Chapter 22: Mitigation

A. INTRODUCTION

The preceding chapters of this environmental impact statement (EIS) discuss the potential for significant adverse impacts to result from the proposed Cornell NYC Tech project. Where such potential impacts have been identified—in the areas of historic and cultural resources, transportation (i.e., traffic, transit [bus line haul], and pedestrian conditions), and construction (i.e., construction-period transportation and noise impacts on open space)—measures are examined to minimize or eliminate the anticipated impacts to the fullest extent practicable. These mitigation measures are discussed below.

Areas in which the proposed project would result in significant adverse impacts that cannot be fully mitigated through reasonably practicable measures are discussed in Chapter 23, "Unavoidable Adverse Impacts."

In addition, this chapter analyzes the potential effects of the proposed traffic mitigation measures on pedestrian conditions.

B. HISTORIC AND CULTURAL RESOURCES

As discussed in Chapter 7, "Historic and Cultural Resources," the Goldwater Hospital complex has been determined eligible for listing on the State/National Registers of Historic Places (S/NR-eligible). The proposed project would demolish the Goldwater Hospital complex, which would constitute a significant adverse impact on this architectural resource. Cornell is consulting with the New York State Office of Parks, Recreation and Historic Preservation (OPRHP) and the Landmarks Preservation Commission (LPC) regarding appropriate measures to partially mitigate the significant adverse impact. These measures, which would include preservation of the Works Progress Administration (WPA) murals to the extent practicable, are being developed and will be implemented by Cornell, as set forth in a Letter of Resolution (LOR) to be signed by Cornell, OPRHP, LPC, and Roosevelt Island Operating Corporation (RIOC).

C. TRANSPORTATION

TRAFFIC

As discussed in Chapter 14, "Transportation," the proposed project would result in significant adverse traffic impacts at a number of locations in the traffic study area. This section describes the mitigation measures that could eliminate significant impacts. **Tables 22-1a** and **22-1b** summarize the significant adverse traffic impacts and identify if they could be fully or partially mitigated with the implementation of traffic improvement measures, or could not be mitigated.

Table 22-1a
Phase 1—2018 Analysis Year (2018 With Action Condition)
Traffic Impact Mitigation Summary

Intersections	AM Peak Hour	Midday Peak Hour	PM Peak Hour
No significant impact	7	10	10
Impact could be fully mitigated	6	4	4
Impact could be partially mitigated	0	0	0
Unmitigated impact	1	0	0

Table 22-1b Full Build—2038 Analysis Year (2038 With Action Condition) Traffic Impact Mitigation Summary

Intersections	AM Peak Hour	Midday Peak Hour	PM Peak Hour
	Alvi Feak Houl	Wilduay Feak Hour	Houi
No significant impact	4	7	3
Impact could be fully mitigated	5	3	7
Impact could be partially mitigated	0	0	0
Unmitigated impact	5	4	4

Details of the intersection capacity analyses and all traffic mitigation measures (e.g., signal timing changes, parking regulation changes, lane reconfigurations, etc.) are summarized in the level of service (LOS) tables presented in **Table 22-2** and **Table 22-3** at the end of the "Traffic" section.

The overall finding of the traffic mitigation analysis is that all but one of the intersections under the 2018 With Action condition and all but five under the 2038 With Action condition that would experience impacts could be fully mitigated with readily implementable traffic improvement measures, including signal timing and phasing changes, new traffic signals, parking regulation changes to gain or widen a travel lane at key intersections, and lane restriping. These measures represent some of the standard traffic capacity improvements that are typically implemented by the New York City Department of Transportation (NYCDOT). Additional review of potential mitigation measures that may fully or partially mitigate other significant impact locations that are identified as unmitigatable in this Draft EIS will be undertaken for the Final EIS.

The following sections describe the potential mitigation measures in detail.

PHASE 1-2018 ANALYSIS YEAR (2018 WITH ACTION CONDITION)

As shown in **Table 22-1a**, in the weekday AM peak hour, seven of the 14 intersections would be impacted and could be fully mitigated with the exception of one intersection; in the weekday midday peak hour, four intersections would be impacted and could be fully mitigated; and in the weekday PM peak hour, four intersections would be impacted and could be fully mitigated.

Traffic mitigation measures needed for each intersection are described below; details of signal timing modifications are summarized in **Table 22-2a** and **Table 22-2b**.

Roosevelt Island Bridge/36th Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared left-turn/through/right-turn lane movement would occur during all three peak hours. Impacts on the southbound Vernon Boulevard shared

left-turn/through/right-turn movement would occur during the AM peak hour. Both conditions could be mitigated by modifying the signal timing.

36th Avenue and 21st Street

Impacts on the eastbound 36th Avenue shared left-turn/through/right-turn movement would occur during the AM and midday peak hours. Impacts on the westbound 36th Avenue shared left-turn/through/right-turn movement would occur during the AM peak hour. Both conditions could be mitigated by modifying the signal timing.

Broadway and 21st Street

Impacts on the eastbound Broadway shared left-turn/through/right-turn movement would occur during the AM, midday, and PM peak hours. Impacts would be experienced during the same peak hours in the westbound direction for the same movements. Both conditions could be mitigated by modifying the signal timing.

41st Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared through/right-turn movement would occur during the PM peak hour. Impacts on the southbound Vernon Boulevard shared left-turn/through movement would occur during the AM peak hour. Both conditions could be mitigated by modifying the signal timing.

Broadway and Vernon Boulevard/11th Street

Impacts on the westbound Broadway shared left-turn/through/right-turn movement and southbound shared left-turn/through/right-turn movement would occur during the AM peak hour. These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

Astoria Boulevard/27th Avenue/Newtown Avenue and 21st Street

Impacts on the northbound 21st Street shared left-turn/through/right-turn movement would occur during the midday and PM peak hours. Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and midday peak hours. Both conditions could be mitigated by modifying the signal timing and signal phasing to allow an eastbound/westbound exclusive left-turn phase.

Hoyt Avenue South and 21st Street

Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM peak hour and could be mitigated by modifying the signal timing and allowing through movements and left turns from the 11-foot wide exclusive left-turn lane on the eastbound approach of Hoyt Avenue South.

FULL BUILD-2038 ANALYSIS YEAR (2038 WITH ACTION CONDITION)

As shown in **Table 22-1b**, in the weekday AM peak hour, 10 of the 14 intersections would be impacted, five of which could be fully mitigated and the other five could not be mitigated; in the weekday midday peak hour, seven intersections would be impacted, three of which could be fully mitigated and four could not be mitigated; and in the weekday PM peak hour, 11 intersections would be impacted, seven of which could be fully mitigated and four could not be mitigated.

Traffic mitigation measures needed for each intersection are described below; details of signal timing modifications are summarized **Table 22-3a** and **Table 22-3b**.

West Road and Main Street

Impacts on the eastbound West Road shared left-turn/right-turn movement would occur during the PM peak hour and could be mitigated by installing a traffic signal. Because installing a single traffic signal would not control all the traffic movements at this triangle-shaped intersection, and it is desirable to eliminate the observed, illegal northbound movements occurring against southbound traffic on the north leg of the triangle, it is recommended to "normalize" this intersection to eliminate superfluous vehicular turning conflicts and pedestrian conflicts so that the south leg no longer carries vehicular traffic and is "pedestrianized." This improvement would allow vehicular and pedestrian movements to occur at the intersection of West Road and Main Street and be under the control of a single new traffic signal. This would also provide unrestricted pedestrian access to the existing triangle from west of Main Street and east of West Road. It should be noted that this would divert existing trips (mainly passenger vehicles) that use the traffic triangle as a U-turn to one block south to the traffic circle at East Road; about 80 vehicles per hour in the AM peak hour and about 40 vehicles per hour in the midday and PM peak hours would be diverted in the 2038 Full Build condition. An analysis of the Main Street at East Road/West Road traffic circle with this traffic diversion is included in the detailed level of service summary tables at the end of the chapter. The mitigation currently identified will be further reviewed for the Final EIS by RIOC and NYCDOT. If the mitigation measures are not feasible, and no other measures are available to fully mitigate the impacts, the intersection may be identified as partially mitigated or unmitigatable in the Final EIS.

Roosevelt Island Bridge Ramp and Main Street

Impacts on the westbound Roosevelt Island Bridge Ramp shared left-turn/right-turn movement would occur during the AM peak hour. Impacts on the northbound Main Street right-turn lane would occur during the PM peak hour. Both conditions could be mitigated by installing a traffic signal. The mitigation currently identified will be further reviewed for the Final EIS by RIOC and NYCDOT. If the mitigation measures are not feasible, and no other measures are available to fully mitigate the impacts, the intersection may be identified as partially mitigated or unmitigatable in the Final EIS.

Roosevelt Island Bridge/36th Avenue and Vernon Boulevard

Impacts on the eastbound Roosevelt Island Bridge shared through/right-turn movement would occur during the PM peak hour. Impacts on the northbound Vernon Boulevard shared left-turn/through/right-turn movement would occur during all peak hours. In the southbound direction of Vernon Boulevard, the shared left-turn/through/right-turn movement would experience impacts during the AM and PM peak hours. These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

36th Avenue and 21st Street

Impacts were identified on the following approaches:

• The eastbound 36th Avenue shared left-turn/through/right-turn movement during all peak hours,

- The westbound 36th Avenue shared left-turn/through/right-turn movement during all peak hours,
- The northbound 21st Street shared left-turn/through/right-turn movement during the midday peak hour, and,
- The southbound 21st Street shared left-turn/through/right-turn movement during the AM peak hour.

Overall, the impacts could be mitigated by modifying the signal timing and making the following modifications:

- Shifting the eastbound approach centerline six feet to the north and restriping the approach from one 25-foot wide travel lane to one 11-foot wide exclusive left-turn lane and one 20-foot wide shared through/right-turn lane, with parking for a distance of 200 feet back from the intersection, and
- Shifting the westbound approach centerline six feet to the south and restriping the approach from one 25-foot wide travel lane to one 11-foot wide exclusive left-turn lane and one 20-foot wide shared through/right-turn lane with parking for a distance of 125 feet back from the intersection.

Broadway and 21st Street

Impacts were identified on the following approaches:

- The eastbound Broadway shared left-turn/through/right-turn movement during all peak hours,
- The westbound Broadway shared left-turn/through/right-turn movement during all peak hours,
- The northbound 21st Street shared left-turn/through/right-turn movement during the PM peak hour, and,
- The southbound 21st Street shared left-turn/through/right-turn movement during the AM peak hour.
- These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

36th Avenue and 31st Street

Impacts on the eastbound 36th Avenue shared left-turn/through/right-turn movement would occur during the midday and PM peak hours and could be mitigated by modifying the signal timing.

41st Avenue and Vernon Boulevard

Impacts on the northbound Vernon Boulevard shared through/right-turn movement would occur during the PM peak hour. Impacts on the southbound Vernon Boulevard shared left-turn/through movement would occur during the AM and PM peak hour. Both conditions could be mitigated by modifying the signal timing.

30th Avenue and 21st Street

An impact on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM peak hour. This impact is currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate the significant impact will be undertaken for the Final EIS.

Broadway and Vernon Boulevard/11th Street

Impacts on the westbound Broadway shared left-turn/through/right-turn movement would occur during all peak hours. Impacts on the southbound Vernon Boulevard shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

Astoria Boulevard/27th Avenue/Newtown Avenue and 21st Street

Impacts were identified on the following approaches:

- The eastbound Astoria Boulevard shared through/right-turn lane during the AM and PM peak hours,
- The westbound Astoria Boulevard shared through/right-turn lane during the PM peak hour,
- The northbound 21st Street shared left-turn/through/right-turn movement during the AM and midday peak hours,
- The northbound 21st Street shared through/right-turn lane during the PM peak hour, and
- The southbound 21st Street shared left-turn/through/right-turn movement during all peak hours.

Overall, the intersection could be mitigated by modifying the signal timing and signal phasing to allow an eastbound/westbound exclusive left-turn phase.

Hoyt Avenue North and 21st Street

Impacts were identified on the following approaches:

- The westbound Hoyt Avenue North left-turn lane during all peak hours,
- The northbound 21st Street through lane during the AM and PM peak hours, and
- The southbound 21st Street shared through/right-turn lane during the AM and PM peak hours.
- These impacts are currently identified as unmitigatable, but additional review of potential mitigation measures that may fully or partially mitigate these significant impacts will be undertaken for the Final EIS.

