	
Configuration			TR	LT			
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				30		36	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	40	0	48	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration					LR		
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT		LR			
v (veh/h)		37		88			
C (m) (veh/h)		1159		437			
v/c		0.03		0.20			
95% queue length		0.10		0.74			
Control Delay (s/veh)		8.2		15.3			
LOS		A		C			
Approach Delay (s/veh)	--	--	15.3				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	10	5	356	27	538	5	193	426	306	243	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			416	585		685			603	
Lane Group Capacity, c		726			547	539		744			325	
v/c Ratio, X		0.02			0.76	1.09		0.92			1.86	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.9	17.5		16.6			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.1			8.2	61.1		16.5			396.6	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0			0.0	
Control Delay		10.4			23.2	78.6		33.1			414.1	
Lane Group LOS		B			C	E		C			F	
Approach Delay		10.4		55.6			33.1		414.1			
Approach LOS		B		E			C		F			
Intersection Delay		142.4		$X_c = 1.47$			Intersection LOS		F			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	No Bridge St & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1			1	
Lane Group					LR			T			T	
Volume, V (vph)				510		9		616			606	
% Heavy Vehicles, %HV				1		1		5			4	
Peak-Hour Factor, PHF				0.86		0.86		0.91			0.91	
Pretimed (P) or Actuated (A)				P		P		P			P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.483			0.865			0.367	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0		0	0	
Lane Width					16.0			16.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b					0			0			2	
Min. Time for Pedestrians, G _p	13.5			13.8			3.2			3.2		
Phasing	WB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					603			677			666	
Lane Group Capacity, c					676			1026			1027	
v/c Ratio, X					0.89			0.66			0.65	
Total Green Ratio, g/C					0.33			0.50			0.50	
Uniform Delay, d ₁					19.0			11.2			11.1	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					9.0			2.9			1.2	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				27.9			14.1			12.3	
Lane Group LOS				C			B			B	
Approach Delay				27.9				14.1			
Approach LOS				C				B			
Intersection Delay	17.8		$X_c = 0.75$		Intersection LOS			B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	So Bridge St & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		616	270	192	921		
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	676	296	210	1012	0	
Percent Heavy Vehicles	0	--	--	6	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	1	1	1	1	0	
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)							
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	0	0	0	
Configuration							
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L					
v (veh/h)		210					
C (m) (veh/h)		693					
v/c		0.30					
95% queue length		1.28					
Control Delay (s/veh)		12.4					
LOS		B					
Approach Delay (s/veh)	--	--					
Approach LOS	--	--					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	0	1	
Lane Group					LR			TR			LT	
Volume, V (vph)				108		222		645	94	157	735	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0			2.0	
Extension of Effective Green, e					2.0			2.0			2.0	
Arrival Type, AT					3			3			3	
Unit Extension, UE					3.0			3.0			3.0	
Filtering/Metering, I					0.940			0.789			0.272	
Initial Unmet Demand, Q _b					0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0			15.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2			2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					370			812			981	
Lane Group Capacity, c					393			1132			643	
v/c Ratio, X					0.94			0.72			1.53	
Total Green Ratio, g/C					0.27			0.57			0.57	
Uniform Delay, d ₁					21.5			9.5			13.0	
Progression Factor, PF					1.000			1.000			1.000	
Delay Calibration, k					0.50			0.50			0.50	
Incremental Delay, d ₂					31.5			3.1			238.7	
Initial Queue Delay, d ₃					0.0			0.0			0.0	

Control Delay				53.1			12.6			251.7		
Lane Group LOS				<i>D</i>			<i>B</i>			<i>F</i>		
Approach Delay				53.1				12.6				251.7
Approach LOS				<i>D</i>				<i>B</i>				<i>F</i>
Intersection Delay	128.0			$X_c = 1.34$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Richmond Valley Rd & Page Ave						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/9/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0	0	1	0	1	1	0	0	2	0	
Lane Group		LTR			LTR		L	TR			LTR		
Volume, V (vph)	103	52	116	31	95	73	163	783	144	58	622	101	
% Heavy Vehicles, %HV	0	0	0	1	1	1	1	1	1	1	1	1	
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.73	0.73	0.73	0.92	0.92	0.92	0.92	0.92	0.92	
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0			2.0		2.0	2.0			2.0		
Extension of Effective Green, e		2.0			2.0		2.0	2.0			2.0		
Arrival Type, AT		3			3		3	3			3		
Unit Extension, UE		3.0			3.0		3.0	3.0			3.0		
Filtering/Metering, I		0.533			1.000		0.561	0.561			0.957		
Initial Unmet Demand, Q _b		0.0			0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	20	
Lane Width		16.0			15.0		10.0	14.0			10.5		
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	Y	
Parking Maneuvers, N _m						5						5	
Buses Stopping, N _b		0			0		0	0			0		
Min. Time for Pedestrians, G _p		16.6			21.5			16.3			21.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 31.0	G =	G =	G =	G = 49.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		348			272		177	1008			827		
Lane Group Capacity, c		487			538		272	1067			959		
v/c Ratio, X		0.71			0.51		0.65	0.94			0.86		
Total Green Ratio, g/C		0.34			0.34		0.54	0.54			0.54		
Uniform Delay, d ₁		25.7			23.4		14.5	19.2			17.6		
Progression Factor, PF		1.000			1.000		1.000	1.000			1.000		
Delay Calibration, k		0.50			0.50		0.50	0.50			0.50		
Incremental Delay, d ₂		4.8			3.4		6.6	11.2			9.7		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0
Control Delay	30.4	26.8	21.1	30.4
Lane Group LOS	C	C	C	C
Approach Delay	30.4	26.8	29.0	27.3
Approach LOS	C	C	C	C
Intersection Delay	28.4	$X_c = 0.86$	Intersection LOS	C

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	S Br St & Page Ave/Boscome Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1							2			2	
Lane Group	L							T			T	
Volume, V (vph)	405							959			723	
% Heavy Vehicles, %HV	2							2			3	
Peak-Hour Factor, PHF	0.86							0.96			0.92	
Pretimed (P) or Actuated (A)	P							P			P	
Start-up Lost Time, I ₁	2.0							2.0			2.0	
Extension of Effective Green, e	2.0							2.0			2.0	
Arrival Type, AT	3							3			3	
Unit Extension, UE	3.0							3.0			3.0	
Filtering/Metering, I	1.000							0.567			0.957	
Initial Unmet Demand, Q _b	0.0							0.0			0.0	
Ped / Bike / RTOR Volumes	0	0		0	0		0	0		0	0	
Lane Width	16.0							14.5			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0							0			0	
Min. Time for Pedestrians, G _p	3.2			20.1			3.2			3.2		
Phasing	EB Only	02	03	04	Thru Only	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 50.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	471							999			786	
Lane Group Capacity, c	669							2134			1951	
v/c Ratio, X	0.70							0.47			0.40	
Total Green Ratio, g/C	0.33							0.56			0.56	
Uniform Delay, d ₁	26.1							12.0			11.5	
Progression Factor, PF	1.000							1.000			1.000	
Delay Calibration, k	0.50							0.50			0.50	
Incremental Delay, d ₂	6.1							0.4			0.6	
Initial Queue Delay, d ₃	0.0							0.0			0.0	

Control Delay	32.3						12.4			12.0
Lane Group LOS	C						B			B
Approach Delay	32.3						12.4		12.0	
Approach LOS	C						B		B	
Intersection Delay	16.4		$X_C = 0.56$		Intersection LOS		B			

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	S Bridge St RT & Page Ave		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>South Bridge Street Right Turn</i>				North/South Street: <i>Page Avenue</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					723		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.92	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	785	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Raised curb						
RT Channelized			0				0
Lanes	0	0	0	0	2		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)			57				
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	64	0	0	0	
Percent Heavy Vehicles	0	0	5	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	1	0	0		0
Configuration			R				
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration							R
v (veh/h)							64
C (m) (veh/h)							650
v/c							0.10
95% queue length							0.33
Control Delay (s/veh)							11.1
LOS							B
Approach Delay (s/veh)	--	--				11.1	
Approach LOS	--	--				B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD					Analysis Year	2020 With-Action Alt 2					
	Peak					Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	176	354	221	273	536	102	277	705	293	243	140	160
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	189	567		284	804			1417		289	357	
Lane Group Capacity, c	119	596		83	1184			966		384	359	
v/c Ratio, X	1.59	0.95		3.42	0.68			1.47		0.75	0.99	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	30.0	29.3		29.5	25.2			32.5		32.7	34.9	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	290.7	21.1		1117	2.8			215.8		11.1	42.6	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	320.7	50.4		1146	28.1			248.3		43.8	77.5	
Lane Group LOS	F	D		F	C			F		D	E	
Approach Delay	117.9			320.0				248.3		62.4		
Approach LOS	F			F				F		E		
Intersection Delay	212.3			$X_c = 2.15$				Intersection LOS		F		

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					633		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	680	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				236			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	262	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			262				
C (m) (veh/h)			418				
v/c			0.63				
95% queue length			4.15				
Control Delay (s/veh)			27.0				
LOS			D				
Approach Delay (s/veh)	--	--	27.0				
Approach LOS	--	--	D				

ALL-WAY STOP CONTROL ANALYSIS									
General Information					Site Information				
Analyst	KM				Intersection	Bricktown Way/Tyrellan Avenue			
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY			
Date Performed	2/9/13				Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd								
Project ID <i>Charleston EIS</i>									
East/West Street: <i>Bricktown Way</i>					North/South Street: <i>Tyrellan Avenue</i>				
Volume Adjustments and Site Characteristics									
Approach	Eastbound				Westbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	138	265	59		111	258	128		
%Thrus Left Lane	50				50				
Approach	Northbound				Southbound				
Movement	L	T	R		L	T	R		
Volume (veh/h)	61	59	95		111	269	83		
%Thrus Left Lane					50				
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Configuration	LT	TR	LT	TR	LT	R	LT	TR	
PHF	0.85	0.85	0.91	0.91	0.89	0.89	0.92	0.92	
Flow Rate (veh/h)	317	225	262	281	134	106	265	236	
% Heavy Vehicles	0	0	0	1	1	1	0	0	
No. Lanes	2		2		2		2		
Geometry Group	5		5		5		5		
Duration, T	0.25								
Saturation Headway Adjustment Worksheet									
Prop. Left-Turns	0.5	0.0	0.5	0.0	0.5	0.0	0.5	0.0	
Prop. Right-Turns	0.0	0.3	0.0	0.5	0.0	1.0	0.0	0.4	
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
hLT-adj	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	
hRT-adj	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	-0.7	
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7	
hadj, computed	0.3	-0.2	0.2	-0.3	0.3	-0.7	0.2	-0.3	
Departure Headway and Service Time									
hd, initial value (s)	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	
x, initial	0.28	0.20	0.23	0.25	0.12	0.09	0.24	0.21	
hd, final value (s)	8.21	7.74	8.20	7.64	8.92	7.97	8.33	7.84	
x, final value	0.72	0.48	0.60	0.60	0.33	0.23	0.61	0.51	
Move-up time, m (s)	2.3		2.3		2.3		2.3		
Service Time, t _s (s)	5.9	5.4	5.9	5.3	6.6	5.7	6.0	5.5	
Capacity and Level of Service									
	Eastbound		Westbound		Northbound		Southbound		
	L1	L2	L1	L2	L1	L2	L1	L2	
Capacity (veh/h)	433	456	432	463	384	356	424	449	
Delay (s/veh)	29.53	17.47	22.33	21.00	16.00	13.09	23.37	18.53	
LOS	D	C	C	C	C	B	C	C	
Approach: Delay (s/veh)	24.52		21.64		14.71		21.09		
LOS	C		C		B		C		
Intersection Delay (s/veh)	21.44								
Intersection LOS	C								

