Inspection of Old Retaining Walls

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CONDITION ASSEMENT -Objectives

- Classification of retaining walls.
- Basic elements of a visual inspection of a retaining wall.
- Issues related to the assessment of old masonry retaining walls.



Older Building Codes - Retaining Wall

- A wall designed to prevent the lateral displacement of soil or other materials
- Too Broad
 - Basement Walls
 - Tunnel Walls
 - Pools



Short History

- The early building codes used the term retaining wall – mainly to refer to basement walls.
- 1915 Building Code used the term closer to present day meaning and also required to be designed for water pressure.
- There was no significant requirement or reference to retaining walls in the 1938 Code –
- The 1968 code introduced the facot r of safety of 1.5 for overturning.



2014 Code Definition

 RETAINING WALL. A wall that resists lateral or other forces caused by soil, rock, water or other materials, thereby limiting lateral displacement and the movement of the supported materials. Basement walls and vault walls that are parts of buildings and underground structures, including but not limited to utility vault structures, tunnels and transit stations, are not considered retaining walls.



What Height?





Supporting Roads or Building Lots







NAVFAC CLASSIFICATION

- Gravity
- Semigravity
- Flexible
- Counterfort



Gravity Walls

- Gravity walls resist overturning and sliding by the weight of the wall itself. These walls are usually constructed of solid concrete or rock rubble mortared together. These walls are not usually reinforced with steel since the massive nature of these walls develops little or no tension in the mass. Gravity walls are seldom constructed any more...
- The vast majority of retaining walls were gravity walls.



Fill Wall

- Earth retaining structure supporting specified soil or aggregate backfill. Fill walls are typically located below roadway grade on the outboard side of the roadway or parking area, but may also exist above travelway grade in locations commonly associated with cut walls.
- Were there "specifications" for the backfill of the old wall.
- Were they compacted or thrown down?



Causes Unsatisfactory Retaining Walls



 Foundation and backfill material of unsatisfactory retaining walls (Ireland 1964)

From EM-1110-2

Cut Walls



• Earth retaining structure directly supporting natural ground; either constructed directly against the excavated soil/rock mass, or against a minor volume of drainage backfill.



Cut Walls

Cut walls are constructed in areas where the finished grade will be substantially below existing grade. Cut walls are constructed with a top down construction sequence, which eliminates the need for temporary shoring...





Bars or cables which pass through the face of the wall which are anchored to a large object buried behind the wall referred to as a "deadman, " which is usually either concrete or sheet piling. The force holding the wall back is generated by passive soil pressures acting on the deadman. The deadman must be located far enough behind the wall so that the active failure zone and the passive resistance wedge in front of the deadman do not overlap.



When excavating on top – protect deadmen and cables !



Old Bulkhead on Hudson Sea Wall







Revetment – retains fill









Is this a retaining wall?







Dry Retaining Wall – no mortar





Rockery Installation





http://flh.fhwa.dot.gov/innovation/td/geotech/rockeries/d ocuments/09_Chapter_6_Construction_Inspection.pdf



Mortared Retaining Wall – Rubble Masonry





????? Ashlar it is a veneer





Bearing Masonry Construction Requires Headers









Rubble wall





WIP – Wall Types

RETAINING WALL INVENTORY AND CONDITION ASSESSMENT PROGRAM (WIP) National Park Service Procedures Manual