Hoyt Avenue South and 21st Street

Impacts on the northbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. Impacts on the southbound 21st Street shared left-turn/through/right-turn movement would occur during the AM and PM peak hours. Both conditions could be mitigated by modifying the signal timing and allowing through movements and left turns from the 11-foot wide exclusive left-turn lane on the eastbound approach of Hoyt Avenue South.

CONCLUSION

The overall finding of the traffic mitigation analysis is that all but one of the 14 intersections analyzed under the 2018 With Action condition and all but five under the 2038 With Action condition would either not be significantly impacted or could be fully mitigated with readily implementable traffic improvement measures, including signal timing and phasing changes, new traffic signals, parking regulation changes to gain or widen a travel lane at key intersections, and lane restriping. Additional review of potential mitigation measures that may fully or partially mitigate the significant impacts that are identified as unmitigatable will be undertaken for the Final EIS.

Table 22-2a 2018 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

	(Unsignal									alız	ed In	ters	ections)	
			No	Action			With	Action		V	Vith N	litigatio	<u>n</u>	
				Control				Control				Control		<u>Mitigation</u>
Intersection	Approach	M∨t.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	<u>Measure</u>
				AM Pe	ak Hou	ır								
1. EAST/WEST MAIN STREE	T & MAIN STRE	ET												
West Road	EB	LT	-	7.1	Α	LT	-	7.4	Α	-	-	-	-	-Mitigation
Main Street	SB	LR	-	7.3	Α	LR	-	8.3	Α	-	-	-	-	not
	Overall Interse	ection	-	7.3	Α	-	-	8.3	Α	-	-	-	-	required.
2. WEST ROAD & MAIN STR														
West Road	EB	LR	-	9.1	Α	LR	-	10.4	В	-	-	-	-	
West Road (south of island)	EB	LR	-	11.3	В	LR	-	12.5	В	-	-	-	-	-Mitigation
Main Street	NB	LT	-	9.9	Α	LT	-	10.6	В	-	-	-	-	not
Wall Street	SB	TR	-	9.3	Α	TR	-	11.7	В	-	-	-	-	required.
	Overall Interse		-	9.8	Α	-	-	11.3	В	-	-	-	-	
3. ROOSEVELT ISLAND BRI	DGE RAMP & N	<u>IAIN S</u>	TREE	Ţ										
Roosevelt Island Bridge Ramp	WB	LR	-	14.6	В	LR	-	23.2	С	-	-	-	-	B. 4111
	NB	Т	-	10.2	В	Т	•	10.7	В	•	-	-	•	-Mitigation
Main Street		R	-	10.8	В	R	-	13.2	В	-	-	-	-	not required.
	SB	LT	-	12.2	В	LT	-	13.5	В	-	-	-	-	required.
	Overall Interse	ection	-	12.8	В	•	•	18.0	C	•	-	-	-	
4. ROOSEVELT ISLAND BR	IDGE & MOTOR	GATE	GARA	GE ENT	RANCE	/ EXI	T							
Roosevelt Island Bridge	EB	LT	-	8.4	Α	Ľ	•	8.8	Α	•	-	-	•	-Mitigation
Motorgate Garage Exit	NB	LR	-	11.2	В	LR	•	11.9	В	•	-	-	-	not
	Overall Interse	ection	-	1.4	Α	•	•	1.4	Α	•	-	-	-	required.
Midday Peak Hour														
1. EAST/WEST MAIN STREE	T & MAIN STRE	ET												
West Road	EB	LT	-	7.6	Α	LT	-	7.9	Α	-	-	-	-	-Mitigation
Main Street	SB	LR	-	7.3	Α	LR	-	8.3	Α	-	-	-	-	not
	Overall Interse	ction	-	7.4	Α	-		8.2	Α		-	-	-	required.
2. WEST ROAD & MAIN STR	EET													
West Road	EB	LR	-	8.4	Α	LR	-	10.1	В	-	-	-	-	
West Road (south of island)	EB	LR	-	10.7	В	LR	-	11.9	В	-	-	-	-	-Mitigation
Main Chroat	NB	LT	-	9.2	Α	LT	-	10.1	В	-	-	-	-	not
Main Street	SB	TR	-	8.6	Α	TR	-	10.8	В	-	-	-	-	required.
	Overall Interse	ection	-	9.2	Α	-	-	10.6	В	-	-	-	-	
3. ROOSEVELT ISLAND BRI	DGE RAMP & N	IAIN S	TREE	T										
Roosevelt Island Bridge Ramp	WB	LR	-	10.1	В	LR	-	13.7	В	_	_	_	_	
	NB	Т	-	9.2	Α	Т	-	9.8	Α	-	-	-	-	-Mitigation
Main Street		Ŕ	-	9.0	Α	R	-	11.4	В	-	-	-	-	not
	SB	LT	-	10.5	В	LT	-	11.7	В	-	-	-	-	required.
	Overall Interse	ection	-	9.8	A	-	-	12.2	В	-	-	-	-	
4. ROOSEVELT ISLAND BRI			GARA					,				1		
Roosevelt Island Bridge	EB	LT	-	7.7	A	LT	-	7.9	Α	-	-	-	-	-Mitigation
Motorgate Garage Exit	NB	LR	-	9.9	Α	LR	-	10.6	В	-	-	-	-	not
	Overall Interse		-	0.9	Α	-	-	0.7	Α	-	-	-	-	required.
				PM Pe		r								
1. EAST/WEST MAIN STREE			ı				1			1	1	ı		
West Road	EB	LT	-	7.4	A	LT	-	7.6	A	-	-	-	-	-Mitigation
Main Street	SB	LR	-	7.2	Α	LR	-	8.0	Α	-	-	-	-	not
	Overall Interse	ection	-	7.3	Α	-	-	7.9	Α	-	-	-	-	required.
2. WEST ROAD & MAIN STR				_			1			1	1			
West Road	EB	LR	-	8.7	A	LR	-	13.0	В	-	-	-	-	
West Road (south of island)	EB	LR	-	10.6	В	LR	-	11.4	В	-	-	-	-	-Mitigation
Main Street	NB	LT	-	9.9	Α	LT	-	11.9	В	-	-	-	-	not
	SB	TR	-	8.7	A	TR	-	11.1	В	-	-	-	-	required.
	Overall Interse	ection	-	9.6	Α	-	-	12.0	В	-	-	-	-	

Table 22-2a (cont'd) 2018 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

		No Action					With	Action		V	/ith N	litigatio	<u>n</u>	
				Control				Control				Control		Mitigation
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	<u>Measure</u>
			PM	Peak Hou	ır (con	tinue	d)							
3. ROOSEVELT ISLAND BRI	DGE RAMP & N	IAIN S	TREE	Τ										
Roosevelt Island Bridge Ramp	WB	LR	1	11.0	В	LR	-	13.9	В	-	-	1	-	Midweller
	NB	Т	-	9.6	Α	Т	•	10.1	В	-	-	-	-	-Mitigation
Main Street		R	-	9.6	Α	R	-	13.0	В	-	-	-		not required.
	SB	LT	-	14.2	В	LT	-	16.7	С	-	-	-		required.
	Overall Interse	ction	-	11.9	В	-	-	14.4	В	-	-	-		
4. ROOSEVELT ISLAND BRI	DGE & MOTOR	GATE	GARA	GE ENTF	RANCE	/EXI	T							
Roosevelt Island Bridge	EB	LT	-	7.9	Α	LT	-	8.1	Α	-	-	-		-Mitigation
Motorgate Garage Exit	NB	LR	-	12.5	В	LR	-	14.1	В	-	-	-		not
_	Overall Interse	ction	-	1.0	Α	-	•	0.9	Α	-	-	-	-	required.

Notes:
(1) Control delay is measured in seconds per vehicle.
(2) Overall intersection V/C ratio is the critical lane groups' V/C ratio.

Table 22-2b 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

No Action	nal timing: n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
Intersection Approach Myt. V/C Delay LOS Myt. V/C Delay LOS Myt. V/C Delay LOS Myt. Myt. V/C Delay LOS Myt. Myt. U/C Delay LOS U/C LOS U/C LOS U/C U/C	nal timing: n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
AM Peak Hour	nal timing: n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
S. ROOSEVELT ISLAND BRIDGE / 36TH AVENUE & VERNON BOULEVARD Roosevelt Island Bridge EB	n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
Roosevelt Island Bridge EB	n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
TR 0.59 16.9 B TR 0.67 19.0 B TR 0.80 28.0 C Shift 4's gree EB/WB havenue WB LTR 0.37 13.7 B LTR 0.44 14.7 B LTR 0.59 21.3 C EB/WB havenue SB LTR 1.12 75.0 E LTR 1.18 1.18 1.10 71.1 E* LTR 0.94 24.3 C Shifts from 2 C Shifts from 2 SB LTR 1.06 57.4 E LTR 1.10 71.1 E* LTR 0.94 24.3 C Shifts from 2 SB LTR 1.06 57.4 E LTR 1.10 71.1 E* LTR 0.94 24.3 C Shifts from 2 SB LTR 0.73 44.1 D LTR 0.90 58.2 E* LTR 0.97 32.6 C Shifts from 2 SB LTR 0.91 48.0 D LTR 0.97 55.6 E* LTR 0.91 46.1 D SB LTR 0.91 48.0 D LTR 0.97 55.6 E* LTR 0.91 46.1 D SB LTR 0.98 28.9 C LTR 1.00 33.4 C LTR 1.03 43.4 D Shifts from 2 SB LTR 0.98 28.9 C LTR 1.00 33.4 C LTR 1.03 43.4 D Shifts from 2 Shifts from 3 Shifts from 6 Shifts from	n time from e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
Set	e to NB/SB WB green m 25 s to 21 reen time 5 s to 29 s].
Vernon Boulevard NB	WB green m 25 s to 21 reen time 5 s to 29 s].
SB	m 25 s to 21 reen time 5 s to 29 s].
Signature Columbia Columbia	reen time 5 s to 29 s].
6. 36TH AVENUE & 21ST STREET 36th Avenue	
Set Avenue	al timing:
WB	val timina.
NB	
SB	
Overall Intersection	
New Color New Color New Color New Color New Color	
T. BROADWAY & 21ST STREET	
Broadway EB	3 s to 71 s].
WB	
21st Street	
SB	
Overall Intersection O.98 36.5 D - 1.01 40.7 D - 1.00 41.7 D time shifts from 6	
Overall Intersection 0.98 36.5 D - 1.01 40.7 D - 1.00 41.7 D s; NB/SB c shifts from 6 8. 36TH AVENUE & 31ST STREET 36th Avenue EB LTR 0.68 32.0 C LTR 0.70 32.7 C -	
8. 36TH AVENUE & 31ST STREET 36th Avenue	
36th Avenue EB LTR 0.68 32.0 C LTR 0.70 32.7 C	
0.68 32.0 0.70 32.7	
0.68 32.0 0.70 32.7	
WB	
31st Street NB	nt required
SB LTR 0.65 17.6 B LTR 0.65 17.6 B - - - - Overall Intersection 0.66 22.5 C - 0.68 23.1 C - - - -	n required.
9. 41ST AVENUE & VERNON BOULEVARD	
41st Avenue WB LR 0.26 16.0 B LR 0.27 16.1 B LR 0.30 17.8 B -Modify sig	
Vernon Boulevard NB TR 0.65 13.1 B TR 0.69 14.0 B TR 0.65 12.0 B shift 1.8 s g	
SB LT 1.06 46.1 D LT 1.09 57.7 E* LT 1.03 34.9 C from WB pha	
shifts from 3	
Overall Intersection 0.75 31.7 C - 0.77 38.2 D - 0.77 25.1 C S;WB green	
from 19.8	
10. 30TH AVENUE & 21ST STREET	
30th Avenue	
30th Avenue	
SB LTR 1.00 30.9 C LTR 1.01 34.4 C	at required
Overall Intersection 0.90 28.3 C - 0.91 30.3 C	ot required.
11. BROADWAY & VERNON BOULEVARD / 11TH STREET	ot required.
Park Entrance EB LTR 0.01 28.2 C LTR 0.01 28.2 C	ot required.
Broadway WB LTR 1.04 62.8 E LTR 1.06 71.8 E*	ot required.
Vernon Boulevard NB LT 0.25 7.9 A LT 0.25 7.9 A	ot required.
R 0.04 6.4 A R 0.05 6.4 A Unmitigatab	ot required.
SB LTR 1.02 58.3 E LTR 1.04 65.0 E*	
11th Street NB LTR 0.38 41.2 D LTR 0.38 41.2 D	
Overall Intersection 1.01 47.6 D - 1.03 52.9 D	