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	2	0
Lane Group		LTR		DefL	TR		DefL	TR			LTR	
Volume, V (vph)	82	175	593	165	432	25	322	269	115	26	834	168
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT		3		3	3		3	3			3	
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I		0.830		0.865	0.865		1.000	1.000			1.000	
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		13.0	13.0			9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b		0		0	0		0	0			0	
Min. Time for Pedestrians, G _p		18.6		16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		924		212	586		343	422			1106	
Lane Group Capacity, c		1142		172	829		128	825			1346	
v/c Ratio, X		0.81		1.23	0.71		2.68	0.51			0.82	
Total Green Ratio, g/C		0.44		0.44	0.44		0.44	0.44			0.44	
Uniform Delay, d ₁		21.7		25.0	20.3		25.0	18.0			21.9	
Progression Factor, PF		1.000		1.000	1.000		1.000	1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50		0.50	0.50			0.50	
Incremental Delay, d ₂		5.2		140.4	4.4		777.7	2.3			5.8	

Initial Queue Delay, d_3		0.0		0.0	0.0		0.0	0.0			0.0	
Control Delay		26.9		165.4	24.6		802.7	20.2			27.6	
Lane Group LOS		C		F	C		F	C			C	
Approach Delay		26.9		62.0			371.1				27.6	
Approach LOS		C		E			F				C	
Intersection Delay		108.2		$X_c = 1.96$			Intersection LOS				F	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	892	472			508	1077				233		216
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 22.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	920	492			540	988				253		235
Lane Group Capacity, c	895	1170			460	391				576		962
v/c Ratio, X	1.03	0.42			1.17	2.53				0.44		0.24
Total Green Ratio, g/C	0.72	0.71			0.24	0.24				0.18		0.64
Uniform Delay, d ₁	21.4	5.4			34.0	34.0				33.0		6.8
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	28.9	0.6			82.7	688.4				2.4		0.6
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

Control Delay	50.3	5.9			116.7	722.4				35.4		7.4
Lane Group LOS	<i>D</i>	<i>A</i>			<i>F</i>	<i>F</i>				<i>D</i>		<i>A</i>
Approach Delay	34.8			508.3						21.9		
Approach LOS	<i>C</i>			<i>F</i>						<i>C</i>		
Intersection Delay	244.1			$X_c = 1.32$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	660	46	0	0	46	58	0	0	1	84	0	1539
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	688	48			99			1			90	1227
Lane Group Capacity, c	736	1066			1633			1184			544	622
v/c Ratio, X	0.93	0.05			0.06			0.00			0.17	1.97
Total Green Ratio, g/C	0.50	0.50			0.50			0.39			0.39	0.39
Uniform Delay, d ₁	21.1	11.5			11.6			16.8			18.0	27.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	20.1	0.1			0.1			0.0			0.6	442.7

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	41.2	11.6			11.7			16.8			18.5	470.2
Lane Group LOS	<i>D</i>	<i>B</i>			<i>B</i>			<i>B</i>			<i>B</i>	<i>F</i>
Approach Delay	39.3			11.7			16.8			439.3		
Approach LOS	<i>D</i>			<i>B</i>			<i>B</i>			<i>F</i>		
Intersection Delay	282.7			$X_c = 1.39$			Intersection LOS			<i>F</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Bricktown Ctr Rd @ Vets Rd W					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1				0	2			2	0
Lane Group	L		R					LT			TR	
Volume, V (vph)	436		36				5	311			587	684
% Heavy Vehicles, %HV	3		3				1	1			1	1
Peak-Hour Factor, PHF	0.85		0.85				0.88	0.88			0.89	0.89
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁	2.0		2.0					2.0			2.0	
Extension of Effective Green, e	2.0		2.0					2.0			2.0	
Arrival Type, AT	3		3					3			3	
Unit Extension, UE	3.0		3.0					3.0			3.0	
Filtering/Metering, I	0.892		0.892					0.995			0.487	
Initial Unmet Demand, Q _b	0.0		0.0					0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	9	0	0		0	0		0	0	0
Lane Width	13.0		12.0					11.5			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0					0			0	
Min. Time for Pedestrians, G _p	21.8			20.3			3.2			19.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 19.0	G =	G =	G =	G = 31.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	513		32					359			1429	
Lane Group Capacity, c	573		497					1694			1928	
v/c Ratio, X	0.90		0.06					0.21			0.74	
Total Green Ratio, g/C	0.32		0.32					0.52			0.52	
Uniform Delay, d ₁	19.6		14.3					7.9			11.4	
Progression Factor, PF	1.000		1.000					1.000			1.000	
Delay Calibration, k	0.50		0.50					0.50			0.50	
Incremental Delay, d ₂	17.5		0.2					0.3			1.3	
Initial Queue Delay, d ₃	0.0		0.0					0.0			0.0	

Control Delay	37.0		14.5					8.2			12.6	
Lane Group LOS	<i>D</i>		<i>B</i>					<i>A</i>			<i>B</i>	
Approach Delay	35.7							8.2		12.6		
Approach LOS	<i>D</i>							<i>A</i>		<i>B</i>		
Intersection Delay	17.3			$X_c = 0.80$				Intersection LOS		<i>B</i>		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/9/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	798	2		5			28	474	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.75	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		1		658	221		6				565		
Lane Group Capacity, c		784		607	584		289				2037		
v/c Ratio, X		0.00		1.08	0.38		0.02				0.28		
Total Green Ratio, g/C		0.42		0.42	0.42		0.42				0.42		
Uniform Delay, d ₁		10.2		17.5	12.1		10.3				11.5		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		60.9	1.8		0.1				0.3		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	10.2	78.4	13.9	10.4	11.9
Lane Group LOS	<i>B</i>	<i>E</i>	<i>B</i>	<i>B</i>	<i>B</i>
Approach Delay	10.2	62.2	10.4	11.9	
Approach LOS	<i>B</i>	<i>E</i>	<i>B</i>	<i>B</i>	
Intersection Delay	42.3	$X_c = 0.68$	Intersection LOS	<i>D</i>	

TWO-WAY STOP CONTROL SUMMARY								
General Information				Site Information				
Analyst	KM			Intersection	Englewood Ave & Vets Rd W RT			
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY			
Date Performed	2/9/13			Analysis Year	2020 With-Action Alt 2			
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project Description <i>Charleston EIS</i>								
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Veterans Rd W Right Turn Bay</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		29						
Peak-Hour Factor, PHF	1.00	0.75	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	38	0	0	0	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	0		0	
Configuration		T						
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			742					
Peak-Hour Factor, PHF	1.00	1.00	0.88	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	843	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		0			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	0	1	0	0		0	
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					843			
C (m) (veh/h)					1040			
v/c					0.81			
95% queue length					9.31			
Control Delay (s/veh)					21.3			
LOS					C			
Approach Delay (s/veh)	--	--	21.3					
Approach LOS	--	--	C					

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd & Veterans Rd W		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	2/9/13				Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road West</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	0	91	36		347	66	0	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	0	0	0		17	120	23	
%Thrus Left Lane					50			
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	TR		LT				LT TR	
PHF	0.75		0.85				0.89	0.89
Flow Rate (veh/h)	169		485				86	92
% Heavy Vehicles	2		4				0	0
No. Lanes	1		1		0		2	
Geometry Group	2		2				1	
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.0		0.8				0.2	0.0
Prop. Right-Turns	0.3		0.0				0.0	0.3
Prop. Heavy Vehicle	0.0		0.0				0.0	0.0
hLT-adj	0.2	0.2	0.2	0.2			0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6			-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7			1.7	1.7
hadj, computed	-0.1		0.2				0.0	-0.2
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20				3.20	3.20
x, initial	0.15		0.43				0.08	0.08
hd, final value (s)	4.86		4.83				5.54	5.33
x, final value	0.23		0.65				0.13	0.14
Move-up time, m (s)	2.0		2.0				2.0	
Service Time, t _s (s)	2.9		2.8				3.5	3.3
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	419		735				336	342
Delay (s/veh)	9.29		16.37				9.38	9.17
LOS	A		C				A	A
Approach: Delay (s/veh)	9.29		16.37				9.27	
LOS	A		C				A	
Intersection Delay (s/veh)	13.41							
Intersection LOS	B							

ALL-WAY STOP CONTROL ANALYSIS								
General Information					Site Information			
Analyst	KM				Intersection	Sharrotts Rd @ Veterans Rd E		
Agency/Co.	AECOM				Jurisdiction	Staten Island, NY		
Date Performed	2/9/13				Analysis Year	2020 With-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd							
Project ID <i>Charleston EIS</i>								
East/West Street: <i>Sharrotts Road</i>					North/South Street: <i>Veterans Road East</i>			
Volume Adjustments and Site Characteristics								
Approach	Eastbound				Westbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	26	83	0		0	358	24	
%Thrus Left Lane								
Approach	Northbound				Southbound			
Movement	L	T	R		L	T	R	
Volume (veh/h)	53	107	139		0	0	0	
%Thrus Left Lane	50							
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LT		TR		LT		TR	
PHF	0.75		0.85		0.95	0.95		
Flow Rate (veh/h)	144		449		110	202		
% Heavy Vehicles	2		2		1	1		
No. Lanes	1		1		2		0	
Geometry Group	2		2		1			
Duration, T	0.25							
Saturation Headway Adjustment Worksheet								
Prop. Left-Turns	0.2		0.0		0.5	0.0		
Prop. Right-Turns	0.0		0.1		0.0	0.7		
Prop. Heavy Vehicle	0.0		0.0		0.0	0.0		
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2		
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6		
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7		
hadj, computed	0.1		-0.0		0.1	-0.4		
Departure Headway and Service Time								
hd, initial value (s)	3.20		3.20		3.20	3.20		
x, initial	0.13		0.40		0.10	0.18		
hd, final value (s)	5.37		4.88		5.52	4.98		
x, final value	0.21		0.61		0.17	0.28		
Move-up time, m (s)	2.0		2.0		2.0			
Service Time, t_s (s)	3.4		2.9		3.5	3.0		
Capacity and Level of Service								
	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	394		699		360	452		
Delay (s/veh)	9.83		15.17		9.63	9.90		
LOS	A		C		A	A		
Approach: Delay (s/veh)	9.83		15.17		9.80			
LOS	A		C		A			
Intersection Delay (s/veh)	12.47							
Intersection LOS	B							

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	218	353	199	0	85	15	716	66	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 21.0	G =	G =	G =	G = 29.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		762	177		120			844				
Lane Group Capacity, c		552	560		561			2266				
v/c Ratio, X		1.38	0.32		0.21			0.37				
Total Green Ratio, g/C		0.35	0.35		0.35			0.48				
Uniform Delay, d ₁		19.5	14.3		13.7			9.8				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		182.3	1.5		0.9			0.5				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		201.8	15.7		14.6			10.2			
Lane Group LOS		F	B		B			B			
Approach Delay		166.7			14.6			10.2			
Approach LOS		F			B			B			
Intersection Delay		87.7			$X_c = 0.80$			Intersection LOS			F