Publication No. FHWA-CFL/TD-10-003

Wall Function Codes		
[FW] Fill Wall [CW] Cut Wall [BW] Bridge Wall [SW] Switchback Wall	[HW] Head Wall [SP] Slope Protection	[FL] Flood Wall
Wall Type Codes		
[AH] Anchor, Tieback H-Pile	[CC] Crib, Concrete	[MG] MSE, Geosynthetic Wrapped Face
[AM] Anchor, Micropile	[CM] Crib, Metal	[MP] MSE, Precast Panel
[AS] Anchor, Tieback Sheet Pile	[CT] Crib, Timber	[MS] MSE, Segmental Block
[BC] Bin, Concrete	[GB] Gravity, Concrete Block/ Brick	[MW] MSE, Welded Wire Face
[BM] Bin, Metal	[GC] Gravity, Mass Concrete	[SN] Soil Nail
[CL] Cantilever, Concrete	[GD] Gravity, Dry Stone	[TP] Tangent/ Secant Pile
[CP] Cantilever, Soldier Pile	[GG] Gravity, Gabion	[OT] Other, User Defined
[CS] Cantilever, Sheet Pile	[GM] Gravity, Mortared Stone	[NO] None
Architectural Facing Type Codes		
[BV] Brick Veneer	[PF] Planted Face	[SS] Simulated Stone
[CO] Cementitious Overlay	[SC] Sculpted Shotcrete	[SV] Stone Veneer
[FF] Fractured Fin Concrete	[SH] Shotcrete (nozzle finish)	[TI] Timber
[FL] Formlined Concrete	[SM] Steel/Metal	[OT] Other, User Defined
[PC] Plain Concrete (float finish or light texture)	[SO] Stone	[NO] None



WIP Design Criteria

The engineer should be knowledgeable of AASHTO wall design standards and aware of historic construction practices and workmanship sufficient to select from one of the following levels of applied design criteria:

- None: Does not meet any known design standards systematic construction methods commonly use the time of construction;
- Non-AASHTO: Does not meet AASHTO des standards, but is consistent with other structures of its type and period of construction exhibiting established construction workmanship and good performance; or
- AASHTO: Appears to meet AASHTO geometric, design, materials, and construction



WIP Consequence of Failure – NOT NYC

- Low No loss of roadway, no to low public risk, no impact to traffic during wall repair/replacement
- Moderate Hourly to short-term closure of roadway lowto-moderate public risk, multiple alternate routes available
- High Seasonal to long-term loss of roadway, substantial loss-of-life risk, no alternate routes available.



WIP Data Reliability – NOT NYC

- Estimate of how well observed conditions represent wall performance and if additional investigations may be warranted.
 - 1-Poor Conditions cannot be sufficiently observed to rate element(s), warranting additional investigations to better define element performances and/or to determine the cause(s) or poor performance.
 - 2-Good Observed conditions are sufficient to rate the conditions of wall element(s); however, additional investigations would be useful to better understand element performance.
 - 3-Very Good Observed conditions clearly describe wall performance. Additional investigations are not needed.



Castle Village





Wall Displacement – Castle Village



Displacement evolution in time of several points of wall on same vertical.

EVERY MONITORING NEEDS PREESTABLISHED ACTION PLANS





Figure 1 The above drawing represents a cross section of the tastle Village retaining wall after construction.

The darker outline in this drawing shows how the Castle Village retaining wall had moved and the deformed shape it had taken in the months prior to its collapse.



This drawing shows the lower portion of the wall once the bulging front face has been removed and illustrates how the wall was in imminent danger of collapse without that support.



Important lessons

- Monitoring has to have a plan that include limits that will trigger immediate protective actions.
- Call 311 in case of emergencies



Sections through wall





Back of Face Wythe





Method of Construction



Backfill is dry to support construction operation. Less pressure originally.



Collapse after Noreaster (water from the top)






Gabion vs Placed Stone







Slope Failure After Sandy (water at base)





General Burgoyne's

Experiments 1853

Dry walls with same volume of stone.

Walls type C & D collapsed at 15 ft



Type A – INCLINED WALL & FACE BATTER MOST STABLE

D. Rectangular Wall

form the rectangular

The form the rectangular wall assumed when falling



Empirical Methods – English Rule

547 English Rule. "Experience has shown that a wall [to sustain earth having a level top surface], whose thickness is one fourth of its height, and which batters 1 or 2 inches per foot on the face, possesses sufficient stability when the backing and foundation are both favorable. This allows a factor of safety of about two to cover contingencies. It has also been proved by experience that under no ordinary conditions of surcharge or heavy backing is it necessary to make a retaining wall on a solid foundation more than double the above, or one half of the height in thickness. Within these limits the engineer must vary the strength according to the conditions affecting the particular case. Outside of these limits, the structure ceases to be a retaining wall in the ordinary acceptation of the term.the thickness of retaining walls in ground of an average character equal to one third of the height from the top of the footings.