Table 22-2b (cont'd)
2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison
(Signalized Intersections)

											ed Intersections)			
<u> </u>			No A	Action			With	Action			With M	itigation		
	A	NA4	WO	Control	100	NA: -4	V//C	Control	100	NA4	\//C	Control		Mitimation Massacc
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
12. ASTORIA BOULEVA	DD / 27TU AVE	NIIE / N	JEW/TA				r (cont							
Astoria Boulevard	EB	NUE / N	0.84	61.6	E E	1612	0.84	61.6	E	1 1	0.78	48.7	D	-Modify signal phasing:
ASIONA DOUIEVANO	ED	TR	0.84	54.6	D	TR	0.84	55.4	E	TR	0.78	34.6	С	Add a new lag phase for
	WB	L	0.88	63.8	E	L	0.87	63.8	E	L	0.55	59.8	E	the EB/WB exclusive left
	VVD	TR	0.86	46.8	D	TR	0.86	47.0	D	TR	0.65	35.7	D	turns. The existing signal
21st Street	NB	LTR	0.86	39.2	D	LTR	0.89	42.1	D	LTR	0.74	29.2	C	phasing [WB has 30 s
2131 011001	SB	LTR	1.08	72.0	E	LTR	1.10	81.6	F*	LTR	0.99	35.7	D	green time; EB has 25 s
	0.5		1.00	72.0			1.10	01.0			0.00	00.7		green time; NB/SB has
														50 s green time] would be
														modified to have the following: EB/WB will
														have 39 s green time;
	Overall Interse	oction	1.00	58.7	E	_	1.01	63.0	Е	_	0.99	37.7	D	EB/WB exclusive left-turn
	Overall lillers	ection	1.00	30.7	-	_	1.01	03.0	_	_	0.99	31.1	, D	phase will have 10 s
														green time; NB/SB will
														have 56 s green time
														[each phase will have 3 s
42 HOVE AVENUE NOD	TU 9 24 CT CTC	CCT			<u> </u>			l]		<u> </u>	l		amber and 2 s all red].
13. HOYT AVENUE NOR	EB		0.02	40.4	D		0.02	40.4	D	-	1	1		1
Hoyt Avenue North	EB	L	0.02	40.4 47.5	D	L R	0.02	40.4 47.5	D	-	-	-	-	-
	WB	R L	0.37	47.5	D	L	0.37	47.5	D	-	-	-	-	-
	VVD	TR	0.90	14.8	В	TR	0.92	14.8	В	-	-	-	-	1
21st Street	NB	L	0.25	31.5	С	L	0.25	31.7	С	-	-	-	-	-Mitigation not required.
2100 00000	140	T	1.04	85.7	F	T	1.04	85.7	F		-		-	†
	SB	TR	1.00	53.9	D	TR	1.04	55.8	E	- -	-	-	-	†
	Overall Interse		0.85	53.1	D	-	0.86	54.2	D	1	-	-	-	1
14. HOYT AVENUE SOU			0.00	55.1		1	0.00	J-7.2		1	<u> </u>	1	1	I.
Hoyt Avenue South	EB	L	0.13	30.0	С	L	0.13	30.0	С	LTR	0.61	36.2	D	-Restripe EB approach of
		TR	1.06	75.0	E	TR	1.06	75.0	E	-	-	-	-	Hoyt Avenue South from
21st Street	NB	LTR	0.55	15.1	В	LTR	0.55	15.2	В	LTR	0.54	14.6	В	one 11-ft exclusive left-
	SB	LTR	1.03	46.1	D	LTR	1.05	52.3	D*	LTR	1.03	45.5	D	turn lane and one 11-ft
		•												shared through-right lane
														to two 11-ft shared left-
														through-right lanes for 250 ft.
														250 πModify signal timing:
	Overall Interse	oction	1.04	42.3	D	_	1.05	45.7	D	_	0.89	35.5	D	shift 1 s green time from
	Overall litters	CCHOIL	1.04	42.3	'	-	1.03	43.7	٠	-	0.09	33.3	"	EB phase to NB/SB
														phase [EB green time
														shifts from 37 s to 36 s;
														NB/SB green time shifts
					L	ما ما د د ۲۰۰۰	aak III:	<u> </u>			<u> </u>	<u> </u>		from 73 s to 74 s].
E DOOGEVELT IOLAND	BRIDGE / SCT	1 A\/E**	IIE e v	EDNO			eak Ho	ur						
5. ROOSEVELT ISLAND								12.4	Р		0.24	17.0	В	Modify oignal timin
Roosevelt Island Bridge	EB	L TD	0.22	12.4	В	L	0.28	13.1	В	L	0.34	17.0	С	-Modify signal timing: shift 4 s green time from
26th Avenue	WB	TR LTR	0.41	14.3 13.5	B B	TR LTR	0.53	16.3 15.1	B B	TR LTR	0.64	22.0 21.3	C	EB/WB phase to NB/SB
36th Avenue Vernon Boulevard	NB				С	LTR	1.06	15.1 62.9	E*	LTR				phase [EB/WB green
vernon boulevard	SB	LTR LTR	0.89	26.6	В		0.72	20.3	E [*]		0.88	22.7 14.6	C B	time shifts from 25 s to 21
				19.0		LTR				LTR		14.0	_ D	s; NB/SB green time
	Overall Interse	ection	0.65	19.4	В	-	0.80	31.1	С	-	0.78	19.8	В	shifts from 25 s to 29 s].
6. 36TH AVENUE & 21S														
36th Avenue	EB	LTR	0.78	46.5	D	LTR	0.97	71.5	E*	LTR	0.84	47.9	D	-Modify signal timing:
	WB	LTR	0.86	50.5	D	LTR	0.89	53.7	D	LTR	0.78	42.3	D	shift 4 s green time from
21st Street	NB	LTR	0.67	17.3	В	LTR	0.79	21.5	С	LTR	0.85	27.0	С	NB/SB phase to EB/WB
	SB	LTR	0.61	16.1	В	LTR	0.62	16.5	В	LTR	0.66	19.5	В	phase [EB/WB green
	Overell late		0.70	24.2			0.05	20.7	_		0.05	20.0	_	time shifts from 37 s to 41 s; NB/SB green time
	Overall Interse	ection	0.73	24.2	С	-	0.85	29.7	С	-	0.85	28.8	С	shifts from 73 s to 69 s].
	l				l									ərintə ironi 73 8 to 69 S].

Table 22-2b (cont'd) 2018 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

												` 0		ed intersections)
			No A	<u>Action</u>			With	Action			With M	itigation		
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
				M	lidday I	Peak He	our (co	ntinued))					
7. BROADWAY & 21S	T STREET													
Broadway	EB	LTR	0.95	64.9	Е	LTR	0.98	71.5	E*	LTR	0.95	63.7	Е	-Modify signal timing:
	WB	LTR	1.01	77.1	E	LTR	1.02	80.7	F*	LTR	0.97	66.9	E	shift 1 s green time from
21st Street	NB	LTR	0.78	22.2	С	LTR	0.80	22.7	С	LTR	0.81	23.7	С	NB/SB phase to EB/WB
	SB	LTR	0.74	21.2	С	LTR	0.78	22.5	С	LTR	0.79	23.5	С	phase [EB/WB green
	Overall Inters	ection	0.85	32.7	С	-	0.87	34.5	С	-	0.86	32.9	С	time shifts from 31 s to 32 s; NB/SB green time shifts from 69 s to 68 s].
8. 36TH AVENUE & 31	ST STREET				1	1			1				<u> </u>	
36th Avenue	EB	LTR	0.81	35.8	D	LTR	0.82	36.8	D	-	-	-	-	
	WB	LTR	0.74	33.2	С	LTR	0.76	34.1	С	-	-	-	-	1
31st Street	NB	LTR	0.57	16.2	В	LTR	0.57	16.2	В	-	-	-	-	-Mitigation not required.
	SB	LTR	0.48	14.4	В	LTR	0.48	14.4	В	-	-	-	-	1 3 1 1 1 1
	Overall Inters	ection	0.66	23.9	С	-	0.67	24.4	С	-	-	-	-	1
9. 41ST AVENUE & VE	RNON BOULEV	ARD												•
41st Avenue	WB	LR	0.18	15.2	В	LR	0.18	15.2	В	LR	0.20	16.7	В	-Mitigation not required.
Vernon Boulevard	NB	TR	0.66	13.6	В	TR	0.70	14.4	В	TR	0.66	12.3	В	-Modify signal timing:
	SB	LT	0.64	13.2	В	LT	0.68	14.0	В	LT	0.64	12.1	В	shift 1.8 s green time
	Overall Inters	ection	0.48	13.6	В	-	0.50	14.3	В	-	0.50	12.5	В	from WB phase to NB/SE phase [NB/SB green time shifts from 31.8 s to 33.6 s; WB green time shifts from 19.8 s to 18 s]. [Measures reflect improvements needed for
														the weekday AM and PM peak periods.]
10. 30TH AVENUE & 2														T
30th Avenue	EB	LTR	0.33	34.4	С	LTR	0.33	34.4	С	-	-	-	-	
	WB	LTR	0.50	38.9	D	LTR	0.50	38.9	D	-	-	-	-	
21st Street	NB	LTR	0.72	18.6	В	LTR	0.73	19.1	В	-	-	-	-	-Mitigation not required.
	SB	LTR	0.78	20.0	В	LTR	0.80	20.7	С	-	-	-	-	
	Overall Inters		0.68	21.5	С	-	0.70	21.9	С	-	-	-	-	
11. BROADWAY & VE					_		0.05		_					T
Park Entrance	EB	LTR	0.02	26.2	C	LTR	0.02	26.2	C	-	-	-	-	
Broadway	WB	LTR	0.89	47.0	D	LTR	0.91	49.2	D	-	-	-	-	
Vernon Boulevard	NB	LT	0.26	8.3	Α	LT	0.27	8.4	Α	-	-	-	-	-Mitigation not required.
		R	0.17	7.6	Α	R	0.18	7.6	Α	-	-	-	-	gation not roquilou.
	SB	LTR	0.56	27.2	С	LTR	0.57	27.7	С	-	-	-	-	
11th Street	NB	LTR	0.22	32.9	С	LTR	0.22	32.9	С	-	-	-	-	
	Overall Inters	ection	0.72	25.6	С	-	0.74	26.3	С	-	-	-	-	

Table 22-2b (cont'd)