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Av & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	300		56				17	404			384	82
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.92		0.92				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.937						0.937			0.937	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0
Lane Width		16.0						14.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	14.9			15.5			3.2			17.5		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		387						457			512	
Lane Group Capacity, c		467						1110			1179	
v/c Ratio, X		0.83						0.41			0.43	
Total Green Ratio, g/C		0.27						0.57			0.57	
Uniform Delay, d ₁		20.7						7.3			7.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		14.7						1.1			1.1	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		35.4					8.4			8.6	
Lane Group LOS		<i>D</i>					<i>A</i>			<i>A</i>	
Approach Delay		35.4					8.4			8.6	
Approach LOS		<i>D</i>					<i>A</i>			<i>A</i>	
Intersection Delay		16.2		$X_c = 0.56$			Intersection LOS			<i>B</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Sharrotts Rd & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0		0				0	1			1	0
Lane Group		LR						LT			TR	
Volume, V (vph)	156		67				43	597			409	339
% Heavy Vehicles, %HV	1		1				1	1			1	1
Peak-Hour Factor, PHF	0.75		0.75				0.92	0.92			0.91	0.91
Pretimed (P) or Actuated (A)	P		P				P	P			P	P
Start-up Lost Time, I ₁		2.0						2.0			2.0	
Extension of Effective Green, e		2.0						2.0			2.0	
Arrival Type, AT		3						3			3	
Unit Extension, UE		3.0						3.0			3.0	
Filtering/Metering, I		0.932						0.850			0.905	
Initial Unmet Demand, Q _b		0.0						0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	20
Lane Width		16.0						16.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m			5						5			
Buses Stopping, N _b		0						0			0	
Min. Time for Pedestrians, G _p	18.6			18.6			3.2			16.6		
Phasing	EB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G =	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		297						696			800	
Lane Group Capacity, c		576						780			1003	
v/c Ratio, X		0.52						0.89			0.80	
Total Green Ratio, g/C		0.33						0.50			0.50	
Uniform Delay, d ₁		16.1						13.5			12.5	
Progression Factor, PF		1.000						1.000			1.000	
Delay Calibration, k		0.50						0.50			0.50	
Incremental Delay, d ₂		3.1						12.8			6.0	
Initial Queue Delay, d ₃		0.0						0.0			0.0	

Control Delay		19.2					26.4			18.5	
Lane Group LOS		<i>B</i>					<i>C</i>			<i>B</i>	
Approach Delay	19.2						26.4		18.5		
Approach LOS	<i>B</i>						<i>C</i>		<i>B</i>		
Intersection Delay	21.7			$X_C = 0.74$			Intersection LOS		<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			1	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		180	302	617	173	150	229			422	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, l	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			16.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		254		1255		163	249			488	
Lane Group Capacity, c	222		220		1338		138	621			707	
v/c Ratio, X	0.12		1.15		0.94		1.18	0.40			0.69	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.9		26.0		21.0		20.0	15.4			17.3	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.8		100.0		7.7		130.3	1.8			4.1	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	23.7		126.0		28.7		150.3	17.2			21.4	
Lane Group LOS	C		F		C		F	B			C	
Approach Delay	116.2		28.7		69.8		21.4					
Approach LOS	F		C		E		C					
Intersection Delay	44.3		$X_C = 1.09$		Intersection LOS		D					

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	S Service Rd & Bloomingdale Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/9/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁	0	1	0				0	1	0	1	1	0	
Lane Group		LTR						LTR		L	TR		
Volume, V (vph)	31	33	5				3	347	153	316	539	47	
% Heavy Vehicles, %HV	2	2	2				2	2	2	1	1	1	
Peak-Hour Factor, PHF	0.76	0.76	0.76				0.92	0.92	0.92	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	P	P	P				P	P	P	P	P	P	
Start-up Lost Time, l ₁		2.0						2.0		2.0	2.0		
Extension of Effective Green, e		2.0						2.0		2.0	2.0		
Arrival Type, AT		3						3		3	3		
Unit Extension, UE		3.0						3.0		3.0	3.0		
Filtering/Metering, I		1.000						0.899		0.644	0.644		
Initial Unmet Demand, Q _b		0.0						0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0	0	0	11	
Lane Width		13.0						16.0		12.0	12.0		
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m			5						5				
Buses Stopping, N _b		0						0		0	0		
Min. Time for Pedestrians, G _p		17.5			16.9			12.6			12.9		
Phasing	EB Only	02	03	04	NS Perm	06	07	08					
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		91						546		351	639		
Lane Group Capacity, c		462						972		447	1025		
v/c Ratio, X		0.20						0.56		0.79	0.62		
Total Green Ratio, g/C		0.28						0.55		0.55	0.55		
Uniform Delay, d ₁		16.3						8.8		10.7	9.2		
Progression Factor, PF		1.000						1.000		1.000	1.000		
Delay Calibration, k		0.50						0.50		0.50	0.50		
Incremental Delay, d ₂		1.0						2.1		8.7	1.9		

Initial Queue Delay, d_3		0.0					0.0		0.0	0.0	
Control Delay		17.3					10.9		19.4	11.1	
Lane Group LOS		B					B		B	B	
Approach Delay		17.3					10.9		14.0		
Approach LOS		B					B		B		
Intersection Delay		13.2		$X_c = 0.59$		Intersection LOS		B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/9/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	8	18	294	18	106	13	347	242	86	444	8
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		41		327	20	118		654			598	
Lane Group Capacity, c		631		578	808	601		743			606	
v/c Ratio, X		0.06		0.57	0.02	0.20		0.88			0.99	
Total Green Ratio, g/C		0.44		0.44	0.44	0.44		0.44			0.44	
Uniform Delay, d ₁		14.3		18.6	14.0	15.2		22.8			24.7	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.5	0.1	0.6		8.0			22.9	

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay	14.5	22.1	14.1	15.9	30.8	47.6
Lane Group LOS	B	C	B	B	C	D
Approach Delay	14.5	20.2	30.8	47.6		
Approach LOS	B	C	C	D		
Intersection Delay	33.3	$X_c = 0.78$	Intersection LOS	C		

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	4/11/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 With-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	0	1		0		0				
Lane Group		TR			LT			LR					
Volume, V (vph)		229	240	283	234		160		233				
% Heavy Vehicles, %HV		2	2	4	4		1		1				
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85				
Pretimed (P) or Actuated (A)		P	P	P	P		P		P				
Start-up Lost Time, I ₁		2.0			2.0			2.0					
Extension of Effective Green, e		2.0			2.0			2.0					
Arrival Type, AT		3			3			3					
Unit Extension, UE		3.0			3.0			3.0					
Filtering/Metering, I		0.927			0.992			0.953					
Initial Unmet Demand, Q _b		0.0			0.0			0.0					
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0		
Lane Width		16.0			16.0			11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N	
Parking Maneuvers, N _m									5				
Buses Stopping, N _b		2			0			0					
Min. Time for Pedestrians, G _p		18.3			3.2			15.2			17.2		
Phasing	EW Perm	02	03	04	NB Only	06	07	08					
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		530			581			462					
Lane Group Capacity, c		984			435			558					
v/c Ratio, X		0.54			1.34			0.83					
Total Green Ratio, g/C		0.50			0.50			0.39					
Uniform Delay, d ₁		15.4			22.5			24.8					
Progression Factor, PF		1.000			1.000			1.000					
Delay Calibration, k		0.50			0.50			0.50					
Incremental Delay, d ₂		2.0			165.9			12.7					
Initial Queue Delay, d ₃		0.0			0.0			0.0					

Control Delay	17.4	188.4	37.5	
Lane Group LOS	<i>B</i>	<i>F</i>	<i>D</i>	
Approach Delay	17.4	188.4	37.5	
Approach LOS	<i>B</i>	<i>F</i>	<i>D</i>	
Intersection Delay	86.4	$X_C = 1.11$	Intersection LOS	<i>F</i>

Alternative 2

Mitigation

2020

Weekday

AM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/15/13			Analysis Year	2020 Miti-Action Alt 2		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		216	178	158	221		
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.83	0.83	1.00	
Hourly Flow Rate, HFR (veh/h)	0	260	214	190	266	0	
Percent Heavy Vehicles	0	--	--	11	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				147		146	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.70	
Hourly Flow Rate, HFR (veh/h)	0	0	0	196	0	208	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L	L		R		
v (veh/h)		190	196		208		
C (m) (veh/h)		1043	253		784		
v/c		0.18	0.77		0.27		
95% queue length		0.66	5.72		1.07		
Control Delay (s/veh)		9.2	55.3		11.2		
LOS		A	F		B		
Approach Delay (s/veh)	--	--	32.6				
Approach LOS	--	--	D				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	3	6	1	174	34	382	0	224	313	119	255	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887	0.887	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4	0	0	3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			239	439		264	368	137	303	
Lane Group Capacity, c		717			539	514		702	606	381	678	
v/c Ratio, X		0.02			0.44	0.85		0.38	0.61	0.36	0.45	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42	0.42	0.42	0.42	
Uniform Delay, d ₁		10.3			12.5	15.8		12.1	13.7	12.0	12.5	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.0			2.5	15.6		1.4	4.0	2.6	2.1	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Control Delay		10.3			15.0	31.4		13.5	17.7	14.6	14.7	
Lane Group LOS		B			B	C		B	B	B	B	
Approach Delay		10.3			25.6			15.9			14.7	
Approach LOS		B			C			B			B	
Intersection Delay		19.3			$X_C = 0.73$			Intersection LOS			B	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				62		204		549	74	144	358	
% Heavy Vehicles, %HV				3		3		11	11	9	9	
Peak-Hour Factor, PHF				0.93		0.93		0.85	0.85	0.87	0.87	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.980			0.769		0.842	0.842	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	3	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					286			733		166	411	
Lane Group Capacity, c					381			1033		265	911	
v/c Ratio, X					0.75			0.71		0.63	0.45	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					20.2			9.4		8.7	7.6	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					12.5			3.2		9.1	1.4	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				32.7			12.6		17.8	8.9	
Lane Group LOS				C			B		B	A	
Approach Delay				32.7	12.6			11.5			
Approach LOS				C	B			B			
Intersection Delay	15.8			$X_c = 0.72$			Intersection LOS		B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	74	175	169	203	367	80	192	324	138	68	32	41
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 33.0	G =	G =	G =	G = 25.0	G = 17.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	84	348		233	509			702		101	109	
Lane Group Capacity, c	261	639		245	1197			903		320	298	
v/c Ratio, X	0.32	0.54		0.95	0.43			0.78		0.32	0.37	
Total Green Ratio, g/C	0.37	0.37		0.37	0.37			0.28		0.19	0.19	
Uniform Delay, d ₁	20.5	22.6		27.7	21.4			29.9		31.5	31.8	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	2.5	2.6		45.5	1.1			6.5		2.5	3.4	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay	23.0	25.1		73.3	22.5			36.5		34.0	35.2	
Lane Group LOS	C	C		E	C			D		C	D	
Approach Delay	24.7			38.4			36.5			34.6		
Approach LOS	C			D			D			C		
Intersection Delay	34.5			$X_c = 0.76$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR			LTR		DefL	TR		L	TR	
Volume, V (vph)	54	39	275	115	285	6	264	93	51	11	160	102
% Heavy Vehicles, %HV	8	8	8	3	3	3	4	4	4	8	8	8
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.87	0.87	0.87	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3			3		3	3		3	3	
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.916	0.916			0.953		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0			12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 40.0	G =	G =	G =	G = 40.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	61	357			467		303	166		14	345	
Lane Group Capacity, c	340	1293			1136		450	794		476	1285	
v/c Ratio, X	0.18	0.28			0.41		0.67	0.21		0.03	0.27	
Total Green Ratio, g/C	0.44	0.44			0.44		0.44	0.44		0.44	0.44	
Uniform Delay, d ₁	15.1	15.8			17.0		19.8	15.3		14.1	15.8	
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	1.1	0.5			1.0		7.8	0.6		0.1	0.5	