Empirical Methods - Trautwine Proportions

Retaining-Walls 263 Table III. Proportions of Retaining-Walls (Thickness of wall at the base in parts of the height, A.B., Fig. 16)				
height of the earth and with the height will above ground	Wall of cut stone in mortar	Wall of rubble or brick, good mortar	Wall of good, dry rubble	
	0.35	0.40	0.50	
the second second	0.42	0.47	0.50	
	0.46	0.51	0.61	
	0.49	0.54	0.61	
1.3	0.51	0.56	0.66	
11	0.52	0.57	0.60	
16	0.54	0.50	0.60	
1.7	0,55	0,60	0.70	
1.8	0.56	0.61	0.75	
	0.58	0.63	0.71	
2.5	0.60	0.65	0.75	
	0.62	0.67	0.77	
	0.63	0.68	0.75	
6	0.65	0.69	0.70	
14	0.65	0.70	0.80	
25	0.66	0.71	0.81	
or more	0.68	0.71	0.81	



Engineering calculations based on theories





Graphic Analysis







COEFFICIENTS OF FRICTION.

Historic Data

Materials.	Coefficients.	Materials.	Coefficients.
Dry masonry on dry masonry Masonry on masonry with wet mortar Timber on stone Iron on stone Timber on timber.	0.6 to 0.7 0.75 0.4 0.3 to 0.7 0.2 to 0.5	Masonry on dry clay Masonry on moist clay Earth on earth Hard brick on hard brick Concrete blocks on concrete blocks	0.5 to 0.6 0.33 0.25 to 1.0 0.7 0.65

TABLE II.

Angles of Repose, ϕ , for Materials.

Materials.	\$	Materials.	\$
Earth, loam.	30° to 45°	Clay.	25° to 45°
Sand, dry.	25° to 35°	Gravel.	30° to 40°
Sand, moist.	30° to 45°	Cinders.	25° to 40°
Sand, wet.	15° to 30°	Coke.	30° to 45°

TABLE III.

ALLOWABLE PRESSURE ON FOUNDATIONS.

Material.	Pressure in Tons per Sq. Ft.
Soft clay.	1 to 2
Ordinary clay and dry sand mixed with clay.	2 to 3
Dry sand and clay.	3 to 4
Hard clay and firm, coarse sand	4 to 6
Firm, coarse sand and gravel	6 to 8
Bed rock	15 and up.

TABLE IV.

ALLOWABLE PRESSURE ON MASONRY.

Materials.	Pressure in Tons per Sq. Ft.
Common brick, Portland cement mortar	12
Paving brick, Portland cement mortar	15
Rubble masonry, Portland cement mortar	12 .
Sandstone, first class masonry	20
Limestone, first class masonry	25
Granite, first class masonry	30
Portland cement concrete, 1-2-4	25
Portland cement concrete, 1-3-6	20



Inspection of Old Retaining Walls

Coef. Friction Angle of Repose Allowable Pressure

- foundation

- masonry

Old Drainage Systems –

Influence of water pressure on failures has been recognized at least since 1900.







Rankine Theory

The Rankine Theory is based on the assumptions that the wall introduces no changes in the shearing stresses at the surface of contact between the wall and the soil. It is also assumed that the ground surface is a straight line (horizontal or sloping surface) and that a plane failure surface develops.

Coulomb Theory

An inherent assumption of the Rankine Theory is that the presence of the wall does not affect the shearing stresses at the surface of wall contact. However, since the friction between the retaining wall and the soil has a significant effect on the vertical shear stresses in the soil, the lateral stresses on the wall are actually different than those assumed by the Rankine Theory. Most of this error can be avoided by using the Coulomb Theory, which considers the changes in tangential stress along the contact surface due to wall friction.







Shear Displacement along the Bed





Buckling Under Compression

 FACE OF STONE MIGHT SEPARATE –largest compression is on exterior wall





Plane of sliding

Sliding is not possible



 Rotation of stones and movement creates a plane where sliding is possible





Improper Place Triangular Stones Under Compression

AT FACE -DISLODGES

INTERIOR STONE BULGES









+ interlock



Failures Modes of Retaining Walls

- Sliding
- Rotating
- Crushing
- General Sliding of the Soil



Possible Modes Sliding Failures

- Sliding along a slip surface that cuts through the wall.
- Sliding along a slip surface that runs behind and beneath the wall.
- Sliding along the base of the wall.