2018 No Action vs With Action Traffic Levels of Service Comparison (Signalized Intersections)

			No A	Action			With	Action		1	With Mi	itigation		
		-	.10 /	Control		1	**1011	Control		 	· · · · · · · · · · · · · · · · · · ·	Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
	7.450.000			,				ntinued)			.,.	20.00		ganonoaoa.o
12. ASTORIA BOULEVA	RD / 27TH AVE	NUF/N	IFWTO											
Astoria Boulevard	EB	L	0.26	34.9	С	L	0.26	34.9	С	L	0.26	24.0	С	-Modify signal phasing:
		TR	0.40	36.3	D	TR	0.41	36.5	D	TR	0.36	32.0	C	Add a new lag phase for
	WB	L	0.86	53.0	D	L	0.86	53.0	D	L	0.87	47.3	D	the EB/WB exclusive left
		TR	0.43	36.2	D	TR	0.44	36.3	D	TR	0.38	31.9	С	turns. The existing signal
21st Street	NB	LTR	1.13	102.1	F	LTR	1.17	121.8	F*	LTR	0.67	25.2	С	phasing [WB has 34 s
	SB	LTR	1.00	56.1	Е	LTR	1.04	65.9	E*	LTR	0.68	25.5	С	green time; EB has 34 s green time; NB/SB has
	Overall Interse		0.81	63.9	E		0.82	73.0	E	-	0.79	29.4	С	37 s green time, Nb/Sb rias 37 s green time] would be modified to have the following: EB/WB will have 39 s green time; EB/WB exclusive left-turn phase will have 10 s green time; NB/SB will have 56 s green time [each phase will have 3 s amber and 2 s all red].
13. HOYT AVENUE NOR	TH & 21ST STR	REET	•						•			•		-
Hoyt Avenue North	EB	L	0.11	42.0	D	L	0.11	42.0	D	-	-	-	-	
		R	0.13	42.5	D	R	0.13	42.5	D	-	-	-	-	
	WB	L	0.69	38.5	D	L	0.72	39.3	D	-	-	-	-	
		TR	0.17	14.2	В	TR	0.17	14.2	В	-	-	-	-	Mitigation not required
21st Street	NB	L	0.11	25.2	С	L	0.11	25.2	С	-	-	-	-	,g
	0.0	T	0.77	43.0	D	T	0.77	43.1	D	-	-	-	-	
	SB	TR	0.57	33.4	С	TR	0.58	33.5	С	-	-	-	-	
14. HOYT AVENUE SOU	Overall Interse		0.61	36.6	D	-	0.62	37.1	D	-	-	-	-	
Hoyt Avenue South	EB	LEEI	0.21	31.6	С		0.21	31.6	С	LTR	0.32	32.8	С	-Mitigation not required.
Hoyt Avenue South	LD	TR	0.41	35.5	D	TR	0.41	35.5	D	LIK	-	-	-	-Restripe EB approach of
21st Street	NB	LTR	0.43	13.3	В	LTR	0.44	13.4	В	LTR	0.44	13.4	В	Hoyt Avenue South from
2101 011001	SB	LTR	0.61	15.9	В	LTR	0.62	16.2	В	LTR	0.62	16.2	В	one 11-ft exclusive left-
	Overall Interse	ection	0.54	17.8	В	-	0.55	17.9	В	-	0.52	17.7	В	turn lane and one 11-ft shared through-right lane to two 11-ft shared left- through-right lanes for 250 ft. [Measures reflect improvements needed for the weekday AM peak period.]
						PM Pea	k Hour							
5. ROOSEVELT ISLAND		I AVEN				EVARD		1						
Roosevelt Island Bridge	EB	L	0.46	14.3	В	L	0.51	15.1	В	L	0.60	19.5	В	-Modify signal timing:
00th A	14/5	TR	0.59	15.6	В	TR	0.78	19.8	В	TR	0.92	33.2	С	shift 4 s green time from EB/WB phase to NB/SB
36th Avenue	WB	LTR	0.28	12.8	В	LTR	0.34	13.6	B F*	LTR	0.50	20.3	С	phase [EB/WB green
Vernon Boulevard	NB SB	LTR	1.15	88.6	F	LTR	1.39	194.5		LTR	1.10	63.5	E B	time shifts from 25 s to 21
	SB	LTR	0.85	25.3	C	LTR	0.87	27.4	C	LTR	0.75	17.5		s; NB/SB green time
	Overall Interse	ection	0.88	42.8	D	-	1.09	77.4	E	-	1.03	36.4	D	shifts from 25 s to 29 s].
6. 36TH AVENUE & 21S				1				1						
		LITD	0.51	35.1	D	LTR	0.78	42.0	D	-	-	-	-	
36th Avenue	EB	LTR		-		LTR	0.83	48.4	D	-	-	-	-	
	WB	LTR	0.79	45.5	D			,	_					A Abd a control of the control of th
21st Street	WB NB	LTR LTR	0.79 0.92	24.8	С	LTR	0.92	24.8	С	-	-	-	-	-Mitigation not required.
	WB NB SB	LTR LTR LTR	0.79 0.92 0.69	24.8 17.8	C B	LTR LTR	0.92 0.70	18.2	В	-	-	-	-	-Mitigation not required.
21st Street	WB NB SB Overall Interse	LTR LTR LTR	0.79 0.92	24.8	С	LTR	0.92							-Mitigation not required.
21st Street 7. BROADWAY & 21ST	WB NB SB Overall Interse	LTR LTR LTR ection	0.79 0.92 0.69 0.87	24.8 17.8 25.5	С В С	LTR LTR	0.92 0.70 0.89	18.2 26.7	В С	-	-	-	-	
21st Street	WB NB SB Overall Interse STREET EB	LTR LTR LTR ection	0.79 0.92 0.69 0.87	24.8 17.8 25.5	С В С	LTR LTR -	0.92 0.70 0.89	18.2 26.7 120.4	В С	- - LTR	1.12	102.5	- - F	-Modify signal timing:
21st Street 7. BROADWAY & 21ST : Broadway	WB NB SB Overall Interse STREET EB WB	LTR LTR LTR ection	0.79 0.92 0.69 0.87 1.13 1.17	24.8 17.8 25.5 107.6 125.7	C B C F F	LTR LTR - LTR LTR	0.92 0.70 0.89 1.16 1.19	18.2 26.7 120.4 134.4	В С F	LTR	- - 1.12 1.13	- 102.5 108.0	- - F	
21st Street 7. BROADWAY & 21ST	WB NB SB Overall Interse STREET EB	LTR LTR LTR ection	0.79 0.92 0.69 0.87	24.8 17.8 25.5	С В С	LTR LTR -	0.92 0.70 0.89	18.2 26.7 120.4	В С	- - LTR	1.12	102.5	- - F	-Modify signal timing:

Table 22-2b (cont'd)

2018 No Acti		1		Action				Action				itigation		l ·
			NO A				with				with ivi		1	
Intanaatian	A	84.4	V//C	Control		NA4	V//C	Control	LOS	NA. 4	V//C	Control	LOS	Mission asian Managemen
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LUS	Mvt.	V/C	Delay	LUS	Mitigation Measure
					PINI PE	ak Hou	r (cont	inuea)						
8. 36TH AVENUE & 31		LTD	0.00	00.0		LTD	0.05	040	_	1		1		ı
36th Avenue	EB	LTR	0.80	32.0	С	LTR	0.85	34.9	С	-	-	-	-	-
04 - 1 - 01 1	WB	LTR	0.71	31.6	С	LTR	0.73	32.2	С		-		-	NACCO CONTRACTOR OF THE PARTY O
31st Street	NB	LTR	0.69	19.0	В	LTR	0.70	19.2	В	-	-	-	-	 -Mitigation not required.
	SB	LTR	0.48	14.5	В	LTR	0.48	14.5	В	-	-	-	-	
	Overall Inters		0.73	23.2	С	-	0.76	24.2	С	-	-	-	-	
9. 41ST AVENUE & VE					_			40.0	_				_	1 14 17 1 141 1
41st Avenue	WB	LR	0.26	16.1	В	LR	0.32	16.9	В	LR	0.35	18.7	В	-Modify signal timing:
Vernon Boulevard	NB	TR	1.03	39.7	D	TR	1.05	47.7	D*	TR	1.00	29.9	С	shift 1.8 s green time from WB phase to NB/SI
	SB	LT	0.93	24.7	С	LT	0.99	35.4	D	LT	0.88	18.5	В	phase [NB/SB green tim
	Overall Inters	ection	0.73	31.8	С	-	0.77	40.1	D	-	0.77	24.3	С	shifts from 31.8 s to 33.6 s; WB green time shifts from 19.8 s to 18 s
10. 30TH AVENUE & 2	1ST STREET		1			1				l		l	l .	110111 13.0 3 to 10 3
30th Avenue	EB	LTR	0.32	34.1	С	LTR	0.33	34.2	С	_	_	_		
JOHN / WORKE	WB	LTR	0.32	38.0	D	LTR	0.33	38.0	D	-	-	-	-	
21st Street	NB	LTR	0.48	20.6	С	LTR	0.48	21.4	C	-			-	-Mitigation not required.
2131 311661	SB	LTR	0.63	16.5	В	LTR	0.65	16.8	В	-			-	-willigation not required.
	Overall Inters		0.70	20.8	C	-	0.71	21.2	C	-	-	-	-	
11. BROADWAY & VE					C	_	0.71	21.2	C	_	-	-	-	
Park Entrance	EB	LTR	0.03	33.2	С	LTR	0.03	33.2	С	_	-	Ι.	-	
Broadway	WB	LTR	0.84	52.6	D	LTR	0.86	54.2	D	-	-	-	-	
Vernon Boulevard	NB	LT	0.46	9.3	A	LT	0.86	9.4	A	-	-	-	-	1
vernon boulevaru	IND	R	0.46	6.3	A	R	0.47	6.4	A	-		-	-	Mitigation not required
	SB	LTR	0.13	29.3	C	LTR	0.14	29.6	C	-	-	-	-	 -Mitigation not required.
11th Ctroot	NB	LTR	0.02	38.3	D	LTR	0.03	38.3	D	-	-	-	-	1
11th Street	Overall Inters		0.33	24.6	C	LIK	0.33	24.9	C	-	-	-	-	
12. ASTORIA BOULE\						2467	STREE		U	_	-	_		
Astoria Boulevard	EB		0.47	42.4	D D		0.47	42.4	D	L	0.41	28.0	С	Modify signal phosing:
ASiona boulevaru	ED	L				L							D	-Modify signal phasing: Add a new lag phase for
	WB	TR	0.78	48.9	D E	TR	0.80	49.6	D E	TR	0.57	35.1	D	the EB/WB exclusive lef
	WB	L	0.89	64.8		L	0.89	64.8		L	0.76	43.9	С	turns. The existing signa
04 -4 044	ND	TR	0.78	51.5	D	TR	0.78	51.7	D E*	TR	0.45	32.9	D	phasing [WB has 24 s
21st Street	NB	LTR	1.04	54.2	D D	LTR	1.08	69.6	E* D	LTR	0.99	38.3	С	green time; EB has 28 s
	SB	LTR	0.90	36.3	U	LTR	0.93	38.1	U	LTR	0.88	32.7	C	green time; NB/SB has 53 s green time] would b modified to have the
	Overall Interse	ection	0.94	48.0	D		0.96	53.6	D	-	0.93	35.5	D	following: EB/WB will have 39 s green time; EB/WB exclusive left-tur phase will have 10 s
														green time; NB/SB will have 56 s green time [each phase will have 3 amber and 2 s all red].
13. HOYT AVENUE NO	ORTH & 21ST STE	REET												
Hoyt Avenue North	EB	L	0.09	41.8	D	L	0.09	41.8	D	-	-	-	-	
		R	0.17	43.1	D	R	0.17	43.1	D	-	-	-	-	
	WB	L	0.61	36.8	D	L	0.63	37.3	D	-	-	-	-	
		TR	0.29	15.7	В	TR	0.29	15.7	В	-	-	-	-	Mitigation
21st Street	NB	L	0.17	26.1	С	L	0.17	26.1	С	-	-	-	-	-Mitigation not required
		Т	1.09	90.0	F	Т	1.09	92.4	F	-	-	-	-	1
	SB	TR	0.76	39.0	D	TR	0.76	39.0	D	-	-	-	-	1

Table 22-2b (cont'd)

2018 No Action vs With Action Traffic Levels of Service Comparison (Signalized Intersections)

			No A	<u>Action</u>			With	Action		1	With M	itigation		
Intersection	Approach	Mvt.	V/C	Control Delay	LOS	Mvt.	V/C	Control Delay	LOS	Mvt.	V/C	Control Delay	LOS	Mitigation Measure
					PM Pe	ak Hou	r (cont	inued)						
14. HOYT AVENUE SO	UTH & 21ST STR	EET												
Hoyt Avenue South	EB	L	0.17	30.8	С	L	0.17	30.8	С	LTR	0.47	34.5	С	-Mitigation not required.
		TR	0.75	44.3	D	TR	0.75	44.3	D	-	-	-	-	-Restripe EB approach of
21st Street	NB	LTR	0.92	26.3	С	LTR	0.94	29.5	С	LTR	0.94	29.5	С	Hoyt Avenue South from
	SB	LTR	0.89	28.0	С	LTR	0.91	29.3	С	LTR	0.91	29.3	С	one 11-ft exclusive left-
	Overall Interse	ction	0.86	29.4	С	-	0.87	31.3	С	-	0.78	30.2	С	turn lane and one 11-ft shared through-right lane to two 11-ft shared left- through-right lanes for 250 ft. [Measures reflect improvements needed fo the weekday AM peak period.]