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0		0.0	0.0	
Control Delay	16.1	16.3			18.0		27.6	15.9		14.2	16.3	
Lane Group LOS	<i>B</i>	<i>B</i>			<i>B</i>		<i>C</i>	<i>B</i>		<i>B</i>	<i>B</i>	
Approach Delay	16.3		18.0		23.5		16.2					
Approach LOS	<i>B</i>		<i>B</i>		<i>C</i>		<i>B</i>					
Intersection Delay	18.7		$X_c = 0.54$		Intersection LOS		<i>B</i>					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	3		70	244	395	164	62	200			475	22
% Heavy Vehicles, %HV	3		3	3	3	3	7	7			7	7
Peak-Hour Factor, PHF	0.79		0.79	0.88	0.88	0.88	0.90	0.90			0.76	0.76
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.974		0.974		0.725		0.946	0.946			0.293	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	7			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 8.0	G = 17.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		89		912		69	222			654	
Lane Group Capacity, c	218		216		1314		186	575			1115	
v/c Ratio, X	0.02		0.41		0.69		0.37	0.39			0.59	
Total Green Ratio, g/C	0.13		0.13		0.28		0.33	0.33			0.33	
Uniform Delay, d ₁	22.6		23.8		19.2		15.2	15.3			16.6	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.2		5.6		2.2		5.3	1.9			0.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.7		29.4		21.4		20.5	17.2			17.2	
Lane Group LOS	C		C		C		C	B			B	
Approach Delay	29.1		21.4		17.9		17.2					
Approach LOS	C		C		B		B					
Intersection Delay	19.9		$X_c = 0.60$		Intersection LOS		B					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	20	13	172	16	105	10	203	131	139	313	9
% Heavy Vehicles, %HV	3	3	3	5	5	5	7	7	7	7	7	7
Peak-Hour Factor, PHF	0.68	0.68	0.68	0.88	0.88	0.88	0.90	0.90	0.90	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.966	0.966	0.966		0.850			0.090	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 37.0	G =	G =	G =	G = 43.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		60		195	18	119		383			607	
Lane Group Capacity, c		597		504	719	531		748			594	
v/c Ratio, X		0.10		0.39	0.03	0.22		0.51			1.02	
Total Green Ratio, g/C		0.41		0.41	0.41	0.41		0.48			0.48	
Uniform Delay, d ₁		16.3		18.6	15.8	17.2		16.2			23.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		2.2	0.1	0.9		2.1			17.2	

Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		16.6		20.7	15.8	18.1		18.4			40.7	
Lane Group LOS		B		C	B	B		B			D	
Approach Delay		16.6		19.5				18.4			40.7	
Approach LOS		B		B				B			D	
Intersection Delay		28.4		$X_c = 0.73$				Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		174	189	173	215		282		151			
% Heavy Vehicles, %HV		10	10	13	13		9		9			
Peak-Hour Factor, PHF		0.92	0.92	0.89	0.89		0.94		0.94			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.964		0.986	0.986			0.858				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		347		194	242			461				
Lane Group Capacity, c		900		377	813			529				
v/c Ratio, X		0.39		0.51	0.30			0.87				
Total Green Ratio, g/C		0.50		0.50	0.50			0.39				
Uniform Delay, d ₁		13.9		15.1	13.2			25.4				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		1.2		4.9	0.9			15.6				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Control Delay		15.1		20.0	14.1			41.0			
Lane Group LOS		B		C	B			D			
Approach Delay		15.1		16.8				41.0			
Approach LOS		B		B				D			
Intersection Delay		25.3		$X_c = 0.67$				Intersection LOS		C	

Weekday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/15/13			Analysis Year	2020 Miti-Action Alt 2		
Analysis Time Period	Weekday 12-1 MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		310	29	28	324		
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.91	0.91	1.00	
Hourly Flow Rate, HFR (veh/h)	0	364	34	30	356	0	
Percent Heavy Vehicles	0	--	--	15	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				54		24	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.94	1.00	0.94	
Hourly Flow Rate, HFR (veh/h)	0	0	0	57	0	25	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		1
Configuration				L			R
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L	L		R		
v (veh/h)		30	57		25		
C (m) (veh/h)		1093	357		685		
v/c		0.03	0.16		0.04		
95% queue length		0.08	0.56		0.11		
Control Delay (s/veh)		8.4	17.0		10.5		
LOS		A	C		B		
Approach Delay (s/veh)	--	--	15.0				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Conditions					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	1	17	6	255	24	490	1	161	287	241	200	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0			2.0			2.0			2.0		
Extension of Effective Green, e	2.0			2.0			2.0			2.0		
Arrival Type, AT	3			3			3			3		
Unit Extension, UE	3.0			3.0			3.0			3.0		
Filtering/Metering, I	1.000			0.887			0.931			1.000		
Initial Unmet Demand, Q _b	0.0			0.0			0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	15.5			11.0			12.0			10.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m	5			5								
Buses Stopping, N _b	0			0			3			0		
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 28.0	G =	G =	G =	G = 22.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	28			310			176			280		
Lane Group Capacity, c	799			586			619			393		
v/c Ratio, X	0.04			0.53			0.28			0.71		
Total Green Ratio, g/C	0.47			0.47			0.37			0.37		
Uniform Delay, d ₁	8.7			11.3			13.4			15.3		
Progression Factor, PF	1.000			1.000			1.000			1.000		
Delay Calibration, k	0.50			0.50			0.50			0.50		
Incremental Delay, d ₂	0.1			3.0			1.1			4.3		

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Control Delay		8.8			14.3	37.7		14.5	19.6	26.8	15.7	
Lane Group LOS		A			B	D		B	B	C	B	
Approach Delay		8.8			29.3			17.8			21.8	
Approach LOS		A			C			B			C	
Intersection Delay		23.9			$X_C = 0.84$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				102		201		473	80	188	467	
% Heavy Vehicles, %HV				3		3		8	8	6	6	
Peak-Hour Factor, PHF				0.87		0.87		0.92	0.92	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.944			0.880		0.500	0.500	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	4	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					348			601		219	543	
Lane Group Capacity, c					385			1058		332	933	
v/c Ratio, X					0.90			0.57		0.66	0.58	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					21.3			8.3		9.0	8.4	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					25.9			1.9		5.1	1.3	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				47.2			10.3		14.1	9.7	
Lane Group LOS				D			B		B	A	
Approach Delay				47.2				10.3			
Approach LOS				D				B			
Intersection Delay	18.1		$X_c = 0.74$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	62	140	509	134	316	18	281	201	88	23	528	226
% Heavy Vehicles, %HV	2	2	2	3	3	3	3	3	3	12	12	12
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.87	0.87	0.87	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.889	0.889		0.932	0.932		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0		12.0	12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	NB Only	07	08				
Timing	G = 40.0	G =	G =	G =	G = 30.0	G = 7.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	74	773		149	371		323	332		26	838	
Lane Group Capacity, c	424	1391		224	813		338	808		275	943	
v/c Ratio, X	0.17	0.56		0.67	0.46		0.96	0.41		0.09	0.89	
Total Green Ratio, g/C	0.44	0.44		0.44	0.44		0.44	0.44		0.33	0.33	
Uniform Delay, d ₁	15.1	18.4		19.7	17.4		34.9	17.0		20.7	28.4	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	0.8	1.4		13.6	1.7		38.9	1.5		0.7	12.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	15.9	19.9		33.4	19.1		73.8	18.5		21.3	40.7	
Lane Group LOS	<i>B</i>	<i>B</i>		<i>C</i>	<i>B</i>		<i>E</i>	<i>B</i>		<i>C</i>	<i>D</i>	
Approach Delay	19.5			23.2			45.8			40.1		
Approach LOS	<i>B</i>			<i>C</i>			<i>D</i>			<i>D</i>		
Intersection Delay	32.3			$X_c = 0.95$			Intersection LOS			<i>C</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Hour					Analysis Year	2020 Miti-Action Alt					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR		DefL	TR			LT	R
Volume, V (vph)	504	38	0	2	65	71	4	3	0	65	6	1120
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	6	6	6
Peak-Hour Factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.58	0.58	0.58	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0		2.0	2.0			2.0	2.0
Arrival Type, AT	3	3			3		3	3			3	3
Unit Extension, UE	3.0	3.0			3.0		3.0	3.0			3.0	3.0
Filtering/Metering, I	0.962	0.962			1.000		1.000	1.000			0.889	0.889
Initial Unmet Demand, Q _b	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	17	0	0	0	0	0	310
Lane Width	16.0	16.0			11.5		12.0	12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0		0	0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 28.0	G = 12.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	42			132		7	5			79	900
Lane Group Capacity, c	818	1056			955		516	732			538	881
v/c Ratio, X	0.68	0.04			0.14		0.01	0.01			0.15	1.02
Total Green Ratio, g/C	0.50	0.50			0.31		0.39	0.39			0.39	0.58
Uniform Delay, d ₁	18.5	11.5			22.3		16.9	16.9			17.8	19.0
Progression Factor, PF	1.000	1.000			1.000		1.000	1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50		0.50	0.50			0.50	0.50
Incremental Delay, d ₂	4.3	0.1			0.3		0.0	0.0			0.5	34.2

Initial Queue Delay, d_3	0.0	0.0			0.0		0.0	0.0			0.0	0.0
Control Delay	22.8	11.5			22.6		16.9	16.9			18.3	53.2
Lane Group LOS	C	B			C		B	B			B	D
Approach Delay	22.0		22.6		16.9		50.3					
Approach LOS	C		C		B		D					
Intersection Delay	38.2		$X_c = 1.01$		Intersection LOS		D					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	10		148	231	471	127	112	171			354	32
% Heavy Vehicles, %HV	1		1	2	2	2	6	6			2	2
Peak-Hour Factor, PHF	0.77		0.77	0.87	0.87	0.87	0.89	0.89			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.830		0.830		0.725		0.937	0.937			0.817	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			2	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 9.0	G = 17.0	G =	G =	G = 19.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	13		192		953		126	192			429	
Lane Group Capacity, c	250		248		1339		272	563			1105	
v/c Ratio, X	0.05		0.77		0.71		0.46	0.34			0.39	
Total Green Ratio, g/C	0.15		0.15		0.28		0.32	0.32			0.32	
Uniform Delay, d ₁	21.8		24.5		19.3		16.4	15.7			16.0	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.3		17.6		2.4		5.2	1.5			0.8	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	22.2		42.1		21.7		21.7	17.2			16.8	
Lane Group LOS	C		D		C		C	B			B	
Approach Delay	40.9		21.7		19.0		16.8					
Approach LOS	D		C		B		B					
Intersection Delay	22.2		$X_c = 0.62$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		198	189	189	226		196		174			
% Heavy Vehicles, %HV		8	8	16	16		2		2			
Peak-Hour Factor, PHF		0.90	0.90	0.91	0.91		0.88		0.88			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.961		0.989	0.989			0.922				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	44	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 45.0	G =	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		381		208	248			421				
Lane Group Capacity, c		921		347	792			559				
v/c Ratio, X		0.41		0.60	0.31			0.75				
Total Green Ratio, g/C		0.50		0.50	0.50			0.39				
Uniform Delay, d ₁		14.2		16.1	13.3			23.8				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		1.3		7.4	1.0			8.4				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Control Delay		15.5		23.4	14.4			32.2			
Lane Group LOS		<i>B</i>		<i>C</i>	<i>B</i>			<i>C</i>			
Approach Delay		15.5		18.5				32.2			
Approach LOS		<i>B</i>		<i>B</i>				<i>C</i>			
Intersection Delay		22.2		$X_c = 0.67$				Intersection LOS		<i>C</i>	