Sliding failure is a failure at the soil at the base.

Buckling or swelling of soil at the bottom of RW usually accompanies it.

In some cases there is a separation of soil at top of the wall.

Sliding of portions of the wall will be accompanied by warping of the RW face.

Note that sliding can occur also by rupture of surface of the bottom of the wall itself.



Rotation of the RW could occur inward or outward as the wall is overcome by passive or active earth pressure. Bearing failure of the underlying soil usually precedes it.

- OUTWARD rotation is preceded and accompanied by sink holes and tensions cracks at the top of the wall. At the bottom of the wall one could observe swelling and sloping towards the wall.
- INWARD rotation could be accompanied also by swelling at top of RW. Observation of the alignment of the top of the wall can indicate rotation of wall segments. It will also allow clarify any confusion between walls built with batter or walls inclined inward.



General Failure & Loss of Stability

 General loss of stability describes a failure where the general area that includes soil and retaining wall fails. It is very much similar to a loss of slope stability



Heaving at base / Settlement on top



Unsatisfactory Behavior





Crushing

Crushing is a traditional terms for RW failure that covers failure of RW structural components due to stresses exceeding the carrying capacity. Such failures could be the result

- of design errors,
- stresses due to loads greater then those considered at design time
- or reduction of the carrying capacity of the RW caused by aging,
- exposure to corrosive atmosphere, etc.

Increase in loads can be due to

- unexpected water accumulation behind the wall,
- traffic and vibrations from traffic,
- undesired pressure from improper backfill -especially clay.



Crushing

Reduction in bearing capacity can be the result of

- loss or separation of interlock,
- loss of mortar,
- corrosion of cables of anchors,
- corrosion of reinforcement.

Many of the distress symptoms such as cracking are the same as to those of concrete, masonry and stone building walls or facades.

Gravity walls fail usually in shear –horizontal or vertical. Observation of cracks on the face of RWs can often indicate the type of crushing.

Note that rotations of anchored RWs are often preceded by punching or cracks around the area of the anchor attachment to the stem.



Settlement (Wall Dropping) Reverses Friction

Resultant may change direction





Slope Failure after Rush Flood





Slope Failure





Slope Failures

Water Related Causes

- Intense rainfall
- Perched water table
- Rapid draw-down
- Flood
- Extreme infiltration
- Seepage

Geological Causes

- Erosion
- Weathered materials
- Weak materials
- Contrast in permeability
- Contrast in stiffness



Slope Failures Human Causes

- Excavation of slope at it's toe
- Loading of slope at it's crest
- Irrigation
- Deforestation
- Artificial vibration (blasting, piling, etc.)
- Water leakage from utilities



Causes Unsatisfactory Retaining Walls



 Causes of failure of rigid concrete retaining walls (Techeng and Iseux 1972)

From EM-1110-2

Inspection Instructions for Rockeries

- Each rock is in contact with at least two rocks below it.
- The first contact point between an upper rock and a lower rock is located within 150 mm (6 in) of the face of the rockery.
- There are no "columns" of rocks; i.e., no continuous vertical seams exist.
- There are no continuous horizontal planes in the rockery.
- Rocks are inclined back into the slope.
- Rocks are free of obvious signs of distress, including significant weathering, fracturing, or disintegration.

http://flh.fhwa.dot.gov/innovation/td/geotech/rockeries/documents/09_Chapter_6_Construction_Inspection .pdf

Inspection Instructions for Rockeries

- All voids greater than 150 mm (6 in) are chinked.
- Chink rocks, where present, cannot be removed by hand.
- There are no loose cap rocks or rocks that can otherwise be moved by hand.
- There is no soil spalling or piping through the voids in the face of the rockery.
- Base rocks are larger than upper rocks









Limitations of the Visual Inspection of Retaining Walls

- When the RW has a veneer, in most cases the condition of the backup cannot be observed. On the positive side experience shows that usually veneers deteriorate faster than the backups.
- Many RWs were built on plies. (The literature shows that even rubble RW's could have been placed on pile foundations). The presence or condition of piles cannot be observed.