Table 22-3a 2038 No Action, With Action, and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

	No Action										_ \	0		zeu intersections)
			No Ac				With	Action			With I	Mitigation	1	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay		Mvt.		Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
					AM	Peak	Hour							
1. EAST/WEST ROAD & MAIN	STREET													
West Road	EB	LT	-	7.2	Α	LT	-	8.0	Α	LT	-	8.2	Α	 Mitigation not required.
Main Street	SB	LR	-	7.4	Α	LR	-	11.3	В	LR	-	13.9	В	-Conditions shown reflect
	Overall Inte	ersection	-	7.3	A	-	-	11.2	В	-	-	13.9	В	additional U-turns that would use this intersection because of the proposed elimination of the traffic triangle at West Road and Main Street.
2. WEST ROAD & MAIN STRE	ET			ı				l l						
West Road	EB	LR	-	9.3	Α	LR	-	12.7	В	LR	0.36	15.8	В	-Mitigation not required.
West Road (south of island)	EB	LR	-	11.5	В	LR	-	16.4	С	-	-	-	-	-Install traffic signal with
	NB	LT	-	10.1	В	LT	-	12.1	В	Т	0.28	10.7	В	the following timing plan:
Main Street	SB	TR	_	9.6	A	TR	-	25.2	D	Т	0.69	17.3	В	EB will have 22 s green
3. ROOSEVELT ISLAND BRID	Overall Inte		-	10.1	В	-	-	19.4	С	-	0.55	15.6	В	time; NB/SB will have 28 s green time [each phase will have 3 s amber and 2 s all red time]Reconfigure to eliminate traffic triangle and consolidate turning movements at one intersection. [Measures reflect improvements needed for the weekday PM peak period.]
				400	_			440.0	Г*	1.0	0.04	20.0		landall tareffice of our of collection
Roosevelt Island Bridge Ramp	WB	LR T	-	16.2	С	LR	-	110.6	F*	LR	0.91	30.9	С	-Install traffic signal with
La constant	NB	T	-	10.4	В	T	-	11.6	В	T	0.12	13.1	В	the following timing plan: WB will have 28 s green
Main Street		R	-	11.4	В	R	-	17.9	С	R	0.69	24.1	С	time; NB/SB will have 22 s
	SB	LT	-	12.8	В	LT	-	15.9	С	LT	0.47	17.8	В	green time [each phase will
	Overall Inte	ersection	-	13.9	В	-	-	67.9	F	-	0.81	26.4	С	have 3 s amber and 2 s all red time].

⁽¹⁾ Control delay is measured in seconds per vehicle.
(2) Overall intersection V/C ratio is the critical lane groups' V/C ratio.

Denotes a significant impact.

Table 22-3a (cont'd)
2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison
(Unsignalized Intersections)

	1	ı				1		•		1				zea miersecuons)
			No A				With	Action	1		With I	Mitigatio		-
Intersection	Approach	Mvt.	V/C	Control Delay	Los	Mvt.	V/C	Control Delay	100	Mvt.	V/C	Control Delay	LOS	Mitigation Measure
intersection	Approach	IVI V L.	V/C	_		Hour		_	LUS	IVI V L.	V/C	Delay	LUS	<u>wittgation weasure</u>
4. ROOSEVELT ISLAND BRID	CE & MOTO	DC ATE C	ADAC					iuea)						
Roosevelt Island Bridge	EB	LT	-	8.5	ANCE	LT	-	9.6	Α		-			1
Motorgate Garage Exit	NB	LR	-	11.5	В	LR	-	13.0	В	H	-	-	-	-Mitigation not required.
Wolfigate Garage Exit	Overall Inte		-	1.5	A	-	 -	1.4	A	+-	-	-	-	-ivilligation not required.
	Overall lille	ersection	-	1.0		ay Pea			А	-	-	-	-	ı
4 FACTAMENT DOAD & MAIN	CTREET				WIIGO	ay Pea	ак пос	ır						
1. EAST/WEST ROAD & MAIN West Road	EB	LT		7.6	Α	LT	Ι -	8.1	Α	LT	-	8.2	Α	-Mitigation not required.
Main Street	SB	LR	-	7.4	A	LR	-	9.1	A	LR	-	9.7	A	-Conditions shown reflect
Iviairi Street	SB	LK	-	7.4	А	LK	- -	9.1	A	LK	-	9.7	A .	additional U-turns that
														would use this intersection
														because of the proposed
	Overall Inte	ersection	-	7.5	Α	-	-	9.0	Α	-	-	9.5	Α	elimination of the traffic
														triangle at West Road and
														Main Street.
2. WEST ROAD & MAIN STRE								40.0	_		0	4	-	Batter of the state of the stat
West Road	EB	LR	-	8.5	A	LR	-	12.6	В	LR	0.48	17.7	В	-Mitigation not required.
West Road (south of island)	EB	LR	-	10.9	В	LR	-	13.4	В	-	-	-	-	 Install traffic signal with the following timing plan:
Main Street	NB	LT	-	9.4	A	LT	-	11.4	В	T	0.22	10.1	В	EB will have 22 s green
	SB	TR	-	8.7	Α	TR	-	14.5	В	Т	0.47	12.8	В	time; NB/SB will have 28 s
														green time [each phase will
														have 3 s amber and 2 s all
														red time].
														-Reconfigure to eliminate
	0													traffic triangle and
	Overall Inte	ersection	-	9.4	Α	-	-	13.1	В	-	0.47	13.7	В	consolidate turning
														movements at one
														intersection.
														[Measures reflect improvements needed for
														the weekday PM peak
														period.]
3. ROOSEVELT ISLAND BRID	GE RAMP &	MAIN ST	REET											, , , , ,
Roosevelt Island Bridge Ramp	WB	LR	-	10.4	В	LR	-	21.5	С	LR	0.54	14.1	В	-Mitigation not required.
	NB	Т	-	9.3	Α	Т	-	10.5	В	Т	0.14	13.2	В	-Install traffic signal with
Main Street		R	-	9.2	Α	R	-	16.9	С	R	0.82	32.6	С	the following timing plan:
	SB	LT	•	10.8	В	LT	-	13.4	В	LT	0.47	18.2	В	WB will have 28 s green
														time; NB/SB will have 22 s
														green time [each phase will have 3 s amber and 2 s all
														red time].
	Overall Inte	ersection	-	10.0	В	-	-	17.6	С	-	0.67	21.2	С	[Measures reflect
														improvements needed for
														the weekday AM and PM
														peak periods.]
4. ROOSEVELT ISLAND BRID			ARAC											
Roosevelt Island Bridge	EB	LT	-	7.7	Α	LT	-	8.2	Α	-	-	-	-	
Motorgate Garage Exit	NB	LR	-	10.1	В	LR	-	11.4	В	-	-	-	-	-Mitigation not required.
	Overall Inte	ersection	-	0.9	Α	-	-	0.7	Α	-	-	-	-	
					PΝ	l Peak	Hour							
1. EAST/WEST ROAD & MAIN		1 .												1
West Road	EB	LT	-	7.4	Α	LT	-	7.8	Α	LT	-	7.9	Α	-Mitigation not required.
Main Street	SB	LR	-	7.3	Α	LR	-	8.5	Α	LR	-	9.0	Α	-Conditions shown reflect
	_	·												additional U-turns that
	1													would use this intersection
	Overall Inte	ersection	-	7.3	Α	-	-	8.4	Α	-	-	8.8	Α	because of the proposed elimination of the traffic
	1													triangle at West Road and
	1													Main Street.
	I			1	l	l	1		1	<u> </u>				Main Olicel.

Table 22-3a (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Unsignalized Intersections)

											- (C 11016		eu intersections,
			No Ac	ction_			With	Action			With I	<u>Mitigation</u>	1	
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
				PM	Peak	Hour ((conti	nued)						
2. WEST ROAD & MAIN STRE	ET													
West Road	EB	LR	-	9.0	Α	LR	-	84.8	F*	LR	0.96	42.4	D	-Install traffic signal with
West Road (south of island)	EB	LR	-	10.9	В	LR	-	12.2	В	-	-	-	-	the following timing plan:
Main Street	NB	LT	-	10.4	В	LT	-	17.3	С	Т	0.22	11.9	В	EB will have 25 s green
Main Street	SB	TR	-	9.0	Α	TR	-	16.8	С	Т	0.36	13.4	В	time; NB/SB will have 25 s
	Overall Intersection		-	9.9	A	-	-	48.6	E	-	0.66	30.6	С	green time [each phase wil have 3 s amber and 2 s all red time]Reconfigure to eliminate traffic triangle and consolidate turning movements at one intersection.
3. ROOSEVELT ISLAND BRID			REET											T
Roosevelt Island Bridge Ramp	WB	LR	-	11.5	В	LR	-	20.2	С	LR	0.58	19.1	В	-Install traffic signal with
	NB	Т	-	9.7	Α	- 1	-	10.7	В	T	0.08	9.1	Α	the following timing plan:
Main Street		R	-	10.0	Α	R	-	37.6	E*	R	0.86	28.6	С	WB will have 22 s green time; NB/SB will have 28 s
	SB	LT	-	15.4	С	LT	-	23.5	С	LT	0.63	16.9	В	green time [each phase wil
	Overall Inte	ersection	-	12.7	В	-	-	28.0	D	-	0.74	22.0	С	have 3 s amber and 2 s all red time].
4. ROOSEVELT ISLAND BRID	GE & MOTO	RGATE G	ARAC	E ENTR	ANCE	/ EXIT								
Roosevelt Island Bridge	EB	LT	-	7.9	Α	LT	-	8.3	Α			,		
								~ ~ ~)					
Motorgate Garage Exit	NB	LR	-	13.0	В	LR	-	20.0	С					 Mitigation not required.

Table 22-3b 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

			No A	<u>Action</u>			With	Action		,	Nith M	itigation		
				Control				Control				Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
						AM	Peak H	our						
5. ROOSEVELT ISLA	ND BRIDGE / 3	6TH AV	/ENUE	& VERNO	N BOU	LEVAF	RD							
Roosevelt Island Bridge	EB	L	0.31	13.3	В	L	0.46	16.3	В					
_		TR	0.62	17.7	В	TR	0.77	22.5	С					
		-	-	-	-	-	-	-	-					Linguisia atable imposto
36th Avenue	WB	LTR	0.43	14.5	В	LTR	0.64	18.8	В					Unmitigatable Impacts
Vernon Boulevard	NB	LTR	1.20+	214.4	F	LTR	1.20+	556.3	F*					
	SB	LTR	1.20+	336.7	F	LTR	1.20+	385.7	F*					
	Overall Interse	ection	1.16	199.1	F	-	1.20+	287.0	F					

⁽¹⁾ Control delay is measured in seconds per vehicle.
(2) Overall intersection V/C ratio is the critical lane groups' V/C ratio.

Denotes a significant impact.

Table 22-3b (cont'd)
2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison
(Signalized Intersections)

			No	Action			With	Action		,	With M	itigation		
				Control				Control				Control		
Intersection	Approach	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	Mitigation Measure
					AM	Peak I	lour (c	ontinued)						
6. 36TH AVENUE &	21ST STREET													
36th Avenue	EB	LTR	0.91	59.0	Е	LTR	1.20+	169.1	F*	L	0.83	72.8	Е	-Shift centerline 6 ft to the
		-	-	-	-	-	-	-	-	TR	0.85	54.3	D	north and restripe EB
	WB	LTR	1.02	68.4	Е	LTR	1.17	123.1	F*	L	0.47	38.4	D	approach from one 25-ft
		-	-	-	-	-	-	-	-	TR	1.03	72.6	Е	travel lane to 11-ft exclusive
21st Street	NB	LTR	0.40	13.0	В	LTR	0.41	13.2	В	LTR	0.38	10.8	В	left-turn lane, one 20-ft
	SB	LTR	1.14	88.8	F	LTR	1.20	115.1	F*	LTR	1.14	85.0	F	shared through-right lane with parking for 200 ft.
7 PROADWAY 9 20	Overall Inters	section	1.10	69.5	E	-	1.20+	104.5	F	-	1.10	66.9	E	-Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ftModify signal timing: shift 4 s green time from EB/WB phase to NB/SB phase [EB/WB] green time shifts from 37 s to 33 s; NB/SB green time shifts from 73 s to 77 s].
7. BROADWAY & 21			•	•	•	1						•		
Broadway	EB	LTR	1.20+	331.7	F	LTR	1.20+	356.0	F*					
	VA/D		-	- 474.6	-	-	-	- 045.6	-					
	WB	LTR	1.20+	171.3	F	LTR	1.20+	215.4	F*					Hamitiantable lane of
Od at Ctra at	ND	LTR	- 0.55	17.1	- D	- LTR	0.59	18.1	- В					Unmitigatable Impacts
21st Street	NB SB	LTR	0.55 1.16	97.9	B F	LTR		18.1 130.4	E*			-		1
	_			97.9 115.7	F		1.20+	130.4	F					1
8. 36TH AVENUE &	Overall Inters	section	1.20+	115./	<u> </u>	-	1.20+	140.7	<u> </u>			<u> </u>		l
36th Avenue	EB	LTR	0.79	38.1	_ <u> </u>	LTR	0.84	42.0	D	1		1		T
Sour Avenue	WB	LTR	0.79	38.1	D C	LTR	0.84	42.0 35.8	D					1
31st Street	NB	LTR	0.74	20.8	C	LTR	0.80	25.8	С					-Mitigation not required.
3131 311661	SB	LTR	0.73	20.8	C	LTR	0.82	20.6	C			1		-willigation not required.
	Overall Inters		0.73	25.7	C	LIK -	0.74	28.7	C					1
	_ overall lillers	CUUII	0.70	23.1	U		0.03	20.7	U			ĺ		l