Weekday

PM

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/15/13			Analysis Year	2020 Miti-Action Alt 2		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		338	53	43	342		
Peak-Hour Factor, PHF	1.00	0.87	0.87	0.81	0.81	1.00	
Hourly Flow Rate, HFR (veh/h)	0	388	60	53	422	0	
Percent Heavy Vehicles	0	--	--	7	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				55		49	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68	
Hourly Flow Rate, HFR (veh/h)	0	0	0	80	0	72	
Percent Heavy Vehicles	0	0	0	7	0	7	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L	L		R		
v (veh/h)		53	80		72		
C (m) (veh/h)		1086	282		649		
v/c		0.05	0.28		0.11		
95% queue length		0.15	1.14		0.37		
Control Delay (s/veh)		8.5	22.7		11.2		
LOS		A	C		B		
Approach Delay (s/veh)	--	--	17.3				
Approach LOS	--	--	C				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	1	11	2	301	20	380	4	167	349	300	279	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0			2.0			2.0			2.0		
Extension of Effective Green, e	2.0			2.0			2.0			2.0		
Arrival Type, AT	3			3			3			3		
Unit Extension, UE	3.0			3.0			3.0			3.0		
Filtering/Metering, I	1.000			0.933			0.933			0.919		
Initial Unmet Demand, Q _b	0.0			0.0			0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	15.5			11.0			10.0			12.0		
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m	5			5								
Buses Stopping, N _b	0			0			3			0		
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	16			391			463			184		
Lane Group Capacity, c	727			530			523			740		
v/c Ratio, X	0.02			0.74			0.89			0.25		
Total Green Ratio, g/C	0.42			0.42			0.42			0.42		
Uniform Delay, d ₁	10.3			14.7			16.2			11.4		
Progression Factor, PF	1.000			1.000			1.000			1.000		
Delay Calibration, k	0.50			0.50			0.50			0.50		
Incremental Delay, d ₂	0.1			8.3			18.3			0.7		

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Control Delay		10.4			23.1	34.5		12.1	17.1	28.4	14.7	
Lane Group LOS		B			C	C		B	B	C	B	
Approach Delay		10.4			29.2			15.4			21.8	
Approach LOS		B			C			B			C	
Intersection Delay		23.1			$X_C = 0.84$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				135		180		588	106	149	747	
% Heavy Vehicles, %HV				1		1		4	4	6	6	
Peak-Hour Factor, PHF				0.84		0.84		0.93	0.93	0.86	0.86	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.901			0.798		0.114	0.114	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			4		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 17.0	G =	G =	G =	G = 33.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					375			746		173	869	
Lane Group Capacity, c					421			1065		251	913	
v/c Ratio, X					0.89			0.70		0.69	0.95	
Total Green Ratio, g/C					0.28			0.55		0.55	0.55	
Uniform Delay, d ₁					20.6			9.9		9.8	12.7	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					21.8			3.1		1.8	3.8	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				42.4			13.0		11.6	16.5	
Lane Group LOS				D			B		B	B	
Approach Delay				42.4				13.0			
Approach LOS				D				B			
Intersection Delay	19.4		$X_c = 0.93$		Intersection LOS			B			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	53	154	557	123	334	10	271	145	124	22	534	124
% Heavy Vehicles, %HV	2	2	2	2	2	2	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.884	0.884		0.927	0.927		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	10.0	12.0		12.0	12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	02	03	04	NS Perm	NB Only	07	08				
Timing	G = 40.0	G =	G =	G =	G = 23.0	G = 14.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	60	808		150	419		291	289		23	700	
Lane Group Capacity, c	384	1391		212	824		476	804		244	807	
v/c Ratio, X	0.16	0.58		0.71	0.51		0.61	0.36		0.09	0.87	
Total Green Ratio, g/C	0.44	0.44		0.44	0.44		0.44	0.44		0.26	0.26	
Uniform Delay, d ₁	14.9	18.7		20.3	17.9		28.7	16.5		25.6	32.0	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	0.8	1.6		16.9	2.1		5.8	1.3		0.8	12.1	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	15.7	20.3		37.1	20.0		34.5	17.8		26.3	44.2	
Lane Group LOS	B	C		D	C		C	B		C	D	
Approach Delay	20.0			24.5			26.2			43.6		
Approach LOS	B			C			C			D		
Intersection Delay	28.5			$X_c = 0.85$			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Hour					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	499	37	0	1	37	43	3	3	2	62	1	1176
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.95	0.95	0.95	0.50	0.50	0.50	0.94	0.94	0.94
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.952	0.952			1.000			1.000			0.937	0.937
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	10	0	0	0	0	0	294
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 29.0	G = 11.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	554	41			75			16			67	938
Lane Group Capacity, c	845	1066			1007			1220			548	897
v/c Ratio, X	0.66	0.04			0.07			0.01			0.12	1.05
Total Green Ratio, g/C	0.50	0.50			0.32			0.39			0.39	0.57
Uniform Delay, d ₁	17.8	11.5			21.2			16.9			17.6	19.5
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	3.8	0.1			0.1			0.0			0.4	41.8

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	21.6	11.5			21.3			16.9			18.1	61.3
Lane Group LOS	C	B			C			B			B	E
Approach Delay	20.9				21.3				16.9		58.4	
Approach LOS	C				C				B		E	
Intersection Delay	43.1				$X_C = 1.03$				Intersection LOS		D	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	4		162	284	466	276	99	207			501	5
% Heavy Vehicles, %HV	1		1	1	1	1	2	2			4	4
Peak-Hour Factor, PHF	0.95		0.95	0.88	0.88	0.88	0.92	0.92			0.87	0.87
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.848		0.848		0.500		0.930	0.930			0.865	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	2			7	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 9.0	G = 17.0	G =	G =	G = 19.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	4		171		1167		108	225			582	
Lane Group Capacity, c	250		248		1329		210	585			1084	
v/c Ratio, X	0.02		0.69		0.88		0.51	0.38			0.54	
Total Green Ratio, g/C	0.15		0.15		0.28		0.32	0.32			0.32	
Uniform Delay, d ₁	21.7		24.2		20.5		16.7	16.0			16.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.1		12.5		4.5		8.1	1.8			1.7	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	21.8		36.7		25.0		24.9	17.7			18.5	
Lane Group LOS	C		D		C		C	B			B	
Approach Delay	36.4		25.0		20.0		18.5					
Approach LOS	D		C		C		B					
Intersection Delay	23.5		$X_c = 0.70$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	6	13	20	267	29	110	18	315	175	94	452	10
% Heavy Vehicles, %HV	2	2	2	3	3	3	1	1	1	2	2	2
Peak-Hour Factor, PHF	0.70	0.70	0.70	0.87	0.87	0.87	0.92	0.92	0.92	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.905	0.905	0.905		0.663			0.229	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		2		0	0	2		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		57		307	33	126		552			639	
Lane Group Capacity, c		618		545	773	570		751			646	
v/c Ratio, X		0.09		0.56	0.04	0.22		0.74			0.99	
Total Green Ratio, g/C		0.43		0.43	0.43	0.43		0.46			0.46	
Uniform Delay, d ₁		15.1		19.1	14.7	16.0		20.1			24.3	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.3		3.8	0.1	0.8		4.3			14.6	

Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		15.3		22.9	14.8	16.8		24.3			38.9	
Lane Group LOS		<i>B</i>		<i>C</i>	<i>B</i>	<i>B</i>		<i>C</i>			<i>D</i>	
Approach Delay		15.3		20.7				24.3			38.9	
Approach LOS		<i>B</i>		<i>C</i>				<i>C</i>			<i>D</i>	
Intersection Delay		28.5		$X_c = 0.78$				Intersection LOS			<i>C</i>	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		203	253	208	224		158		200			
% Heavy Vehicles, %HV		5	5	8	8		1		1			
Peak-Hour Factor, PHF		0.82	0.82	0.83	0.83		0.67		0.67			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.927		0.989	0.989			0.917				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	59	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		5		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 30.0	G = 10.0	G =	G =	G = 37.0	G =	G =	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		485		251	270			535				
Lane Group Capacity, c		626		374	813			592				
v/c Ratio, X		0.77		0.67	0.33			0.90				
Total Green Ratio, g/C		0.33		0.48	0.48			0.41				
Uniform Delay, d ₁		27.0		30.3	14.6			24.8				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		8.5		9.1	1.1			18.4				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Control Delay		35.4		39.4	15.7			43.2			
Lane Group LOS		<i>D</i>		<i>D</i>	<i>B</i>			<i>D</i>			
Approach Delay		35.4		27.1				43.2			
Approach LOS		<i>D</i>		<i>C</i>				<i>D</i>			
Intersection Delay		35.3		$X_c = 0.87$				Intersection LOS			<i>D</i>

Saturday

Midday

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	Englewood Ave & Arthur Kill Rd		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	4/15/13			Analysis Year	2020 Miti-Action Alt 2		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>Englewood Avenue</i>				North/South Street: <i>Arthur Kill Road</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		321	33	33	387		
Peak-Hour Factor, PHF	1.00	0.92	0.92	0.88	0.88	1.00	
Hourly Flow Rate, HFR (veh/h)	0	348	35	37	439	0	
Percent Heavy Vehicles	0	--	--	5	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	1	1	1	1		0
Configuration		T	R	L	T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				30		36	
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.75	1.00	0.75	
Hourly Flow Rate, HFR (veh/h)	0	0	0	40	0	48	
Percent Heavy Vehicles	0	0	0	3	0	3	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	1	
Configuration				L		R	
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		L	L		R		
v (veh/h)		37	40		48		
C (m) (veh/h)		1159	315		693		
v/c		0.03	0.13		0.07		
95% queue length		0.10	0.43		0.22		
Control Delay (s/veh)		8.2	18.1		10.6		
LOS		A	C		B		
Approach Delay (s/veh)	--	--	14.0				
Approach LOS	--	--	B				

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	1	1	1	0
Lane Group	LTR			LT R			LT R			L TR		
Volume, V (vph)	0	10	5	356	27	538	5	193	426	306	243	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0	2.0	2.0	2.0	
Arrival Type, AT		3			3	3		3	3	3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861	0.861	1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		12.0	12.0	10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2	0	0	2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 28.0	G =	G =	G =	G = 22.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			416	585		217	468	336	267	
Lane Group Capacity, c		813			613	603		654	564	389	649	
v/c Ratio, X		0.02			0.68	0.97		0.33	0.83	0.86	0.41	
Total Green Ratio, g/C		0.47			0.47	0.47		0.37	0.37	0.37	0.37	
Uniform Delay, d ₁		8.6			12.5	15.6		13.7	17.3	17.6	14.2	
Progression Factor, PF		1.000			1.000	1.000		1.000	1.000	1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50	0.50	0.50	0.50	
Incremental Delay, d ₂		0.0			5.1	27.2		1.2	11.6	21.7	1.9	

Initial Queue Delay, d_3		0.0			0.0	0.0		0.0	0.0	0.0	0.0	
Control Delay		8.7			17.6	42.8		14.9	28.9	39.3	16.1	
Lane Group LOS		A			B	D		B	C	D	B	
Approach Delay		8.7			32.3			24.5			29.0	
Approach LOS		A			C			C			C	
Intersection Delay		29.0			$X_C = 0.92$			Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Rch Valley Rd & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:15-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁				0		0		1	0	1	1	
Lane Group					LR			TR		L	T	
Volume, V (vph)				108		222		645	94	157	735	
% Heavy Vehicles, %HV				1		1		2	2	2	2	
Peak-Hour Factor, PHF				0.89		0.89		0.91	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)				P		P		P	P	P	P	
Start-up Lost Time, I ₁					2.0			2.0		2.0	2.0	
Extension of Effective Green, e					2.0			2.0		2.0	2.0	
Arrival Type, AT					3			3		3	3	
Unit Extension, UE					3.0			3.0		3.0	3.0	
Filtering/Metering, I					0.940			0.789		0.272	0.272	
Initial Unmet Demand, Q _b					0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0	0	0	0	0	0	
Lane Width					12.0			15.0		10.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b					0			2		0	2	
Min. Time for Pedestrians, G _p	11.8			11.8			14.1			3.2		
Phasing	WB Only	02	03	04	NS Perm	06	07	08				
Timing	G = 16.0	G =	G =	G =	G = 34.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					370			812		173	808	
Lane Group Capacity, c					393			1132		252	977	
v/c Ratio, X					0.94			0.72		0.69	0.83	
Total Green Ratio, g/C					0.27			0.57		0.57	0.57	
Uniform Delay, d ₁					21.5			9.5		9.2	10.6	
Progression Factor, PF					1.000			1.000		1.000	1.000	
Delay Calibration, k					0.50			0.50		0.50	0.50	
Incremental Delay, d ₂					31.5			3.1		4.1	2.3	
Initial Queue Delay, d ₃					0.0			0.0		0.0	0.0	