Backfill & Foundation Materials







Hides condition RW



Actually deteriorates RW




Wall thickness? Fill?











Various accidents





More





Effect of Vegetation





Mortar Condition







Cracks







Wall Movement - Corner Condition







Crest symptoms







Crest symptoms







Tension Cracks





Stone Fragments at Base of Wall







SINKHOLES





Out of Plumb





Bulging





Irregularities at coping level





Stone Spalling







Stone Masonry RW - Form Rapid Assessment

	Rubble			□ Coursed		□ Random		Rough		Dry
										[no mortar]
	Cutstone			Coursed		Random		Rough		Dry
										[no mortar]
		Counterfort		Don't Know		No		Yes		
		Buttressed [w. piers]								
		Tiedback [anchored] wall		Rock Anchor				Deadman		
		RW on piles		Don't Know		Νο		Yes		
	□ RW foundation on soil type					Don't Know				
								•		
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I. Structural Condition Assessment continued...

				-			<i>∕</i> ∖	-	
CMU Retaining Wall							·ty,		
	Brick Retaining Wall						"MS	•	
	Vene	ers on	Concrete		Ashlar		Brick 🗸	Ŕ	СМИ
Ashlar Veneer on Rubble									
	Veneer on Natural Rock								
		Coun	terfort		Don't Know		No		Yes
		Buttre	essed [w. piers]						
		Tiedb	ack [anchored] wall		Rock Anchor		Deadman		
		RW o	n piles		Don't Know		No		Yes
		Steel	Reinforced		Don't Know		No		Yes
		RW fo	oundation on soil type				Don't Know		
During the course of the Visual Inspection, the condition of the back-up structure was:									
			Reliably Assessed						
Partially Assessed									
			Not Assessed						



I. Structural Condition Assessment

ι.	Structural Co	on	diti	on	Asse	es	smen	t	Ŷ	ANID,
S1	Top of Wall Outward		No		Out of plum	b/heigł	nt	Not	e #	$\langle \langle \rangle$
S2	Top of Wall Inward		No		Out of plumb/height		nt	Note #		·
S3	Bulging/Warping of Wall		Νο		Minor		Moderate		Severe	Note #
S4	Top of Wall Aligned		No	Note	e #	Des	C			
S5	Tiebacks		No		Loose		Corroded		Missing %	Note #
S6	Settlement of Wall		No		Minor		Moderate		Severe	Note #
S 7	Displaced Large Stone		No		Minor		Moderate		Severe	Note #
S8	Displaced Small Stone		No		Minor		Moderate		Severe	Note #
S9	Horizontal Cracks		No		Minor		Moderate		Severe	Note #
S10	Vertical Cracks		No		Minor		Moderate		Severe	Note #
S11	Diag. Cracks at Mortar Joint Only		No		Minor		Moderate		Severe	Note #
S12	Diag. Cracks through Joint & Stone		No		Minor		Moderate		Severe	Note #
S13	Cracked Stones		No		Minor		Moderate		Severe	Note #
S14	Spalled Stone		No		Minor		Moderate		Severe	Note #
S15	Condition of Mortar		Good		Sandy	Miss	sing pct			
S16	Coping of Wall		None		Sound		Deteriorated		Displace	Note #
S17	Corner Cracks		No		Both Sides		One Side		Mortar	Mortar & Stone Note #
S18	Previous Repair		None Visible		Minor		Moderate		Major	Failed Note #