Table 22-3b (cont'd)
2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison
(Signalized Intersections)

	1		No	Action			With	Action		,	With M	itigation	,	
			110	Control			<u> </u>	Control	1			Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	M∨t.	V/C		LOS	Mitigation Measure
		1			AM	Peak I	lour (c	ontinued)						
9. 41ST AVENUE & \	ERNON BOUL	EVARD												
41st Avenue	WB	LR	0.31	16.7	В	LR	0.34	17.1	В	LR	0.39	19.8	В	-Modify signal timing: shift
Vernon Boulevard	NB	TR	0.72	15.0	В	TR	0.83	19.4	В	TR	0.77	14.9	В	2.4 s green time from WB
	SB	LT	1.20+	110.2	F	LT	1.20+	157.1	F*	LT	1.18	96.7	F	phase to NB/SB phase
	Overall Inters	ection	0.86	68.4	E	-	0.94	93.0	F	-	0.91	59.2	E	[NB/SB green time shifts from 31.8 s to 34.2 s; WB green time shifts from 19.8 s to 17.4 s].
10. 30TH AVENUE &														
30th Avenue	EB	LTR	0.82	56.4	Е	LTR	0.82	56.4	Е					
	WB	LTR	0.94	72.2	Е	LTR	0.95	75.2	Е					
21st Street	NB	LTR	0.59	16.1	В	LTR	0.61	16.5	В					Unmitigatable Impact
	SB	LTR	1.09	64.9	Е	LTR	1.13	82.5	F*					
	Overall Inters		1.04	51.6	D	-	1.07	62.0	E					
11. BROADWAY & V		VARD												
Park Entrance	EB	LTR	0.01	28.2	С	LTR	0.01	28.2	С					
Broadway	WB	LTR	1.20+	146.1	F	LTR	1.20+	175.4	F*					
		-	-	-	1	-	-	-	-					
Vernon Boulevard	NB	LT	0.30	8.4	Α	LT	0.31	8.5	Α					Unmitigatable Impacts
		R	0.10	6.8	Α	R	0.11	6.8	Α					Onmittigatable impacts
	SB	LTR	1.20+	284.7	F	LTR	1.20+	318.0	F*					
11th Street	NB	LTR	0.43	42.6	D	LTR	0.43	42.6	D					
	Overall Inters	ection	1.20+	177.5	F	-	1.20+	200.9	F					
12. ASTORIA BOULI	EVARD / 27TH A	VENUE	E / NEW	/TOWN A	VENUE	& 21S	T STRE	ET						
Astoria Boulevard	EB	L	1.06	106.3	F	L	1.06	106.3	F	L	0.86	54.7	D	-Modify signal phasing: Add a new lag phase for the EB/WB. The existing signal phasing
		TR	1.20+	471.4	F	TR	1.20+	480.3	F*	TR	1.20+	305.9	F	[WB has 30 s green time; EB
	WB	L	1.05	81.9	F	L	1.05	81.9	F	L	1.04	67.4	Е	has 25 s green time; NB/SB
		TR	0.98	57.7	Е	TR	0.99	60.7	Е	TR	0.96	53.7	D	has 50 s green time] would be
21st Street	NB	LTR	1.20+	212.7	F	LTR	1.20+	234.1	F*	LTR	1.20+	188.6	F	modified to have the following:
	SB	LTR	1.20+	173.3	F	LTR	1.20+	200.5	F*	LTR	1.20+	164.6	F	EB/WB will have 31 s green
	Overall Inters	ection	1.20+	217.4	F	-	1.20+	232.1	F	-	1.20+	169.6	F	time; EB/WB exclusive left- turn phase will have 21 s green time; NB/SB will have 53 s green time [each phase will have 3 s amber and 2 s all red].

Table 22-3b (cont'd)
2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison
(Signalized Intersections)

			No	Action			With	Action		1	With M	itigation		
			1	Control		1		Control			<u> </u>	Control		
Intersection	Approach	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay	LOS	M∨t.	V/C	Delay		Mitigation Measure
		•	•		AM	Peak I	Hour (c	ontinued)		•	•		•	
13. HOYT AVENUE I	NORTH & 21ST	STREE	Т				•							
Hoyt Avenue North	EB	L	0.02	40.4	D	L	0.02	40.4	D					
		R	0.39	48.2	D	R	0.39	48.2	D					
	WB	L	1.10	91.1	F	L	1.16	116.3	F*					
		TR	0.26	15.0	В	TR	0.26	15.0	В					Unmitigatable Impacts
21st Street	NB	L	0.36	35.0	С	L	0.37	36.2	D					Onmitigatable impacts
		Т	1.20+	188.8	F	Т	1.20+	191.5	F*					
	SB	TR	1.13	585.7	F	TR	1.16	600.0+	F*					
	Overall Inters	section	1.04	218.6	F	-	1.06	259.5	F					
14. HOYT AVENUE S	SOUTH & 21ST	STREE	T											
Hoyt Avenue South	EB	L	0.36	32.5	С	L	0.36	32.5	С	LTR	0.95	45.3	D	-Restripe EB approach of
		TR	1.20+	219.6	F	TR	1.20+	219.6	F	-	-	-	-	Hoyt Avenue South from one
21st Street	NB	LTR	0.80	45.0	D	LTR	0.83	54.1	D*	LTR	0.78	40.0	D	11-ft exclusive left-turn lane
	SB	LTR	1.20+	147.4	F	LTR	1.20+	170.0	F*	LTR	1.20+	138.1	F	and one 11-ft shared through-right lane to two 11-
	NB LTR	1.20+	129.3	F	-	1.20+	143.6	F	-	1.16	92.7	F	ft shared left-through-right lanesModify signal timing: shift 3 s green time from EB phase to NB/SB phase [EB green time shifts from 37 s to 34 s NB/SB green time shifts fror 73 s to 76 s].	
F DOOCEVELTIC	AND DDIDOE /	OCTIL AN	/ENUIE	e VEDNO	N DOL		y Peak	Hour						
5. ROOSEVELT ISLA Roosevelt Island		O IH A	ENUE	& VEKNO	N ROU	LEVA	עא	ı	1		1	1		
Bridge Island	EB	L	0.24	12.5	В	L	0.37	14.7	В					
		TR	0.44	14.6	В	TR	0.68	20.0	В					
001 1	14/5	-	-	-	-	-	-	-	-					
36th Avenue	WB	LTR	0.37	14.0	В	LTR	0.59	18.2	В					
Vernon Boulevard	NB	LTR	1.04	54.5	D	LTR	1.20+	219.6	F*					
	SB	LTR	0.85	27.4	С	LTR	0.93	36.0	D					
	Overall Inters	section	0.74	31.4	С	-	1.06	82.1	F			l		Unmitigatable Impact

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		1	No	Action			\A/i+h	Action			Mith M	itigation	,	izeu miersechons)
			<u>NO</u>	Control			VVILII	Control	1		VVILII IVI	Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
					Midd	ay Pea	k Hour	(continue	d)					
6. 36TH AVENUE & 2														
36th Avenue	EB	LTR	0.89	56.6	Е	LTR	1.20+	193.8	F*	L	0.72	51.2	D	-Shift centerline 6 ft to the
		-	-	-	-	-	-	-	-	TR	0.68	42.8	D	north and restripe EB
	WB	LTR	0.96	63.8	Е	LTR	1.02	78.2	E*	L	0.38	36.8	D	approach from one 25-ft travel
		-	-	-	-	-	-	-	-	TR	0.84	50.8	D	lane to 11-ft exclusive left-turn lane. one 20-ft shared
21st Street	NB	LTR	0.75	19.4	В	LTR	1.02	52.1	D*	LTR	0.99	41.3	D	through-right lane with parking
	SB	LTR	0.69	18.1	В	LTR	0.73	19.3	В	LTR	0.71	17.5	В	for 200 ft.
	Overall Inters	ection	0.82	28.5	С	-	1.12	61.7	E	-	0.94	34.4	С	-Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft. '-Modify signal timing: shift 2 s green time from EB/WB phase to NB/SB phase [EB/WB] green time shifts from 37 s to 35 s; NB/SB green time shifts from 73 s to 75 s].
7. BROADWAY & 21	ST STREET													
Broadway	EB	LTR	1.20+	191.5	F	LTR	1.20+	227.0	F*					
·		-	-	-	-	-	-	-	-					
	WB	LTR	1.20+	161.7	F	LTR	1.20+	179.6	F*					
		-	-	-	-	-	-	-	-					Unmitigatable Impacts
21st Street	NB	LTR	0.90	28.3	С	LTR	0.93	30.9	С					-
	SB	LTR	0.87	27.4	С	LTR	0.94	34.3	С					
	Overall Inters	ection	1.03	63.3	E	-	1.08	73.0	Е					
8. 36TH AVENUE & 3	31ST STREET			•				•	•		•	•		
36th Avenue	EB	LTR	0.88	42.3	D	LTR	0.93	48.3	D*	LTR	0.82	34.4	С	-Modify signal timing: shift 3
	WB	LTR	0.80	36.2	D	LTR	0.83	38.5	D	LTR	0.76	31.6	С	s green time from NB/SB
31st Street	NB	LTR	0.63	17.9	В	LTR	0.64	18.2	В	LTR	0.69	21.5	С	phase to EB/WB phase
	SB	LTR	0.53	15.3	В	LTR	0.53	15.4	В	LTR	0.56	17.9	В	[EB/WB green time shifts
	Overall Inters	ection	0.73	26.5	С	-	0.76	28.6	С	-	0.75	25.6	С	from 31 s to 34 s; NB/SB green time shifts from 49 s to 46 s].
9. 41ST AVENUE & \	VERNON BOUL	EVARD					•			•		•		
41st Avenue	WB	LR	0.21	15.5	В	LR	0.21	15.5	В	LR	0.23	17.7	В	-Mitigation not required.
Vernon Boulevard	NB	TR	0.74	15.7	В	TR	0.80	17.8	В	TR	0.74	14.0	В	-Modify signal timing: shift
	SB	LT	0.73	15.3	В	LT	0.80	18.1	В	LT	0.74	14.2	В	2.4 s green time from WB
	Overall Inters	ection	0.53	15.5	В	-	0.57	17.8	В	-	0.57	14.3	В	phase to NB/SB phase [NB/SB green time shifts from 31.8 s to 34.2 s; WB green time shifts from 19.8 s to 17.4 s]. [Measures reflect improvements needed for the weekday AM and PM peak periods.]