Control Delay				53.1			12.6		13.4	12.9	
Lane Group LOS				<i>D</i>			<i>B</i>		<i>B</i>	<i>B</i>	
Approach Delay				53.1	12.6			13.0			
Approach LOS				<i>D</i>	<i>B</i>			<i>B</i>			
Intersection Delay	19.7		$X_c = 0.86$		Intersection LOS			<i>B</i>			

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	SFR NB-DefL = 2050					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	2	0	0	2	0	0	2	0	1	2	0
Lane Group	L	TR		DefL	TR		DefL	TR		L	TR	
Volume, V (vph)	82	175	593	165	432	25	322	269	115	26	834	168
% Heavy Vehicles, %HV	2	2	2	1	1	1	1	1	1	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.78	0.78	0.78	0.94	0.91	0.91	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3		3	3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Filtering/Metering, I	0.830	0.830		0.865	0.865		1.000	1.000		1.000	1.000	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	11.0	12.0		12.0	12.0		13.0	13.0		10.0	9.5	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0		0	0		0	0	
Min. Time for Pedestrians, G _p	18.6			16.6			20.1			19.8		
Phasing	EW Perm	WB Only	03	04	SB Only	NS Perm	NB Only	08				
Timing	G = 25.0	G = 6.0	G =	G =	G = 15.0	G = 14.0	G = 11.0	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 3	Y = 3	Y = 5	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	89	835		212	586		343	422		28	1078	
Lane Group Capacity, c	152	871		302	705		447	578		370	1150	
v/c Ratio, X	0.59	0.96		0.70	0.83		0.77	0.73		0.08	0.94	
Total Green Ratio, g/C	0.28	0.28		0.38	0.38		0.31	0.31		0.36	0.36	
Uniform Delay, d ₁	28.0	32.0		34.7	25.4		32.1	27.6		19.4	28.0	
Progression Factor, PF	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50		0.50	0.50		0.50	0.50	
Incremental Delay, d ₂	13.0	19.4		11.2	9.6		11.9	7.9		0.4	15.2	

Initial Queue Delay, d_3	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay	41.0	51.4		45.9	35.0		44.0	35.5		19.8	43.2	
Lane Group LOS	<i>D</i>	<i>D</i>		<i>D</i>	<i>D</i>		<i>D</i>	<i>D</i>		<i>B</i>	<i>D</i>	
Approach Delay	50.4			37.9			39.4			42.7		
Approach LOS	<i>D</i>			<i>D</i>			<i>D</i>			<i>D</i>		
Intersection Delay	42.9			$X_c = 1.39$			Intersection LOS			<i>D</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & OBX Off-Ramp					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Sat Flw Rate EB-L = 2100					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1			1	1				2		1
Lane Group	L	T			T	R				L		R
Volume, V (vph)	892	472			508	1077				233		216
% Heavy Vehicles, %HV	1	1			1	1				1		1
Peak-Hour Factor, PHF	0.97	0.96			0.94	0.97				0.92		0.92
Pretimed (P) or Actuated (A)	P	P			P	P				P		P
Start-up Lost Time, I ₁	2.0	2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e	3.0	2.0			2.0	2.0				2.0		2.0
Arrival Type, AT	3	3			3	3				3		3
Unit Extension, UE	3.0	3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I	0.508	0.508			0.176	0.176				1.000		1.000
Initial Unmet Demand, Q _b	0.0	0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	119	0	0		0	0	0
Lane Width	11.0	12.0			12.0	12.0				10.0		10.0
Parking / Grade / Parking	N	0	Y	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m			5									
Buses Stopping, N _b	0	0			0	0				0		0
Min. Time for Pedestrians, G _p	14.6			24.6			3.2			20.1		
Phasing	EB Only	EW Perm	03	04	SB Only	06	07	08				
Timing	G = 37.0	G = 24.0	G =	G =	G = 14.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	920	492			540	988				253		235
Lane Group Capacity, c	895	1207			502	764				504		928
v/c Ratio, X	1.03	0.41			1.08	1.29				0.50		0.25
Total Green Ratio, g/C	0.74	0.73			0.27	0.48				0.16		0.62
Uniform Delay, d ₁	9.5	4.6			33.0	23.5				34.8		7.6
Progression Factor, PF	1.000	1.000			1.000	1.000				1.000		1.000
Delay Calibration, k	0.50	0.50			0.50	0.50				0.50		0.50
Incremental Delay, d ₂	28.9	0.5			41.4	133.7				3.5		0.7
Initial Queue Delay, d ₃	0.0	0.0			0.0	0.0				0.0		0.0

Control Delay	38.4	5.1			74.4	157.2				38.4		8.3
Lane Group LOS	<i>D</i>	<i>A</i>			<i>E</i>	<i>F</i>				<i>D</i>		<i>A</i>
Approach Delay	26.8			128.0						23.9		
Approach LOS	<i>C</i>			<i>F</i>						<i>C</i>		
Intersection Delay	71.5			$X_c = 1.16$			Intersection LOS			<i>E</i>		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Boscombe Ave & Tyrellan Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 mITI-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	2	0	0	2	0	0	2	0	0	1	1
Lane Group	DefL	TR			LTR			LTR			LT	R
Volume, V (vph)	660	46	0	0	46	58	0	0	1	84	0	1539
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.96	0.96	0.96	0.91	0.91	0.91	0.90	0.90	0.90	0.93	0.93	0.93
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0			2.0			2.0			2.0	2.0
Extension of Effective Green, e	2.0	2.0			2.0			2.0			2.0	2.0
Arrival Type, AT	3	3			3			3			3	3
Unit Extension, UE	3.0	3.0			3.0			3.0			3.0	3.0
Filtering/Metering, I	0.967	0.967			1.000			1.000			0.855	0.855
Initial Unmet Demand, Q _b	0.0	0.0			0.0			0.0			0.0	0.0
Ped / Bike / RTOR Volumes	0	0	0	0	0	14	0	0	0	0	0	398
Lane Width	16.0	16.0			11.5			12.0			12.0	12.0
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0			0			0			0	0
Min. Time for Pedestrians, G _p	20.3			17.5			20.3			20.3		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 26.0	G = 14.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	688	48			99			1			90	1227
Lane Group Capacity, c	853	1066			944			1184			544	959
v/c Ratio, X	0.81	0.05			0.10			0.00			0.17	1.28
Total Green Ratio, g/C	0.50	0.50			0.29			0.39			0.39	0.60
Uniform Delay, d ₁	20.7	11.5			23.5			16.8			18.0	18.0
Progression Factor, PF	1.000	1.000			1.000			1.000			1.000	1.000
Delay Calibration, k	0.50	0.50			0.50			0.50			0.50	0.50
Incremental Delay, d ₂	7.8	0.1			0.2			0.0			0.6	132.7

Initial Queue Delay, d_3	0.0	0.0			0.0			0.0			0.0	0.0
Control Delay	28.5	11.6			23.7			16.8			18.5	150.7
Lane Group LOS	C	B			C			B			B	F
Approach Delay	27.4				23.7				16.8		141.7	
Approach LOS	C				C				B		F	
Intersection Delay	97.1				$X_C = 1.19$				Intersection LOS		F	

HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	KM					Intersection	Englewood Ave @ Veterans Rd W						
Agency or Co.	AECOM					Area Type	All other areas						
Date Performed	2/12/13					Jurisdiction	Staten Island, NY						
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2						
						Project ID	Charleston EIS						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N ₁		1	0	1	1		1			0	3	0	
Lane Group		TR		L	LT		L				LTR		
Volume, V (vph)		1	0	798	2		5			28	474	1	
% Heavy Vehicles, %HV		1	1	2	2		0			2	2	2	
Peak-Hour Factor, PHF		0.75	0.25	0.91	0.91		0.88			0.89	0.89	0.89	
Pretimed (P) or Actuated (A)		P	P	P	P		P			P	P	P	
Start-up Lost Time, l ₁		2.0		2.0	2.0		2.0				2.0		
Extension of Effective Green, e		2.0		2.0	2.0		2.0				2.0		
Arrival Type, AT		3		3	3		3				3		
Unit Extension, UE		3.0		3.0	3.0		3.0				3.0		
Filtering/Metering, I		1.000		0.971	0.971		0.951				1.000		
Initial Unmet Demand, Q _b		0.0		0.0	0.0		0.0				0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0		0	0	0	
Lane Width		12.0		13.0	13.0		10.0				11.0		
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N	
Parking Maneuvers, N _m													
Buses Stopping, N _b		0		0	0		0				0		
Min. Time for Pedestrians, G _p		31.2			3.2			18.1			11.8		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 28.0	G =	G =	G =	G = 22.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		1		658	221		6				565		
Lane Group Capacity, c		878		680	654		243				1793		
v/c Ratio, X		0.00		0.97	0.34		0.02				0.32		
Total Green Ratio, g/C		0.47		0.47	0.47		0.37				0.37		
Uniform Delay, d ₁		8.5		15.6	10.1		12.1				13.6		
Progression Factor, PF		1.000		1.000	1.000		1.000				1.000		
Delay Calibration, k		0.50		0.50	0.50		0.50				0.50		
Incremental Delay, d ₂		0.0		27.0	1.4		0.2				0.5		

Initial Queue Delay, d_3	0.0	0.0	0.0	0.0	0.0
Control Delay	8.5	42.5	11.5	12.3	14.1
Lane Group LOS	A	D	B	B	B
Approach Delay	8.5	34.7	12.3	14.1	
Approach LOS	A	C	B	B	
Intersection Delay	26.6	$X_c = 0.68$	Intersection LOS	C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Englewood Ave @ Vets Rd E					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/12/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	1	0	1	0	0	3	0			
Lane Group		LT	R		LTR			LTR				
Volume, V (vph)	218	353	199	0	85	15	716	66	3			
% Heavy Vehicles, %HV	1	1	1	0	0	0	2	2	2			
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.83	0.83	0.83	0.93	0.93	0.93			
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P			
Start-up Lost Time, l ₁		2.0	2.0		2.0			2.0				
Extension of Effective Green, e		2.0	2.0		2.0			2.0				
Arrival Type, AT		3	3		3			3				
Unit Extension, UE		3.0	3.0		3.0			3.0				
Filtering/Metering, I		0.997	0.997		0.993			0.967				
Initial Unmet Demand, Q _b		0.0	0.0		0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	66	0	0	0	0	0	0	0	0	
Lane Width		13.0	12.0		11.5			11.0				
Parking / Grade / Parking	N	0	N	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m						5						
Buses Stopping, N _b		0	0		0			0				
Min. Time for Pedestrians, G _p		21.2			12.9			14.3			17.5	
Phasing	EW Perm	02	03	04	NB Only	06	07	08				
Timing	G = 26.0	G =	G =	G =	G = 24.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		762	177		120			844				
Lane Group Capacity, c		693	693		694			1875				
v/c Ratio, X		1.10	0.26		0.17			0.45				
Total Green Ratio, g/C		0.43	0.43		0.43			0.40				
Uniform Delay, d ₁		17.0	10.8		10.4			13.2				
Progression Factor, PF		1.000	1.000		1.000			1.000				
Delay Calibration, k		0.50	0.50		0.50			0.50				
Incremental Delay, d ₂		64.6	0.9		0.5			0.8				