S19 Other



3B1 Top of Wall Outward	Νο		Out of plumb	/height		Note	* \{	1/2
3B2 Top of Wall Inward	No		Out of plumb	/height		Note	#	10
3B3 Bulging/Warping of Wall	No		Minor		Moderate		Severe	Not
3B4 Top of Wall Aligned	No	Note	. #	Desc.				
3B5 Tiebacks	No		Loose		Corroded		Missing %	Note #
B6 Settlement of Wall	No		Minor		Moderate		Severe	Note #
B7 Expansion Construction Joint	None		Sound		Deteriorated	% Det		Note #
B8 Horizontal Crack	None		Sound		Deteriorated	% Det		Note #
B9 Vertical Cracks	No		Minor		Moderate		Severe	Note #
310 Corner Crack	No		Both Sides		One Side		Mortar Jt. Mortar Jt. & Blk.	Note #
311 Stepped Cracks at Mort. Jnt. only	No		Minor		Moderate		Severe	Note #
12 Stepped Crack through Jnts & Blk	No		Minor		Moderate		Severe	Note #
813 Crack Due Steel Corrosion	No		Minor		Moderate		Severe	Note #
314 Steel Reinforcement Exposed	No		Rusted		Sect. Loss	Note	#	
B15 Displaced Blocks/Bricks	No		Minor		Moderate		Severe	Note #
B16 Spalled Brick/Block	No		Minor		Moderate		Severe	Note #
B17 Conditions of Mortar	Sound		Sandy		Missing %			
B18 Freeze/Thaw Damage [crazing]	No		Minor		Moderate		Severe	Note #
B19 Efflorescence/Calcium/Chloride	Yes		Description					
320 Veneer Not Attached Back-up	None		Sound		Deteriorated		Displaced	Note #
S21 Veneer Separated Back-up	None		Sound		Deteriorated		Displaced	Note #
322 Coping of Wall	None		Sound		Deteriorated		Displaced	Note #
B23 Previous Repair	None Visible		Minor		Moderate		Major Failed	Note #
B24 Other								



II. Condition Assessment of Soil / Pavement Adjoining Wall

1. Buckling of Road Sidewalk at Bottom	□ No □ Minor □ Moderate □ Severe Note #
2. Tension Cracks in Soil at Top	Yes – Width of Crack
3. Sink Holes in Soil/Pavement	□ Yes – Dimensions
4. Soil/Pavement at Base of Wall	□ Acceptable □ Defective Describ.
5. Soil/Pavement at Top of Wall	Acceptable Defective Describ.
6. Spoil Separating from Back of Wall	□ No – Width of Separation

7. Other _____



III. Condition Assessment of Water Management Area Surrounding Wall

	III. Condition Assessment of water									
	Management	Area	Surrounding Wall							
1.	Weeps	□ No	Functioning?							
2.	Erosion of Wall/Soil by Water	□ No								
3.	Water/Silt Filtering through Water	□ No	□ Minor □ Moderate □ Severe Note #							
4.	Area Drains/Piping Present	□ No	Functioning?							
5.	Drywell Catch Basin	□ No	Describe							
6.	Hydrant	□ No	Describe							
7.	Downspouts/adj/ Buildings	□ No	Describe							
8.	Soil Drains Away from Wall	□ No	Describe							
9.	Other									



IV. Attachments to Wall

	V. Attachments	all	Et AND,	
1.	Balustrade/Handrail	□ No	Describe	
2.	Ramp/Steps		Describe	Condition
3.	Tunnels	□ No	Describe	Condition
4.	Light Structure (shed/garage)	□ No	Functioning?	
5.	Fence	□ No	Describe	
6.	Trees/Vegetation	□ No	Describe	
7.	Equipment/Storage at near Top of Wall	□ No	Describe	
8.	Other			



Stabilization -Shoring





Bibliography

- Naval Facilites Engineering Command (1967) Design Manual: Structural Engineering
 : NAVFAC DM-2 Dec. 1967 Department of the Navy
- Trautwine J., (1886) The Civil Engineer's Pocket-Book -Revised, New York, John Wiley
- Baker Ira, (1905) A Treatise of Masonry Construction, 9th ed. New York , John Wiley
- Paaswell G., (1920) Retaining Walls: Their Design and Construction, New York, McGraw Hill
- FHWA-CFL/TD-10-003 (2010) Retaining Wall Inventory and Condition Assessment Program (WIP) National Park Service Procedures Manual, Lakewood, CO
- Chan Y C (1996). Study of Old Masonry Retaining Walls in Hong Kong. Geo Report No 31. Hong Kong: Geotechnical Engineering Office
- K. C. Brady, J. Kavanagh, (2002) Analysis of the Stability of Masonry-faced Earth Retaining Walls, Transport Research Laboratory (Great Britain), Transport Research Foundation (Great Britain)
- New York State Department of Transportation, (2014) Bridge Inspection Manual
- Federal Highway Administration, (2012) Bridge Inspector's Reference Manual FHWA NHI 12-049



•Questions?