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		1	No	Action			With	Action			With M	itigation		zea intersections)
			<u>INU</u>	Control			VVILLI	Control		-	VVILII IVI	Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
					Midda	ay Peal	k Hour	(continue	d)					
10. 30TH AVENUE 8	21ST STREET							•	•					
30th Avenue	EB	LTR	0.50	38.7	D	LTR	0.50	38.7	D	-	-	-	-	
	WB	LTR	0.65	45.0	D	LTR	0.66	45.2	D	-	-	-	-	
21st Street	NB	LTR	0.84	23.4	С	LTR	0.88	25.7	С	-	-	-	-	 Mitigation not required.
	SB	LTR	0.86	23.8	С	LTR	0.89	25.9	С	-		-	-	
	Overall Inters		0.79	26.3	С	ı	0.81	28.1	С	-	-	-	-	
11. BROADWAY & V	ERNON BOULE	VARD	/ 11TH	STREET										
Park Entrance	EB	LTR	0.02	26.2	C	LTR	0.02	26.2	C					
Broadway	WB	LTR	1.09	93.0	F	LTR	1.13	106.3	F*					
		-	-	-	-	-	-	-	-					
Vernon Boulevard	NB	LT	0.31	8.8	Α	LT	0.32	8.9	Α					Unmitigatable Impact
		R	0.20	7.8	Α	R	0.21	7.9	Α					Onmitigatable impact
	SB	LTR	0.75	34.5	С	LTR	0.78	36.2	D					
11th Street	NB	LTR	0.26	33.6	C	LTR	0.26	33.6	C					
	Overall Inters		0.90	41.0	D	٠	0.93	45.1	D					
12. ASTORIA BOUL	EVARD / 27TH A	AVENU	E / NEV	/TOWN A	VENUE	& 21S	T STRE	ET						
Astoria Boulevard	EB	L	0.36	36.8	D	L	0.36	36.8	D	L	0.33	25.3	С	-Modify signal phasing: Add a
		TR	0.71	44.2	D	TR	0.73	45.0	D	TR	0.78	48.8	D	new lag phase for the EB/WB
	WB	L	0.92	59.6	Е	┙	0.92	59.6	Е	L	0.91	54.8	D	The existing signal phasing
		TR	0.56	38.2	D	TR	0.57	38.5	D	TR	0.61	40.7	D	[WB has 34 s green time; EB
21st Street	NB	LTR	1.20+	443.0	F	LTR	1.20+	501.8	F*	LTR	1.18	116.2	F	has 34 s green time; NB/SB
	SB	LTR	1.20+	220.2	F	LTR	1.20+	242.7	F*	LTR	1.03	56.3	Е	has 37 s green time] would be modified to have the following
	Overall Inters		1.19	205.0	F	-	1.20+	229.1	F	-	1.05	67.5	E	EB/WB will have 32 s green time; EB/WB exclusive left-turn phase will have 22 s green time; NB/SB will have 51 s green time; leach phase will have 3 s amber and 2 s a red].
13. HOYT AVENUE I	_	STREE												
Hoyt Avenue North	EB	l L	0.12	42.3	D	L	0.12	42.3	D					
		R	0.14	42.7	D	R	0.14	42.7	D					
	WB	L	0.91	49.9	D	L	0.96	55.7	E*				ļ	
		TR	0.17	14.3	В	TR	0.17	14.3	В					
21st Street	NB	L	0.13	25.6	C	L	0.13	25.6	C				ļ	
		T	0.90	55.3	E	T	0.90	56.2	E				ļ	
	SB	TR	0.65	35.8	D	TR	0.66	36.1	D				ļ	
	Overall Interse	ection	0.74	45.7	D	-	0.76	48.9	D					Unmitigatable Impact

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		1	No	Action		1	\A/i4h	Action			M/ith M	itigation	<u> </u>	izeu intersections)
ĺ		-	<u>INO /</u>	Control			vvitr	Control		 	VVITTI IVI	Control		1
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
14. HOYT AVENUE												,		
Hoyt Avenue South	EB	L	0.28	32.7	С	L	0.28	32.7	С	LTR	0.45	34.8	С	[Measures reflect
,		TR	0.60	40.7	D	TR	0.60	40.7	D	-	-	-	-	improvements needed for
21st Street	NB	LTR	0.52	14.6	В	LTR	0.53	14.8	В	LTR	0.53	14.8	В	the weekday AM and PM
	SB	LTR	0.77	20.2	С	LTR	0.80	21.1	С	LTR	0.80	21.1	С	peak periods.]
	Overall Inters	ection	0.71	21.4	С	-	0.73	21.9	С	-	0.68	21.4	С	-Restripe EB approach of Hoyt Avenue South from one 11-ft excl. left-turn lane and one 11-ft shared through- right lane to two 11-ft shared left-through-right lanes.
						PM	Peak H	lour						
5. ROOSEVELT ISLA	AND BRIDGE / 3	6TH A	/ENUE	& VERNO	N BOU	LEVA	RD							
Roosevelt Island Bridge	EB	L	0.49	14.8	В	L	0.64	17.5	В					
		TR	0.64	16.4	В	TR	1.12	83.2	F*					
		-	-	-	-	-	-	-	-					Unmitigatable Impacts
36th Avenue	WB	LTR	0.34	13.5	В	LTR	0.56	18.8	В					Offittilitigatable impacts
Vernon Boulevard	NB	LTR	1.20+	236.2	F	LTR	1.20+	443.3	F*					
	SB	LTR	1.06	63.3	E	LTR	1.10	80.2	F*					
	Overall Inters	ection	1.07	108.6	F	-	1.20+	185.0	F					
6. 36TH AVENUE & 2							· V.							
36th Avenue	EB	LTR	0.62	37.4	D	LTR	1.20+	250.7	F*	L	0.70	41.9	D	-Shift centerline 6 ft to the
		-	-	-	-	-	-	-	-	TR	0.74	40.3	D	north and restripe EB
	WB	LTR	0.89	54.2	D	LTR	0.99	70.8	E*	L	0.36	34.9	С	approach from one 25-ft
		-	-	-	-	-	-	-	-	TR	0.75	43.7	D	travel lane to 11-ft exclusive left-turn lane, one 20-ft
21st Street	NB	LTR	1.03	44.7	D	LTR	1.03	44.9	D	LTR	1.03	44.9	D	shared through-right lane
	SB	LTR	0.82	22.8	С	LTR	0.85	24.0	С	LTR	0.85	24.0	С	with parking for 200 ft.
	Overall Inters	ection	0.98	38.0	D	-	1.17	66.0	E	-	0.93	37.6	D	-Shift centerline 6 ft to the south and restripe WB approach from one 25-ft travel lane to 11-ft exclusive left-turn lane, one 20-ft shared through-right lane with parking for 125 ft.
7. BROADWAY & 21												•		
Broadway	EB	LTR -	1.20+	293.1	F -	LTR -	1.20+ -	339.4	F* -					
	WB	LTR	1.20+	313.1	F	LTR	1.20+	333.3	F*					
21st Street	NB	-	-	•	-	-	-	-	-					Unmitigatable Impacts
		LTR	1.05	54.1	D	LTR	1.11	78.3	E*]
	SB	LTR	0.84	25.2	С	LTR	0.86	26.6	С					
	Overall Inters	ection	1.20+	99.5	F	-	1.20+	118.7	F]

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		I	No	Action			\A/i+h	Action			Mith M	itigation	,	izeu miersechons)
			<u>NO</u>	Action Control			VVILI	Action Control	ı		With ivi	Control	ı	
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delav	LOS	Mvt.	V/C	Delay	LOS	Mitigation Measure
linter scotter.	Арргоссії	101 0	1,0	Delay				ontinued)		101 V C.	*/-	Delay	LUC	Minganon measure
8. 36TH AVENUE &	24 STREET				Fivi	Peak i	Hour (c	Olitinueu						
36th Avenue	EB	LTR	0.90	38.7	D	LTR	1.08	80.1	F*	LTR	0.95	42.5	D	-Modify signal timing: shift 3
30lii Aveilue	WB	LTR	0.80	35.9	D	LTR	0.83	37.4	D	LTR	0.95	30.8	С	s green time from NB/SB
24 at Ctroat	NB	LTR	0.80	25.2	С	LTR	0.84	25.8	С	LTR	0.75	32.9	C	phase to EB/WB phase
31st Street	SB	LTR	0.83	25.2 16.1	В	LTR	0.84	25.8 16.2	В	LTR	0.89	18.9	В	[EB/WB green time shifts
	SB	LIK	0.56	10.1	ь	LIK	0.56	10.2	ь	LIK	0.60	10.9	ь	from 31 s to 34 s; NB/SB
	Overall Inters	section	0.86	27.7	С	_	0.93	37.7	D	_	0.92	31.1	С	green time shifts from 49 s to
l	0101411111111		0.00		_		0.00	0	_		0.01	J	-	46 s].
9. 41ST AVENUE &	VERNON BOUL	EVARD												
41st Avenue	WB	LR	0.44	18.9	В	LR	0.52	20.4	С	LR	0.59	24.8	С	-Modify signal timing: shift
Vernon Boulevard	NB	TR	1.17	97.4	F	TR	1.20+	114.8	F*	TR	1.13	76.7	Е	2.4 s green time from WB
	SB	LT	1.14	86.1	F	LT	1.20+	164.2	F*	LT	1.11	70.5	Е	phase to NB/SB phase
														[NB/SB green time shifts
	Overall Inters	ection	0.89	85.2	F	_	1.01	125.4	F	_	0.95	68.7	Е	from 31.8 s to 34.2 s; WB
	O Vorum mitore	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0.00	00.2	•			120.4	•		0.00	00.7	_	green time shifts from 19.8 s
10. 30TH AVENUE 8	24CT CTDEET								<u> </u>					to 17.4 s].
30th Avenue	EB	LTR	0.52	39.2	D	LTR	0.53	39.5	D	-	-	l <u>-</u>	l -	
30th Avenue	WB	LTR	0.52	45.3	D	LTR	0.53	45.6	D	-	-		-	
21st Street	NB	LTR	0.67	31.2	С	LTR	1.01	42.4	D	-	-	-	-	-Mitigation not required.
Z ISI Sireei	SB	LTR	0.96	18.1	В	_	_		В	-	-	-	-	-ivilligation not required.
	Overall Inters		0.70	28.0	C	LTR -	0.72 0.90	18.6 33.9	C	-	-	-	-	
11. BROADWAY & V					C	_	0.90	33.9	C	_	-	-	-	
Park Entrance	EB	LTR	0.03	33.2	С	LTR	0.03	33.2	С				1	
Broadway	WB	LTR	1.18	140.0	F	LTR	1.20	150.8	F*					
Dioauway	WD	LIK -	1.10	140.0	-	LIK -	1.20	-	<u>-</u>					
Vernon Boulevard	NB	LT	0.57	11.0	В	LT	0.60	11.5	В					
vernon boulevalu	IND	R	0.37	6.6	A	R	0.00	6.7	A					Unmitigatable Impacts
	SB	LTR	1.15	117.2	F	LTR	1.20	138.4	F*					
11th Street	NB	LTR	0.37	39.1	D	LTR	0.37	39.1	D				-	
T THI SHEEL	Overall Inters		1.17	67.5	E	LIK	1.20	74.5	E					
12. ASTORIA BOUL						. & 21S				1		l	l	<u> </u>
Astoria Boulevard	EB	I	0.59	45.2	D	1	0.59	45.2	D	1	0.59	40.9	D	-Modify signal phasing: Add a
, iotoria Douicvalu		TR	1.20+	162.8	F	TR	1.20+	180.2	F*	TR	1.09	95.3	F	new lag phase for the EB/WB.
	WB	L	0.96	75.9	E	L	0.96	75.9	E	1	0.95	72.6	E	The existing signal phasing
	***	TR	1.15	127.3	F	TR	1.16	132.3	F*	TR	0.82	45.7	D	[WB has 24 s green time; EB
21st Street	NB	DefL	1.20+	526.1	F	DefL	1.20+	526.1	F	DefL	1.20+	524.1	F	has 28 s green time; NB/SB
2100 00000	110	TR	1.20+	434.7	F	TR	1.20+	492.4	F*	TR	1.20+	424.6	F	has 53 s green time] would be
	SB	LTR	1.20+	250.0	F	LTR	1.20+	267.8	F*	LTR	1.20+	214.0	F	modified to have the following:
	1 05	LIIN	1.207	200.0	-	LIIK	1.201	201.0		L 1 1 1	1.20+	217.0		EB/WB will have 33 s green
														time; EB/WB exclusive left-
														turn phase will have 15 s
	Overall Inters	ection	1.20+	254.7	F	-	1.20+	280.9	F	-	1.20+	220.6	F	green time; NB/SB will have 57 s green time [each phase
1														will have 3 s amber and 2 s all
														red].
	1													icuj.