Initial Queue Delay, d_3		0.0	0.0		0.0			0.0			
Control Delay		81.6	11.7		11.0			13.9			
Lane Group LOS		F	B		B			B			
Approach Delay		68.5			11.0			13.9			
Approach LOS		E			B			B			
Intersection Delay		40.6			$X_c = 0.79$			Intersection LOS			D

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Vets Rd E & Bloomingdale Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1		1	0	3	0	1	1			2	0
Lane Group	L		R		LTR		L	T			TR	
Volume, V (vph)	19		180	302	617	173	150	229			422	17
% Heavy Vehicles, %HV	1		1	2	2	2	2	2			1	1
Peak-Hour Factor, PHF	0.71		0.71	0.87	0.87	0.87	0.92	0.92			0.90	0.90
Pretimed (P) or Actuated (A)	P		P	P	P	P	P	P			P	P
Start-up Lost Time, l ₁	2.0		2.0		2.0		2.0	2.0			2.0	
Extension of Effective Green, e	2.0		2.0		2.0		2.0	2.0			2.0	
Arrival Type, AT	3		3		3		3	3			3	
Unit Extension, UE	3.0		3.0		3.0		3.0	3.0			3.0	
Filtering/Metering, I	0.717		0.717		0.483		0.917	0.917			0.747	
Initial Unmet Demand, Q _b	0.0		0.0		0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0		0	0	0
Lane Width	10.0		13.0		11.0		12.0	12.0			12.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0		0		0		0	0			0	
Min. Time for Pedestrians, G _p	17.2			16.9			3.2			20.9		
Phasing	EB Only	WB Only	03	04	NS Perm	06	07	08				
Timing	G = 11.0	G = 17.0	G =	G =	G = 17.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	27		254		1255		163	249			488	
Lane Group Capacity, c	306		303		1338		219	528			1009	
v/c Ratio, X	0.09		0.84		0.94		0.74	0.47			0.48	
Total Green Ratio, g/C	0.18		0.18		0.28		0.28	0.28			0.28	
Uniform Delay, d ₁	20.3		23.6		21.0		19.5	17.8			17.9	
Progression Factor, PF	1.000		1.000		1.000		1.000	1.000			1.000	
Delay Calibration, k	0.50		0.50		0.50		0.50	0.50			0.50	
Incremental Delay, d ₂	0.4		17.7		7.7		18.9	2.8			1.2	

Initial Queue Delay, d_3	0.0		0.0		0.0		0.0	0.0			0.0	
Control Delay	20.7		41.4		28.7		38.4	20.5			19.1	
Lane Group LOS	C		D		C		D	C			B	
Approach Delay	39.4		28.7		27.6		19.1					
Approach LOS	D		C		C		B					
Intersection Delay	27.8		$X_c = 0.84$		Intersection LOS		C					

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Amboy Rd & Bloomingdale Ave					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/13/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	1	1	1	0	1	0	0	1	0
Lane Group		LTR		L	T	R		LTR			LTR	
Volume, V (vph)	8	8	18	294	18	106	13	347	242	86	444	8
% Heavy Vehicles, %HV	2	2	2	1	1	1	0	0	0	1	1	1
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0		2.0	2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0	2.0		2.0			2.0	
Arrival Type, AT		3		3	3	3		3			3	
Unit Extension, UE		3.0		3.0	3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000		0.887	0.887	0.887		0.516			0.500	
Initial Unmet Demand, Q _b		0.0		0.0	0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	50	0	0	0	0	0	50	0	0	0	0	0
Lane Width		12.0		12.0	11.0	11.0		11.0			12.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0		0	0	0		0			0	
Min. Time for Pedestrians, G _p		11.5		11.8			18.9			18.3		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 41.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		41		327	20	118		654			598	
Lane Group Capacity, c		615		563	788	586		762			630	
v/c Ratio, X		0.07		0.58	0.03	0.20		0.86			0.95	
Total Green Ratio, g/C		0.43		0.43	0.43	0.43		0.46			0.46	
Uniform Delay, d ₁		14.9		19.3	14.6	15.8		21.9			23.5	
Progression Factor, PF		1.000		1.000	1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50		0.50	0.50	0.50		0.50			0.50	
Incremental Delay, d ₂		0.2		3.9	0.1	0.7		6.7			15.8	

Initial Queue Delay, d_3		0.0		0.0	0.0	0.0		0.0			0.0	
Control Delay		15.1		23.2	14.7	16.5		28.6			39.3	
Lane Group LOS		B		C	B	B		C			D	
Approach Delay		15.1		21.1				28.6			39.3	
Approach LOS		B		C				C			D	
Intersection Delay		29.9		$X_c = 0.77$				Intersection LOS			C	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Arthur Kill Rd & Bloomingdl Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/15/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Miti-Action Alt 2					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁		1	0	1	1		0		0			
Lane Group		TR		L	T			LR				
Volume, V (vph)		229	240	283	234		160		233			
% Heavy Vehicles, %HV		2	2	4	4		1		1			
Peak-Hour Factor, PHF		0.78	0.78	0.89	0.89		0.85		0.85			
Pretimed (P) or Actuated (A)		P	P	P	P		P		P			
Start-up Lost Time, I ₁		2.0		2.0	2.0			2.0				
Extension of Effective Green, e		2.0		2.0	2.0			2.0				
Arrival Type, AT		3		3	3			3				
Unit Extension, UE		3.0		3.0	3.0			3.0				
Filtering/Metering, I		0.927		0.992	0.992			0.953				
Initial Unmet Demand, Q _b		0.0		0.0	0.0			0.0				
Ped / Bike / RTOR Volumes	0	0	56	0	0		0	0	0	0	0	
Lane Width		16.0		11.0	11.0			11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	Y	N	0	N
Parking Maneuvers, N _m									5			
Buses Stopping, N _b		2		0	0			0				
Min. Time for Pedestrians, G _p		18.3		3.2			15.2		17.2			
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 30.0	G = 12.0	G =	G =	G = 35.0	G =	G =	G =				
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 90.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		530		318	263			462				
Lane Group Capacity, c		656		408	883			558				
v/c Ratio, X		0.81		0.78	0.30			0.83				
Total Green Ratio, g/C		0.33		0.50	0.50			0.39				
Uniform Delay, d ₁		27.4		31.1	13.2			24.8				
Progression Factor, PF		1.000		1.000	1.000			1.000				
Delay Calibration, k		0.50		0.50	0.50			0.50				
Incremental Delay, d ₂		9.6		13.6	0.9			12.7				
Initial Queue Delay, d ₃		0.0		0.0	0.0			0.0				

Control Delay		37.0		44.7	14.1			37.5			
Lane Group LOS		D		D	B			D			
Approach Delay		37.0		30.8				37.5			
Approach LOS		D		C				D			
Intersection Delay		34.9		$X_c = 0.86$				Intersection LOS		C	

Alternative 3
With-Action
2020

Weekday

AM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Alt 3 With-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	3	6	1	167	34	220	0	216	293	104	166	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4			3	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			231	253		599			321	
Lane Group Capacity, c		718			540	514		712			332	
v/c Ratio, X		0.02			0.43	0.49		0.84			0.97	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			12.4	12.8		15.7			17.1	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/7/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 With-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	50	164	169	203	285	80	119	324	138	68	32	27
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	57	335		233	415			624		101	88	
Lane Group Capacity, c	272	579		217	1081			903		376	356	
v/c Ratio, X	0.21	0.58		1.07	0.38			0.69		0.27	0.25	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	21.5	24.8		30.0	22.9			29.0		29.0	28.8	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	1.4	3.3		81.4	1.0			4.3		1.7	1.6	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/7/13			Analysis Year	2020 With-Action Alt 3		
Analysis Time Period	Weekday 8-9 AM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					404		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.95	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	425	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0			0	
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				287			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	308	0	0	
Percent Heavy Vehicles	0	0	60	8	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			308				
C (m) (veh/h)			575				
v/c			0.54				
95% queue length			3.16				
Control Delay (s/veh)			18.2				
LOS			C				
Approach Delay (s/veh)	--	--	18.2				
Approach LOS	--	--	C				

Weekday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Alt 3 With-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	17	6	232	24	364	1	200	246	203	215	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			4	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			285	404		485			487	
Lane Group Capacity, c		713			524	518		717			443	
v/c Ratio, X		0.04			0.54	0.78		0.68			1.10	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.4			13.2	15.1		14.2			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/7/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 With-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	94	250	148	195	446	84	178	448	316	149	67	40
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	112	436		217	575			1013		186	134	
Lane Group Capacity, c	202	597		150	1114			924		380	365	
v/c Ratio, X	0.55	0.73		1.45	0.52			1.10		0.49	0.37	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.28		0.22	0.22	
Uniform Delay, d ₁	24.5	26.4		30.0	24.2			32.5		30.5	29.6	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	9.1	6.6		232.2	1.6			59.5		4.4	2.8	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/7/13			Analysis Year	2020 With-Action Alt 3		
Analysis Time Period	Weekday 12-1 PM MD Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					410		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.87	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	471	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				188			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	202	0	0	
Percent Heavy Vehicles	0	0	60	4	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			202				
C (m) (veh/h)			548				
v/c			0.37				
95% queue length			1.69				
Control Delay (s/veh)			15.4				
LOS			C				
Approach Delay (s/veh)	--	--	15.4				
Approach LOS	--	--	C				

Weekday

PM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Alt 3 With-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	1	11	2	278	20	273	4	207	304	259	285	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			363	333		554			633	
Lane Group Capacity, c		727			531	523		751			410	
v/c Ratio, X		0.02			0.68	0.64		0.74			1.54	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.3	13.9		14.7			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/7/13					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 With-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	109	264	206	248	326	87	205	471	267	223	77	56
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	124	483		276	491			986		293	142	
Lane Group Capacity, c	240	591		240	1132			942		384	373	
v/c Ratio, X	0.52	0.82		1.15	0.43			1.05		0.76	0.38	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	24.2	27.5		29.5	22.7			32.5		32.8	29.7	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	5.5	8.6		103.2	1.2			42.3		13.0	2.8	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/7/13			Analysis Year	2020 With-Action Alt 3		
Analysis Time Period	Weekday 5-6 PM Peak Period						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					532		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.89	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	597	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1		0
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				474			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.93	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	0	
Percent Heavy Vehicles	0	0	60	3	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0		0
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			509				
C (m) (veh/h)			464				
v/c			1.10				
95% queue length			16.91				
Control Delay (s/veh)			100.2				
LOS			F				
Approach Delay (s/veh)	--	--	100.2				
Approach LOS	--	--	F				

Saturday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Alt 3 With-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	0	1	0
Lane Group		LTR			LT	R		LTR			LTR	
Volume, V (vph)	0	10	5	325	27	398	5	252	364	254	261	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁		2.0			2.0	2.0		2.0			2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0			2.0	
Arrival Type, AT		3			3	3		3			3	
Unit Extension, UE		3.0			3.0	3.0		3.0			3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861			1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0			16.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2			2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			382	433		682			566	
Lane Group Capacity, c		726			548	539		755			350	
v/c Ratio, X		0.02			0.70	0.80		0.90			1.62	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42			0.42	
Uniform Delay, d ₁		10.3			14.4	15.3		16.4			17.5	
Progression Factor, PF		1.000			1.000	1.000		1.000			1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50			0.50	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	2/7/13					Jurisdiction	Staten Island					
Time Period	Saturday 12:45-1:45 PM MD					Analysis Year	2020 With-Action Alt 3					
	Peak					Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	99	318	221	273	443	102	266	705	293	243	140	93
% Heavy Vehicles, %HV	1	1	1	3	3	3	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.96	0.78	0.78	0.90	0.90	0.90	0.84	0.84	0.84
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.700	0.700		0.903	0.903			1.000		0.855	0.855	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	48	0	0	11	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 25.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	106	528		284	685			1405		289	278	
Lane Group Capacity, c	160	594		97	1179			966		384	367	
v/c Ratio, X	0.66	0.89		2.93	0.58			1.45		0.75	0.76	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.28		0.22	0.22	
Uniform Delay, d ₁	25.7	28.4		29.5	24.2			32.5		32.7	32.7	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	14.1	13.4		892.3	1.9			210.3		11.1	11.8	