Table 22-3b (cont'd) 2038 No Action, With Action and Mitigated Traffic Levels of Service Comparison (Signalized Intersections)

		I	No	Action			\\/i+h	Action			Mith M	itigation		zea mersections)
			<u>NO</u>	Control			VVILII	Control	1			Control		
Intersection	Approach	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay	LOS	Mvt.	V/C	Delay		Mitigation Measure
					PM	Peak I	lour (c	ontinued))					
13. HOYT AVENUE I	NORTH & 21ST	STREE	Т											
Hoyt Avenue North	EB	L	0.10	41.9	D	L	0.10	41.9	D					
		R	0.18	43.3	D	R	0.18	43.3	D					
	WB	L	1.07	86.0	F	L	1.10	97.2	F*					
		TR	0.30	15.9	В	TR	0.30	15.9	В					Unmitigatable Impacts
21st Street	NB	L	0.21	26.9	С	L	0.21	26.9	С					Offiffiligatable impacts
		Т	1.20+	166.0	F	Т	1.20+	176.4	F*					
	SB	TR	0.87	580.5	F	TR	0.87	600.0+	F*					
	Overall Interse	ection	0.98	191.2	F	-	1.00	212.9	F					
14. HOYT AVENUE S	SOUTH & 21ST	STREE	T											
Hoyt Avenue South	EB	L	0.24	31.7	С	L	0.24	31.7	С	LTR	0.68	40.8	D	-Restripe EB approach of
		TR	0.99	72.1	E	TR	0.99	72.1	E	-	-	-	-	Hoyt Avenue South from one
21st Street	NB	LTR	1.20+	286.8	F	LTR	1.20+	325.9	F*	LTR	1.20+	279.9	F	11-ft exclusive left-turn lane
	SB	LTR	1.20+	185.2	F	LTR	1.20+	204.9	F*	LTR	1.20+	167.6	F	and one 11-ft shared
Notoci	Overall Interse	ection	1.20+	202.3	F	-	1.20+	226.8	F	-	1.12	188.6	F	through-right lane to two 11- ft shared left-through-right lanes for 250 ftModify signal timing: shift 3 s green time from EB phase to NB/SB phase [EB green time shifts from 37 s to 34 s; NB/SB green time shifts from 73 s to 76 s].

Notes:

- (1) Control delay is measured in seconds per vehicle.
- (2) Overall intersection V/C ratio is the critical lane groups' V/C ratio.

Denotes a significant impact.

TRANSIT

As discussed in Chapter 14, "Transportation," the proposed project would not result in any significant adverse subway station or tramway impacts in either the 2018 or 2038 analysis year. However, it would result in significant adverse impacts to bus line-haul levels for the Q102 bus and the Red Bus. In the eastbound and westbound directions the Q102 bus route would experience significant adverse impacts during the PM peak period in the 2018 analysis year and during both the AM and PM peak period in the 2038 analysis year. The Red Bus route would also result in significant adverse impacts to bus line-haul levels for the southbound direction in the AM peak period and the northbound direction in the PM peak period during the 2038 analysis year. Potential measures to mitigate these significant adverse impacts are described below.

BUS LINE HAUL

The proposed project would result in significant adverse bus line-haul impacts on the Q102 route under 2018 Phase 1 and to both the Q102 and Red Bus routes under 2038 Full Build conditions. Under Phase 1-2018, during the PM peak period, the eastbound Q102 would exceed the NYCT guideline capacity. Under Full Build-2038, during the AM and PM peak periods, both the eastbound and westbound Q102 would exceed the guideline capacity while the Red Bus would exceed the RIOC guideline capacity in the southbound and northbound directions during the AM and PM peak periods.

Table 22-4 provides comparisons of existing service and the number of buses required to fully mitigate the identified significant adverse line-haul impacts along the Q102 bus route under Phase 1 and the Q102 and Red Bus route under Full Build of the project. The Full Build-2038 mitigation accounts for all buses needed to accommodate the 2038 projected passenger volumes independent of the Phase 1-2038 mitigation. NYCT and RIOC routinely monitors changes in bus ridership and makes the necessary service adjustments where warranted.

Table 22-4 Mitigated Future With Action Condition (Capacity Improvement): Bus Line Haul Levels

Analysis		Peak	Eastbound/N Buses pe			Southbound per Hour
Year	Route	Period	Existing	Mitigation	Existing	Mitigation
2018	Q102	AM	4	n/a	3	n/a
2010	Q102	PM	2	3	2	n/a
2038	Q102	AM	4	6	3	5
2036	Q102	PM	2	7	2	6
2038	Red	AM	8	n/a	8	10
2036	Bus	PM	8	9	8	n/a

Notes: The Q102 bus route operates standard buses with a guideline capacity of 54 passengers per bus and the Red Bus route operates with a guideline capacity of 55 passengers per bus.

PEDESTRIANS

PHASE 1-2018 ANALYSIS YEAR (2018 WITH ACTION CONDITION)

The proposed project would not result in any significant adverse impacts on pedestrian operations.

FULL BUILD-2038 ANALYSIS YEAR (2038 WITH ACTION CONDITION)

Under Full Build-2038, the proposed project would result in significant adverse pedestrian impacts at the following locations on West Road and West Main Street:

- West Road: The east sidewalk between West Main Street and the subway station; and
- West Main Street: The east sidewalk between the Tram Station West bus stop and the Queensboro Bridge.

West Road between West Main Street and Subway Station

At this location, the east sidewalk would experience the following changes:

- LOS B (1.65 PMF) under the No Action condition to LOS D (9.28 PMF) under the With Action condition during the AM peak period;
- LOS B (1.01 PMF) under the No Action condition to LOS D (7.06 PMF) under the With Action condition during the midday peak period; and
- LOS B (2.72 PMF) under the No Action condition to LOS D (11.48 PMF) under the With Action condition during the PM peak periods.

The significant adverse impacts at this sidewalk would be fully mitigated by widening its existing width of 6.4 feet to 8.9 feet, thereby increasing its effective width from 2.7 feet to 5.2 feet.

West Main Street between the Tram Station West Bus Stop and Queensboro Bridge

At this location, the east sidewalk would experience the following changes:

- LOS B (1.20 PMF) under the No Action condition to LOS D (7.06 PMF) under the With Action condition during the AM peak period; and
- LOS B (1.78 PMF) under the No Action condition to LOS D (8.52 PMF) under the With Action condition during the PM peak period.

The significant adverse impacts at this sidewalk would be fully mitigated by widening its existing width of 6.4 feet to 8.0 feet, thereby increasing its effective width from 3.6 feet to 5.2 feet.

The measures described above and the mitigated conditions are summarized in **Table 22-5**. In the event the proposed sidewalk widening is determined to be infeasible, the projected impacts would be deemed unmitigatable.

Table 22-5 2038 No Action, With Action, and Mitigated Conditions Pedestrian Level of Service Analysis

		Existing Effective	No A	ction	With A	ction	Proposed Effective	Wi Mitig	
Location	Mitigation Measures	Width (ft.)	PMF	LOS	PMF	LOS	Width (ft.)	PMF	LOS
	Weekday Pl	VI Peak 15-M	linutes	;					
West Road, between West Main Street and the Subway Station- East Sidewalk	Sidewalk widening by 2.5 feet	2.7	2.72	В	11.48	D	5.2	5.96	С
West Main Street between the Tram Station West Bus Stop and Queensboro Bridge- East Sidewalk	Sidewalk widening by 1.6 feet	3.6	1.78	В	8.52	D	5.2	5.90	С
Note: PMF = pedestrians	s per minute per foot.	-							

EFFECTS OF TRAFFIC MITIGATIONS ON PEDESTRIAN OPERATIONS

As previously described, intersection operations would be improved with the implementation of the recommended traffic mitigation measures. These measures would include changes to existing signal timings, installation of new signals, and modifications to lane utilization. A review of the effects of these changes on pedestrian circulation and levels of service showed that they would not alter the conclusions made for the pedestrian impact analyses, nor would they result in the potential for any additional significant adverse pedestrian impacts.

At the newly signalized (per traffic mitigation) intersection of Main Street and West Road, a pedestrian crosswalk analysis was conducted to determine if the proposed intersection reconfiguration and signal timing would continue to adequately accommodate pedestrian crossing at the intersection. As shown in **Table 22-6**, the traffic mitigation measures recommended for this intersection would not result in any significant adverse pedestrian impacts.

Table 22-6 2038 With Action Condition Crosswalk Analysis with Traffic Mitigation

		Street	Crosswalk	Conditions with conflicting vehicles					
		Width	Width	AM		Midday		PM	
Location	Crosswalk	(feet)	(feet)	SFP	LOS	SFP	LOS	SFP	LOS
Main Street	West	27.5	12.0	49.9	В	40.0	С	39.1	С
and West Road	Northeast	35.5	12.0	104.5	Α	212.6	Α	206.8	Α
Note: SFP = square feet per pedestrian.									

MITIGATION IMPLEMENTATION

Subject to approvals of the relevant agencies, including NYCDOT, RIOC, and NYCT, the recommended mitigation measures would be implemented to mitigate the projected significant adverse transportation impacts at the completion of the project's Phase 1-2018 and Full Build-2038 conditions.

D. CONSTRUCTION

The analysis undertaken in Chapter 20, "Construction," concludes that the proposed project would result in significant adverse construction impacts related to transportation and noise (i.e., construction noise impacts on open space).

TRANSPORTATION

During Phase 1 construction of the proposed project, significant adverse impacts are expected for traffic and transit conditions. During Phase 2 construction, significant adverse impacts are expected for traffic, transit, and pedestrian conditions. These findings are summarized below.

TRAFFIC

Four intersections (of the seven analyzed) would experience significant adverse traffic impacts during Phase 1 construction. Impacts at two of the four intersections could be mitigated using standard mitigation measures typically implemented by NYCDOT. These measures would also be consistent with those proposed to mitigate the intersection impacts associated with the project's build-out and occupancy. Two impacts are currently identified as unmitigatable, but additional review of potential mitigation measures will be undertaken for the Final EIS that may fully or partially mitigate these significant impacts.

For Phase 2 construction, the cumulative operational and construction traffic would be of lower magnitudes than what the overall project would generate when completed in 2038. Therefore, potential traffic impacts during peak Phase 2 construction would be within the envelope of significant adverse traffic impacts identified for the 2038 With Action condition in Chapter 14, "Transportation," and mitigatable and unmitigatable impacts identified above would apply to Phase 2 construction conditions as well. The required mitigation measures for those locations that could be mitigated are expected to be part of those presented for the 2038 full build-out of the proposed project. These mitigation measures could be implemented at the discretion of NYCDOT during construction of Phase 2.

TRANSIT

During construction of Phase 1, because most construction workers parking at the Motorgate garage would rely on the Red Bus for travel to/from the project site, during off-peak hours when the Red Bus operates at comparatively lower frequencies, there is a potential for a line-haul impact on the Red Bus that would warrant an increase in its service during off-peak hours (i.e., three additional buses during the 6 to 7 AM and 3 to 4 PM construction peak hours).

A significant adverse impact has been identified for the Q102 bus route due to the projected increase in demand from the completed buildings, and this impact would continue during the Phase 2 construction period. Mitigation measures identified above for the operational impact would be proposed to mitigate the construction-period impact.

PEDESTRIANS

Pedestrian trips generated by construction workers are not expected to result in significant adverse pedestrian impacts during Phase 1 construction. After the completion of the Phase 1 and Phase 2A components of the proposed project, the combination of the Phase 2 construction worker pedestrian trips with those generated by the completed Phase 1 and Phase 2A buildings during the commuter peak hours may result in similar significant adverse pedestrian impacts as those discussed in Chapter 14, "Transportation," and may warrant the earlier implementation of the recommended sidewalk widening described above. In the event the widening is determined to be infeasible, the projected impacts would be deemed unmitigatable.

NOISE IMPACTS ON OPEN SPACE

The proposed project would result in significant adverse impacts with respect to construction noise, as follows:

- During construction of Phase 1, the open space areas along Main Street would experience exceedances due to trucks and workers travelling on Main Street to and from the project site during the AM construction traffic peak hour (6 to 7 AM);
- During construction of Phase 2, South Point Park and the waterfront promenades on the east and west sides of the Island adjacent to the project site would experience noise levels in the mid to high 70s of dBA for over 24 months. These exceedances would be due to the operation of on-site construction equipment.

No practical and feasible mitigation measures have been identified that could be implemented to reduce noise levels to below the 55 dBA $L_{10(1)}$ guideline within the impacted open space areas (i.e., the open spaces along Main Street, the waterfront promenade, or South Point Park). Noise levels in these spaces would exceed the 55 dBA $L_{10(1)}$ noise level recommended for outdoor areas requiring serenity and quiet by the *CEQR Technical Manual* noise exposure guidelines. However while the 55 dBA $L_{10(1)}$ guideline is a worthwhile goal for outdoor areas requiring serenity and quiet, due to the level of activity present at most New York City open space areas and parks (except for areas far away from traffic and other typical urban activities) this relatively low noise level is often not achieved. For example, existing noise levels at the waterfront promenade and South Point Park are already above the 55 dBA $L_{10(1)}$ guideline due to noise from vehicular traffic on the Queensboro Bridge and on the FDR Drive. To achieve noise levels that would meet the 55 dBA $L_{10(1)}$ guideline, measures would need to be implemented to control noise from the Queensboro Bridge; the implementation of such barriers on the bridge would not be possible because of the bridge's landmarked status.