TWO-WAY STOP CONTROL SUMMARY							
General Information				Site Information			
Analyst	KM			Intersection	KWVP WB OR UT & No Bridge St		
Agency/Co.	AECOM			Jurisdiction	Staten Island, NY		
Date Performed	2/7/13			Analysis Year	2020 With-Action Alt 3		
Analysis Time Period	Saturday 12:45-1:45 PM Peak Pd						
Project Description <i>Charleston EIS</i>							
East/West Street: <i>KWVP WB Off-Ramp U-Turn</i>				North/South Street: <i>North Bridge Street</i>			
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Northbound			Southbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)					633		
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	0.93	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	680	0	
Percent Heavy Vehicles	0	--	--	0	--	--	
Median Type	Undivided						
RT Channelized			0				0
Lanes	0	0	0	0	1	0	
Configuration					T		
Upstream Signal		0			0		
Minor Street	Eastbound			Westbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)				236			
Peak-Hour Factor, PHF	1.00	1.00	0.90	0.90	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	0	0	0	262	0	0	
Percent Heavy Vehicles	0	0	60	1	0	0	
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	1	0	0	
Configuration				L			
Delay, Queue Length, and Level of Service							
Approach	Northbound	Southbound	Westbound			Eastbound	
Movement	1	4	7	8	9	10	11
Lane Configuration			L				
v (veh/h)			262				
C (m) (veh/h)			418				
v/c			0.63				
95% queue length			4.15				
Control Delay (s/veh)			27.0				
LOS			D				
Approach Delay (s/veh)	--	--	27.0				
Approach LOS	--	--	D				

Alternative 3

Mitigation

2020

Weekday

AM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8-9 AM Peak Period					Analysis Year	2020 Alt 3 Miti-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	1	1	0
Lane Group		LTR			LT	R		LTR		L	TR	
Volume, V (vph)	3	6	1	167	34	220	0	216	293	104	166	9
% Heavy Vehicles, %HV	1	1	1	7	7	7	11	11	11	11	11	11
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.85	0.85	0.85	0.87	0.87	0.87
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0		2.0	2.0	
Arrival Type, AT		3			3	3		3		3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0		3.0	3.0	
Filtering/Metering, I		1.000			0.940	0.940		0.887		1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0		10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		4		0	3	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		11			231	253		599		120	201	
Lane Group Capacity, c		718			540	514		712		162	676	
v/c Ratio, X		0.02			0.43	0.49		0.84		0.74	0.30	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42		0.42	0.42	
Uniform Delay, d ₁		10.3			12.4	12.8		15.7		14.8	11.7	
Progression Factor, PF		1.000			1.000	1.000		1.000		1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50		0.50	0.50	
Incremental Delay, d ₂		0.0			2.3	3.1		10.4		26.0	1.1	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 8 - 9 AM Peak Period					Analysis Year	2020 Miti-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	50	164	169	203	285	80	119	324	138	68	32	27
% Heavy Vehicles, %HV	2	2	2	8	8	8	8	8	8	3	3	3
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.87	0.87	0.87	0.93	0.93	0.93	0.67	0.67	0.67
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.780	0.780		0.979	0.979			1.000		0.991	0.991	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	38	0	0	4	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 32.0	G =	G =	G =	G = 25.0	G = 18.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	57	335		233	415			624		101	88	
Lane Group Capacity, c	295	618		242	1153			903		339	321	
v/c Ratio, X	0.19	0.54		0.96	0.36			0.69		0.30	0.27	
Total Green Ratio, g/C	0.36	0.36		0.36	0.36			0.28		0.20	0.20	
Uniform Delay, d ₁	20.1	23.2		28.4	21.4			29.0		30.6	30.5	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	1.1	2.7		48.4	0.9			4.3		2.2	2.1	

Weekday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Alt 3 Miti-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	1	1	0
Lane Group		LTR			LT	R		LTR		L	TR	
Volume, V (vph)	1	17	6	232	24	364	1	200	246	203	215	1
% Heavy Vehicles, %HV	4	4	4	6	6	6	11	11	11	7	7	7
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.92	0.92	0.92	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0		2.0	2.0	
Arrival Type, AT		3			3	3		3		3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0		3.0	3.0	
Filtering/Metering, I		1.000			0.887	0.887		0.931		1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0		10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3		0	4	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 25.0	G =	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		28			285	404		485		236	251	
Lane Group Capacity, c		713			524	518		717		275	703	
v/c Ratio, X		0.04			0.54	0.78		0.68		0.86	0.36	
Total Green Ratio, g/C		0.42			0.42	0.42		0.42		0.42	0.42	
Uniform Delay, d ₁		10.4			13.2	15.1		14.2		15.9	12.0	
Progression Factor, PF		1.000			1.000	1.000		1.000		1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50		0.50	0.50	
Incremental Delay, d ₂		0.1			3.6	9.9		4.7		27.6	1.4	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 12-1 PM MD Peak Period					Analysis Year	2020 Miti-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	94	250	148	195	446	84	178	448	316	149	67	40
% Heavy Vehicles, %HV	1	1	1	6	6	6	4	4	4	2	2	2
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.90	0.90	0.90	0.93	0.93	0.93	0.80	0.80	0.80
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.856	0.856		0.927	0.927			1.000		0.985	0.985	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	32	0	0	13	0	0	0	0	0	0
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 29.0	G = 16.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	112	436		217	575			1013		186	134	
Lane Group Capacity, c	202	597		150	1114			1072		304	292	
v/c Ratio, X	0.55	0.73		1.45	0.52			0.94		0.61	0.46	
Total Green Ratio, g/C	0.33	0.33		0.33	0.33			0.32		0.18	0.18	
Uniform Delay, d ₁	24.5	26.4		30.0	24.2			29.7		34.1	33.1	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	9.1	6.6		232.2	1.6			17.1		8.8	5.0	

Weekday

PM

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Alt 3 Miti-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	1	1	0
Lane Group		LTR			LT	R		LTR		L	TR	
Volume, V (vph)	1	11	2	278	20	273	4	207	304	259	285	1
% Heavy Vehicles, %HV	3	3	3	5	5	5	5	5	5	7	7	7
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.82	0.82	0.82	0.93	0.93	0.93	0.86	0.86	0.86
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0		2.0	2.0	
Arrival Type, AT		3			3	3		3		3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0		3.0	3.0	
Filtering/Metering, I		1.000			0.933	0.933		0.919		1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0		10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		3		0	2	
Min. Time for Pedestrians, G _p	11.5			13.5			15.2			12.1		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 23.0	G =	G =	G =	G = 27.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			363	333		554		301	332	
Lane Group Capacity, c		669			488	481		813		280	766	
v/c Ratio, X		0.02			0.74	0.69		0.68		1.08	0.43	
Total Green Ratio, g/C		0.38			0.38	0.38		0.45		0.45	0.45	
Uniform Delay, d ₁		11.5			16.0	15.5		13.1		16.5	11.3	
Progression Factor, PF		1.000			1.000	1.000		1.000		1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50		0.50	0.50	
Incremental Delay, d ₂		0.1			9.3	7.5		4.2		75.1	1.8	

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Bricktown/KWVP					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Sat Flow Rate WB-L = 1950					
Time Period	Weekday 5-6 PM Peak Period					Analysis Year	2020 Miti-Action Alt 3					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	1	1	0	1	2	0	0	2	0	1	1	0
Lane Group	L	TR		L	TR			LTR		L	TR	
Volume, V (vph)	109	264	206	248	326	87	205	471	267	223	77	56
% Heavy Vehicles, %HV	1	1	1	7	7	7	3	3	3	1	1	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.90	0.82	0.82	0.93	0.93	0.93	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, I ₁	2.0	2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0		3.0	3.0			2.0		2.0	2.0	
Arrival Type, AT	3	3		3	3			3		3	3	
Unit Extension, UE	3.0	3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I	0.701	0.701		0.949	0.949			1.000		0.970	0.970	
Initial Unmet Demand, Q _b	0.0	0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	45	0	0	11	0	0	25	0	0	25
Lane Width	11.0	12.0		10.0	12.0			12.5		11.0	10.0	
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking Maneuvers, N _m												
Buses Stopping, N _b	0	0		0	0			0		0	0	
Min. Time for Pedestrians, G _p	34.6			18.6			20.3			16.1		
Phasing	EW Perm	02	03	04	NB Only	SB Only	07	08				
Timing	G = 30.0	G =	G =	G =	G = 26.0	G = 19.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 90.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	124	483		276	491			986		293	142	
Lane Group Capacity, c	240	591		240	1132			980		365	355	
v/c Ratio, X	0.52	0.82		1.15	0.43			1.01		0.80	0.40	
Total Green Ratio, g/C	0.33	0.33		0.34	0.34			0.29		0.21	0.21	
Uniform Delay, d ₁	24.2	27.5		29.5	22.7			32.0		33.7	30.6	
Progression Factor, PF	1.000	1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k	0.50	0.50		0.50	0.50			0.50		0.50	0.50	
Incremental Delay, d ₂	5.5	8.6		103.2	1.2			30.2		16.4	3.2	

Saturday

Midday

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	KM					Intersection	Veterans Rd W & Arthur Kill Rd					
Agency or Co.	AECOM					Area Type	All other areas					
Date Performed	4/11/13					Jurisdiction	Staten Island, NY					
Time Period	Saturday 12:45-1:45 PM Peak Pd					Analysis Year	2020 Alt 3 Miti-Action Cond					
						Project ID	Charleston EIS					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N ₁	0	1	0	0	1	1	0	1	0	1	1	0
Lane Group		LTR			LT	R		LTR		L	TR	
Volume, V (vph)	0	10	5	325	27	398	5	252	364	254	261	0
% Heavy Vehicles, %HV	2	2	2	2	2	2	5	5	5	3	3	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.91	0.91	0.91	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	P	P	P	P	P	P	P	P	P	P	P	P
Start-up Lost Time, l ₁		2.0			2.0	2.0		2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0	2.0		2.0		2.0	2.0	
Arrival Type, AT		3			3	3		3		3	3	
Unit Extension, UE		3.0			3.0	3.0		3.0		3.0	3.0	
Filtering/Metering, I		1.000			0.849	0.849		0.861		1.000	1.000	
Initial Unmet Demand, Q _b		0.0			0.0	0.0		0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.5			11.0	10.0		15.0		10.0	11.0	
Parking / Grade / Parking	N	0	Y	N	0	Y	N	0	N	N	0	N
Parking Maneuvers, N _m			5			5						
Buses Stopping, N _b		0			0	0		2		0	2	
Min. Time for Pedestrians, G _p		11.5			13.5			15.2			12.1	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 22.0	G =	G =	G =	G = 28.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		16			382	433		682		279	287	
Lane Group Capacity, c		639			483	474		847		254	826	
v/c Ratio, X		0.03			0.79	0.91		0.81		1.10	0.35	
Total Green Ratio, g/C		0.37			0.37	0.37		0.47		0.47	0.47	
Uniform Delay, d ₁		12.1			16.9	18.1		13.7		16.0	10.2	
Progression Factor, PF		1.000			1.000	1.000		1.000		1.000	1.000	
Delay Calibration, k		0.50			0.50	0.50		0.50		0.50	0.50	
Incremental Delay, d ₂		0.1			10.7	21.8		7.0		85.3	1.2	